

SUPPLEMENTAL DATA REPORT

Proposed Multi-family Development

51-53-55 Summer Street

Walpole, Massachusetts



Prepared by:

Howard Stein Hudson 114 Turnpike Road, Suite 2C Chelmsford, MA 01824

June 2023



Contents

Existing Conditions

Proposed Conditions

Hydrology

Stormwater Management Standards

Standard 1: No new untreated discharges

Standard 2: Post-development peak discharge rates not to exceed pre-development peak discharge rates

Standard 3: Minimize or eliminate loss of annual recharge to groundwater

Standard 4: Stormwater management system to remove 80% of the average annual load of Total Suspended Solids (TSS)

Standard 5: Land uses with higher potential pollutant loads

Standard 6: Stormwater discharges to critical areas

Standard 7: Redevelopment projects

Standard 8: Control construction-related impacts

Standard 9: Long-term operation and maintenance plan

Standard 10: No illicit discharges

Appendix A: Test Pit Information / Falling Head Permeability Test Results

Appendix B: Mounding Analysis

Appendix C: Operation and Maintenance Plan and Long-Term Pollution Prevention Plan

Appendix D: Pre- and Post- Drainage maps

Appendix E: HydroCAD, Stage Storage, and Pocket Wetland Calculations



Existing Conditions

The subject site consists of three parcels totaling 54.73 acres in the Limited Manufacturing – LM zone. The addresses of record for the parcels are 51-53-55 Summer Street. Summer Street has a 50-foot-wide right-of-way along the frontage of the existing lots. There are no existing buildings or improvements on site. The site extends to the east side of the railroad tracks. On the north side there is a Park, School, Recreation & Conservation (PSRC) zoned area encompassing a large wetland area. On the west and south sides there is Residence B (RB) zoned properties.

There is a railroad yard located on the abutting property on the east side of our project site in the LM zone, across from the railroad right-of-way. The PSRC zone does not contain any existing buildings. An RB zone exists to the west and south of the parcel and contain residential dwellings with associated improvements (such as stand-alone garages, pools, sheds, driveways, etc).

The site contains a mix of woodlands, isolated vegetated wetlands, bordering vegetated wetlands, vernal pools, and open grassed areas. All three vernal pools are denoted as potential vernal pools at this time. This property is located within the Area 3 – Primary Recharge Area Water Resource Protection Overlay District and partially within the Large-Scale Ground-Mounted Solar Photovoltaic Overlay District (SPOD). The terrain ranges on site from elevation 186' to 228' Mean Sea Level, with the lower areas generally being wetlands and the higher elevations being upland areas. The site topography decreases from south to north starting at Summer Street and ending at Cedar Swamp Brook at the rear of the site. The site currently accepts direct runoff from abutters on the south and west sides. This runoff flows into a wetland on the northern side of the property. Existing flow patterns are generally from the south and west towards north, with localized flow in other directions due to the site terrain.

The site hydrology consists of upland areas flowing to both isolated and bordering vegetated wetlands existing across the entirety of the site. The entire site drains to four analysis points. The first (AP1) is a small portion of the entrance to the site that drains back onto Summer Street and into the drainage system located within Summer Street. This takes up a very small portion of the site drainage. The second analysis point (AP2) for the site is an isolated wetland which is located adjacent to the existing train tracks on the eastern side of the property. This depression has an outlet which flows under the railroad tracks, but it is currently completely blocked; water collects here and slowly infiltrates into the soil. The third analysis point is another wetland area (AP3). This isolated pocket is located adjacent to the eastern train tracks and the other depressed wetland pocket, AP2, and collects and infiltrates water. The final analysis point (AP4) is Cedar Swamp Brook which runs along the entirety of the northern part of the property.



The only drainage infrastructure located onsite is the blocked outlet pipe which runs under the existing railroad tracks. There is an existing storm drain system in Summer Street with a catch basin located along the site's frontage.

Soil conditions on site are mainly Fine Sandy Loam (Canton, Ridgebury, Whitman, Scituate, and Merrimac) with a smaller area of Hollis-Rock Outcrop-Charlton Complex. The hydrologic soil group for these soils area A, B, C & D with a majority belonging to groups B, C & D.

Both town and private sewer, water, electricity, gas, and communications are currently located within the Summer Street right-of-way, which is the preferred source of utilities to service the project.

Proposed Conditions

This project proposed to construct a multifamily housing development consisting of apartment buildings and townhouses for rent. This project is to be serviced by municipal utilities. An easement was purchased from the abutter located at 87 Summer Street to facilitate a second means of emergency access and looped water service for the development.

The existing site is proposed to be improved with the addition of stormwater best management practices which are designed to treat, detain, and infiltrate the proposed impervious areas on the developed site, directing stormwater to the same four (4) analysis points.

There are eight (8) main stormwater treatment trains proposed within the new development. The first main treatment train drains to Pond P204 which is the proposed Stormtech infiltration system located to the east of the proposed multi-family building #2000. This treatment train takes the clean roof runoff from multi-family building #2000, the adjacent townhouse unit and some of the pavement runoff to the north of multi-family building #2000. This treatment train outlets to the adjacent wetland and flows to Analysis Point #4.

Treatment train #2 drains to Pond P205 which is located adjacent to the emergency access to Summer Street. This infiltration basin accepts runoff from the emergency access road. This treatment train outlets to the adjacent wetland and flows to Analysis Point #4.

Treatment train #3 drains to Pond P206, the second Stormtech chamber system onsite, which is located behind multi-family building #1000. This Stormtech system accepts all the clean roof runoff from multi-family building #1000 as well as the associated pretreated street drainage on the northern side of the building. This drainage is piped into the system, treated, and infiltrated onsite prior to out-letting to the adjacent wetland system and flowing to Analysis Point #4



Treatment train #4 drains to Pond P207 which is located on the western side of multi-family building #1000 and accepts most of the street and open-space drainage located adjacent to multi-family buildings #1000 and #2000. This treatment train outlets to the adjacent wetland and flows to Analysis Point #4.

Treatment train #5 drains to Pond P210 which is located north of Driveway A, on the eastern side of the property adjacent to the railroad tracks. This Pocket Wetland accepts all the associated street drainage from the beginning section of Driveway A until the mail kiosk. This treatment train outlets to the adjacent wetland and flows to Analysis Point #2.

Treatment train #6 drains to Pond P212 which is located between Driveway C, Driveway D, and wetland system C, in the center of the development. This Infiltration Pond takes all the street drainage from the high points of both crossings to more than halfway down Driveway C and D. The three townhouses adjacent to the open space in the center of the development are directed to this infiltration pond and treated. This treatment train outlets to the adjacent wetland and flows to Analysis Point #4.

Treatment train #7 drains to Pond P213, the third Stormtech chamber system onsite, which is located in the south parking area adjacent to multi-family building #13000. This Stormtech system accepts the southern half of Driveway D and all roof runoff from multi-family building #13000. This drainage is piped into the system, treated, and infiltrated prior to discharging to Pond P212, and later to the adjacent wetland system, Analysis Point #4.

Treatment train #8 drains to Pond P214, the fourth Stormtech chamber system onsite, which is located in the north parking area adjacent to multi-family building #13000. This Stormtech system accepts the parking area to the north of multi-family building #13000. This drainage is piped into the system, treated, and infiltrated prior to discharging to Analysis Point #4.

The remainder of the clean roof runoff from the townhouse units are handled with individual drip edge systems which outlet to either Analysis Point #2 or 4 via overland flow.

As of February 28, 2023, tree clearing has occurred as depicted on previously approved plans dated January 10, 2020, revised through April 13, 2021. Tree clearing which has occurred outside of the newly proposed limit of disturbance will grow back to its natural state.

The remainder of the land, which was untouched will flow, as it currently does, to the existing analysis points.



Stormwater Management Standards

Standard 1: No new untreated discharges

The Massachusetts Stormwater Handbook requires that the project demonstrates that no new stormwater conveyances (e.g. outfalls) discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

The proposed project will not discharge stormwater directly to, or cause erosion in, wetlands or water of the Commonwealth and will treat stormwater prior to discharge or infiltration.

BMP's have been proposed to treat stormwater collected from the newly paved areas. Each treatment chain consists of a deep sump hooded catch basin, grassed channel and a sediment forebay which is sized to accommodate the water quality volume per the Massachusetts Stormwater Handbook.

The new discharges have been designed to outlet to flared end sections with riprap to minimize any erosion to the isolated vegetated wetland. The table below shows the average flow rate for the 2-year storm event in feet per second (fps).

Storm Event	2-year
Flared End Section (Pond 204) (fps)	2.71
Flared End Section (Pond 205) (fps)	0.00
Flared End Section (Pond 206) (fps)	0.00
Flared End Section (Pond 207) (fps)	1.10
Flared End Section (Pond 210) (fps)	3.62
Flared End Section (Pond 212) (fps)	0.00
Flared End Section (Pond 213) (fps)	0.00
Flared End Section (Pond 214) (fps)	0.00



Standard 2: Post-development peak discharge rates not to exceed pre-development peak discharge rates.

Post-development peak discharge rates do not exceed the pre-development peak discharge rates and total runoff volumes for all storm events except for a runoff volume increase to Analysis Point #4 during the 100-year storm event (Cedar Brook). The proposed condition reduces rates by collecting and controlling the stormwater runoff within the stormwater management system.

Storm Event	2-year	10-year	25-year	100-year
Pre-Development Rates (cfs) AP1	0.74	1.28	1.71	2.59
Volume (cf) (Summer St)	2,360	4,159	5,619	8,714
Post-Development Rates (cfs) AP1	0.67	1.05	1.35	1.96
Volume (cf) (Summer St)	2,247	3,627	4,720	7,004
Rate Reductions (cfs)	-0.07	-0.23	-0.36	-0.63
Volume Reductions (cf)	- 113	- 532	- 899	-1,710
Pre-Development Rates (cfs) AP2	12.30	27.75	41.12	70.33
Volume (cf) (Wetland at track)	85,349	184,006	270,829	464,971
Post-Development Rates (cfs) AP2	7.31	18.24	28.46	52.47
Volume (cf) (Wetland at track)	75,144	157,893	229,211	394,820
Rate Reductions (cfs)	-4.99	-9.51	-12.66	-17.86
Volume Reductions (cf)	-10,205	-26,113	-41,618	-70,151
Pre-Development Rates (cfs) AP3	2.52	5.96	8.96	15.56
Volume (cf) (Wetland at track)	8,514	18,960	28,279	49,317
Post-Development Rates (cfs) AP3	1.15	2.89	4.43	7.86
Volume (cf) (Wetland at track)	4,009	9,258	14,013	24,862
Rate Reductions (cfs)	-1.37	-3.07	-4.53	-7.70
Volume Reductions (cf)	-4,505	-9,702	-14,266	-24,455
Pre-Development Rates (cfs) AP4	10.77	33.90	56.04	107.72
Volume (cf) (Cedar Brook)	73,247	192,708	306,701	576,512
Post-Development Rates (cfs) AP4	7.79	25.19	43.69	98.54
Volume (cf) (Cedar Brook)	65,174	178,860	300,250	593,282



Rate Reductions (cfs)	-2.98	-8.71	-12.35	-9.18
Volume Reductions (cf)	-8,073	-13,848	-6,451	16,770

Standard 3: Minimize or eliminate loss of annual recharge to groundwater.

Groundwater recharge will be accomplished using the surface infiltration and subsurface practices. As shown in the table summary for Standard 2, the project decreases the total volume of runoff for all storm events except for a runoff volume increase to Analysis Point #4 (Cedar Brook) during the 100-year storm event. This reduction in volume is generated by collecting and infiltrating a significant portion of the impervious surfaces created on site.

Recharge Volume Requirement:

Rv = F x impervious area

Rv = Required Recharge Volume, expressed in Ft³, cubic yards, or acre-feet

F= Target Depth Factor associated with each Hydrologic Soil Group

Impervious Area = pavement and rooftop area <u>on site</u>

Recharge volume for the entire site:

Soil A:

Rv=0.60 in * 55,549 sf * 1 ft / 12 in = 2,778 cf recharge

Soil B:

Rv=0.35 in * 71,802 sf * 1 ft / 12 in = 2,095 cf recharge

Soil C:

Rv=0.25 in * 285,304 sf * 1 ft / 12 in = 5,944 cf recharge

Soil D:

Rv=0.1 in * 45,142 sf * 1 ft / 12 in = 377 cf recharge

Total Recharge Required:

Rv = (2,778 cf) + (2,095 cf) + (5,944 cf) + (377 cf) = 11,194 cf total recharge required

<u>Total recharge provided:</u>

 $Townhouse\ Drip\ Edges\ (4\ Unit-Type\ I) = 96\ cf\ below\ outlet = (96\ cf)*(5\ buildings) = 480\ cf$

 $Townhouse\ Drip\ Edges\ (4\ Unit-Type\ II) = 82\ cf\ below\ outlet = (82\ cf)\ *\ (2\ buildings) = 164\ cf$

 $Townhouse\ Drip\ Edges\ (6\ Unit) = 136\ cf\ below\ outlet = (136\ cf)*(4\ buildings) = 544\ cf$

 $Club\ house\ drip\ edge = 130\ cf\ below\ outlet$

Pond P204 = 2,117 cf below outlet (Stormtech System)

Pond P205 = 7,623 cf below outlet

Pond P206 = 4,970 cf below outlet (Stormtech System)

Pond P207 = 6,345 cf below outlet



Pond P210 = 0 cf below outlet

Pond P212 = 22,538 cf below outlet

Pond P213 = 5,440 cf below outlet (Stormtech System)

Pond P214 = 5,178 cf below outlet (Stormtech System)

Total site recharge provided = 55,529 cf recharge volume > 11,194 cf required

Recharge per Pond

Pond P204

Soil A:

Rv=0.60 in * 8,041 sf * 1 ft / 12 in = 403 cf recharge

Soil C:

Rv=0.25 in * 41,586 sf * 1 ft / 12 in = 867 cf recharge

Soil D:

Rv=0.1 in * 3 sf * 1 ft / 12 in = 1 cf recharge

Total Weighted Average Recharge:

Rv = (403 cf) + (867 cf) + (1 cf) = 1,271 cf recharge required

Recharge provided (including drip edges) = 2,383 cf > 1,270 cf required

Pond P205:

Soil B:

Rv=0.35 in * 16,376 sf * 1 ft / 12 in = 478 cf recharge

Recharge provided = 7,623 cf > 478 cf required

Pond P206:

Soil A:

Rv=0.60 in * 131 sf * 1 ft / 12 in = 7 cf recharge

Soil C:

Rv=0.25 in * 27,225 sf * 1 ft / 12 in = 568 cf recharge

Soil D:

Rv=0.1 in * 20,862 sf * 1 ft / 12 in = 174 cf recharge

Total Weighted Average Recharge:

Rv = (7 cf) + (568 cf) + (174 cf) = 749 cf total recharge required

Recharge provided = 4,970 cf > 749 cf required



Pond P207

Soil A:

Rv=0.60 in * 35,976 sf * 1 ft / 12 in = 1,799 cf recharge

Soil C:

Rv=0.25 in * 25,566 sf * 1 ft / 12 in = 533 cf recharge

Soil D:

Rv=0.1 in * 20,350 sf * 1 ft / 12 in = 170 cf recharge

Total Weighted Average Recharge:

Rv = (1,799 cf) + (533 cf) + (170 cf) = 2,502 cf recharge required

Recharge provided = 6,345 cf > 2,502 cf required

Pond P210

Soil B:

Rv=0.35 in * 30,443 sf * 1 ft / 12 in = 888 cf recharge

Soil C:

Rv=0.25 in * 911 sf * 1 ft / 12 in = 19 cf recharge

Soil D:

Rv=0.1 in * 1,186 sf * 1 ft / 12 in = 10 cf recharge

Total Weighted Average Recharge:

Rv = (888 cf) + (19 cf) + (10 cf) = 917 cf recharge required

Recharge provided = $0 \text{ cf} \neq 917 \text{ cf}$ required

(Overall recharge provided on site still greater than overall recharge required)

Pond P212

Soil B:

Rv=0.35 in * 772 sf * 1 ft / 12 in = 23 cf recharge

Soil C:

Rv=0.25 in * 109,962 sf * 1 ft / 12 in = 2,291 cf recharge

Soil D:

Rv=0.1 in * 1,920 sf * 1 ft / 12 in = 16 cf recharge

Total Weighted Average Recharge:

Rv = (23 cf) + (2,291 cf) + (16 cf) = 2,330 cf recharge required

Recharge provided (including drip edges) = 22,906 cf > 2,330 cf required



Pond P213

Soil C:

Rv=0.25 in * 29,819 sf * 1 ft / 12 in = 622 cf recharge

Recharge provided = 5,440 cf > 622 cf required

Pond P214

Soil A:

Rv=0.6 in * 8,071 sf * 1 ft / 12 in = 404 cf recharge

Soil C:

Rv=0.25 in * 22,900 sf * 1 ft / 12 in = 478 cf recharge

Total Weighted Average Recharge:

Rv = (404 cf) + (478 cf) = 882 cf recharge required

Recharge provided = 5,178 cf > 882 cf required

Drawdown Within 72 Hours

Townhouse Drip Edge (4 unit – Type I): 96 cf / [(0.17 in/hr)(1 ft/12 in)(470 sf)] = 14.4 hours < 72 hours, OK

Townhouse Drip Edge (4 unit – Type II): 82 cf / [(0.17 in/hr)(1 ft/12 in)(404 sf)] = 14.3 hours < 72 hours, OK

Townhouse Drip Edge (6 unit): 136 cf / [0.17 in/hr)(1 ft/12 in)(665 sf)] = 14.4 hours < 72 hours, OK

Pond P204: 2,117 cf / [(0.66 in/hr)(1 ft/12 in)(5,670 sf)] = 6.8 hours < 72 hours, OK

Pond P205: 7,623 cf / [(2.41 in/hr)(1 ft/12 in)(3,939 sf)] = 9.7 hours < 72 hours, OK

Pond P206: 4,970 cf / [(3.5 in/hr)(1 ft/12 in)(6,072 sf)] = 2.8 hours < 72 hours, OK

Pond P207: 6,345 cf / [(3.69 in/hr)(1 ft/12 in)(10,100 sf)] = 2.1 hours < 72 hours, OK

Pond P212: 22,358 cf / [(5.13 in/hr)(1 ft/12 in)(4,354 sf)] = 12.0 hours < 72 hours, OK

Pond P213: 5,440 cf / [(5.13 in/hr)(1 ft/12 in)(3,317 sf)] = 3.9 hours < 72 hours, OK

Pond P214: 5,178 cf / [(8.28 in/hr)(1 ft/12 in)(3,201 sf)] = 2.4 hours < 72 hours, OK



Water Quality Volume

Calculated as Vwq = (Dwg/12 inches/foot) * (Aimp * 43,560 square feet/acre), where:

Vwq =required water quality volume (in cubic feet)

Dwq = water quality depth: one-inch for discharges within a Zone II or Interim Wellhead Protection Area, to or near another critical area, runoff from a LUHPPL, or exfiltration to soils with infiltration rate greater than 2.4 inches/hour or greater; ½ inch for discharges near or to other areas.

Aimp = impervious area (in acres)

Aimp = Impervious Area of Subcatchments onsite = 457,797 SF

Dwq = 1 inch

Vwq = (1 inch / 12 inches / foot) * (308,995 S.F.) = 25,750 C.F.

 $Vwq = (\frac{1}{2} inch / 12 inches / foot) * (148,802 S.F.) = 12,401 C.F.$

Total Water Quality Volumes from proposed BMP's = 55,529 cf > 38,151 cf OK

Pretreatment sizing for flow based devices

Calculated as Vwq = (Dwg/12 inches/foot) * (Aimp * 43,560 square feet/acre), where:

Vwq = required water quality volume (in cubic feet)

Dwq = water quality depth: one-inch for discharges within a Zone II or Interim Wellhead Protection Area, to or near another critical area, runoff from a LUHPPL, or exfiltration to soils with infiltration rate greater than 2.4 inches/hour or greater; ½ inch for discharges near or to other areas.

Aimp = impervious area

Pond P204:

Stormtech Infiltration Chambers = (½ inch / 12 inches / foot) * (49,630 S.F.) = 2,068 C.F.

Designed Infiltration Chambers = 2,117 C.F. below outlet

2,117 CF > 2,068 CF OK

Stormtech Isolator Row:



 $Q(\frac{1}{2})=(752 \text{ csm/in})(1.14 \text{ AC})(0.0015625 \text{ mi2/AC})(\frac{1}{2} \text{ in})$

 $Q(\frac{1}{2})=0.67 \text{ cfs}$

For the SC 740 each chamber is rated for 0.14 cfs:

Design calls for 9 SC 740 Isolator Units = 9 units x 0.14 cfs = 1.26 cfs

1.26 cfs > 0.67 cfs OK

Volume Provided = 1.26 cfs

1.26 cfs > 0.67 cfs O.K.

Pond P205:

Infiltration pond = (1 inch / 12 inches / foot) * (16,376 S.F.) = 1,365 C.F.

Designed Infiltration Pond = 7,623 C.F. below outlet

7,623 CF > 1,365 CF OK

Sediment forebay = 0.1 * 1,365 C.F = 137 C.F

Designed sediment forebays = 144 CF

144 CF > 137 CF OK

Pond P206:

Stormtech Infiltration Chambers = (1 inch / 12 inches / foot) * (48,218 S.F.) = 4,019 C.F.

Designed Infiltration Chambers = 4,970 C.F. below outlet

4,970 CF > 4,019 CF OK

Stormtech Isolator Row:

Q(1)=(774 csm/in)(1.11 AC)(0.0015625 mi2/AC)(1 in)

Q(1)=1.35 cfs

For the SC 740 each chamber is rated for 0.14 cfs:

Design calls for 16 SC 740 Isolator Units = 16 units x 0.14 cfs = 2.24 cfs

2.24 cfs > 1.35 cfs OK

Volume Provided = 2.24 cfs

2.24 cfs > 1.35 cfs O.K.



Pond P207:

Infiltration pond = (1 inch / 12 inches / foot) * (64,794 S.F.) = 5,400 C.F.

Designed Infiltration Pond = 6,345 C.F. below outlet

6,345 CF > 5,400 CF OK

Sediment forebay = 0.1 * 5,400 C.F = 540 C.F

Designed sediment forebays = 1,257 CF

1,257 CF > 540 CF OK

Pond P210:

Pocket Wetland #1 = $(\frac{1}{2} \text{ inch} / 12 \text{ inches} / \text{ foot}) * (32,540 \text{ S.F.}) = 1,356 \text{ C.F.}$

Micropool and Low / High Marsh Volume = 2,619 C.F. (See attached design criteria)

 $2,619 \text{ CF} \ge 1,356 \text{ CF OK}$

Sediment forebay = 0.1 * 1,356 C.F = 136 C.F

Designed sediment forebays = 267 CF

267 CF > 136 CF OK

Pond P212:

Infiltration pond = (1 inch / 12 inches / foot) * (112,654 S.F.) = 9,388 C.F.

Designed Infiltration Pond = 22,538 C.F. below outlet

22,538 CF > 9,388 CF OK

Sediment forebay = 0.1 * 9,388 C.F = 939 C.F

Designed sediment forebays = 2,406 CF

2,406 CF > 939 CF OK



Pond P213:

Stormtech Infiltration Chambers = (1 inch / 12 inches / foot) * (29,819 S.F.) = 2,485 C.F.

Designed Infiltration Chambers = 5,440 C.F. below outlet

5,440 CF > 2,485 CF OK

Stormtech Isolator Row:

Q(1)=(774 csm/in)(0.69 AC)(0.0015625 mi2/AC)(1 in)

Q(1)=0.84 cfs

For the SC 740 each chamber is rated for 0.14 cfs:

Design calls for 9 SC 740 Isolator Units = $9 \text{ units } \times 0.14 \text{ cfs} = 1.26 \text{ cfs}$

1.26 cfs > 0.84 cfs OK

Volume Provided = 1.26 cfs

1.26 cfs > 0.84 cfs O.K.

Pond P214:

Stormtech Infiltration Chambers = (1 inch / 12 inches / foot) * (30,971 S.F.) = 2,581 C.F.

Designed Infiltration Chambers = 5,178 C.F. below outlet

5,178 CF > 2,581 CF

Stormtech Isolator Row:

Q(1)=(774 csm/in)(0.72 AC)(0.0015625 mi2/AC)(1 in)

Q(1)=0.87 cfs

For the SC 740 each chamber is rated for 0.14 cfs:

Design calls for 9 SC 740 Isolator Units = $9 \text{ units } \times 0.14 \text{ cfs} = 1.26 \text{ cfs}$

Volume Provided = 1.26 cfs

1.26 cfs > 0.87 cfs O.K.

Contech Stormceptor STC-900 Water Quality Unit

Q(1) = (774 csm/in)(0.39 AC)(0.0015625 mi2/AC)(1 in)

Q = 0.48 cfs

STC-900 Water Quality Unit is rated for 0.89 cfs

Volume Provided = 0.89 cfs

0.89 cfs > 0.48 cfs O.K.



Standard 4: Stormwater management system to remove 80% of the average annual load of Total Suspended Solids (TSS)

The stormwater management system is designed to remove >80% annual total suspended solids (TSS) from the proposed roadway, driveways, and sidewalks.

The stormwater management system is designed to remove 80% of the average annual total suspended solids (TSS) from the proposed development.

TSS Removal Calculation

Pretreatment Train #1 to Pond P204

Deep Sump Hooded Catch Basin:

$$100\% * 25\% = 25\%$$

 $100\% - 25\% = 75\%$

• Stormtech Isolator Row:

Pretreatment TSS Removal = 25% + 19% = 44%

Treatment Train #1 to Pond P204

• Stormtech Isolator Row:

$$100\% * 25\% = 25\%$$

 $100\% - 25\% = 75\%$

• Stormtech Infiltration Chambers

TSS Removal of the proposed drainage = 25% + 60% = 85%Site impervious percentage = 7%



Pretreatment Train #2 to Pond P205

• Deep Sump Hooded Catch Basins:

• Sediment Forebay:

Pretreatment TSS Removal = 25% + 19% = 44%

Treatment Train #2 to Pond P205

• Sediment Forebay:

$$100\% * 25\% = 25\%$$

 $100\% - 25\% = 75\%$

• Infiltration Pond

TSS Removal of the proposed drainage = 25% + 60% = 85%Site impervious percentage = 5%

Pretreatment Train #3 to Pond P206

• Deep Sump Hooded Catch Basin:

• Stormtech Isolator Row:

Pretreatment TSS Removal = 25% + 19% = 44%



Treatment Train #3 to Pond P206

• Stormtech Isolator Row:

• Stormtech Infiltration Chambers

TSS Removal of the proposed drainage = 25% + 60% = 85%Site impervious percentage = 8%

Pretreatment Train #4 to Pond P207

• Deep Sump Hooded Catch Basins:

$$100\% * 25\% = 25\%$$

 $100\% - 25\% = 75\%$

• Sediment Forebay / CDS Water Quality Unit (Calculation based on minimum treatment from Deep Sump Hooded Catch Basin only. CDS Water Quality unit will see a higher TSS treatment removal rate.):

Pretreatment TSS Removal = 25% + 19% = 44%

Treatment Train #4 to Pond P207

• Sediment Forebay / CDS Water Quality Unit (Calculations based on minimum treatment from Deep Sump Hooded Catch Basin only. CDS Water Quality Unit will see a higher TSS treatment removal rate.):

$$100\% * 25\% = 25\%$$

 $100\% - 25\% = 75\%$



• Infiltration Pond:

TSS Removal of the proposed drainage = 25% + 60% = 85%Site impervious percentage = 25%

Pretreatment Train #5 to Pond P210

• Deep Sump Hooded Catch Basins:

• Sediment Forebay:

Pretreatment TSS Removal = 25% + 19% = 44%

Treatment Train #5 to Pond P210

• Sediment Forebay:

• Pocket Wetland

TSS Removal of the proposed drainage = 25% + 60% = 85%Site impervious percentage = 10%



Pretreatment Train #6 to Pond P212

• Deep Sump Hooded Catch Basin

• Sediment Forebay:

Pretreatment TSS Removal = 25% + 19% = 44%

Treatment Train #6 to Pond P212

• Sediment Forebay:

$$100\% * 25\% = 25\%$$

 $100\% - 25\% = 75\%$

• Infiltration Pond:

TSS Removal of the proposed drainage = 25% + 60% = 85%Site impervious percentage = 28%

Pretreatment Train #7 to Pond P213

• Deep Sump Hooded Catch Basin:

• Stormtech Isolator Row:



Pretreatment TSS Removal = 25% + 19% = 44%

Treatment Train #7 to Pond P213

• Stormtech Isolator Row:

$$100\% * 25\% = 25\%$$

 $100\% - 25\% = 75\%$

• Stormtech Infiltration Chambers:

TSS Removal of the proposed drainage = 25% + 60% = 85%

Site impervious percentage = 8%

Pretreatment Train #8 to Pond P214

• Deep Sump Hooded Catch Basin:

$$100\% * 25\% = 25\%$$

 $100\% - 25\% = 75\%$

• Stormtech Isolator Row:

Pretreatment TSS Removal = 25% + 19% = 44%

Treatment Train #8 to Pond P214

• Stormtech Isolator Row:



• Stormtech Infiltration Chambers:

TSS Removal of the proposed drainage = 25% + 60% = 85%

Site impervious percentage = 3%

Treatment Train #9 to Existing Summer Street CB

• Deep Sump Hooded Catch Basins:

$$100\% * 25\% = 25\%$$

 $100\% - 25\% = 75\%$

TSS Removal of the proposed drainage = 25% Site impervious percentage = 3%

Treatment Train #10 Overland flow from rooftops and pavement

TSS Removal of the proposed drainage = 0% Site impervious percentage = 3%

Total weighted TSS Removal rate = [(7% * 85%) + (5% * 85%) + (8% * 85%) + (25% * 85%) + (10% * 85%) + (28% * 85%) + (8% * 85%) + (3% * 85%) + (3% * 25%) + (3% * 0%)

$$= 5.9 + 4.3 + 6.8 + 21.3 + 8.5 + 23.8 + 6.8 + 2.6 + 0.8 + 0 = 80.8\% > 80\%$$
, OK



Standard 5: Land uses with higher potential pollutant loads.

The development is not considered a land use that generally produces higher potential pollutant loads.

Standard 6: Stormwater discharges to critical areas

There are three potential vernal pools located on the property. Potential vernal pool #1 is located on the eastern side of the property just north of Pocket Wetland #1 and adjacent to the railroad. Potential vernal pool #2 is isolated and located just to the north of PVP#1, also adjacent to the railroad. Potential vernal pool #3 is located on the southern side of the parcel to the south of both wetland crossings. A pocket wetland is proposed to outlet more than 180' upslope from potential vernal pool #1. All treatment trains that discharge to critical areas have been designed to meet the pretreatment requirement of 44% TSS removal prior to entering the treatment system such as an Infiltration Pond or Pocket Wetland.

Standard 7: Redevelopment projects

The project is not considered a redevelopment project.

Standard 8: Control construction-related impacts

The project will install erosion and sediment controls prior to any earthwork activity. Erosion control barriers will be placed down slope from the proposed construction to prevent erosion and sedimentation into the surrounding areas. The barriers will be maintained and inspected periodically during construction; sediment buildup will be removed, and any damaged barrier will be replaced as needed. See site plan and SWPPP.

Standard 9: Long-term operation and maintenance plan

See Appendix A for the operation and maintenance requirements of the stormwater management system.

Standard 10: No illicit discharges

An illicit discharge compliance statement has been provided by the property owner under separate cover.



Appendix A: Test Pit Information and Falling Head Permeability Test Results

MASTER LOG - TEST PIT INFORMATION





Test Pits Performed on 12/4/2019, 12/5/2019, 12/6/2019, 01/09/2020, 10/20/2020, 10/21/2020 Test Pits Performed By Kasey Ferreira, E.I.T.

Test Pits Witnessed By Chris Johnson, Town of Walpole

TP-1 (Drainage)			
0"-13"	Ap	Loam/Organics	
13"-23"	Bw	Sandy Loam	
23"-120"	С	Loamy Sand	
Mottles at 24"			
HSG C			

TP-2 (Drainage)				
0"-10"	Ap	Loam/Organics		
10"-118"	С	Loamy Sand		
Weeping at 71", Mottles at 33"				
HSG B				

TP-3 (Drainage)				
0"-12"	Ap	Loam		
12"-20"	Bw	Sandy Loam		
20"-98"	С	Loamy Sand		
Standing at 94", Mottles at 31"				
HSG C				

114 TURNPIKE ROAD, SUITE 2C | CHELMSFORD, MASSACHUSETTS 01824 | 617.482.7080

TP-4 (Drainage)			
0"-11"	Ap	Loam/Organics	
11"-96"	С	Loamy Sand	
Standing at 97", Mottles at 49"			
HSG A			

TP-5 (Drainage)			
0"-12"	Ap	Loam/Organics	
12"-109"	С	Loamy Sand	
Standing at 85", Mottles at 41"			
HSG A			

TP-6 (Drainage)			
0"-11"	Ap	Loam/Organics	
11"-20"	Bw	Sandy Loam	
20"-99"	С	Loamy Sand	
Standing at 60", Mottles at 26"			
HSG C			

TP-7 (Drainage)				
0"-7"	Ap	Loam		
7"-122"	С	Loamy Sand		
Weeping at 103", Mottles at 40"				
HSG B				

TP-8 (Drainage)			
0"-7"	Ap	Loam	
7"-18"	Bw	Sandy Loam	
18"-139"	С	Loamy Sand	
Standing at 130", Weeping at 125", Mottles at 44"			
HSG B			

TP-9 (Drainage)				
0"-40"	Fill			
40"-117"	С	Medium Sand		
Weeping at 34", Mottles at 40"				
HSG A				

TP-10 (Drainage)		
0"-9"	Ap	Loam
9"-23"	B_w	Sandy Loam
23"-96"	С	Coarse Sand
Standing at 96", Mottles at 34"		
HSG C		

TP-11 (Drainage)		
0"-8"	Ap	Loam
8"-17"	Bw	Loamy Sand
17"-122"	С	Medium Sand
Standing at 115", Weeping at 46", Mottles at 30"		
HSG B		

TP-12 (Drainage)		
0"-9"	Ap	Loam
9"-18"	B _w	Loamy Sand
18"-120"	С	Medium Sand
Weeping at 24", Mottles at 29"		
HSG B		

TP-13 (Drainage)		
0"-13"	Ap	Loam
13"-28"	Bw	Sandy Loam
28"-135"	С	Loamy Sand
Weeping at 115", Mottles at 43"		
HSG B		

TP-14 (Drainage)		
0"-7"	Ap	Loam
7"-13"	Bw	Loamy Fine Sand
13"-95"	C ₁	Coarse Sand
95"-120"	C ₂	Gravel
Standing at 104", Mottles at 95"		
HSG A		

TP-15 (Drainage)		
0"-5"	Ap	Loam
5"-22"	Bw	Sandy Loam
22"-120"	С	Loamy Sand
Weeping at 30"		
HSG C		

TP-16 (Drainage)		
0"-5"	Ap	Loam
5"-20"	Bw	Sandy Loam
20"-120"	С	Loamy Sand
Standing at 96", Mottles at 36"		
HSG C		

TP-17 (Drainage)			
0"-9"	Ap	Loam/Organics	
9"-108"	С	Loamy Sand	
Weeping at 18"			
HSG A/D (D)			

TP-18 (Drainage)		
0"-8"	Ap	Loam
8"-17"	B _w	Sandy Loam
17"-120"	С	Loamy Sand
Weeping at 29", Mottles at 26"		
HSG C		

TP-19 (Building)		
0"-8"	Ap	Loam
8"-20"	B _w	Loamy Sand
20"-120"	С	Sand
Mottles at 50"		
HSG A		

TP-20 (Building)		
0"-7"	Ap	Loam
7"-24"	Bw	Sandy Loam
24"-103"	С	Loamy Sand
Mottles at 48"		
HSG B		

TP-21 (Exploratory)		
0"-9"	Ap	Loam
9"-24"	Bw	Loamy Sand
24"-102"	С	Sand
Mottles at 43"		
HSG A		

TP-22 (Exploratory)		
0"-6"	Ap	Loam
6"-23"	Bw	Sandy Loam
23"-66"	С	Loamy Sand
Mottles at 32"		
HSG C		

TP-23 (Building)		
0"-12"	Ap	Loam
12"-24"	Bw	Loamy Sand
24"-118"	С	Sand
Standing at 96", Mottles at 36"		
HSG B		

TP-24 (Building)			
0"-11"	Ap	Loam	
11"-24"	Bw	Loamy Sand	
24"-102"	С	Sand	
Weeping at 100", Mottles at 39", Refusal at 102"			
HSG B			



TP-25 (Building)		
0"-10"	Ар	Loam
10"-20"	Bw	Loamy Sand
20"-69"	С	Sand
Mottles at 41", Refusal at 69"		
HSG A		

TP-25A (Exploratory)		
0"-8"	Ap	Loam
8"-15"	Bw	Sandy Loam
15"-108"	С	Loamy Sand
Mottles at 32"		
HSG C		

	TP-26 (Building)		
0"-10"	Ap	Loam	
10"-18"	Bw	Sandy Loam	
18"-75"	С	Loamy Sand	
Mottles at 49", Refusal at 75"			
HSG B			

TP-27 (Building)		
0"-8"	Ap	Loam
8"-25"	Bw	Sandy Loam
25"-48"	C ₁	Loamy Sand
48"-110	C ₂	Loamy Sand
Standing at 100", Weeping at 54", Mottles at 30"		
HSG C		

TP-28 (Building)		
0"-6"	Ap	Loam
6"-24"	Bw	Sandy Loam
24"-99"	С	Loamy Sand
Standing at 90", Weeping at 65", Mottles at 32"		
HSG C		

TP-29 (Exploratory)		
0"-13"	Ap	Loam
13"-18"	Bw	Sandy Loam
18"-132"	С	Loamy Sand
Mottles at 43"		
HSG B		



TP-30 (Exploratory)		
0"-12"	Ap	Loam/Organics
12"-30"	Bw	Loam
30"-128"	С	Loamy Sand
Weeping at 102", Mottles at 36"		
HSG C		

TP-31 (Exploratory)			
0"-32"	Fill		
32"-96"	С	Gravelly Loamy sand	
Mottles at 42"			
HSG A			

TP-32 (Drainage)		
0"-14"	А	Sandy Loam
14"-20"	В	Sandy Loam
20"-88"	С	Sandy Loam
Mottles at 30", No Standing		
HSG C		

TP-33 (Drainage)		
0"-12"	А	Sandy Loam
12"-30"	В	Sandy Loam
30"-87"	С	Loamy Sand
Mottles at 30"		
HSG C		

TP-34 (Drainage)		
0"-10"	А	Sandy Loam
10"-24"	Bw	Sandy Loam
24"-72"	С	Loamy Sand
Mottles at 37"		
HSG C		

TP-36 (Drainage)		
0"-10"	А	Sandy Loam
10"-22"	Bw	Sandy Loam
22"-62"	С	Sandy Loam
Mottles at 21"		
HSG C		

TP-37 (Drainage)		
0"-11"	А	Sandy Loam
11"-28"	Bw	Sandy Loam
28"-52"	С	Sandy Loam
Seasonal high at 28"		
HSG C		



 TP-38 (Drainage)

 0"-12"
 A
 Sandy Loam

 12"-28"
 B
 Sandy Loam

 28"-72"
 C
 Sand

 Mottles at 42"
 HSG B

TP-39 (Drainage)		
0"-14"	А	Sandy Loam
14"-37"	В	Sandy Loam
37"-66"	С	Loamy Sand
Mottles at 36"		
HSG C		

TP-40 (Drainage)		
0"-14"	А	Sandy Loam
14"-30"	Bw	Sandy Loam
30"-59"	C1	Sand
59"-98"	C2	Loamy Sand
Seasonal high at 28"		
HSG C		

TP-40A (Drainage)			
0"-14"	А	Sandy Loam	
14"-23"	Bw	Sandy Loam	
23"-80"	С	Sand	
Seasonal high at 40"			
HSG C			

TP-41 (Drainage)			
0"-9"	А	Sandy Loam	
9"-20"	В	Sandy Loam	
20"-88"	С	Sand	
Seasonal high at 45"			
HSG B			

TP-42 (Drainage)			
0"-10"	Α	Sandy Loam	
10"-28"	Bw	Sandy Loam	
28"-86"	С	Sand	
Seasonal high at 48", presence of color change			
HSG B			



TP-43 (Drainage)			
0"-10"	А	Sandy Loam	
10"-26"	Bw	Sandy Loam	
26"-64"	C1	Sandy Loam	
64"-100"	C2	Loamy Sand	
Mottles at 26"			
HSG C			

TP-43A (Drainage)			
0"-10"	А	Sandy Loam	
10"-19"	Bw	Sandy Loam	
19"-89"	С	Sandy Loam	
Mottles at 16"			
HSG B/D (B)			

TP-44 (Drainage)			
0"-10"	А	Sandy Loam	
10"-35"	Bw	Sandy Loam	
35"-52"	C1	Sandy Loam	
52"-76"	C2	Loamy Sand	
Seasonal high at 35"			
HSG C			

TP-45 (Drainage)			
0"-12"	А	Sandy Loam	
12"-27"	Bw	Sandy Loam	
27"-56"	C1	Sandy Loam	
56"-91"	C2	Loamy Sand	
TBD			

TP-46 (Drainage)			
0"-12"	А	Sandy Loam	
12"-27"	Bw	Sandy Loam	
27"-52"	C1	Sandy Loam	
52"-100"	C2	Loamy Sand	
Seasonal high at 18"			
HSG B/D (B)			

TP-47 (Drainage)			
0"-12"	А	Sandy Loam	
12"-34"	Bw	Sandy Loam	
34"-48"	C1	Sandy Loam	
48"-102"	C2	Loamy Sand	
Seasonal high at 30"			
HSG C			



TP-48 (Drainage)			
0"-12"	А	Sandy Loam	
12"-29"	Bw	Sandy Loam	
29"-80"	С	Loamy Sand	
Seasonal high at 36"			
HSG C			

TP-49 (Drainage)			
0"-10"	Α	Sandy Loam	
10"-24"	Bw	Sandy Loam	
24"-60"	С	Loamy Sand	
Refusal at 60". No seasonal high present.			
HSG B			

TP-50 (Drainage)			
0"-12"	Α	Sandy Loam	
12"-25"	Bw	Sandy Loam	
25"-67"	С	Loamy Sand	
No seasonal high.			
HSG B			

TP-51 (Drainage)							
0"-12"	А	Sandy Loam					
12"-34"	В	Sandy Loam					
34"-65"	С	Loamy Sand					
Seasonal high at 21	33						
HSG B/D							

TP-52 (Drainage)							
0"-10"	А	Sandy Loam					
10"-28"	Bw	Sandy Loam					
28"-72"	С	Loamy Sand					
Seasonal high at 53)"						
HSG B							

	TP-53 (Drainage)							
0"-10"	А	Sandy Loam						
10"-32"	В	Sandy Loam						
32"-78"	С	Sandy Loam						
Seasonal high at 32	,,,							
HSG C								



TP-2A (Drainage)							
0"-14"	А	Sandy Loam					
14"-28"	Bw	Sandy Loam					
28"-72"	С	Sandy Loam					
Mottles at 25"							
HSG C							

HSG-1 (Drainage)							
0"-10"	А	Sandy Loam					
10"-28"	Bw	Sandy Loam					
28"-32"	С	Loamy Sand					
Refusal at 32". No s	easonal high.						
HSG C							

	HSG-2 (Drainage)							
0"-10"	А	Sandy Loam						
10"-30"	Bw Sandy Loam							
30"-42"	С	Loamy Sand						
Seasonal high at 32	,,,							
HSG C								

HSG-3 (Drainage)							
0"-10"	А	Sandy Loam					
10"-22"	Bw	Sandy Loam					
22"-41"	С	Sandy Loam					
Seasonal high at 34							
HSG C							

| 19 |

Falling Head Permeability Test

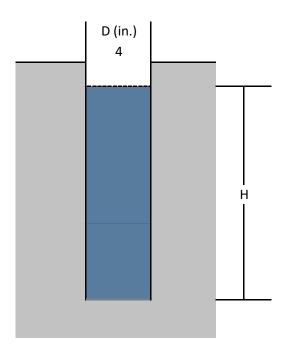
Project: Summer Street, Walpole

Location: OTH 1
Test Date: 28-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 8" below existing grade



	н	Т	H ₁ /H ₂	t ₂ -t ₁				
	(inches)	(seconds)	(inches)	(seconds)	In(H	H ₁ /H ₂)	k (in/l	nr)
$\frac{\pi D}{11(t_2-t_4)} \ln(H_1/H_2)$	24	0	n/a	n/a				
11(t ₂ -t ₁) ""("1,""2)	23	600	1.04	600	0.	043	0.3	
	22	1320	1.05	720	0.	044	0.3	
Ref: Fig. 19.3	21	2340	1.05	1020	0.	047	0.2	
Lambe and Whitman,	20	3360	1.05	1020	0.	049	0.2	
Soil Mechanics,1969	19	4440	1.05	1080	0.	051	0.2	
Falling Head	18	5460	1.06	1020	0.	054	0.2	
4" Sch. 40 PVC Test Pipe		Average		0.22	in/hr			

Average 0.22 in/hr
Safety Factor 2
Design K 0.11 in/hr

Falling Head Permeability Test

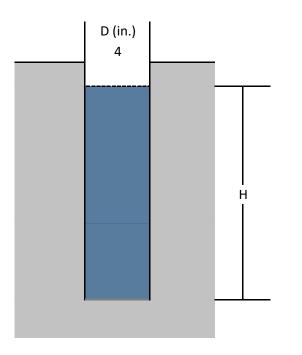
Project: Summer Street, Walpole

Location: OTH 2A
Test Date: 28-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 8" below existing grade



	н	Т		H ₁ /H ₂	t_2 - t_1			
	(inches)	(seconds)		(inches)	(seconds)		$ln(H_1/H_2)$	
$k = \frac{\pi D}{11(H_1)} \ln(H_1/H_2)$	24	0		n/a	n/a			
11(t ₂ -t ₁) ""("1,""2)	23	300		1.04	300		0.043	
	22	660		1.05	360		0.044	
Ref: Fig. 19.3	21	1140		1.05	480		0.047	
Lambe and Whitman,	20	1740		1.05	600		0.049	
Soil Mechanics,1969	19	2340		1.05	600		0.051	
Falling Head	18	3060		1.06	720		0.054	
4" Sch. 40 PVC Test Pipe	Average 0.41 in/hr							

Average 0.41 in/hr
Safety Factor 2

Design K 0.21 in/hr

k (in/hr)

0.6 0.5 0.4 0.3 0.4 0.3

Falling Head Permeability Test

Project: Summer Street, Walpole

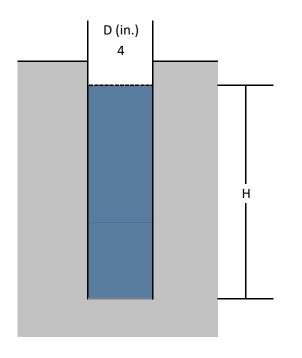
Location: OTH 32
Test Date: 20-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 32" below existing grade

4" Sch. 40 PVC Test Pipe



	Н	Т	H ₁ /H ₂	t_2 - t_1			
	(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)	
$k = \frac{\pi D}{11(t_2-t_1)} \ln(H_1/H_2)$	24	0	n/a	n/a			
11(t ₂ -t ₁) (((1))	23	90	1.04	90	0.043	1.9	
	22	180	1.05	90	0.044	2.0	
Ref: Fig. 19.3	21	285	1.05	105	0.047	1.8	
Lambe and Whitman,	20	390	1.05	105	0.049	1.9	
Soil Mechanics,1969	19	570	1.05	180	0.051	1.2	
Falling Head	18	690	1.06	120	0.054	1.9	

Average 1.79 in/hr
Safety Factor 2
Design K 0.89 in/hr

Falling Head Permeability Test

Project: Summer Street, Walpole

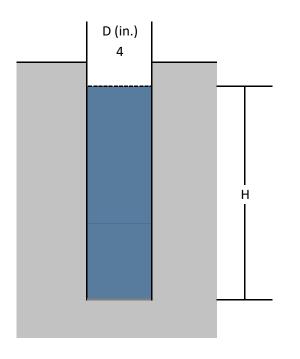
Location: OTH 33
Test Date: 20-Oct-20

By: Daniel J. Merrikin, P.E./Kasey Ferreira

Test apparatus

4" Sch. 40 PVC Test Pipe

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 37" below existing grade



		Н	Т	H_1/H_2	t_2 - t_1			
	(ir	ches)	(seconds)	(inches)	(seconds)	$ln(H_1/H_2)$	k (in/hr)	
$K = \frac{\pi D}{44(H_1 + h_2)} \ln(H_1/H_2)$		24	0	n/a	n/a			
11(t ₂ -t ₁) ""("11/")"		23	600	1.04	600	0.043	0.3	
		22	1740	1.05	1140	0.044	0.2	
Ref: Fig. 19.3		21	2820	1.05	1080	0.047	0.2	
Lambe and Whitman,		20	4140	1.05	1320	0.049	0.2	
Soil Mechanics,1969		19	5580	1.05	1440	0.051	0.1	
Falling Head		18	7140	1.06	1560	0.054	0.1	

Average 0.18 in/hr
Safety Factor 2
Design K 0.09 in/hr

Falling Head Permeability Test

Project: Summer Street, Walpole

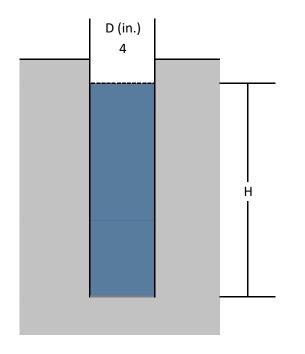
Location: OTH 34
Test Date: 20-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

4" Sch. 40 PVC Test Pipe

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 32" below existing grade



	Н	Т	H ₁ /H ₂	t ₂ -t ₁			
	(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)	
$k = \frac{\pi D}{11(t_2 - t_1)} \ln(H_1/H_2)$	24	0	n/a	n/a			
11(t ₂ -t ₁)	23	300	1.04	300	0.043	0.6	
	22	600	1.05	300	0.044	0.6	
Ref: Fig. 19.3	21	900	1.05	300	0.047	0.6	
Lambe and Whitman,	20	1260	1.05	360	0.049	0.6	
Soil Mechanics,1969	19	1620	1.05	360	0.051	0.6	
Falling Head	18	1980	1.06	360	0.054	0.6	

Average 0.60 in/hr
Safety Factor 2
Design K 0.30 in/hr

Falling Head Permeability Test

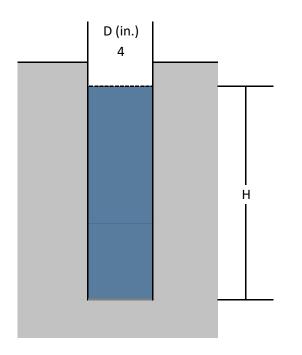
Project: Summer Street, Walpole

Location: OTH 16
Test Date: 20-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 32" below existing grade



	н	Т	H ₁ /H ₂	t ₂ -t ₁			
	(inch	es) (seconds)	(inches)	(seconds)		In(H ₁ /H ₂)	k (in/hr)
$K = \frac{\pi D}{14(1+1)} \ln(H_1/H_2)$	24	0	n/a	n/a			
11(t ₂ -t ₁)	23	300	1.04	300		0.043	0.6
	22	660	1.05	360		0.044	0.5
Ref: Fig. 19.3	21	1140	1.05	480		0.047	0.4
Lambe and Whitman,	20	1680	1.05	540		0.049	0.4
Soil Mechanics,1969	19	2160	1.05	480		0.051	0.4
Falling Head	18	2700	1.06	540		0.054	0.4
4" Sch. 40 PVC Test Pipe		Average		0.45	in/	hr	

Safety Factor

Design K

2

0.23 in/hr

Falling Head Permeability Test

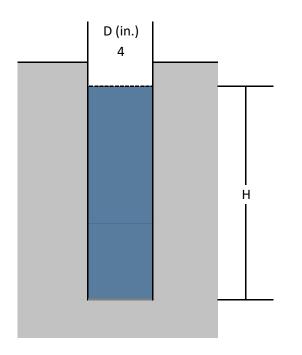
Project: Summer Street, Walpole

Location: OTH 38
Test Date: 20-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 38" below existing grade



	Н	T	
	(inches)	(seconds)	
k=πDIn(H_/H_)	24	0	
$\frac{\pi D}{11(t_2-t_1)} \ln(H_1/H_2)$	22	18	
	20	36	
Ref: Fig. 19.3	18	56	
Lambe and Whitman,	16	81	
Soil Mechanics,1969	14	107	
Falling Head	12	137	
4" Sch. 40 PVC Test Pine		Average	

L6	81		1.13	25		0.118
L4	107		1.14	26		0.134
L2	137		1.17	30		0.154
	Average			20.8	in/	hr
	Safety Fact	or		2		

 H_1/H_2

n/a 1.09

1.10

1.11

Design K

 t_2 - t_1

n/a

18

18

20

10.41 in/hr

(inches) (seconds)

In(H₁/H₂)

0.087

0.095

0.105

k (in/hr)

19.9

21.8 21.7

19.4 21.1 21.1

Falling Head Permeability Test

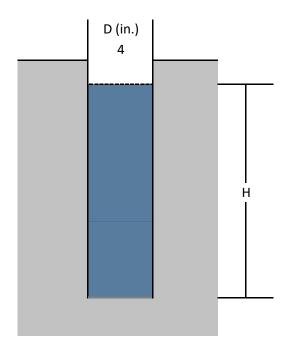
Project: Summer Street, Walpole

Location: OTH 40
Test Date: 20-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 32" below existing grade



	(inches)	(seco
k=πDIn(H_/H_)	24	0
$\frac{k = \frac{\pi D}{11(t_2 - t_1)} \ln(H_1/H_2)}{11(t_2 - t_1)}$	22	33
	20	75
Ref: Fig. 19.3	18	11
Lambe and Whitman,	16	16
Soil Mechanics,1969	14	21
Falling Head	12	27
4" Sch 40 PVC Test Pine		Avera

Н	Т	H_1/H_2	t_2 - t_1			
(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)	
24	0	n/a	n/a			
22	33	1.09	33	0.087	10.8	
20	75	1.10	42	0.095	9.3	
18	118	1.11	43	0.105	10.1	
16	166	1.13	48	0.118	10.1	
14	219	1.14	53	0.134	10.4	
12	277	1.17	58	0.154	10.9	

Average 10.3 in/hr
Safety Factor 2
Design K 5.13 in/hr

Falling Head Permeability Test

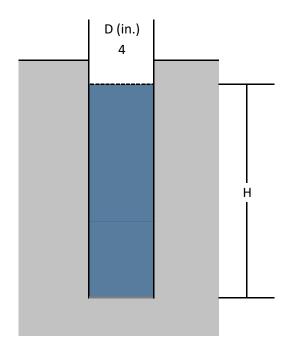
Project: Summer Street, Walpole

Location: OTH 40A
Test Date: 20-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 42" below existing grade



	н	Т	H ₁ /H ₂	t ₂ -t ₁				
	(inches)	(seconds)	(inches)	(seconds)		In(H ₁ /H ₂)	k (in/hr)	
$k = \frac{\pi D}{\ln(H_1/H_2)}$	24	0	n/a	n/a				
11(t ₂ -t ₁) (((1))	22	34	1.09	34		0.087	10.5	
	20	60	1.10	26		0.095	15.1	
Ref: Fig. 19.3	18	93	1.11	33		0.105	13.1	
Lambe and Whitman,	16	129	1.13	36		0.118	13.4	
Soil Mechanics,1969	14	171	1.14	42		0.134	13.1	
Falling Head	12	217	1.17	46		0.154	13.8	
4" Sch. 40 PVC Test Pipe		Average		13.2	in/l	nr		

Average 13.2 in/hr
Safety Factor 2
Design K 6.58 in/hr

Falling Head Permeability Test

Project: Summer Street, Walpole

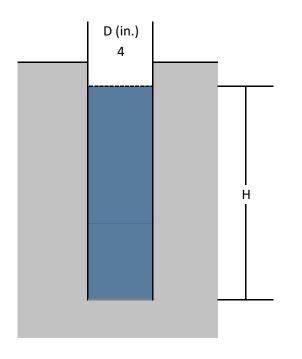
Location: OTH 41
Test Date: 20-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

4" Sch. 40 PVC Test Pipe

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 42" below existing grade



	Н	T	H_1/H_2	t ₂ -t ₁			
	(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)	
$k = \frac{\pi D}{11(1+1)} \ln(H_1/H_2)$	24	0	n/a	n/a			
11(t ₂ -t ₁)	22	15	1.09	15	0.087	23.8	
	20	37	1.10	22	0.095	17.8	
Ref: Fig. 19.3	18	63	1.11	26	0.105	16.7	
Lambe and Whitman,	16	102	1.13	39	0.118	12.4	
Soil Mechanics,1969	14	145	1.14	43	0.134	12.8	
Falling Head	12	185	1.17	40	0.154	15.8	

Average 16.6 in/hr
Safety Factor 2
Design K 8.28 in/hr

Falling Head Permeability Test

Project: Summer Street, Walpole

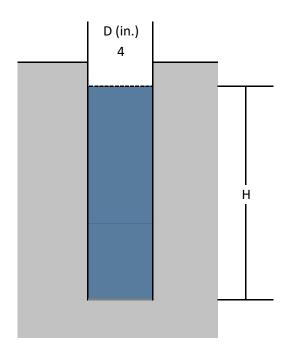
Location: OTH 42 (in C1)
Test Date: 20-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

4" Sch. 40 PVC Test Pipe

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 38" below existing grade



	Н	Т	H ₁ /H ₂	t_2 - t_1			
	(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)	
$\kappa = \frac{\pi D}{14(1+1)} \ln(H_1/H_2)$	24	0	n/a	n/a			
11(t ₂ -t ₁) ""("1)" 11	22	56	1.09	56	0.087	6.4	
	20	114	1.10	58	0.095	6.8	
Ref: Fig. 19.3	18	175	1.11	61	0.105	7.1	
Lambe and Whitman,	16	251	1.13	76	0.118	6.4	
Soil Mechanics,1969	14	352	1.14	101	0.134	5.4	
Falling Head	12	440	1.17	88	0.154	7.2	

Average 6.5 in/hr
Safety Factor 2
Design K 3.27 in/hr

Falling Head Permeability Test

Project: Summer Street, Walpole **Location:** OTH 42 (in C2) (remove C1)

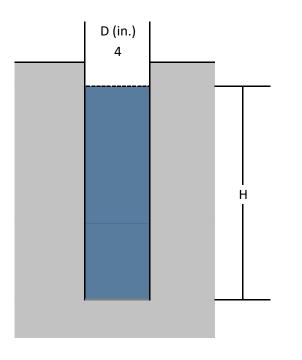
Test Date: 21-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

4" Sch. 40 PVC Test Pipe

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 52" below existing grade



	Н	T	H ₁ /H ₂	t ₂ -t ₁		
	(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)
$k = \frac{\pi D}{11(H_1/H_2)} \ln(H_1/H_2)$	24	0	n/a	n/a		
11(t ₂ -t ₁) (((11)/(12)/(12)/(12)/(12)/(12)/(12)/(12	22	20	1.09	20	0.087	17.9
	20	38	1.10	18	0.095	21.8
Ref: Fig. 19.3	18	61	1.11	23	0.105	18.8
Lambe and Whitman,	16	88	1.13	27	0.118	17.9
Soil Mechanics,1969	14	110	1.14	22	0.134	24.9
Falling Head	12	133	1.17	23	0.154	27.5

Average 21.5 in/hr
Safety Factor 2

Design K 10.74 in/hr

Falling Head Permeability Test

Project: Summer Street, Walpole **Location:** OTH 43 (in C2) (remove C1)

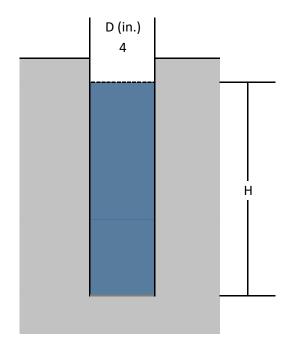
Test Date: 21-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

4" Sch. 40 PVC Test Pipe

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 74" below existing grade



	Н	T	H ₁ /H ₂	t ₂ -t ₁			
	(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)	
$k = \frac{\pi D}{11(1+1)} \ln(H_1/H_2)$	24	0	n/a	n/a			
11(t ₂ -t ₁)	23	75	1.04	75	0.043	2.3	
	22	164	1.05	89	0.044	2.1	
Ref: Fig. 19.3	21	390	1.05	226	0.047	0.8	
Lambe and Whitman,	20	600	1.05	210	0.049	1.0	
Soil Mechanics,1969	19	870	1.05	270	0.051	0.8	
Falling Head	18	1110	1.06	240	0.054	0.9	

Average 1.3 in/hr
Safety Factor 2
Design K 0.66 in/hr

Falling Head Permeability Test

Project: Summer Street, Walpole **Location:** OTH 44 (in C2) (remove C1)

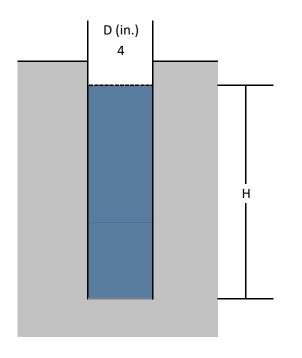
Test Date: 21-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

4" Sch. 40 PVC Test Pipe

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 56" below existing grade



	н	T	H ₁ /H ₂	t ₂ -t ₁			
	(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)	
$k = \frac{\pi D}{11(t_2 - t_1)} \ln(H_1/H_2)$	24	0	n/a	n/a			
11(t ₂ -t ₁)	22	23	1.09	23	0.087	15.6	
	20	48	1.10	25	0.095	15.7	
Ref: Fig. 19.3	18	75	1.11	27	0.105	16.0	
Lambe and Whitman,	16	98	1.13	23	0.118	21.1	
Soil Mechanics,1969	14	120	1.14	22	0.134	24.9	
Falling Head	12	165	1.17	45	0.154	14.1	

Average 17.9 in/hr
Safety Factor 2

Design K 8.95 in/hr

Falling Head Permeability Test

Project: Summer Street, Walpole **Location:** OTH 45 (in C2) (remove C1)

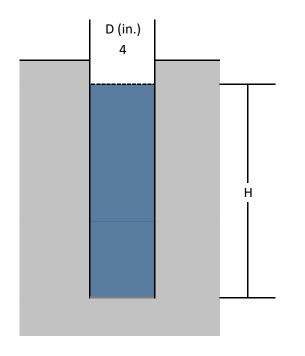
Test Date: 21-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

4" Sch. 40 PVC Test Pipe

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 62" below existing grade



	Н	Т	H_1/H_2	t_2 - t_1			
	(inches)	(seconds)	(inches)	(seconds)	$ln(H_1/H_2)$	k (in/hr)	
$k = \frac{\pi D}{14(1+1)} \ln(H_1/H_2)$	20	0	n/a	n/a			
11(t ₂ -t ₁) ""(11/112)	19	390	1.05	390	0.051	0.5	
	18	690	1.06	300	0.054	0.7	
Ref: Fig. 19.3	17	1050	1.06	360	0.057	0.7	
Lambe and Whitman,	16	1410	1.06	360	0.061	0.7	
Soil Mechanics,1969	15	1770	1.07	360	0.065	0.7	
Falling Head	14	2130	1.07	360	0.069	0.8	

Average 0.7 in/hr
Safety Factor 2
Design K 0.35 in/hr

Falling Head Permeability Test

Project: Summer Street, Walpole **Location:** OTH 46 (in C2) (remove C1)

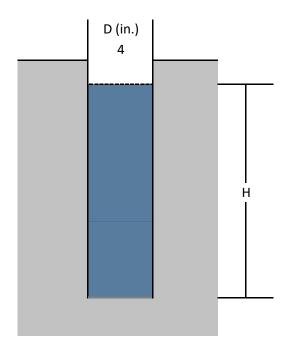
Test Date: 21-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

4" Sch. 40 PVC Test Pipe

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 64" below existing grade



	Н	T	H_1/H_2	t ₂ -t ₁			
	(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)	
$k = \frac{\pi D}{11(1+1)} \ln(H_1/H_2)$	24	0	n/a	n/a			
11(t ₂ -t ₁)	23	120	1.04	120	0.043	1.5	
	22	300	1.05	180	0.044	1.0	
Ref: Fig. 19.3	21	480	1.05	180	0.047	1.1	
Lambe and Whitman,	20	660	1.05	180	0.049	1.1	
Soil Mechanics,1969	19	840	1.05	180	0.051	1.2	
Falling Head	18	1020	1.06	180	0.054	1.2	

Average 1.2 in/hr
Safety Factor 2

Design K 0.59 in/hr

Falling Head Permeability Test

Project: Summer Street, Walpole **Location:** OTH 47 (in C2) (remove C1)

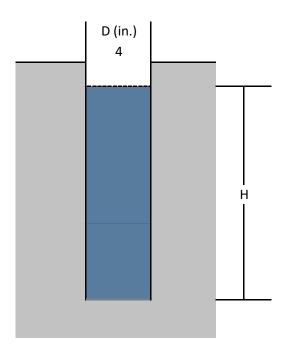
Test Date: 21-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

4" Sch. 40 PVC Test Pipe

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 72" below existing grade



	Н	Т	H_1/H_2	t_2 - t_1			
	(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)	
$ = \frac{\pi D}{14(1+1)} \ln(H_1/H_2) $	24	0	n/a	n/a			
11(t ₂ -t ₁)	23	20	1.04	20	0.043	8.7	
	22	40	1.05	20	0.044	9.1	
Ref: Fig. 19.3	21	60	1.05	20	0.047	9.6	
Lambe and Whitman,	20	90	1.05	30	0.049	6.7	
Soil Mechanics,1969	19	130	1.05	40	0.051	5.3	
Falling Head	18	175	1.06	45	0.054	4.9	

Average 7.4 in/hr
Safety Factor 2
Design K 3.69 in/hr

Falling Head Permeability Test

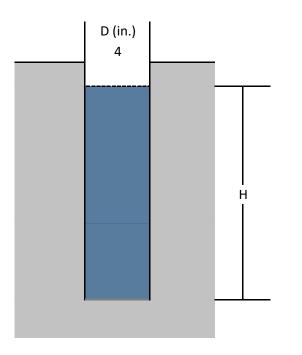
Project: Summer Street, Walpole

Location: OTH 48
Test Date: 21-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 48" below existing grade



k=	$\frac{\pi D}{11(t_2-t_1)} \ln(H_1/H_2)$
	Ref: Fig. 19.3 Lambe and Whitman, Soil Mechanics,1969 Falling Head
	4" Sch. 40 PVC Test Pipe

H (inches)	T (seconds)	H ₁ /H ₂ (inches)	t ₂ -t ₁ (seconds)	In(H ₁ /H ₂)	k (in/hr)	
24	0	n/a	n/a			
23	7	1.04	7	0.043	25.0	
22	15	1.05	8	0.044	22.8	
21	35	1.05	20	0.047	9.6	
20	59	1.05	24	0.049	8.4	
19	105	1.05	46	0.051	4.6	
18	145	1.06	40	0.054	5.6	

Average 12.6 in/hr
Safety Factor 2

Design K 6.32 in/hr

Falling Head Permeability Test

Project: Summer Street, Walpole

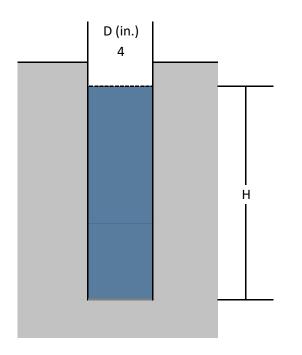
Location: OTH 49
Test Date: 21-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

4" Sch. 40 PVC Test Pipe

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 32" below existing grade



	Н	T	H_1/H_2	t ₂ -t ₁			
	(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)	
$k = \frac{\pi D}{11(H_1/H_2)}$	24	0	n/a	n/a			
11(t ₂ -t ₁)	23	35	1.04	35	0.043	5.0	
	22	65	1.05	30	0.044	6.1	
Ref: Fig. 19.3	21	100	1.05	35	0.047	5.5	
Lambe and Whitman,	20	135	1.05	35	0.049	5.7	
Soil Mechanics,1969	19	165	1.05	30	0.051	7.0	
Falling Head	18	205	1.06	40	0.054	5.6	

Average 5.8 in/hr
Safety Factor 2

Design K 2.91 in/hr

Falling Head Permeability Test

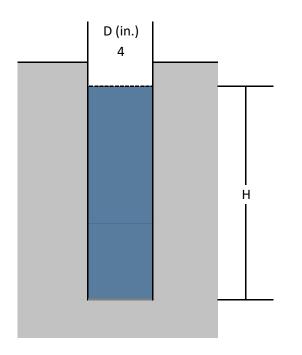
Project: Summer Street, Walpole

Location: OTH 50
Test Date: 21-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 42" below existing grade



	Н	T	H_1/H_2	t ₂ -t ₁			
	(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)	
$k = \frac{\pi D}{44(1+1)} \ln(H_1/H_2)$	24	0	n/a	n/a			
11(t ₂ -t ₁) (((1))	23	120	1.04	120	0.043	1.5	
	22	285	1.05	165	0.044	1.1	
Ref: Fig. 19.3	21	450	1.05	165	0.047	1.2	
Lambe and Whitman,	20	630	1.05	180	0.049	1.1	
Soil Mechanics,1969	19	820	1.05	190	0.051	1.1	
Falling Head	18	1010	1.06	190	0.054	1.2	
4" Sch. 40 PVC Test Pipe		Average		1.2 i	n/hr		

Average 1.2 in/hr
Safety Factor 2

Design K 0.59 in/hr

Falling Head Permeability Test

Project: Summer Street, Walpole **Location:** OTH 51 (in C2) (remove C1)

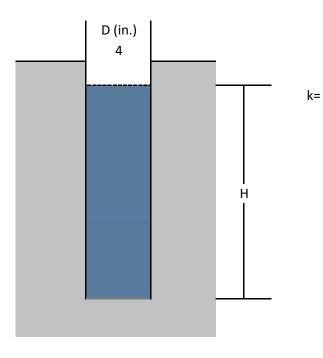
Test Date: 21-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

4" Sch. 40 PVC Test Pipe

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 58" below existing grade



	Н	Т	H ₁ /H ₂	t_2 - t_1			
	(inches)	(seconds)	(inches)	(seconds)	$ln(H_1/H_2)$	k (in/hr)	
$= \frac{\pi D}{11(t_2-t_1)} \ln(H_1/H_2)$	24	0	n/a	n/a			
11(t ₂ -t ₁)	23	45	1.04	45	0.043	3.9	
	22	85	1.05	40	0.044	4.6	
Ref: Fig. 19.3	21	150	1.05	65	0.047	2.9	
Lambe and Whitman,	20	240	1.05	90	0.049	2.2	
Soil Mechanics,1969	19	375	1.05	135	0.051	1.6	
Falling Head	18	510	1.06	135	0.054	1.6	

Average 2.8 in/hr
Safety Factor 2

Design K 1.40 in/hr

Falling Head Permeability Test

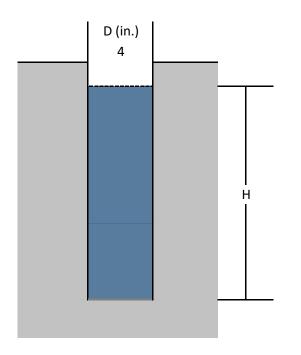
Project: Summer Street, Walpole

Location: OTH 52
Test Date: 21-Oct-20

By: Daniel J. Merrikin, P.E.

Test apparatus

24" long x 4" diameter schedule 40 pvc pipe Bottom of pipe set 42" below existing grade



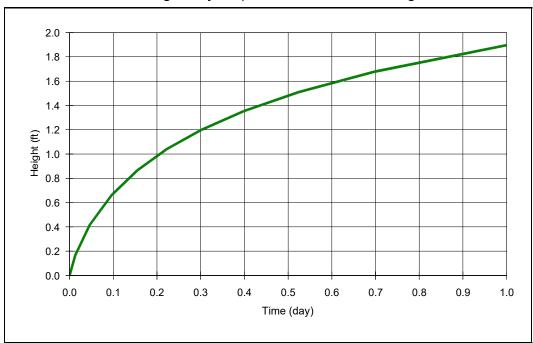
k=	$\frac{\pi D}{11(t_2-t_1)} \ln(H_1/H_2)$
	Ref: Fig. 19.3 Lambe and Whitman, Soil Mechanics,1969 Falling Head
	4" Sch. 40 PVC Test Pipe

н	Т	H_1/H_2	t ₂ - t ₁			
(inches)	(seconds)	(inches)	(seconds)	In(H ₁ /H ₂)	k (in/hr)	
24	0	n/a	n/a			
23	30	1.04	30	0.043	5.8	
22	55	1.05	25	0.044	7.3	
21	85	1.05	30	0.047	6.4	
20	110	1.05	25	0.049	8.0	
19	140	1.05	30	0.051	7.0	
18	170	1.06	30	0.054	7.4	

Average 7.0 in/hr
Safety Factor 2
Design K 3.50 in/hr



Appendix B: Mounding Analysis



COMPANY: Howard Stein Hudson

PROJECT: Infiltration Pond #1

ANALYST: Matthew Baker

DATE: 6/12/2023 TIME: 1:06:46 PM

INPUT PARAMETERS

Application rate: 5.92 c.ft/day/sq. ft Duration of application: 1 day
Total simulation time: 1 day
Fillable porosity: 0.4

Hydraulic conductivity: 90 ft/day Initial saturated thickness: 35 ft Length of application area: 150 ft Width of application area: 29 ft No constant head boundary used Groundwater mounding @

X coordinate: 0 ft
Y coordinate: 0 ft
Total volume applied: 25752 cft

Time (day)	Mound Height (ft)
0	0
0	0.17
0	0.41
0.1	0.66
0.2	0.86
0.2	1.04
0.3	1.2
0.4	1.35
0.5	1.51
0.7	1.68
1	1.9



COMPANY: Howard Stein Hudson

PROJECT: Infiltration Pond #2

ANALYST: Matthew Baker

DATE: 6/12/2023 TIME: 1:04:22 PM

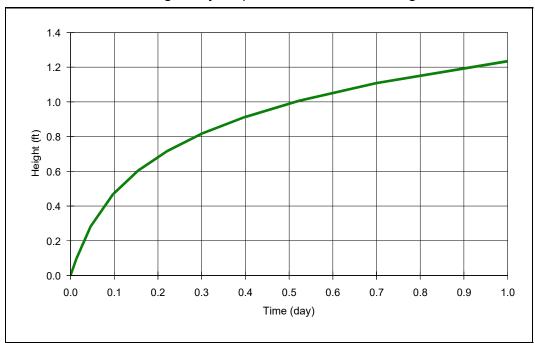
INPUT PARAMETERS

Application rate: 3.13 c.ft/day/sq. ft Duration of application: 1 day Total simulation time: 1 day Fillable porosity: 0.4 Hydraulic conductivity: 90 ft/day

Hydraulic conductivity: 90 ft/day Initial saturated thickness: 20 ft Length of application area: 165 ft Width of application area: 40 ft No constant head boundary used Groundwater mounding @

X coordinate: 0 ft Y coordinate: 0 ft Total volume applied: 20658 cft

Time (day)	Mound Height (ft)
0 0 0 0.1 0.2 0.2 0.3 0.4 0.5 0.7	0 0.1 0.3 0.53 0.73 0.92 1.1 1.28 1.48 1.69 1.97



COMPANY: Howard Stein Hudson

PROJECT: Infiltration Pond #3

ANALYST: Matthew Baker

DATE: 6/7/2023 TIME: 12:08:37 PM

INPUT PARAMETERS

Application rate: 2.93 c.ft/day/sq. ft Duration of application: 1 day Total simulation time: 1 day Fillable porosity: 0.4

Hydraulic conductivity: 70 ft/day Initial saturated thickness: 35 ft Length of application area: 77 ft Width of application area: 50 ft No constant head boundary used Groundwater mounding @

X coordinate: 0 ft
Y coordinate: 0 ft
Total volume applied: 11280.5 cft

Time (day)	Mound Height (ft)
0	0
0	0.1
0	0.28
0.1	0.47
0.2	0.6
0.2	0.72
0.3	0.82
0.4	0.91
0.5	1.01
0.7	1.11
1	1.24



COMPANY: Howard Stein Hudson

PROJECT: Stormtech #1

ANALYST: Matthew Baker

DATE: 6/7/2023 TIME: 12:17:27 PM

INPUT PARAMETERS

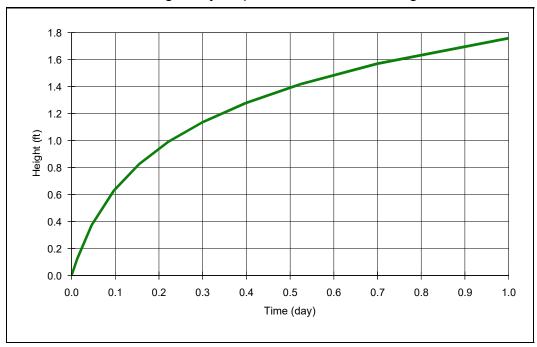
Application rate: 0.99 c.ft/day/sq. ft Duration of application: 1 day Total simulation time: 1 day Fillable porosity: 0.4 Hydraulic conductivity: 12 ft/day Initial saturated thickness: 35 ft Length of application area: 82 ft Width of application area: 67 ft

Groundwater mounding @ X coordinate: 0 ft

Y coordinate: 0 ft Total volume applied: 5439.06 cft

No constant head boundary used

Time (day)	Mound Height (ft)
0	0
0	0.03
0	0.11
0.1	0.24
0.2	0.37
0.2	0.52
0.3	0.67
0.4	0.84
0.5	1.02
0.7	1.23
1	1.52



COMPANY: Howard Stein Hudson

PROJECT: Stormtech #2

ANALYST: Matthew Baker

DATE: 6/7/2023 TIME: 12:16:03 PM

INPUT PARAMETERS

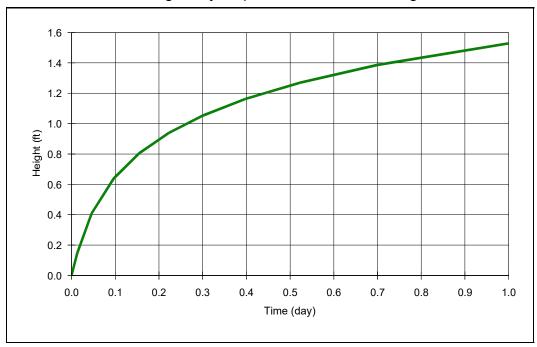
Application rate: 3.86 c.ft/day/sq. ft Duration of application: 1 day Total simulation time: 1 day Fillable porosity: 0.4

Hydraulic conductivity: 70 ft/day Initial saturated thickness: 35 ft Length of application area: 92 ft Width of application area: 48 ft No constant head boundary used Groundwater mounding @

X coordinate: 0 ft Y coordinate: 0 ft

Total volume applied: 17045.76 cft

Time (day)	Mound Height (ft)
0	0
0	0.12
0	0.37
0.1	0.63
0.2	0.83
0.2	0.99
0.3	1.14
0.4	1.28
0.5	1.42
0.7	1.57



COMPANY: Howard Stein Hudson

PROJECT: Stormtech #3

ANALYST: Matthew Baker

DATE: 6/7/2023 TIME: 12:21:05 PM

INPUT PARAMETERS

Application rate: 4.61 c.ft/day/sq. ft Duration of application: 1 day Total simulation time: 1 day Fillable porosity: 0.4 Hydraulic conductivity: 85 ft/day Initial saturated thickness: 35 ft

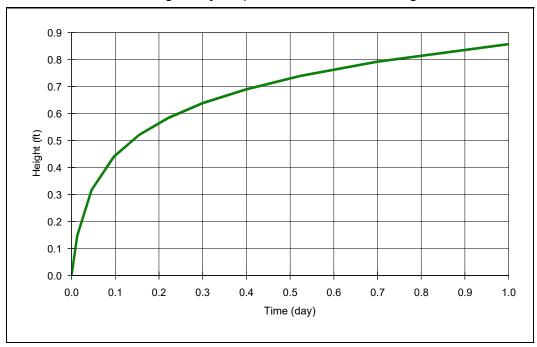
Initial saturated thickness: 35 ft Length of application area: 68 ft Width of application area: 49 ft No constant head boundary used Groundwater mounding @

X coordinate: 0 ft Y coordinate: 0 ft

Total volume applied: 15360.52 cft

Time (day)	Mound Height (ft)
0	0
0	0.15
0	0.41
0.1	0.64
0.2	8.0
0.2	0.94
0.3	1.05
0.4	1.16
0.5	1.27
0.7	1.39
1	1.53

Groundwater Mounding Analysis (Hantush's Method using Glover's Solution)



COMPANY: Howard Stein Hudson

PROJECT: Stormtech #4

ANALYST: Matthew Baker

DATE: 6/7/2023 TIME: 12:23:42 PM

INPUT PARAMETERS

Application rate: 5.42 c.ft/day/sq. ft Duration of application: 1 day Total simulation time: 1 day

Fillable porosity: 0.4

Hydraulic conductivity: 200 ft/day Initial saturated thickness: 35 ft Length of application area: 68 ft Width of application area: 44 ft No constant head boundary used Groundwater mounding @

X coordinate: 0 ft Y coordinate: 0 ft

Total volume applied: 16216.64 cft

MODEL RESULTS

Time (day)	Mound Height (ft)
0	0
0	0.15
0	0.32
0.1	0.44
0.2	0.52
0.2	0.58
0.3	0.64
0.4	0.69
0.5	0.74
0.7	0.79
1	0.86



Appendix C: Operation and Maintenance Plan



Operation and Maintenance Plan and Long-Term Pollution Prevention Plan

51-53-55 Summer Street

Walpole, Massachusetts

Prepared by:

Howard Stein Hudson 114 Turnpike Road, Suite 2C Chelmsford, MA 01824

June 2023



Contents

Эр	peration and Maintenance Plan	3
[Deep Sump Hooded Catch Basins	3
(Contech Water Quality Unit – STC 450i	5
	Subsurface Infiltration System	6
I	solator Row	8
•	Sediment Forebay	9
I	nfiltration Ponds	11
I	Pocket Wetland	13
(Outfalls and Riprap	15
I	Drip-edge System	16
Ξr	osion and Sediment Control Notes	18
Эe	eneral Construction Sequence	20
n۱	asive Vegetation Control	21
_0	ng-Term Pollution Prevention Plan	23
7	Trash and Litter Cleanup	23
I	Paved Roadway	23
I	Deep Sump Hooded Catch Basins	23
(Contech Water Quality Unit – STC 450i	24
I	Landscape Maintenance of Maintained Landscape Areas	24
•	Snow Plowing and Disposal	24
•	Subsurface Infiltration Systems	24
	solator Row	25

OPERATIONS AND MAINTENENACE PLAN





June 20, 2023

Sediment Forebays	25
Infiltration Pond	25
Pocket Wetlands	26
Outfalls and Riprap	26
Drip-edge System	27
DEP Standard 4: Water Quality	27
Appendix A: Stormtech Construction Guide	30
Appendix B: Stormtech Isolator Row Operation and Maintenance Manual	31
Appendix C: Stormceptor STC Operation and Maintenance Guide	32
Annendiy D: O&M and I TPPP Plan	33

Operation and Maintenance Plan

Deep Sump Hooded Catch Basins

System Owner: Fairfield Residential (until ownership is transferred)

Robert Hewitt rhewitt@ffres.com

Estimated Annual Maintenance: \$10,200.00 - \$15,300.00

(Per DEP Stormwater Structural BMP's Vol 2)

Inspect or clean deep sump basins at least four times per year and at the end of the foliage and snow removal seasons. Sediments must also be removed four times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin. If handling runoff from land uses with higher potential pollutant loads or discharging runoff near or to a critical area, more frequent cleaning may be necessary. Clamshell buckets are typically used to remove sediment in Massachusetts. However, vacuum trucks are preferable because they remove more trapped sediment and supernatant than clamshells. Vacuuming is also a speedier process and is less likely to snap the cast iron hood within the deep sump catch basin.

Date	Inspector	Condition	Maintenance Performed*

^{*}Evidence of maintenance (i.e. receipts) must be provided.

Contech Water Quality Unit - STC 450i

System Owner: Fairfield Residential (until ownership is transferred)

Robert Hewitt rhewitt@ffres.com

Estimated Annual Maintenance: \$200.00 - \$300.00 (Per Stormceptor Inspection and Maintenance Guide)

Inspect or clean pos-construction prior to being put in service. The unit is to be cleaned by a vacuum truck. The unit should be cleaned once the sediment depth reaches 15% of the storage capacity, or when about 8 inches of sediment has been accumulated. Additional information regarding the maintenance of the unit can be found within the attached product Maintenance Manual.

Date	Inspector	Condition	Maintenance Performed*

^{*}Evidence of maintenance (i.e. receipts) must be provided.

OPERATIONS AND MAINTENENACE PLAN MULTIFAMILY DEVELOPMENT - 51-53-55 SUMMER STREET

June 20, 2023

Subsurface Infiltration System

System Owner: Fairfield Residential (until ownership is transferred)

Robert Hewitt rhewitt@ffres.com

Estimated Annual Maintenance: \$200.00 - \$300.00

(Per DEP Stormwater Structural BMP's Vol 2)

For the first 3 months after construction, the subsurface infiltration system should be inspected after every storm greater than 1" for standing water for periods more than 72 hours. Therein after, the subsurface infiltration system should be inspected biannually. If standing water is observed for longer than 72 hours, a pump should be placed in the basin and discharged through the outlet pipe. After the system is dewatered, it should be observed by a Professional Engineer. A Professional Engineer should provide an opinion as to why the infiltration system is not draining and provide recommendations to restore infiltration capacity to the system.

Date	Inspector	Condition	Maintenance Performed*

^{*}Evidence of maintenance (i.e. receipts) must be provided.

Isolator Row

System Owner: Fairfield Residential (until ownership is transferred)

Robert Hewitt rhewitt@ffres.com

Estimated Annual Maintenance: \$600.00 - \$900.00

(Per DEP Stormwater Structural BMP's Vol 2)

In the first year of operation, the Isolator Row should be inspected every 6 months for depth of sediment. Therein after, the Isolator Row should be inspected annually. If sediment is present, a stadia rod should be inserted into the inspection port to determine depth of sediment. If/when the depth exceeds 3 inches throughout the length of the Isolator Row, clean out should be performed. Please see the Isolator Row Maintenance Manual for cleanout procedures.

Date	Inspector	Condition	Maintenance Performed*

^{*}Evidence of maintenance (ie. receipts) must be provided.

OPERATIONS AND MAINTENENACE PLAN MULTIFAMILY DEVELOPMENT – 51-53-55 SUMMER STREET

June 20, 2023

Sediment Forebay

System Owner: Fairfield Residential (until ownership is transferred)

Robert Hewitt rhewitt@ffres.com

Estimated Annual Maintenance: \$1,200.00 (Per DEP Stormwater Structural BMP's Vol 2)

In many cases, a landscaping contractor working elsewhere on the site can complete maintenance tasks. Stabilize the floor and sidewalls of the sediment forebay before making it operational, otherwise the practice will discharge excess amounts of suspended sediments.

Inspect and clean out the sediment forebay to assure that sediments and associated pollutants are cleaned out. Frequently removing accumulated sediments will make it less likely that sediments will be resuspended. At a minimum, inspect the sediment forebays monthly and clean them out at least four times a year.

Mow the grass areas and keep the grass height no greater than 6 inches. Check for signs of rilling and gullying and repair as needed. After removing the sediment, replace any vegetation damaged during the clean-out by either reseeding or resodding. When reseeding, incorporate practices such as hydroseeding with a tackifier, blanket, or similar practice to ensure that no scour occurs in the forebay, while the seeds germinate and develop roots.

*Paying careful attention to the pretreatment and operation and maintenance can extend the life of the soil media.

Date	Inspector	Condition	Maintenance Performed*

^{*}Evidence of maintenance (ie. receipts) must be provided.

OPERATIONS AND MAINTENENACE PLAN MULTIFAMILY DEVELOPMENT – 51-53-55 SUMMER STREET

June 20, 2023

Infiltration Ponds

System Owner: Fairfield Residential (until ownership is transferred)

Robert Hewitt rhewitt@ffres.com

Estimated Annual Maintenance: \$580.00 (Per DEP Stormwater Structural BMP's Vol 2)

In many cases, a landscaping contractor working elsewhere on the site can complete maintenance tasks. Inspect the basin and outlet structure to ensure no structural damage has occurred and that they are functioning properly and up to design standards.

Inspection and preventive maintenance are required at least twice per year, and after each major storm event. Note how long water remains standing in the basin after a storm. If water remains standing after 48 to 72 hours after a storm, the infiltration basin may be clogged.

At least twice per year, mow the buffer area, side slopes, and basin bottom. Remove grass clippings, accumulated organic matter, trash and debris at this time.

Remove sediment from the basin as necessary when the basin is dry. Use light equipment when removing the top layer, as to not compact the underlying soil. Use deep tilling to break and remove any clogged surfaces and revegetate immediately.

Important items to check during inspections include:

- Signs of differential settlement
- Cracking
- Erosion
- Leakage in the embankments
- Tree growth on the embankments
- Condition of rip rap
- Sediment accumulation
- Health of vegetation, turf

^{*}Paying careful attention to the pretreatment and operation and maintenance can extend the life of the soil media.



Inspector	Condition	Maintenance Performed*	
			_
		Inspector Condition	Inspector Condition Maintenance Performed*

^{*}Evidence of maintenance (ie. receipts) must be provided.

OPERATIONS AND MAINTENENACE PLAN MULTIFAMILY DEVELOPMENT – 51-53-55 SUMMER STREET

June 20, 2023

Pocket Wetland

System Owner: Fairfield Residential (until ownership is transferred)

Robert Hewitt rhewitt@ffres.com

Estimated Annual Maintenance: \$580.00 (Per DEP Stormwater Structural BMP's Vol 2)

The constructed stormwater wetland must be observed over time. In the first 3 years after construction, inspect the constructed stormwater wetland twice a year during both the growing and non-growing seasons. Following the maturation of the plant community within the treatment wetland(s), in addition to annual forebay inspection and clean-outs, the plant communities will be assessed to verify that the desired species composition is retained, and that woody or pernicious species do not become substantially established. In the event that undesirable plant specimens become established these will be removed either by hand pulling, puller-bear extraction, or by targeted herbicide application by a Licensed Applicator. A brief, annual photolog (1-2 photos per community type from consistent view positions) will provide documentation under the O&M plan. During these inspections, record and map the following information:

- The types and distribution of the dominant wetland plants in the marsh;
- The presence and distribution of planted wetland species;
- The presence and distribution of invasive wetland species (invasive species must be removed);
- Indications that other species are replacing the planted wetland species;
- Percentages of standing water that is unvegetated (excluding the deep-water cells which are not suitable for emerging plant growth);
- The maximum elevation and the vegetative condition in this zone if the design elevation of the normal pool is being maintained for wetlands with extended zones;
- Stability of the original depth zones and the micro-topographic features; and
- Accumulation of sediment in the forebay and micro pool; and survival rate of plants (cells with dead plants must be replanted).

Date	Inspector	Condition	Maintenance Performed*

^{*}Evidence of maintenance (ie. receipts) must be provided.

Outfalls and Riprap

System Owner: Fairfield Residential (until ownership is transferred)

Robert Hewitt rhewitt@ffres.com

Estimated Annual Maintenance: \$750.00 - \$1,000.00

(Per DEP Stormwater Structural BMP's Vol 2)

Inspections should be performed annually and after major storm events. If riprap has been damaged, repairs should be made promptly to prevent a progressive failure. Channel obstructions, such as trees and sediment bars, can change flow patterns and cause erosive forces which may damage riprap and the integrity of the outfall.

Date	Inspector	Condition	Maintenance Performed*

^{*}Evidence of maintenance (ie. receipts) must be provided.

OPERATIONS AND MAINTENENACE PLAN MULTIFAMILY DEVELOPMENT – 51-53-55 SUMMER STREET

June 20, 2023

Drip-edge System

System Owner: Fairfield Residential (until ownership is transferred)

Robert Hewitt rhewitt@ffres.com

Estimated Annual Maintenance: \$2,500.00 - \$3,000.00

(Per DEP Stormwater Structural BMP's Vol 2)

Inspect the infiltration trench after the first several rainfall events, after all major storms, and on regularly scheduled dates every six months. Inspect the trench 24 hours or several days after a rain event, to look for ponded water. If there is ponded water at the surface of the trench, it is likely that the trench surface is clogged. To address surface clogging, remove and replace the topsoil or first layer of stone aggregate and the filter fabric. If water is ponded inside the trench, it may indicate that the bottom of the trench has failed. To rehabilitate a failed trench, all accumulated sediment must be stripped from the bottom, the bottom of the trench must be scarified and tilled to induce infiltration, and all of the stone aggregate and filter fabric or media must be removed and replaced.

Inspector	Condition	Maintenance Performed*	
		Inspector Condition	Inspector Condition Maintenance Performed*

^{*}Evidence of maintenance (ie. receipts) must be provided.

Erosion and Sediment Control Notes

- A. Erosion and sediment control measures must be installed prior to the start of construction and maintained and upgraded as necessary during construction by the contractor. It is the contractor's responsibility to inspect and install additional control measures as needed during construction.
- B. All catch basins receiving drainage from the project site must be provided with a catch basin filter.
- C. Stabilization of all re-graded and soil stockpile areas must be maintained during all phases of construction.
- D. Sediment removed from erosion and sediment control devices must be properly removed and disposed. All damaged controls must be removed and replaced.
- E. The contractor is responsible for implementing the erosion and sediment control plan which will be submitted as part of the SWPPP. This includes the installation and maintenance of control measures, informing all parties engaged on the construction site of the requirements and objectives of the plant, and notifying the proper city agency of any transfer of this responsibility.
- F. The contractor shall be responsible for controlling wide erosion and dust throughout the life of his contract. Dust control may include, but is not limited to, sprinkling of water on exposed soils and street sweeping adjacent roadways.
- G. If final grading is to be delayed for more than 21 days after land disturbance activates cease, temporary vegetation or mulch shall be used to stabilize soils within 14 days of the last disturbance.
- H. If a disturbed area will be exposed for greater than one year, permanent grasses or other approved cover must be installed.
- I. The contractor must keep on-site at all times additional silt fence and straw wattle for the installation at the direction of the engineer or the city to mitigate any emergency condition.
- J. The constriction fencing and erosion and sediment controls as shown may not be practical during all stages of construction. Earthwork activity on-site must be done in a manner such that runoff is directed to a sediment control device or infiltrated to the ground.

OPERATIONS AND MAINTENENACE PLAN MULTIFAMILY DEVELOPMENT – 51-53-55 SUMMER STREET June 20, 2023

- K. Demolition and construction debris must be properly contained and disposed of.
- L. Disposal of all demolished materials is the responsibility of the contractor and must be hauled off-site in accordance with all federal, state, and local requirements.





Construction sequence to comply with the submitted and approved SWPPP.

Construction

A seven-step process to prepare a wetland bed prior to planting (Shueler 1992):

- Prepare final pond-scaping and grading plans for the constructed stormwater wetland. At the same time, order wetland plant stocks from aquatic nurseries.
- Once the constructed stormwater wetland volume has been excavated, grade the wetland to create the major internal features (pool, aquatic bench, deep water channels, etc.).
- Because deep subsoils often lack the nutrients and organic matter needed to support vigorous plant growth, add topsoil and/or wetland mulch to the wetland excavation. If available, wetland mulch is preferable to topsoil.
- 4. After the mulch or topsoil has been added, grade the constructed stormwater wetland to its final elevations. Temporarily stabilize all wetland features above the normal pool. After final grading, close the pool drain to allow the pool to fill. MassDEP recommends evaluating the wetland elevations during a standing period of approximately six months to assess how the constructed stormwater wetland responds to storm flows and inundation, where the pond-scaping zones are located, and whether the final grade and micro-topography will persist over time.
- Before planting, measure the constructed stormwater wetland depths to the nearest inch to confirm planting depth. If necessary, modify the pond-scape plan at this time to reflect altered depths or availability of plant stock.
- Aggressively apply erosion controls during the standing and planting periods. Stabilize the vegetation in all areas above the normal pool elevation during the standing period (typically by hydroseeding).
- Dewater the constructed stormwater wetland at least three days before planting, because a dry wetland is easier to plant than a wet one.

Invasive Vegetation Control

A. Wetland Replication Areas

1. Undesirable exotic vegetation, including all species from the Massachusetts Department of Agriculture, "Massachusetts Prohibited Plant List" (MA DAR, 2017) will be removed from areas where compensatory wetland replication will occur, including the adjacent upland work areas if applicable. Throughout the anticipated two (2) growing season monitoring period, undesirable plants will be removed by hand-pulling or mechanical means if necessary. Non-invasive wetland plants that are not desirable to the replication plan may also be controlled manually during the two seasons of maturation for the replication areas.

B. Upland Project Area Buffer Zone Invasive Species Control Plan

- The most pernicious invasions of exotic vegetation currently occur within areas of former
 pasture within uplands that lie within the project area. These materials will be removed
 during site preparation and either chipped and composted and disposed of offsite or
 taken off site to a processing facility.
- 2. Undesirable exotic vegetation will be removed from areas of upland Buffer Zone within the project development areas (buildings, roads and drainage, landscaped areas). In general lawns and public areas with landscaping will be kept free of undesirable vegetation during normal landscape maintenance. Particular attention will be paid to areas not associated with private dwellings to assure that seed sources do not develop within areas beyond exclusive use zones. No management of exotic vegetation will be conducted within areas left in their natural state.

C. Invasive Plant Species Maintenance Plan

- 1. Throughout the areas improved beyond exclusive use and public use areas (lawns, gardens, planting islands, sidewalks) annual mowing will be performed, after August 1, for the purpose of controlling invasive plant species, both woody and herbaceous.
- 2. Areas beyond typical public view or visitation are particularly susceptible to overgrowth by undesirable species; examples of these can be found at: http://www.massnrc.org/MIPAG/invasive.htm. These species can generally be controlled or eliminated by regular mowing and destruction of propagules prior to ripening. The areas of greatest concern are low/no activity areas such as detention basin berms, and

OPERATIONS AND MAINTENENACE PLAN MULTIFAMILY DEVELOPMENT – 51-53-55 SUMMER STREET June 20, 2023

areas cleared and graded for project construction, but not part of normal habitation and use activities. Example areas include:

- 2.1 East of Basin #1
- 2.2 East of Building #1 and parking
- 2.3 North and East of Basin
- 2.4 North of Maintenance Facilities
- 3. Regular, annual mowing of these areas will discourage establishment and propagation of undesirable plant species. In the event that species on the MIPAG list (or other current recognized lists) become established despite mowing, conventional measures will be employed to eradicate these from herbaceous communities functioning to stabilize areas disturbed in the course of project construction. Hand, or mechanical pulling, or licensed herbicide application, as appropriate will be used to minimize advancement of undesirable plant species in "back areas" not typically used for recreation or enjoyment by the residents.

Long-Term Pollution Prevention Plan

This Long-Term Pollution Prevention Plan is prepared to comply with the provisions set forth in the Massachusetts Department of Environmental Protection (DEP) Stormwater Management Standards. Structural Best Management Practices (BMP's) require periodic maintenance to ensure proper function and efficiency in pollutant removal from stormwater discharges that would otherwise reach wetland resource areas untreated.

Maintenance schedules found below are as recommended in Department of Environmental Protection's Massachusetts Stormwater Handbook and as recommended in manufacturer's specifications.

■ Transfer of Ownership

After the project is completed the site ownership and maintenance responsibilities will be transferred to a from Fairfield Residential. The new owner will be responsible for the operation, maintenance, and inspection of all components of the onsite stormwater management system.

Trash and Litter Cleanup

The owner (or maintainer) shall perform trash and litter cleanup once per month in and around the site. Trash and litter shall be disposed of in the on-site dumpsters during construction, and after construction shall be collected and properly disposed of.

Paved Roadway

Impervious surfaces (pavement and sidewalks) shall be swept on an annual basis between April 1 and May 31. Raised sidewalks shall be swept by hand prior to any mechanical (rotary brush) sweeping. Sand and sediment deposits shall be collected from the site and disposed of by a licensed contractor, who must dispose of the material off-site in a manner consistent with all local, state, and federal regulations.

Deep Sump Hooded Catch Basins

Catch Basins shall be inspected on a bi-annual basis. Any sediment accumulations in excess of half the unit's sump depth shall be removed. Material shall be removed by a licensed contractor, who shall be responsible for disposing of the material off-site in a manner consistent with all local, state, and federal regulations.

Contech Water Quality Unit - STC 450i

Inspect or clean pos-construction prior to being put in service. The unit is to be cleaned by a vacuum truck. The unit should be cleaned once the sediment depth reaches 15% of the storage capacity, or when about 8 inches of sediment has been accumulated. Additional information regarding the maintenance of the unit can be found within the attached product Maintenance Manual.

Landscape Maintenance of Maintained Landscape Areas

Landscaped surfaces in and around the proposed development in maintained areas shall be kept healthy and maintained. All lawn areas, as shown in maintained upland areas, shall be kept cut and watered to maintain groundcover. Clippings shall be contained and disposed of at an offsite location. Care should be taken as to not dump or dispose of any clippings and or plant material into the replication area and/or the Town of Walpole Conservation Areas. All landscaping on the site shall be installed so as to not require ongoing use of fertilizers and pesticides. Fertilizers containing phosphorus shall not be used on the property. All landscape waste accumulated in the course of grounds maintenance, including grass clippings and leaves, shall be disposed of at a proper upland composting or disposal area, and shall not be disposed of in any wetland area or buffer zone without the approval of the conservation commission. Non-organic fertilizers and pesticides and landscape care chemicals within 100 feet of the wetlands are prohibited. Signs must be posted as referenced in condition #49 of the Decision and dog curbing rules must be implemented (pick up and proper disposal) to further reduce nutrient loading within wetland resource areas.

Shrubs and trees shall be maintained and shall be replaced immediately if the plant has died.

Snow Plowing and Disposal

All snow shall be plowed and stored within the areas designated on the O&M and LTPPP Plan. Once these areas can no longer accept additional snow, all excess snow shall be removed from the site. Absolutely no deposition of snow shall enter directly into the wetlands or placed over any stormwater management facility. This shall be maintained in perpetuity.

Subsurface Infiltration Systems

For the first 3 months after construction, the subsurface infiltration system should be inspected after every storm greater than 1" for standing water for periods more than 72 hours. Therein after, the subsurface infiltration system should be inspected biannually. If standing water is observed for longer than 72 hours, a pump should be placed in the basin and discharged through the outlet pipe. After the system is dewatered, it should be observed by a Professional Engineer. A Professional

OPERATIONS AND MAINTENENACE PLAN MULTIFAMILY DEVELOPMENT – 51-53-55 SUMMER STREET

June 20, 2023

Engineer should provide an opinion as to why the infiltration system is not draining and provide recommendations to restore infiltration capacity to the system.

Isolator Row

In the first year of operation, the Isolator Row should be inspected every 6 months for depth of sediment. Therein after, the Isolator Row should be inspected annually. If sediment is present, a stadia rod should be inserted into the inspection port to determine depth of sediment. If/when the depth exceeds 3 inches throughout the length of the Isolator Row, clean out should be performed. Please see the Isolator Row Maintenance Manual for cleanout procedures.

Sediment Forebays

In many cases, a landscaping contractor working elsewhere on the site can complete maintenance tasks. Stabilize the floor and sidewalls of the sediment forebay before making it operational, otherwise the practice will discharge excess amounts of suspended sediments.

Inspect and clean out the sediment forebay to assure that sediments and associated pollutants are cleaned out. Frequently removing accumulated sediments will make it less likely that sediments will be resuspended. At a minimum, inspect the sediment forebays monthly and clean them out at least four times a year.

Mow the grass areas and keep the grass height no greater than 6 inches. Check for signs of rilling and gullying and repair as needed. After removing the sediment, replace any vegetation damaged during the clean-out by either reseeding or resodding. When reseeding, incorporate practices such as hydroseeding with a tackifier, blanket, or similar practice to ensure that no scour occurs in the forebay while the seeds germinate and develop roots.

Infiltration Pond

In many cases, a landscaping contractor working elsewhere on the site can complete maintenance tasks. Inspect the basin and outlet structure to ensure no damage has occurred and that they are functioning properly and up to design standards.

Inspection and preventive maintenance is required at least twice per year, and after each major storm event. Note how long water remains standing in the basin after a storm. If water remains standing after 48 to 72 hours after a storm, the infiltration basin may be clogged.

At least twice per year, mow the buffer area, side slopes, and basin bottom. Remove grass clippings, accumulated organic matter, trash and debris at this time.

OPERATIONS AND MAINTENENACE PLAN MULTIFAMILY DEVELOPMENT – 51-53-55 SUMMER STREET

June 20, 2023

Remove sediment from the basin as necessary when the basin is dry. Use light equipment when removing the top layer, as not to compact the underlying soil. Use deep tilling to break and remove any clogged surfaces and revegetate immediately.

Important items to check during inspections include:

- Signs of differential settlement
- Cracking
- Erosion
- Leakage in the embankments
- Condition of rip rap
- Sediment accumulation
- Health of vegetation, turf

Pocket Wetlands

Unlike conventional wet basin systems that require large-scale sediment removal at infrequent intervals, constructed stormwater wetlands require small-scale maintenance at regular intervals to evaluate the health and composition of the plant species.

Proponents must carefully observe the constructed stormwater wetland system over time. In the first three years after construction, inspect the constructed stormwater wetlands twice a year during both the growing and non-growing seasons. The sediment forebays should be inspected and cleaned once a year. This requirement must be included in the Operations and Maintenance plan. During these inspections, record and map the following information:

- The types and distribution of the dominant wetland plants in the marsh
- The presence and distribution of planted wetland species
- The presence and distribution of invasive wetland species (invasives must be removed)
- Indications that other species are replacing the planted wetland species
- Percentage of standing water that is unvegetated (excluding the deep water cells which are not suitable for emergent plant growth)
- The maximum elevation and the vegetative condition in this zone, if the design elevation of the normal pool is being maintained for wetlands with extended zones
- Stability of the original depth zones and the micro-topographical features
- Accumulation of sediment in the forebay and micropool; and survival rate of plants (cells with dead plants must be replanted)

Outfalls and Riprap

Inspections should be performed annually and after major storm events. If riprap has been damaged, repairs should be made promptly to prevent a progressive failure. Channel obstructions, such as

trees and sediment bars, can change flow patterns and cause erosive forces which may damage riprap and the integrity of the outfall.

Drip-edge System

Inspect the infiltration trench after the first several rainfall events, after all major storms, and on regularly scheduled dates every six months. Inspect the trench 24 hours or several days after a rain event, to look for ponded water. If there is ponded water at the surface of the trench, it is likely that the trench surface is clogged. To address surface clogging, remove and replace the topsoil or first layer of stone aggregate and the filter fabric. If water is ponded inside the trench, it may indicate that the bottom of the trench has failed. To rehabilitate a failed trench, all accumulated sediment must be stripped from the bottom, the bottom of the trench must be scarified and tilled to induce infiltration, and all of the stone aggregate and filter fabric or media must be removed and replaced.

DEP Standard 4: Water Quality

The Long-Term Pollution Prevention Plan Includes the following:

Good housekeeping practices:

Prevent or reduce pollutant runoff from reaching the wetland resource areas through street sweeping, stabilizing all disturbed areas with vegetative cover and catch basin cleaning.

Provisions for storing materials and waste products inside or under cover:

All materials on site are to be stored in a neat and orderly fashion in their appropriate containers and, if possible, under a roof or other secure enclosure. All waste products are to be placed in secure receptacles until they are emptied by a solid waste management company licensed in the commonwealth of Massachusetts.

Vehicle washing controls:

Vehicle washing will occur on-site as part of standard operations. All contaminants / hazardous waste shall be disposed of in a manner specified by local or state regulations or by the manufacturer. provide an effective means of minimizing the discharge of pollutants from equipment and vehicle washing, wheel wash water, and other types of wash waters. Ensure there is no discharge of soaps, solvents, or detergents in equipment and vehicle wash water; and for storage of soaps, detergents, or solvents, provide either (1) cover (e.g., plastic sheeting, temporary roofs) to minimize the exposure of these detergents to precipitation and to stormwater, or (2) a similarly effective means designed to minimize the discharge of pollutants from these areas. The homeowners shall be instructed in these practices.

OPERATIONS AND MAINTENENACE PLAN MULTIFAMILY DEVELOPMENT – 51-53-55 SUMMER STREET June 20, 2023

Requirements for routine inspections and maintenance of Stormwater BMP's:

Follow the procedures outlined within the Operations and Maintenance Section of this report.

Spill prevention and response plan:

Spill Prevention: As mentioned previously, all materials on site are to be stored in a neat and orderly fashion in their appropriate containers and, if possible, under a roof or other secure enclosure. Products shall be kept in their original containers with the original manufacturer's label. Product should not be mixed unless recommended by the manufacturer. The manufacturer's recommendations for proper use, storage and disposal shall be followed at all times and, if possible, all of the product should be used up before proper disposal.

The manufacturer's recommended methods for cleanup must be followed and spills cleaned up immediately after discovery. Spills shall be kept well ventilated and personnel must wear appropriate protective gear to prevent injury from contact with hazardous substances. Spills of toxic or hazardous materials must be reported to the appropriate local and/ or state agency in accordance with the local and/ or Commonwealth of Massachusetts regulations.

Requirements for storage and use of fertilizers, herbicides and pesticides:

Consult the town of Chelmsford, MA Conservation Commission for any questions regarding these materials.

Fertilizers:

Fertilizers are to be applied at the minimum amounts recommended by the manufacturer and once applied shall be worked into the soil to limit the possibility of entering the storm drains. Storage procedures are to be followed as previously stated and the contents of any partially used bags should be transferred to a sealable container, either bag or bin to avoid spilling.

Herbicides and Pesticides: Storage of these materials are to be as outlined previously and especially out of the reach of pets and children, away from damp areas where their containers may succumb to moisture or rust and should not be stored near food. These materials must not be placed in the trash or washed down the drain. Handle using rubber gloves and use an appropriate mask when using these products for extensive periods of time.

Provisions for maintenance of lawns, gardens, and other landscaped areas:

lawns gardens and other landscape areas are to be maintained in a manner that the ground remains stabilized. All dead plants shall be replaced in a timely manner as to prevent erosion and sedimentation control within the resource areas buffer zones.

Provisions for solid waste management:

All waste products are to be placed in secure receptacles until they are emptied by a solid waste management company licensed in the Commonwealth of Massachusetts.

Snow disposal and plowing plans relative to Wetland Resource Areas:

Snow disposal/removal shall refer to the locations as depicted on the O&M and LTPPP Plan attached to this document.

Winter Road Salt and/or Sand Use and Storage restrictions:

Road Salt use must be in compliance with the Guidelines on Deicing Chemical (Road Salt) Storage effective date December 19, 1997, Guideline No. DWSG97-1 found in the BRP's Drinking Water Program. Sand Use: Encourage the use of environmentally friendly alternatives such as calcium chloride and/or sand instead of road salt for melting ice whenever possible. Use of de-icing agents should be tightly restricted to those absolutely necessary for public safety in consideration of associated vegetated wetlands. Environmentally friendly salt alternatives shall be used for de-icing operations.

Provisions for prevention of illicit discharges to the stormwater management systems:

According to Standard 10 in the Massachusetts Stormwater Handbook, Illicit discharges to the stormwater management system are discharges that are not entirely comprised of stormwater. Notwithstanding the foregoing, an illicit discharge does not include discharges from the following activities or facilities: firefighting, water line flushing, landscape irrigation, uncontaminated groundwater, potable water sources, foundation drains, air conditioning condensation, footing drains, individual resident car washing, flows from riparian habitats and wetlands, dechlorinated water from swimming pools, water used for street washing and water used to clean residential buildings without detergents.

Training for staff or personnel involved with implementing LTPPP:

This responsibility lies with the owner(s) unless a legally-binding agreement is made with another party to perform such duties for the owner(s).

List of Emergency contacts for implementing Long-Term Pollution Prevention Plan:

This responsibility lies with the owner(s) unless a legally-binding agreement is made with another party to perform such duties for the owner(s).

Appendix A: Stormtech Construction Guide



R. P. TO. C. RODG. TOL StormTec

StormTech Construction Guide

REQUIRED MATERIALS AND EQUIPMENT LIST

- Acceptable fill materials per Table 1
- ADS Plus and non-woven geotextile fabrics

- StormTech solid end caps and pre-cored end caps
- StormTech chambers
- StormTech manifolds and fittings

IMPORTANT NOTES:

A. This installation guide provides the minimum requirements for proper installation of chambers. Non-adherence to this guide may result in damage to chambers during installation. Replacement of damaged chambers during or after backfilling is costly and very time consuming. It is recommended that all installers are familiar with this quide, and that the contractor inspects the chambers for distortion, damage and joint integrity as work progresses.

B. Use of a dozer to push embedment stone between the rows of chambers may cause damage to chambers and is not an acceptable backfill method. Any chambers damaged by using the "dump and push" method are not covered under the StormTech standard warranty.

C. Care should be taken in the handling of chambers and end caps. Avoid dropping, prying or excessive force on chambers during removal from pallet and initial placement.

Requirements for System Installation



Excavate bed and prepare subgrade per engineer's plans.



Place non-woven geotextile over prepared soils and up excavation walls. Install underdrains if required.



Place clean, crushed, angular stone foundation 6" (150 mm) min. Compact to achieve a flat surface.

Manifold, Scour Fabric and Chamber Assembly



Install manifolds and lay out ADS PLUS fabric at inlet rows [min. 12.5 ft (3.8 m)] at each inlet end cap. Place a continuous piece along entire length of Isolator® PLUS Row(s).

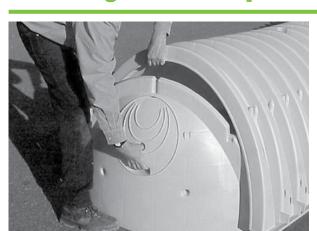


Align the first chamber and end cap of each row with inlet pipes. Contractor may choose to postpone stone placement around end chambers and leave ends of rows open for easy inspection of chambers during the backfill process.



Continue installing chambers by overlapping chamber end corrugations. Chamber joints are labeled "Lower Joint - Overlap Here" and "Build this direction -Upper Joint" Be sure that the chamber placement does not exceed the reach of the construction equipment used to place the stone. Maintain minimum 6" (150 mm) spacing between rows.

Attaching the End Caps



Lift the end of the chamber a few inches off the ground. With the curved face of the end cap facing outward, place the end cap into the chamber's end corrugation.

Prefabricated End Caps



24" (600 mm) inlets are the maximum size that can fit into a SC-740/DC-780 end cap and must be prefabricated with a 24" (600 mm) pipe stub. SC-310 chambers with a 12" (300 mm) inlet pipe must use a prefabricated end cap with a 12" (300 mm) pipe stub. When used on an Isolator Row PLUS, these end caps will contain a welded FLAMP (flared end ramp) that will lay on top of the ADS PLUS fabric (shown above)

Isolator Row PLUS



Place a continuous layer of ADS PLUS fabric between the foundation stone and the Isolator Row PLUS chambers, making sure the fabric lays flat and extends the entire width of the chamber feet. Drape a strip of ADS non-woven geotextile over the row of chambers (not required over DC-780). This is the same type of non-woven geotextile used as a separation layer around the angular stone of the StormTech system. 2

Initial Anchoring of Chambers – Embedment Stone





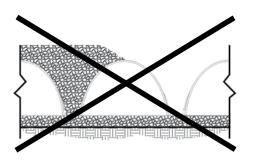
Initial embedment shall be spotted along the centerline of the chamber evenly anchoring the lower portion of the chamber. This is best accomplished with a stone conveyor or excavator reaching along the row.

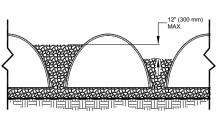




No equipment shall be operated on the bed at this stage of the installation. Excavators must be located off the bed. Dump trucks shall not dump stone directly on to the bed. Dozers or loaders are not allowed on the bed at this time.

Backfill of Chambers – Embedment Stone

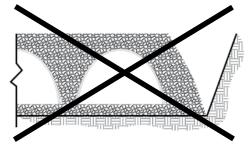




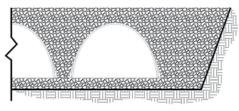
UNEVEN BACKFILL

EVEN BACKFILL

Backfill chambers evenly. Stone column height should never differ by more than 12" (300 mm) between adjacent chamber rows or between chamber rows and perimeter.







PERIMETER FULLY BACKFILLED

Perimeter stone must be brought up evenly with chamber rows. Perimeter must be fully backfilled, with stone extended horizontally to the excavation wall.

Backfill - Embedment Stone & Cover Stone



Continue evenly backfilling between rows and around perimeter until embedment stone reaches tops of chambers. Perimeter stone must extend horizontally to the excavation wall for both straight or sloped sidewalls. Only after chambers have been backfilled to top of chamber and with a minimum 6" (150 mm) of cover stone on top of chambers can small dozers be used over the chambers for backfilling remaining cover stone.



Small dozers and skid loaders may be used to finish grading stone backfill in accordance with ground pressure limits in Table 2. They must push material parallel to rows only. Never push perpendicular to rows. StormTech recommends that the contractor inspect chambers before placing final backfill. Any chambers damaged by construction shall be removed and replaced.

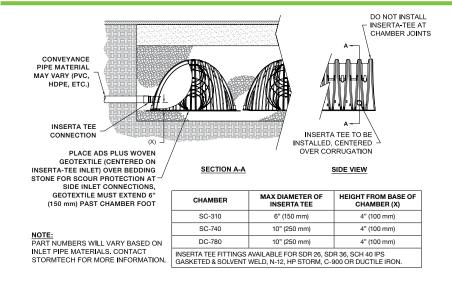
Final Backfill of Chambers – Fill Material





Install non-woven geotextile over stone. Geotextile must overlap 24" (600 mm) min. where edges meet. Compact each lift of backfill as specified in the site design engineer's drawings. Roller travel parallel with rows.

Inserta Tee Detail



StormTech Isolator Row PLUS Detail

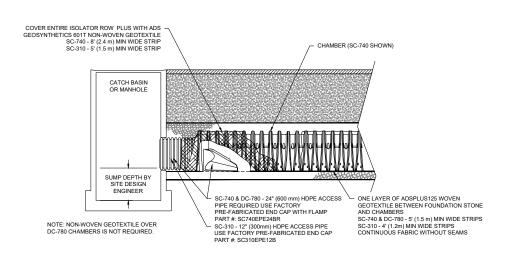


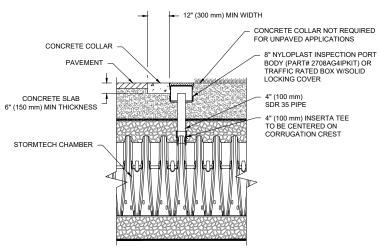
Table 1- Acceptable Fill Materials

Material Location	Description	AASHTO M43 Designation ¹	Compaction/Density Requirement
DFinal Fill: Fill Material for layer 'D' starts from the top of the 'C' layer to the bottom of flexible pavement or unpaved finished grade above. Note that the pavement subbase may be part of the 'D' layer.	Any soil/rock materials, native soils or per engineer's plans. Check plans for pavement subgrade requirements.	N/A	Prepare per site design engineer's plans. Paved installations may have stringent material and preparation requirements.
© Initial Fill: Fill Material for layer 'C' starts from the top of the embedment stone ('B' layer) to 18" (450 mm) above the top of the chamber. Note that pavement subbase may be part of the 'C' layer.	Granular well-graded soil/ aggregate mixtures, <35% fines or processed aggregate. Most pavement subbase materials can be used in lieu of this layer.	AASHTO M45 A-1,A-2-4, A-3 or AASHTO M431 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	Begin compaction after min. 12" (300 mm) of material over the chambers is reached. Compact additional layers in 6" (150 mm) max. lifts to a min. 95% Proctor density for well-graded material and 95% relative density for processed aggregate materials. Roller gross vehicle weight not to exceed 12,000 lbs (53 kN). Dynamic force not to exceed 20,000 lbs (89 kN)
B Embedment Stone: Embedment Stone surrounding chambers from the foundation stone to the 'C' layer above.	Clean, crushed, angular stone	AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57	No compaction required.
(A) Foundation Stone: Foundation Stone below the chambers from the subgrade up to the foot (bottom) of the chamber.	Clean, crushed, angular stone,	AASHTO M43¹ 3, 357, 4, 467, 5, 56, 57	Place and compact in 6" (150 mm) lifts using two full coverages with a vibratory compactor. ^{2,3}

PLEASE NOTE:

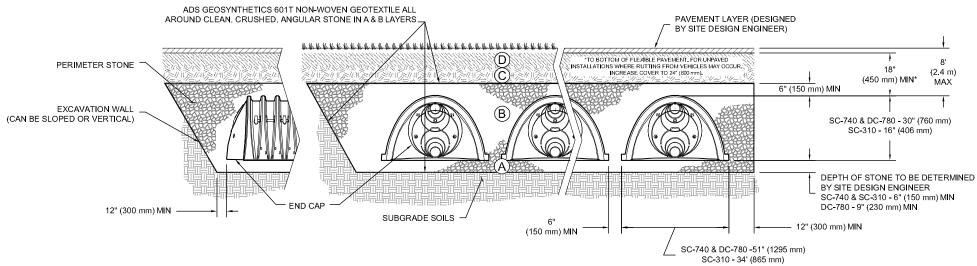
- 1. The listed AASHTO designations are for gradations only. The stone must also be clean, crushed, angular. For example, a specification for #4 stone would state: "clean, crushed, angular no. 4 (AASHTO M43) stone".
- 2. StormTech compaction requirements are met for 'A' location materials when placed and compacted in 6" (150 mm) (max) lifts using two full coverages with a vibratory compactor.
- 3. Where infiltration surfaces may be comprised by compaction, for standard installations and standard design load conditions, a flat surface may be achieved by raking or dragging without compaction equipment. For special load designs, contact StormTech for compaction requirements.

Figure 1- Inspection Port Detail



NOTE: INSPECTION PORTS MAY BE CONNECTED THROUGH ANY CHAMBER CORRUGATION CREST.

Figure 2 - Fill Material Locations



NOTES:

- 1. 36" (900 mm) of stabilized cover materials over the chambers is required for full dump truck travel and dumping.
- 2. During paving operations, dump truck axle loads on 18" (450 mm) of cover may be necessary. Precautions should be taken to avoid rutting of the road base layer, to ensure that compaction requirements have been met, and that a minimum of 18" (450 mm) of cover exists over the chambers. Contact StormTech for additional guidance on allowable axle loads during paving.
- Ground pressure for track dozers is the vehicle operating weight divided by total ground contact area for both tracks. Excavators will exert higher ground pressures based on loaded bucket weight and boom extension.
- 4. Mini-excavators (< 8,000lbs/3,628 kg) can be used with at least 12" (300 mm) of stone over the chambers and are limited by the maximum ground pressures in Table 2 based on a full bucket at maximum boom extension.
- 5. Storage of materials such as construction materials, equipment, spoils, etc. should not be located over the StormTech system. The use of equipment over the StormTech system not covered in Table 2 (ex. soil mixing equipment, cranes, etc) is limited. Please contact StormTech for more information.
- 6. Allowable track loads based on vehicle travel only. Excavators shall not operate on chamber beds until the total backfill reaches 3 feet (900 mm) over the entire bed.

ADS "Terms and Conditions of Sale" are available on the ADS website, www.ads-pipe.com. Advanced Drainage Systems, the ADS logo, and the green stripe are registered trademarks of Advanced Drainage Systems, Inc. StormTech® and the Isolator® Row PLUS are registered trademarks of StormTech, Inc #11010 09/20 CS

©2020 Advanced Drainage Systems, Inc.

Table 2 - Maximum Allowable Construction Vehicle Loads⁵

	Fill Donth	Maximum Allowa	able Wheel Loads	Maximum Allowa	able Track Loads ⁶	Maximum Allowable Roller Loads
Material Location	Fill Depth over Chambers in. [mm]	Max Axle Load for Trucks lbs [kN]	Max Wheel Load for Loaders Ibs [kN]	Track Width in. [mm]	Max Ground Pressure psf [kPa]	Max Drum Weight or Dynamic Force lbs [kN]
① Final Fill Material	36" [900] Compacted	32,000 [142]	16,000 [71]	12" [305] 18" [457] 24" [610] 30" [762] 36" [914]	3420 [164] 2350 [113] 1850 [89] 1510 [72] 1310 [63]	38,000 [169]
© Initial Fill Material	24" [600] Compacted	32,000 [142]	16,000 [71]	12" [305] 18" [457] 24" [610] 30" [762] 36" [914]	2480 [119] 1770 [85] 1430 [68] 1210 [58] 1070 [51]	20,000 [89]
	24" [600] Loose/Dumped	32,000 [142]	16,000 [71]	12" [305] 18" [457] 24" [610] 30" [762] 36" [914]	2245 [107] 1625 [78] 1325 [63] 1135 [54] 1010 [48]	20,000 [89] Roller gross vehicle weight not to exceed 12,000 lbs. [53 kN]
	18" [450]	32,000 [142]	16,000 [71]	12" [305] 18" [457] 24" [610] 30" [762] 36" [914]	2010 [96] 1480 [71] 1220 [58] 1060 [51] 950 [45]	20,000 [89] Roller gross vehicle weight not to exceed 12,000 lbs. [53 kN]
B Embedment Stone	12" [300]	16,000 [71]	NOT ALLOWED	12" [305] 18" [457] 24" [610] 30" [762] 36" [914]	1540 [74] 1190 [57] 1010 [48] 910 [43] 840 [40]	20,000 [89] Roller gross vehicle weight not to exceed 12,000 lbs. [53 kN]
	6" [150]	8,000 [35]	NOT ALLOWED	12" [305] 18" [457] 24" [610] 30" [762] 36" [914]	1070 [51] 900 [43] 800 [38] 760 [36] 720 [34]	NOT ALLOWED

Table 3 - Placement Methods and Descriptions

Material Material	Discoment Methods/ Destrictions	Wheel Load Restrictions	Track Load Restrictions	Roller Load Restrictions
Location	Placement Methods/ Restrictions	See Ta	ble 2 for Maximum Constructi	on Loads
① Final Fill Material	A variety of placement methods may be used. All construction loads must not exceed the maximum limits in Table 2.	36" (900 mm) minimum cover required for dump trucks to dump over chambers.	Dozers to push parallel to rows until 36" (900mm) compaced cover is reached. ⁴	Roller travel parallel to rows only until 36" (900 mm) compacted cover is reached.
© Initial Fill Material	Excavator positioned off bed recommended. Small excavator allowed over chambers. Small dozer allowed.	Asphalt can be dumped into paver when compacted pavement subbase reaches 18" (450 mm) above top of chambers.	Small LGP track dozers & skid loaders allowed to grade cover stone with at least 6" (150 mm) stone under tracks at all times. Equipment must push parallel to rows at all times.	Use dynamic force of roller only after compacted fill depth reaches 12" (300 mm) over chambers. Roller travel parallel to chamber rows only.
(B) Embedment Stone	No equipment allowed on bare chambers. Use excavator or stone conveyor positioned off bed or on foundation stone to evenly fill around all chambers to at least the top of chambers.	No wheel loads allowed. Material must be placed outside the limits of the chamber bed.	No tracked equipment is allowed on chambers until a min. 6" (150 mm) cover stone is in place.	No rollers allowed.
A Foundation Stone	No StormTech restrictions. Contractor responsible for any conditions or requirements by others relative to subgrade bearing capacity, dewatering or protection of subgrade.			

Appendix B: Stormtech Isolator Row Operation and Maintenance Manual



Isolator® Row O&M Manual









THE ISOLATOR® ROW

INTRODUCTION

An important component of any Stormwater Pollution Prevention Plan is inspection and maintenance. The StormTech Isolator Row is a technique to inexpensively enhance Total Suspended Solids (TSS) removal and provide easy access for inspection and maintenance.

THE ISOLATOR ROW

The Isolator Row is a row of StormTech chambers, either SC-160LP, SC-310, SC-310-3, SC-740, DC-780, MC-3500 or MC-4500 models, that is surrounded with filter fabric and connected to a closely located manhole for easy access. The fabric-wrapped chambers provide for settling and filtration of sediment as storm water rises in the Isolator Row and ultimately passes through the filter fabric. The open bottom chambers and perforated sidewalls (SC-310, SC- 310-3 and SC-740 models) allow storm water to flow both vertically and horizontally out of the chambers. Sediments are captured in the Isolator Row protecting the storage areas of the adjacent stone and chambers from sediment accumulation.

Two different fabrics are used for the Isolator Row. A woven geotextile fabric is placed between the stone and the Isolator Row chambers. The tough geotextile provides a media for storm water filtration and provides a durable surface for maintenance operations. It is also designed to prevent scour of the underlying stone and remain intact during high pressure jetting. A non-woven fabric is placed over the chambers to provide a filter media for flows passing through the perforations in the sidewall of the chamber. The non-woven fabric is not required over the SC-160LP, DC-780, MC-3500 or MC-4500 models as these chambers do not have perforated side walls.

The Isolator Row is typically designed to capture the "first flush" and offers the versatility to be sized on a volume basis or flow rate basis. An upstream manhole not only provides access to the Isolator Row but typically includes a high flow weir such that storm water flowrates or volumes that exceed the capacity of the Isolator Row overtop the over flow weir and discharge through a manifold to the other chambers.

The Isolator Row may also be part of a treatment train. By treating storm water prior to entry into the chamber system, the service life can be extended and pollutants such as hydrocarbons can be captured. Pre-treatment best management practices can be as simple as deep sump catch basins, oil-water separators or can be innovative storm water treatment devices. The design of the treatment train and selection of pretreatment devices by the design engineer is often driven by regulatory requirements. Whether pretreatment is used or not, the Isolator Row is recommended by StormTech as an effective means to minimize maintenance requirements and maintenance costs.

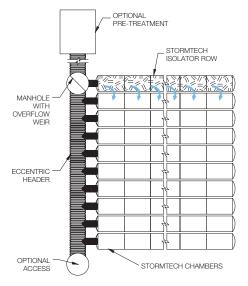
Note: See the StormTech Design Manual for detailed information on designing inlets for a StormTech system, including the Isolator Row.



Looking down the Isolator Row from the manhole opening, woven geotextile is shown between the chamber and stone base.



StormTech Isolator Row with Overflow Spillway (not to scale)





ISOLATOR ROW INSPECTION/MAINTENANCE

INSPECTION

The frequency of inspection and maintenance varies by location. A routine inspection schedule needs to be established for each individual location based upon site specific variables. The type of land use (i.e. industrial, commercial, residential), anticipated pollutant load, percent imperviousness, climate, etc. all play a critical role in determining the actual frequency of inspection and maintenance practices.

At a minimum, StormTech recommends annual inspections. Initially, the Isolator Row should be inspected every 6 months for the first year of operation. For subsequent years, the inspection should be adjusted based upon previous observation of sediment deposition.

The Isolator Row incorporates a combination of standard manhole(s) and strategically located inspection ports (as needed). The inspection ports allow for easy access to the system from the surface, eliminating the need to perform a confined space entry for inspection purposes.

If upon visual inspection it is found that sediment has accumulated, a stadia rod should be inserted to determine the depth of sediment. When the average depth of sediment exceeds 3 inches throughout the length of the Isolator Row, clean-out should be performed.

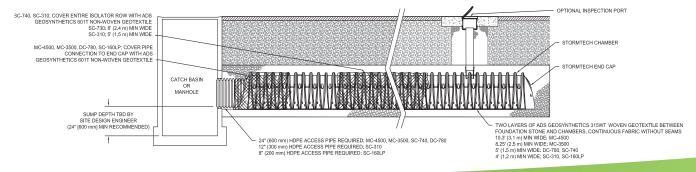
MAINTENANCE

The Isolator Row was designed to reduce the cost of periodic maintenance. By "isolating" sediments to just one row, costs are dramatically reduced by eliminating the need to clean out each row of the entire storage bed. If inspection indicates the potential need for maintenance, access is provided via a manhole(s) located on the end(s) of the row for cleanout. If entry into the manhole is required, please follow local and OSHA rules for a confined space entries.

Maintenance is accomplished with the JetVac process. The JetVac process utilizes a high pressure water nozzle to propel itself down the Isolator Row while scouring and suspending sediments. As the nozzle is retrieved, the captured pollutants are flushed back into the manhole for vacuuming. Most sewer and pipe maintenance companies have vacuum/JetVac combination vehicles. Selection of an appropriate JetVac nozzle will improve maintenance efficiency. Fixed nozzles designed for culverts or large diameter pipe cleaning are preferable. Rear facing jets with an effective spread of at least 45" are best. Most JetVac reels have 400 feet of hose allowing maintenance of an Isolator Row up to 50 chambers long. The JetVac process shall only be performed on StormTech Isolator Rows that have AASHTO class 1 woven geotextile (as specified by StormTech) over their angular base stone.

StormTech Isolator Row (not to scale)

Note: Non-woven fabric is only required over the inlet pipe connection into the end cap for SC-160LP, DC-780, MC-3500 and MC-4500 chamber models and is not required over the entire Isolator Row.





ISOLATOR ROW STEP BY STEP MAINTENANCE PROCEDURES

STEP 1

Inspect Isolator Row for sediment.

- A) Inspection ports (if present)
 - i. Remove lid from floor box frame
 - ii. Remove cap from inspection riser
 - iii. Using a flashlight and stadia rod, measure depth of sediment and record results on maintenance log.
 - iv. If sediment is at or above 3 inch depth, proceed to Step 2. If not, proceed to Step 3.
- B) All Isolator Rows
 - i. Remove cover from manhole at upstream end of Isolator Row
 - ii. Using a flashlight, inspect down Isolator Row through outlet pipe
 - 1. Mirrors on poles or cameras may be used to avoid a confined space entry
 - 2. Follow OSHA regulations for confined space entry if entering manhole
 - iii. If sediment is at or above the lower row of sidewall holes (approximately 3 inches), proceed to Step 2. If not, proceed to Step 3.

STEP 2

Clean out Isolator Row using the JetVac process.

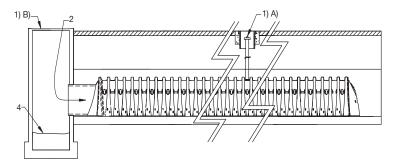
- A) A fixed floor cleaning nozzle with rear facing nozzle spread of 45 inches or more is preferable
- B) Apply multiple passes of JetVac until backflush water is clean
- C) Vacuum manhole sump as required

STEP 3

Replace all caps, lids and covers, record observations and actions.

STEP 4

Inspect & clean catch basins and manholes upstream of the StormTech system.



SAMPLE MAINTENANCE LOG

	Stadia Ro	Stadia Rod Readings			
Date	Fixed point to chamber bottom (1)	Fixed point to top of sediment (2)	Sediment Depth (1)–(2)	Observations/Actions	Inspector
3/15/11	6.3 ft	none		New installation. Fixed point is CI frame at grade	MCG
9/24/11		6.2	0.1 ft	Some grit felt	SM
6/20/13		5,8	o.s ft	Mucky feel, debris visible in manhole and in Isolator Row, maintenance due	Ν
7/7/13	6.3 ft		0	System jetted and vacuumed	MCG

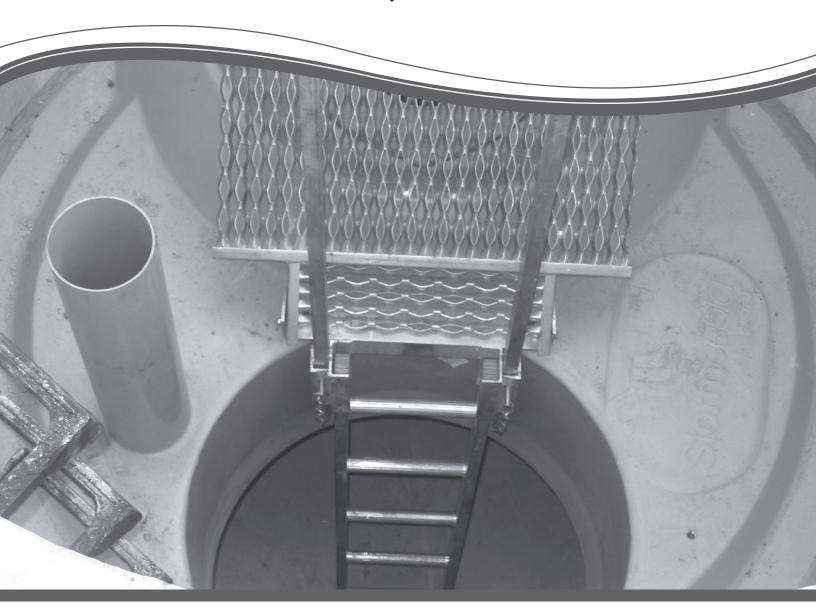




Appendix C: Stormceptor STC Operation and Maintenance Guide



Stormceptor® STC Operation and Maintenance Guide





Stormceptor Design Notes

- Only the STC 450i is adaptable to function with a catch basin inlet and/or inline pipes.
- Only the Stormceptor models STC 450i to STC 7200 may accommodate multiple inlet pipes.

Inlet and outlet invert elevation differences are as follows:

Inlet and Outlet Pipe Invert Elevations Differences			
Inlet Pipe Configuration	STC 450i	STC 900 to STC 7200	STC 11000 to STC 16000
Single inlet pipe	3 in. (75 mm)	1 in. (25 mm)	3 in. (75 mm)
Multiple inlet pipes	3 in. (75 mm)	3 in. (75 mm)	Only one inlet pipe.

Maximum inlet and outlet pipe diameters:

Inlet/Outlet Configuration	Inlet Unit STC 450i	In-Line Unit STC 900 to STC 7200	Series* STC 11000 to STC 16000
Straight Through	24 inch (600 mm)	42 inch (1050 mm)	60 inch (1500 mm)
Bend (90 degrees)	18 inch (450 mm)	33 inch (825 mm)	33 inch (825 mm)

- The inlet and in-line Stormceptor units can accommodate turns to a maximum of 90 degrees.
- Minimum distance from top of grade to crown is 2 feet (0.6 m)
- Submerged conditions. A unit is submerged when the standing water elevation at the proposed location of the Stormceptor unit is greater than the outlet invert elevation during zero flow conditions. In these cases, please contact your local Stormceptor representative and provide the following information:
- Top of grade elevation
- Stormceptor inlet and outlet pipe diameters and invert elevations
- Standing water elevation
- Stormceptor head loss, K = 1.3 (for submerged condition, K = 4)



OPERATION AND MAINTENANCE GUIDE Table of Content

1.	About Stormceptor	4
2.	Stormceptor Design Overview	4
3.	Key Operation Features	6
4.	Stormceptor Product Line.	7
5.	Sizing the Stormceptor System	10
	Spill Controls	
7.	Stormceptor Options	14
8.	Comparing Technologies	17
9.	Testing	18
10.	Installation	18
	Stormceptor Construction Sequence	
12.	Maintenance	19

1. About Stormceptor

The Stormceptor® STC (Standard Treatment Cell) was developed by Imbrium™ Systems to address the growing need to remove and isolate pollution from the storm drain system before it enters the environment. The Stormceptor STC targets hydrocarbons and total suspended solids (TSS) in stormwater runoff. It improves water quality by removing contaminants through the gravitational settling of fine sediments and floatation of hydrocarbons while preventing the re-suspension or scour of previously captured pollutants.

The development of the Stormceptor STC revolutionized stormwater treatment, and created an entirely new category of environmental technology. Protecting thousands of waterways around the world, the Stormceptor System has set the standard for effective stormwater treatment.

1.1. Patent Information

The Stormceptor technology is protected by the following patents:

- Australia Patent No. 693,164 693,164 707,133 729,096 779401
- Austrian Patent No. 289647
- Canadian Patent No 2,009,208 2,137,942 2,175,277 2,180,305 2,180,383 2,206,338 2,327,768 (Pending)
- China Patent No 1168439
- Denmark DK 711879
- German DE 69534021
- Indonesian Patent No 16688
- Japan Patent No 9-11476 (Pending)
- Korea 10-2000-0026101 (Pending)
- Malaysia Patent No PI9701737 (Pending)
- New Zealand Patent No 314646
- United States Patent No 4,985,148 5,498,331 5,725,760 5,753,115 5,849,181 6,068,765 6,371,690
- Stormceptor OSR Patent Pending Stormceptor LCS Patent Pending

2. Stormceptor Design Overview

2.1. Design Philosophy

The patented Stormceptor System has been designed to focus on the environmental objective of providing long-term pollution control. The unique and innovative Stormceptor design allows for continuous positive treatment of runoff during all rainfall events, while ensuring that all captured pollutants are retained within the system, even during intense storm events.

An integral part of the Stormceptor design is PCSWMM for Stormceptor - sizing software developed in conjunction with Computational Hydraulics Inc. (CHI) and internationally acclaimed expert, Dr. Bill James. Using local historical rainfall data and continuous simulation modeling, this software allows a Stormceptor unit to be designed for each individual site and the corresponding water quality objectives.

By using PCSWMM for Stormceptor, the Stormceptor System can be designed to remove a wide range of particles (typically from 20 to 2,000 microns), and can also be customized to remove a specific particle size distribution (PSD). The specified PSD should accurately reflect what is in the stormwater runoff to ensure the device is achieving the desired water quality objective. Since stormwater runoff contains small particles (less than 75 microns), it is important to design a treatment system to remove smaller particles in addition to coarse particles.

2.2. Benefits

The Stormceptor System removes free oil and suspended solids from stormwater, preventing spills and non-point source pollution from entering downstream lakes and rivers. The key benefits, capabilities and applications of the Stormceptor System are as follows:

- Provides continuous positive treatment during all rainfall events
- Can be designed to remove over 80% of the annual sediment load
- Removes a wide range of particles
- Can be designed to remove a specific particle size distribution (PSD)
- Captures free oil from stormwater
- Prevents scouring or re-suspension of trapped pollutants
- · Pre-treatment to reduce maintenance costs for downstream treatment measures (ponds, swales, detention basins, filters)
- Groundwater recharge protection
- Spills capture and mitigation
- Simple to design and specify
- Designed to your local watershed conditions
- Small footprint to allow for easy retrofit installations
- Easy to maintain (vacuum truck)
- Multiple inlets can connect to a single unit
- Suitable as a bend structure
- Pre-engineered for traffic loading (minimum AASHTO HS-20)
- Minimal elevation drop between inlet and outlet pipes
- Small head loss
- Additional protection provided by an 18" (457 mm) fiberglass skirt below the top of the insert, for the containment of hydrocarbons in the event of a spill.

2.3. Environmental Benefit

Freshwater resources are vital to the health and welfare of their surrounding communities. There is increasing public awareness, government regulations and corporate commitment to reducing the pollution entering our waterways. A major source of this pollution originates from stormwater runoff from urban areas. Rainfall runoff carries oils, sediment and other contaminants from roads and parking lots discharging directly into our streams, lakes and coastal waterways.

The Stormceptor System is designed to isolate contaminants from getting into the natural environment. The Stormceptor technology provides protection for the environment from spills that occur at service stations and vehicle accident sites, while also removing contaminated sediment in runoff that washes from roads and parking lots.

3. Key Operation Features

3.1. Scour Prevention

A key feature of the Stormceptor System is its patented scour prevention technology. This innovation ensures pollutants are captured and retained during all rainfall events, even extreme storms. The Stormceptor System provides continuous positive treatment for all rainfall events, including intense storms. Stormceptor slows incoming runoff, controlling and reducing velocities in the lower chamber to create a non-turbulent environment that promotes free oils and floatable debris to rise and sediment to settle.

The patented scour prevention technology, the fiberglass insert, regulates flows into the lower chamber through a combination of a weir and orifice while diverting high energy flows away through the upper chamber to prevent scouring. Laboratory testing demonstrated no scouring when tested up to 125% of the unit's operating rate, with the unit loaded to 100% sediment capacity (NJDEP, 2005). Second, the depth of the lower chamber ensures the sediment storage zone is adequately separated from the path of flow in the lower chamber to prevent scouring.

3.2. Operational Hydraulic Loading Rate

Designers and regulators need to evaluate the treatment capacity and performance of manufactured stormwater treatment systems. A commonly used parameter is the "operational hydraulic loading rate" which originated as a design methodology for wastewater treatment devices.

Operational hydraulic loading rate may be calculated by dividing the flow rate into a device by its settling area. This represents the critical settling velocity that is the prime determinant to quantify the influent particle size and density captured by the device. PCSWMM for Stormceptor uses a similar parameter that is calculated by dividing the hydraulic detention time in the device by the fall distance of the sediment.

$$V_{SC} = \frac{H}{6_H} = \frac{Q}{A_S}$$

Where:

 v_{sc} = critical settling velocity, ft/s (m/s)

H = tank depth, ft (m)

 \emptyset_{\perp} = hydraulic detention time, ft/s (m/s)

Q = volumetric flow rate, ft3/s (m3/s)

 $A_s = surface area, ft^2 (m^2)$

(Tchobanoglous, G. and Schroeder, E.D. 1987. Water Quality. Addison Wesley.)

Unlike designing typical wastewater devices, stormwater systems are designed for highly variable flow rates including intense peak flows. PCSWMM for Stormceptor incorporates all of the flows into its calculations, ensuring that the operational hydraulic loading rate is considered not only for one flow rate, but for all flows including extreme events.

3.3. Double Wall Containment

The Stormceptor System was conceived as a pollution identifier to assist with identifying illicit discharges. The fiberglass insert has a continuous skirt that lines the concrete barrel wall for a depth of 18 inches (457 mm) that provides double wall containment for hydrocarbons storage. This protective barrier ensures that toxic floatables do not migrate through the concrete wall into the surrounding soils.

4. Stormceptor Product Line

4.1. Stormceptor Models

A summary of Stormceptor models and capacities are listed in Table 1.

Table 1. Stormceptor Models

Stormceptor Model	Total Storage Volume U.S. Gal (L)	Hydrocarbon Storage Capacity U.S. Gal (L)	Maximum Sediment Capacity ft³ (L)
STC 450i	470 (1,780)	86 (330)	46 (1,302)
STC 900	952 (3,600)	251 (950)	89 (2,520)
STC 1200	1,234 (4,670)	251 (950)	127 (3,596)
STC 1800	1,833 (6,940)	251 (950)	207 (5,861)
STC 2400	2,462 (9,320)	840 (3,180)	205 (5,805)
STC 3600	3,715 (1,406)	840 (3,180)	373 (10,562)
STC 4800	5,059 (1,950)	909 (3,440)	543 (15,376)
STC 6000	6,136 (23,230)	909 (3,440)	687 (19,453)
STC 7200	7,420 (28,090)	1,059 (4,010)	839 (23,757)
STC 11000	11,194 (42,370)	2,797 (10, 590)	1,086 (30,752)
STC 13000	13,348 (50,530)	2,797 (10, 590)	1,374 (38,907)
STC 16000	15,918 (60,260)	3,055 (11, 560)	1,677 (47,487)

NOTE: Storage volumes may vary slightly from region to region. For detailed information, contact your local Stormceptor representative.

4.2. Inline Stormceptor

The Inline Stormceptor, Figure 1, is the standard design for most stormwater treatment applications. The patented Stormceptor design allows the Inline unit to maintain continuous positive treatment of total suspended solids (TSS) year-round, regardless of flow rate. The Inline Stormceptor is composed of a precast concrete tank with a fiberglass insert situated at the invert of the storm sewer pipe, creating an upper chamber above the insert and a lower chamber below the insert.

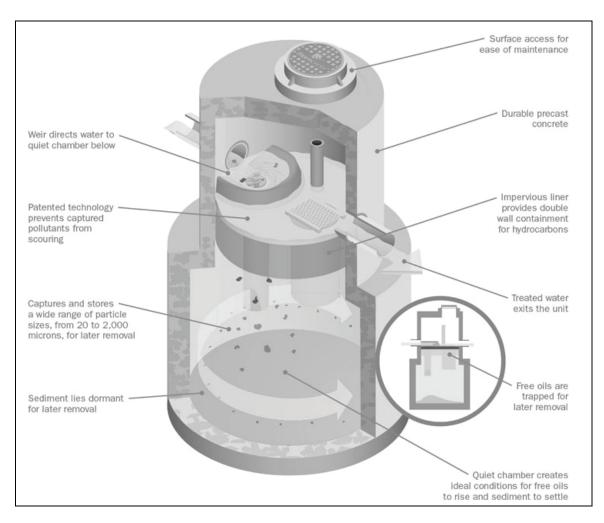


Figure 1. Inline Stormceptor

Operation

As water flows into the Stormceptor unit, it is slowed and directed to the lower chamber by a weir and drop tee. The stormwater enters the lower chamber, a non-turbulent environment, allowing free oils to rise and sediment to settle. The oil is captured underneath the fiberglass insert and shielded from exposure to the concrete walls by a fiberglass skirt. After the pollutants separate, treated water continues up a riser pipe, and exits the lower chamber on the downstream side of the weir before leaving the unit. During high flow events, the Stormceptor System's patented scour prevention technology ensures continuous pollutant removal and prevents re-suspension of previously captured pollutants.

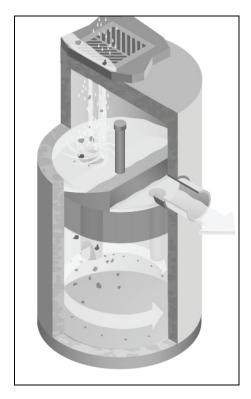


Figure 2. Inlet Stormceptor

4.3. Inlet Stormceptor

The Inlet Stormceptor System, Figure 2, was designed to provide protection for parking lots, loading bays, gas stations and other spill-prone areas. The Inlet Stormceptor is designed to remove sediment from stormwater introduced through a grated inlet, a storm sewer pipe, or both.

The Inlet Stormceptor design operates in the same manner as the Inline unit, providing continuous positive treatment, and ensuring that captured material is not re-suspended.

4.4. Series Stormceptor

Designed to treat larger drainage areas, the Series Stormceptor System, Figure 3, consists of two adjacent Stormceptor models that function in parallel. This design eliminates the need for additional structures and piping to reduce installation costs.

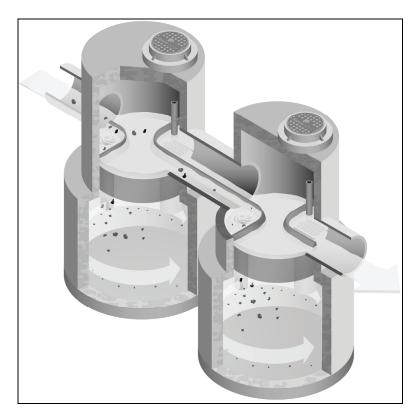


Figure 3. Series System

The Series Stormceptor design operates in the same manner as the Inline unit, providing continuous positive treatment, and ensuring that captured material is not re-suspended.

5. Sizing the Stormceptor System

The Stormceptor System is a versatile product that can be used for many different aspects of water quality improvement. While addressing these needs, there are conditions that the designer needs to be aware of in order to size the Stormceptor model to meet the demands of each individual site in an efficient and cost-effective manner.

PCSWMM for Stormceptor is the support tool used for identifying the appropriate Stormceptor model. In order to size a unit, it is recommended the user follow the seven design steps in the program. The steps are as follows:

STEP 1 – Project Details

The first step prior to sizing the Stormceptor System is to clearly identify the water quality objective for the development. It is recommended that a level of annual sediment (TSS) removal be identified and defined by a particle size distribution.

STEP 2 - Site Details

Identify the site development by the drainage area and the level of imperviousness. It is recommended that imperviousness be calculated based on the actual area of imperviousness based on paved surfaces, sidewalks and rooftops.

STEP 3 – Upstream Attenuation

The Stormceptor System is designed as a water quality device and is sometimes used in conjunction with onsite water quantity control devices such as ponds or underground detention systems. When possible, a greater benefit is typically achieved when installing a Stormceptor unit upstream of a detention facility. By placing the Stormceptor unit upstream of a detention structure, a benefit of less maintenance of the detention facility is realized.

STEP 4 - Particle Size Distribution

It is critical that the PSD be defined as part of the water quality objective. PSD is critical for the design of treatment system for a unit process of gravity settling and governs the size of a treatment system. A range of particle sizes has been provided and it is recommended that clays and silt-sized particles be considered in addition to sand and gravel-sized particles. Options and sample PSDs are provided in PCSWMM for Stormceptor. The default particle size distribution is the Fine Distribution, Table 2, option.

Table 2. Fine Distribution

Particle Size	Distribution	Specific Gravity
20	20%	1.3
60	20%	1.8
150	20%	2.2
400	20%	2.65
2000	20%	2.65

If the objective is the long-term removal of 80% of the total suspended solids on a given site, the PSD should be representative of the expected sediment on the site. For example, a system designed to remove 80% of coarse particles (greater than 75 microns) would provide relatively poor removal efficiency of finer particles that may be naturally prevalent in runoff from the site.

Since the small particle fraction contributes a disproportionately large amount of the total available particle surface area for pollutant adsorption, a system designed primarily for coarse particle capture will compromise water quality objectives.

STEP 5 - Rainfall Records

Local historical rainfall has been acquired from the U.S. National Oceanic and Atmospheric Administration, Environment Canada and regulatory agencies across North America. The rainfall data provided with PCSMM for Stormceptor provides an accurate estimation of small storm hydrology by modeling actual historical storm events including duration, intensities and peaks.

STEP 6 – Summary

At this point, the program may be executed to predict the level of TSS removal from the site. Once the simulation has completed, a table shall be generated identifying the TSS removal of each Stormceptor unit.

STEP 7 - Sizing Summary

Performance estimates of all Stormceptor units for the given site parameters will be displayed in a tabular format. The unit that meets the water quality objective, identified in Step 1, will be highlighted.

5.1. PCSWMM for Stormceptor

The Stormceptor System has been developed in conjunction with PCSWMM for Stormceptor as a technological solution to achieve water quality goals. Together, these two innovations model, simulate, predict and calculate the water quality objectives desired by a design engineer for TSS removal.

PCSWMM for Stormceptor is a proprietary sizing program which uses site specific inputs to a computer model to simulate sediment accumulation, hydrology and long-term total suspended solids removal. The model has been calibrated to field monitoring results from Stormceptor units that have been monitored in North America. The sizing methodology can be described by three processes:

- 1. Determination of real time hydrology
- 2. Buildup and wash off of TSS from impervious land areas
- 3. TSS transport through the Stormceptor (settling and discharge). The use of a calibrated model is the preferred method for sizing stormwater quality structures for the following reasons:
 - » The hydrology of the local area is properly and accurately incorporated in the sizing (distribution of flows, flow rate ranges and peaks, back-to-back storms, inter-event times)
 - » The distribution of TSS with the hydrology is properly and accurately considered in the sizing
 - » Particle size distribution is properly considered in the sizing
 - » The sizing can be optimized for TSS removal
 - » The cost benefit of alternate TSS removal criteria can be easily assessed
 - » The program assesses the performance of all Stormceptor models. Sizing may be selected based on a specific water quality outcome or based on the Maximum Extent Practicable

For more information regarding PCSWMM for Stormceptor, contact your local Stormceptor representative, or visit www.imbriumsystems.com to download a free copy of the program.

5.2. Sediment Loading Characteristics

The way in which sediment is transferred to stormwater can have a considerable effect on which type of system is implemented. On typical impervious surfaces (e.g. parking lots) sediment will build over time and wash off with the next rainfall. When rainfall patterns are examined, a short intense storm will have a higher concentration of sediment than a long slow drizzle. Together with rainfall data representing the site's typical rainfall patterns, sediment loading characteristics play a part in the correct sizing of a stormwater quality device.

Typical Sites

For standard site design of the Stormceptor System, PCSWMM for Stormceptor is utilized to accurately assess the unit's performance. As an integral part of the product's design, the program can be used to meet local requirements for total suspended solid removal. Typical installations of manufactured stormwater treatment devices would occur on areas such as paved parking lots or paved roads. These are considered "stable" surfaces which have non – erodible surfaces.

Unstable Sites

While standard sites consist of stable concrete or asphalt surfaces, sites such as gravel parking lots, or maintenance yards with stockpiles of sediment would be classified as "unstable". These types of sites do not exhibit first flush characteristics, are highly erodible and exhibit atypical sediment loading characteristics and must therefore be sized more carefully. Contact your local Stormceptor representative for assistance in selecting a proper unit sized for such unstable sites.

6. Spill Controls

When considering the removal of total petroleum hydrocarbons (TPH) from a storm sewer system there are two functions of the system: oil removal, and spill capture.

'Oil Removal' describes the capture of the minute volumes of free oil mobilized from impervious surfaces. In this instance relatively low concentrations, volumes and flow rates are considered. While the Stormceptor unit will still provide an appreciable oil removal function during higher flow events and/or with higher TPH concentrations, desired effluent limits may be exceeded under these conditions.

'Spill Capture' describes a manner of TPH removal more appropriate to recovery of a relatively high volume of a single phase deleterious liquid that is introduced to the storm sewer system over a relatively short duration. The two design criteria involved when considering this manner of introduction are overall volume and the specific gravity of the material. A standard Stormceptor unit will be able to capture and retain a maximum spill volume and a minimum specific gravity.

For spill characteristics that fall outside these limits, unit modifications are required. Contact your local Stormceptor Representative for more information.

One of the key features of the Stormceptor technology is its ability to capture and retain spills. While the standard Stormceptor System provides excellent protection for spill control, there are additional options to enhance spill protection if desired.

6.1. Oil Level Alarm

The oil level alarm is an electronic monitoring system designed to trigger a visual and audible alarm when a pre-set level of oil is reached within the lower chamber. As a standard, the oil

level alarm is designed to trigger at approximately 85% of the unit's available depth level for oil capture. The feature acts as a safeguard against spills caused by exceeding the oil storage capacity of the separator and eliminates the need for manual oil level inspection.

The oil level alarm installed on the Stormceptor insert is illustrated in Figure 4.

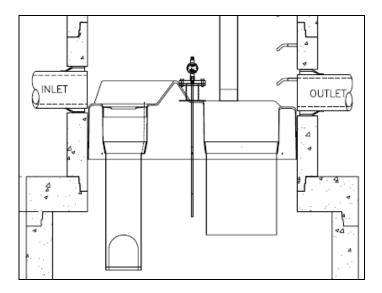


Figure 4. Oil level alarm

6.2. Increased Volume Storage Capacity

The Stormceptor unit may be modified to store a greater spill volume than is typically available. Under such a scenario, instead of installing a larger than required unit, modifications can be made to the recommended Stormceptor model to accommodate larger volumes. Contact your local Stormceptor representative for additional information and assistance for modifications.

7. Stormceptor Options

The Stormceptor System allows flexibility to incorporate to existing and new storm drainage infrastructure. The following section identifies considerations that should be reviewed when installing the system into a drainage network. For conditions that fall outside of the recommendations in this section, please contact your local Stormceptor representative for further guidance.

7.1. Installation Depth Minimum Cover

The minimum distance from the top of grade to the crown of the inlet pipe is 24 inches (600 mm). For situations that have a lower minimum distance, contact your local Stormceptor representative.

7.2. Maximum Inlet and Outlet Pipe Diameters

Maximum inlet and outlet pipe diameters are illustrated in Figure 5. Contact your local Stormceptor representative for larger pipe diameters

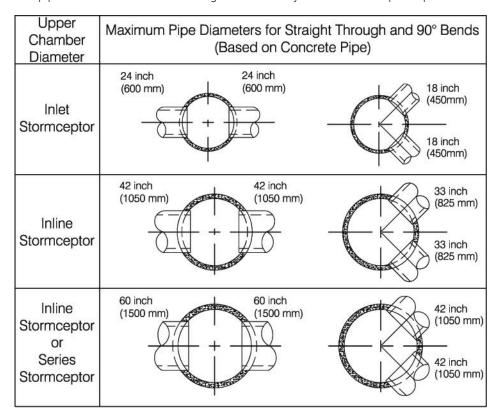


Figure 5. Maximum pipe diameters for straight through and bend applications

7.3. Bends

The Stormceptor System can be used to change horizontal alignment in the storm drain network up to a maximum of 90 degrees. Figure 6 illustrates the typical bend situations of the Stormceptor System. Bends should only be applied to the second structure (downstream structure) of the Series Stormceptor System.

^{*}The bend should only be incorporated into the second structure (downstream structure) of the Series Stormceptor System

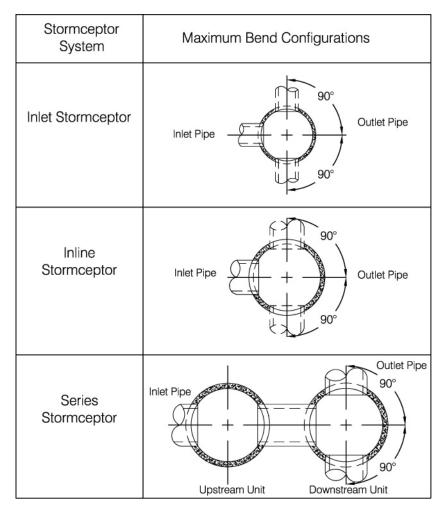


Figure 6. Maximum bend angles

7.4. Multiple Inlet Pipes

The Inlet and Inline Stormceptor System can accommodate two or more inlet pipes. The maximum number of inlet pipes that can be accommodated into a Stormceptor unit is a function of the number, alignment and diameter of the pipes and its effects on the structural integrity of the precast concrete. When multiple inlet pipes are used for new developments, each inlet pipe shall have an invert elevation 3 inches (75 mm) higher than the outlet pipe invert elevation.

7.5. Inlet/Outlet Pipe Invert Elevations

Recommended inlet and outlet pipe invert differences are listed in Table 3.

Table 3. Recommended Drops Between Inlet and Outlet Pipe Inverts

Number of Inlet Pipes	Inlet System	In-Line System	Series System
1	3 inches (75 mm)	1 inch (25 mm)	3 inches (75 mm)
>1	3 inches (75 mm)	3 inches (75 mm)	Not Applicable

7.6. Shallow Stormceptor

In cases where there may be restrictions to the depth of burial of storm sewer systems. In this situation, for selected Stormceptor models, the lower chamber components may be increased in diameter to reduce the overall depth of excavation required.

7.7. Customized Live Load

The Stormceptor system is typically designed for local highway truck loading (AASHTO HS- 20). When the project requires live loads greater than HS-20, the Stormceptor System may be customized structurally for a pre-specified live load. Contact your local Stormceptor representative for customized loading conditions.

7.8. Pre-treatment

The Stormceptor System may be sized to remove sediment and for spills control in conjunction with other stormwater BMPs to meet the water quality objective. For pretreatment applications, the Stormceptor System should be the first unit in a treatment train. The benefits of pre-treatment include the extension of the operational life (extension of maintenance frequency) of large stormwater management facilities, prevention of spills and lower total life- cycle maintenance cost.

7.9. Head loss

The head loss through the Stormceptor System is similar to a 60 degree bend at a manhole. The K value for calculating minor losses is approximately 1.3 (minor loss = k*1.3v2/2g).

However, when a Submerged modification is applied to a Stormceptor unit, the corresponding K value is 4.

7.10. Submerged

The Submerged modification, Figure 7, allows the Stormceptor System to operate in submerged or partially submerged storm sewers. This configuration can be installed on all models of the Stormceptor System by modifying the fiberglass insert. A customized weir height and a secondary drop tee are added.

Submerged instances are defined as standing water in the storm drain system during zero flow conditions. In these instances, the following information is necessary for the proper design and application of submerged modifications:

- Stormceptor top of grade elevation
- Stormceptor outlet pipe invert elevation
- · Standing water elevation

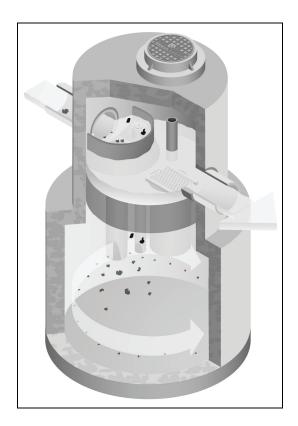


Figure 7. Submerged Stormceptor

8. Comparing Technologies

Designers have many choices available to achieve water quality goals in the treatment of stormwater runoff. Since many alternatives are available for use in stormwater quality treatment it is important to consider how to make an appropriate comparison between "approved alternatives". The following is a guide to assist with the accurate comparison of differing technologies and performance claims.

8.1. Particle Size Distribution (PSD)

The most sensitive parameter to the design of a stormwater quality device is the selection of the design particle size. While it is recommended that the actual particle size distribution (PSD) for sites be measured prior to sizing, alternative values for particle size should be selected to represent what is likely to occur naturally on the site. A reasonable estimate of a particle size distribution likely to be found on parking lots or other impervious surfaces should consist of a wide range of particles such as 20 microns to 2,000 microns (Ontario MOE, 1994).

There is no absolute right particle size distribution or specific gravity and the user is cautioned to review the site location, characteristics, material handling practices and regulatory requirements when selecting a particle size distribution. When comparing technologies, designs using different PSDs will result in incomparable TSS removal efficiencies. The PSD of the TSS removed needs to be standard between two products to allow for an accurate comparison.

8.2. Scour Prevention

In order to accurately predict the performance of a manufactured treatment device, there must be confidence that it will perform under all conditions. Since rainfall patterns cannot be predicted, stormwater quality devices placed in storm sewer systems must be able to withstand extreme events, and ensure that all pollutants previously captured are retained in the system.

In order to have confidence in a system's performance under extreme conditions, independent validation of scour prevention is essential when examining different technologies. Lack of independent verification of scour prevention should make a designer wary of accepting any product's performance claims.

8.3. Hydraulics

Full scale laboratory testing has been used to confirm the hydraulics of the Stormceptor System. Results of lab testing have been used to physically design the Stormceptor System and the sewer pipes entering and leaving the unit. Key benefits of Stormceptor are:

- Low head loss (typical k value of 1.3)
- Minimal inlet/outlet invert elevation drop across the structure
- Use as a bend structure
- Accommodates multiple inlets

The adaptability of the treatment device to the storm sewer design infrastructure can affect the overall performance and cost of the site.

8.4. Hydrology

Stormwater quality treatment technologies need to perform under varying climatic conditions. These can vary from long low intensity rainfall to short duration, high intensity storms. Since a treatment device is expected to perform under all these conditions, it makes sense that any system's design should accommodate those conditions as well.

Long-term continuous simulation evaluates the performance of a technology under the varying conditions expected in the climate of the subject site. Single, peak event design does not provide this information and is not equivalent to long-term simulation. Designers should request long-term simulation performance to ensure the technology can meet the long-term water quality objective.

9. Testing

The Stormceptor System has been the most widely monitored stormwater treatment technology in the world. Performance verification and monitoring programs are completed to the strictest standards and integrity. Since its introduction in 1990, numerous independent field tests and studies detailing the effectiveness of the Stormceptor System have been completed.

- Coventry University, UK 97% removal of oil, 83% removal of sand and 73% removal of peat
- National Water Research Institute, Canada, scaled testing for the development of the Stormceptor System identifying both TSS removal and scour prevention.
- New Jersey TARP Program full scale testing of an STC 900 demonstrating 75% TSS removal of particles from 1 to 1000 microns. Scour testing completed demonstrated that the system does not scour. The New Jersey Department of Environmental Protection was followed.
- City of Indianapolis full scale testing of an STC 900 demonstrating over 80% TSS removal of particles from 50 microns to 300 microns at 130% of the unit's operating rate. Scour testing completed demonstrated that the system does not scour.
- Westwood Massachusetts (1997), demonstrated >80% TSS removal
- Como Park (1997), demonstrated 76% TSS removal
- Ontario MOE SWAMP Program 57% removal of 1 to 25 micron particles
- Laval Quebec 50% removal of 1 to 25 micron particles

10. Installation

The installation of the concrete Stormceptor should conform in general to state highway, or local specifications for the installation of manholes. Selected sections of a general specification that are applicable are summarized in the following sections.

10.1. Excavation

Excavation for the installation of the Stormceptor should conform to state highway, or local specifications. Topsoil removed during the excavation for the Stormceptor should be stockpiled in designated areas and should not be mixed with subsoil or other materials.

Topsoil stockpiles and the general site preparation for the installation of the Stormceptor should conform to state highway or local specifications.

The Stormceptor should not be installed on frozen ground. Excavation should extend a minimum of 12 inches (300 mm) from the precast concrete surfaces plus an allowance for shoring and bracing where required. If the bottom of the excavation provides an unsuitable foundation additional excavation may be required.

In areas with a high water table, continuous dewatering may be required to ensure that the excavation is stable and free of water.

10.2. Backfilling

Backfill material should conform to state highway or local specifications. Backfill material should be placed in uniform layers not exceeding 12 inches (300mm) in depth and compacted to state highway or local specifications.

11. Stormceptor Construction Sequence

The concrete Stormceptor is installed in sections in the following sequence:

- 1. Aggregate base
- 2. Base slab
- 3. Lower chamber sections
- 4. Upper chamber section with fiberglass insert
- 5. Connect inlet and outlet pipes
- 6. Assembly of fiberglass insert components (drop tee, riser pipe, oil cleanout port and orifice plate
- 7. Remainder of upper chamber
- 8. Frame and access cover

The precast base should be placed level at the specified grade. The entire base should be in contact with the underlying compacted granular material. Subsequent sections, complete with joint seals, should be installed in accordance with the precast concrete manufacturer's recommendations.

Adjustment of the Stormceptor can be performed by lifting the upper sections free of the excavated area, re-leveling the base and reinstalling the sections. Damaged sections and gaskets should be repaired or replaced as necessary. Once the Stormceptor has been constructed, any lift holes must be plugged with mortar.

12. Maintenance

12.1. Health and Safety

The Stormceptor System has been designed considering safety first. It is recommended that confined space entry protocols be followed if entry to the unit is required. In addition, the fiberglass insert has the following health and safety features:

- Designed to withstand the weight of personnel
- A safety grate is located over the 24 inch (600 mm) riser pipe opening
- Ladder rungs can be provided for entry into the unit, if required

12.2. Maintenance Procedures

Maintenance of the Stormceptor system is performed using vacuum trucks. No entry into the unit is required for maintenance (in most cases). The vacuum service industry is a well- established sector of the service industry that cleans underground tanks, sewers and catch basins. Costs to clean a Stormceptor will vary based on the size of unit and transportation distances.

The need for maintenance can be determined easily by inspecting the unit from the surface. The depth of oil in the unit can be determined by inserting a dipstick in the oil inspection/cleanout port.

Similarly, the depth of sediment can be measured from the surface without entry into the Stormceptor via a dipstick tube equipped with a ball valve. This tube would be inserted through the riser pipe. Maintenance should be performed once the sediment depth exceeds the guideline values provided in the Table 4.

Particle Size	Specific Gravity	
Model	Sediment Depth inches (mm)	
450i	8 (200)	
900	8 (200)	
1200	10 (250)	
1800	15 (381)	
2400	12 (300)	
3600	17 (430)	
4800	15 (380)	
6000	18 (460)	
7200	15 (381)	
11000	17 (380)	
13000	20 (500)	
16000	17 (380)	
* based on 15% of the Stormceptor unit's total storage		

Table 4. Sediment Depths Indicating Required Servicing*

Although annual servicing is recommended, the frequency of maintenance may need to be increased or reduced based on local conditions (i.e. if the unit is filling up with sediment more quickly than projected, maintenance may be required semi-annually; conversely once the site has stabilized maintenance may only be required every two or three years).

Oil is removed through the oil inspection/cleanout port and sediment is removed through the riser pipe. Alternatively oil could be removed from the 24 inches (600 mm) opening if water is removed from the lower chamber to lower the oil level below the drop pipes.

The following procedures should be taken when cleaning out Stormceptor:

- 1. Check for oil through the oil cleanout port
- 2. Remove any oil separately using a small portable pump
- 3. Decant the water from the unit to the sanitary sewer, if permitted by the local regulating authority, or into a separate containment tank
- 4. Remove the sludge from the bottom of the unit using the vacuum truck
- 5. Re-fill Stormceptor with water where required by the local jurisdiction

12.3. Submerged Stormceptor

Careful attention should be paid to maintenance of the Submerged Stormceptor System. In cases where the storm drain system is submerged, there is a requirement to plug both the inlet and outlet pipes to economically clean out the unit.

12.4. Hydrocarbon Spills

The Stormceptor is often installed in areas where the potential for spills is great. The Stormceptor System should be cleaned immediately after a spill occurs by a licensed liquid waste hauler.

12.5. Disposal

Requirements for the disposal of material from the Stormceptor System are similar to that of any other stormwater Best Management Practice (BMP) where permitted. Disposal options for the sediment may range from disposal in a sanitary trunk sewer upstream of a sewage treatment plant, to disposal in a sanitary landfill site. Petroleum waste products collected in the Stormceptor (free oil/chemical/fuel spills) should be removed by a licensed waste management company.

12.6. Oil Sheens

With a steady influx of water with high concentrations of oil, a sheen may be noticeable at the Stormceptor outlet. This may occur because a rainbow or sheen can be seen at very small oil concentrations (<10 mg/L). Stormceptor will remove over 98% of all free oil spills from storm sewer systems for dry weather or frequently occurring runoff events.

The appearance of a sheen at the outlet with high influent oil concentrations does not mean the unit is not working to this level of removal. In addition, if the influent oil is emulsified the Stormceptor will not be able to remove it. The Stormceptor is designed for free oil removal and not emulsified conditions.



SUPPORT

Drawings and specifications are available at www.ContechES.com. Site-specific design support is available from our engineers.

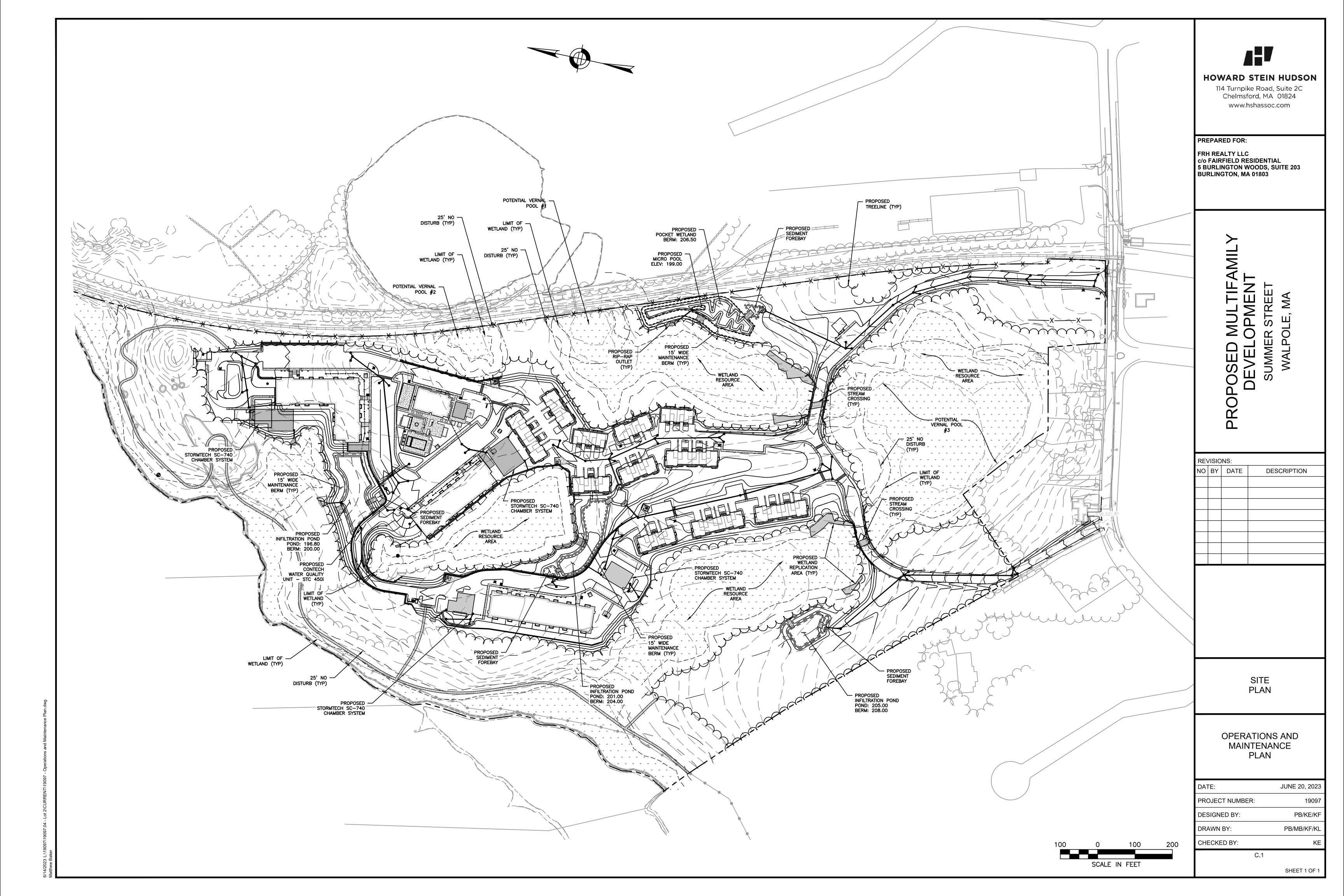
©2020 Contech Engineered Solutions LLC, a QUIKRETE Company

Contech Engineered Solutions LLC provides site solutions for the civil engineering industry. Contech's portfolio includes bridges, drainage, sanitary sewer, stormwater, and earth stabilization products. For information, visit www.ContechES.com or call 800.338.1122

NOTHING IN THIS CATALOG SHOULD BE CONSTRUED AS A WARRANTY. APPLICATIONS SUGGESTED HEREIN ARE DESCRIBED ONLY TO HELP READERS MAKE THEIR OWN EVALUATIONS AND DECISIONS, AND ARE NEITHER GUARANTEES NOR WARRANTIES OF SUITABILITY FOR ANY APPLICATION. CONTECH MAKES NO WARRANTY WHATSOEVER, EXPRESS OR IMPLIED, RELATED TO THE APPLICATIONS, MATERIALS, COATINGS, OR PRODUCTS DISCUSSED HEREIN. ALL IMPLIED WARRANTIES OF MERCHANTABILITY AND ALL IMPLIED WARRANTIES OF FITNESS FOR ANY PARTICULAR PURPOSE ARE DISCLAIMED BY CONTECH. SEE CONTECH'S CONDITIONS OF SALE (AVAILABLE AT WWW.CONTECHES.COM/COS) FOR MORE INFORMATION.

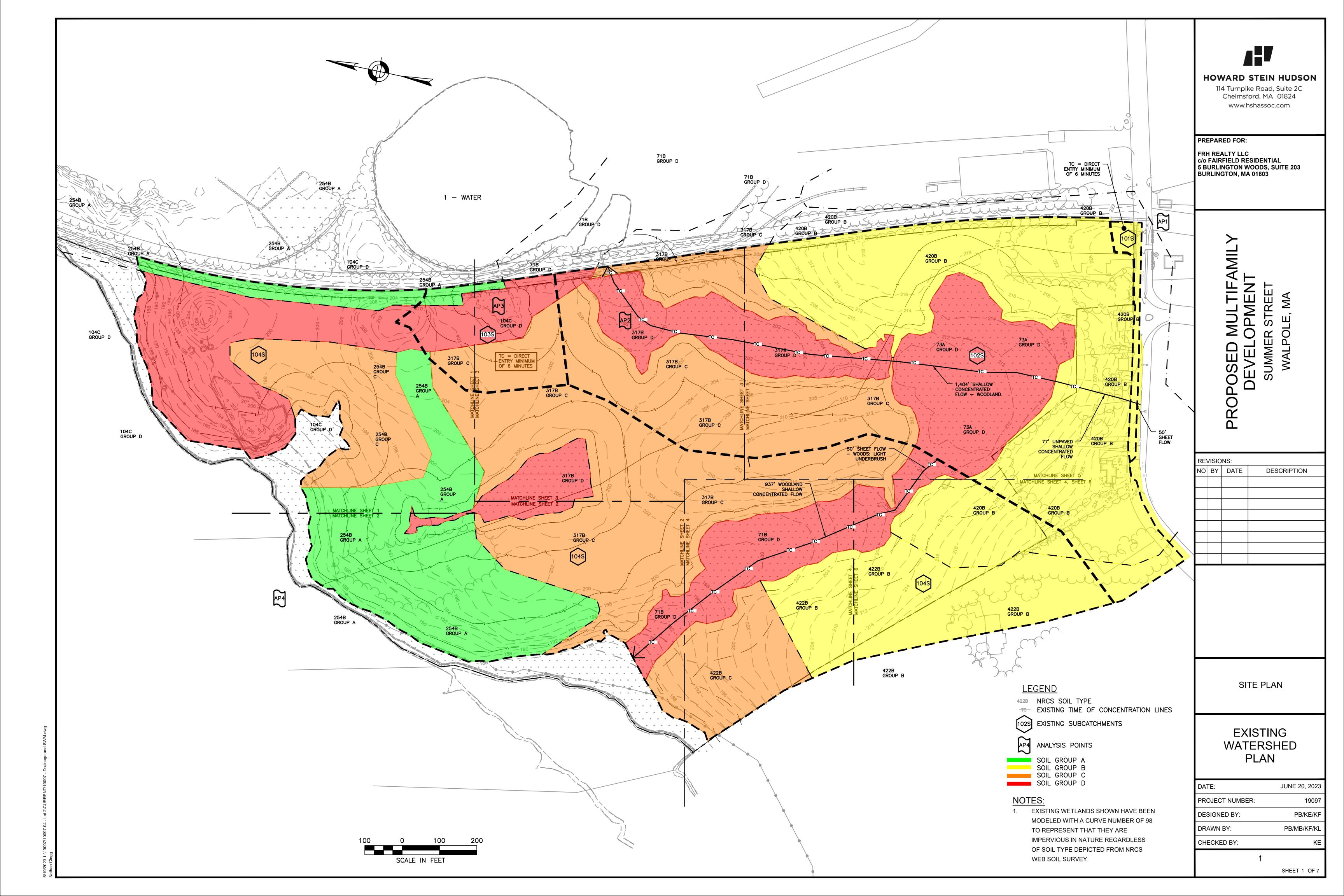


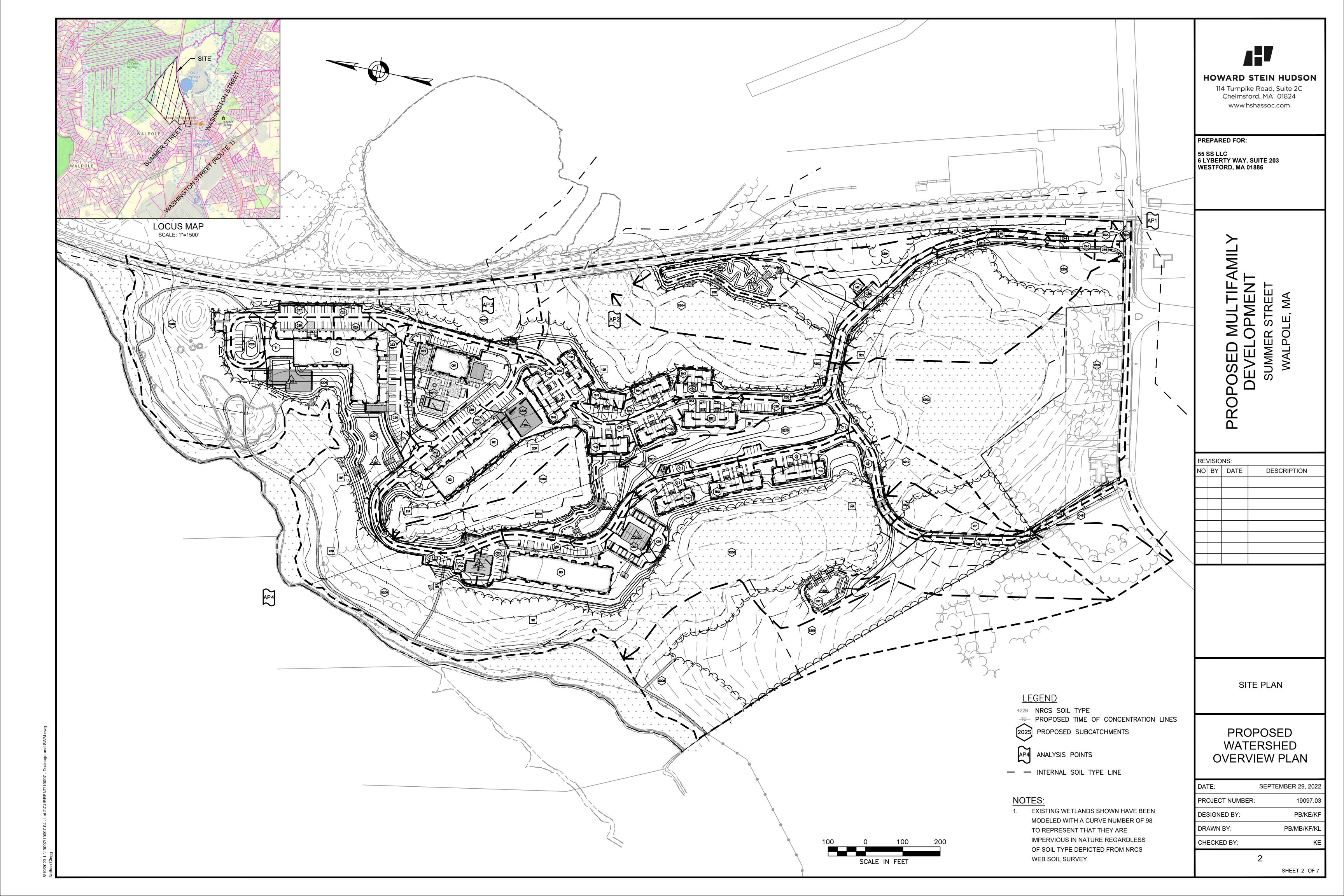
Appendix D: O&M and LTPPP Plan

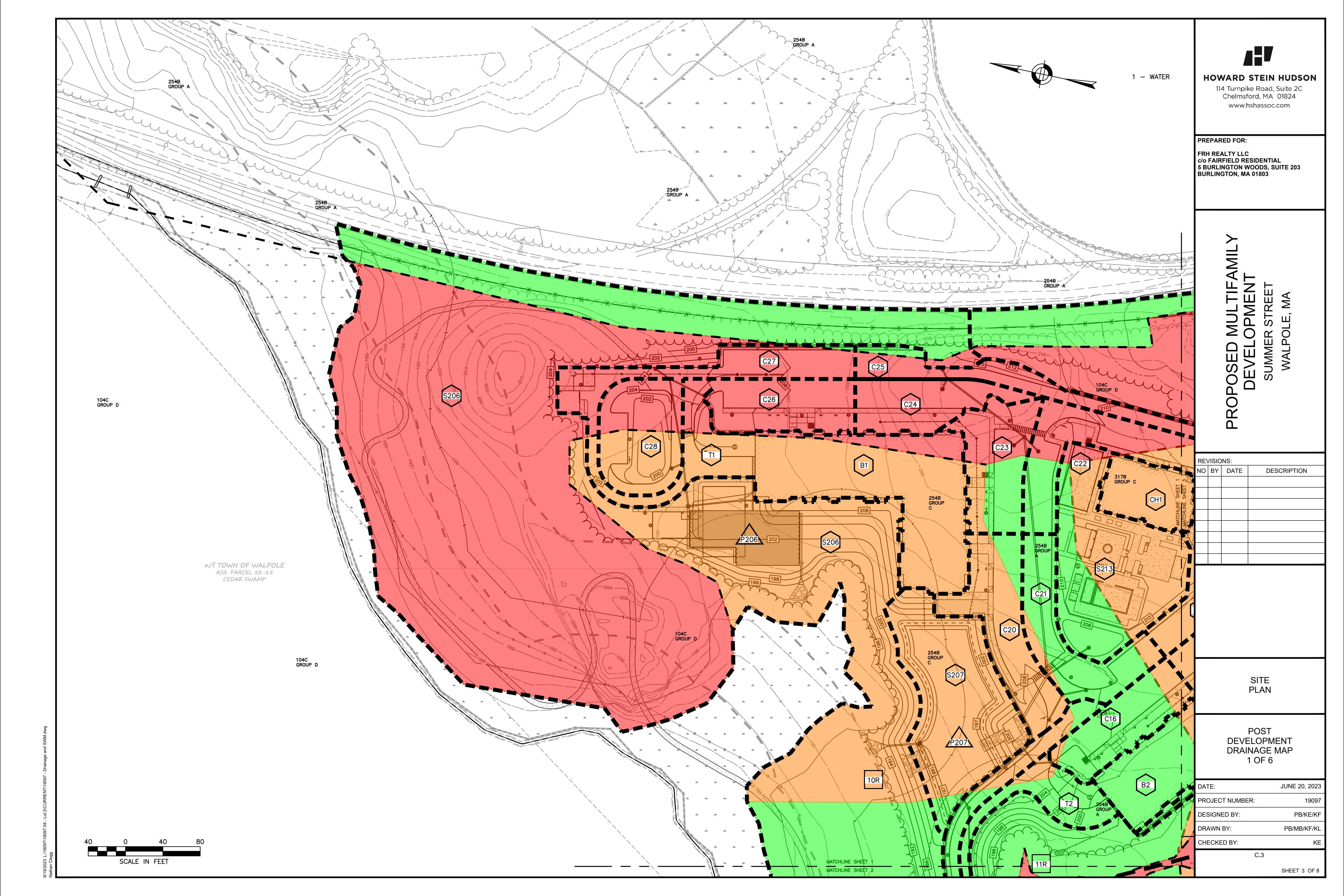


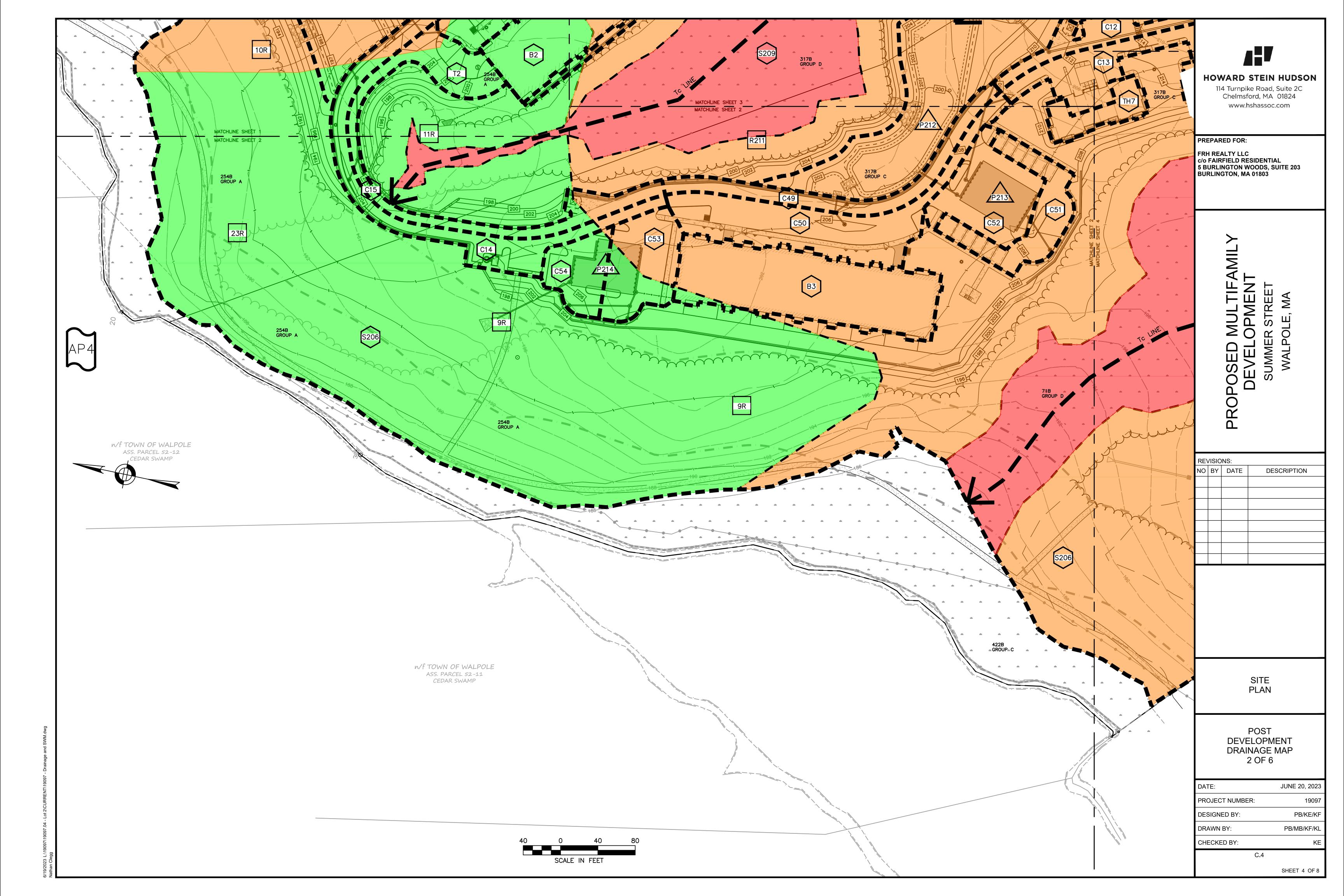


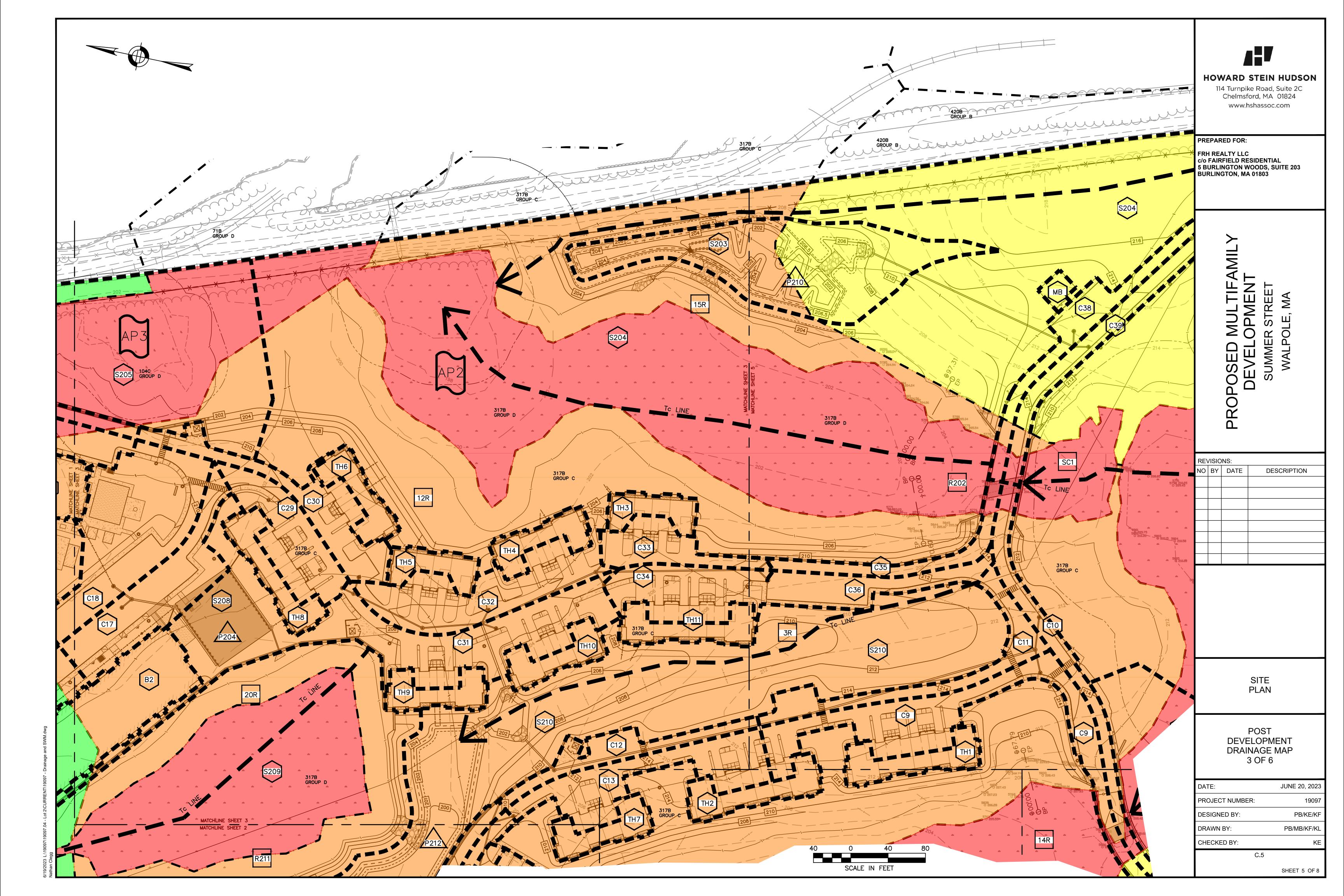
Appendix D: Pre and Post Drainage Maps

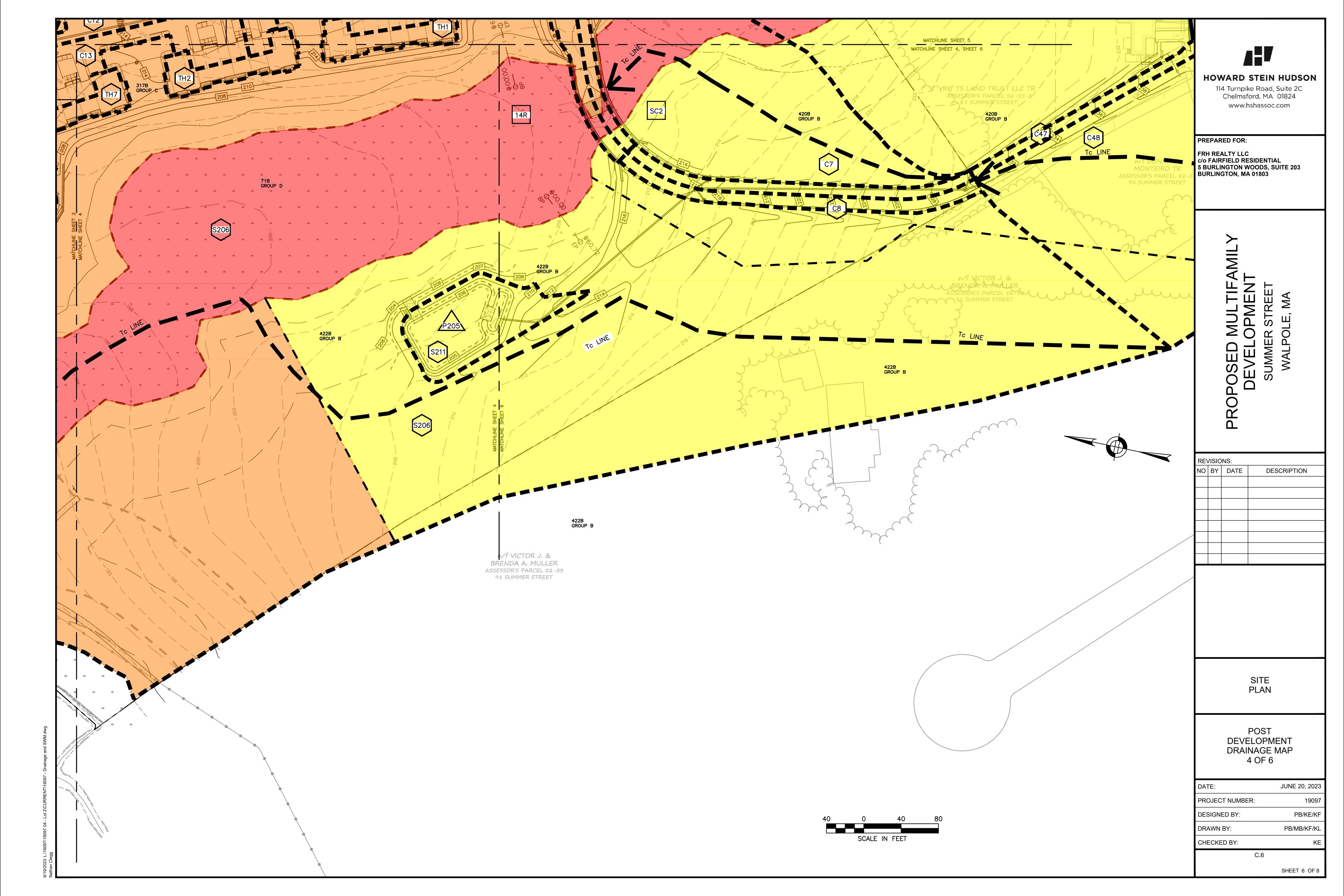


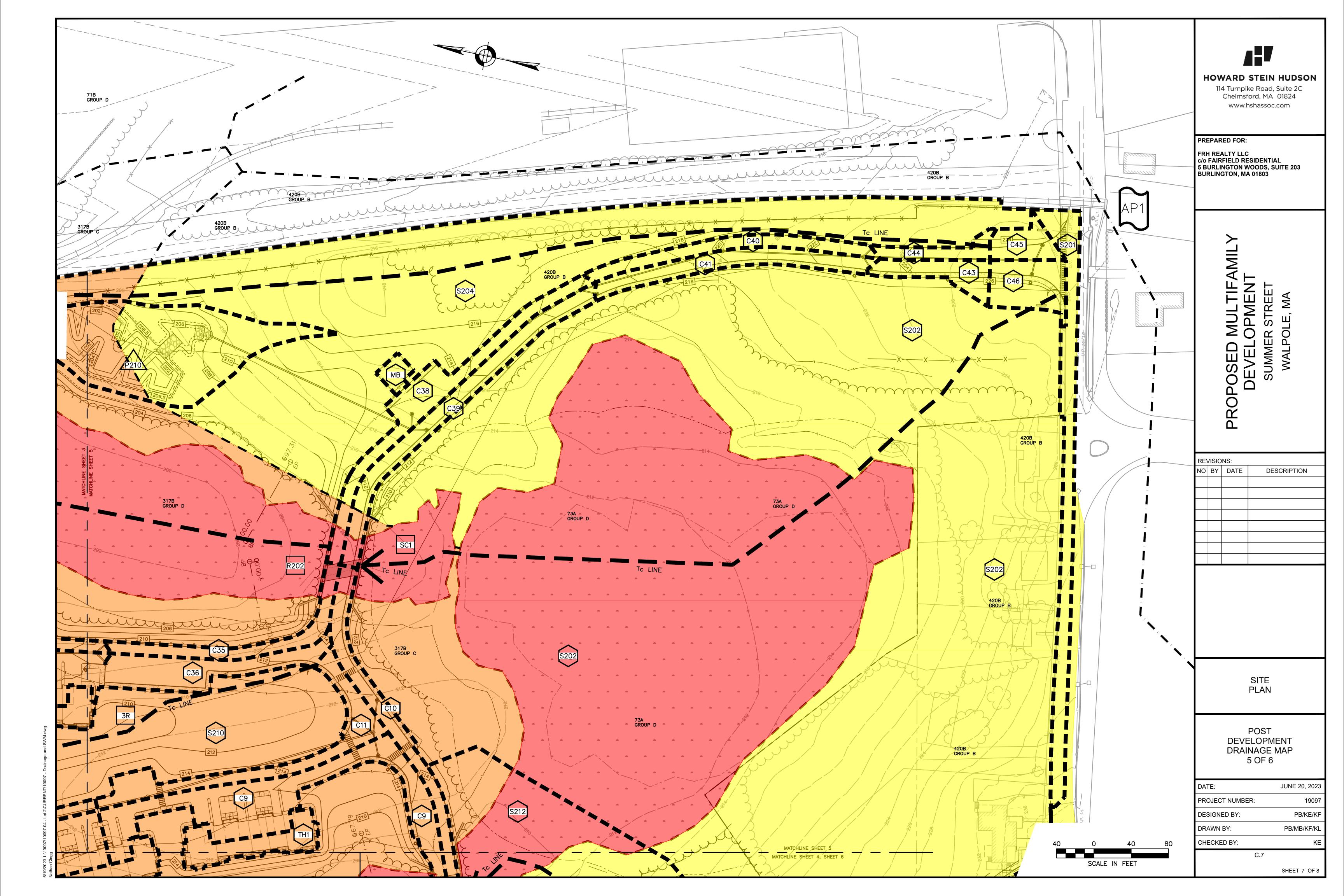


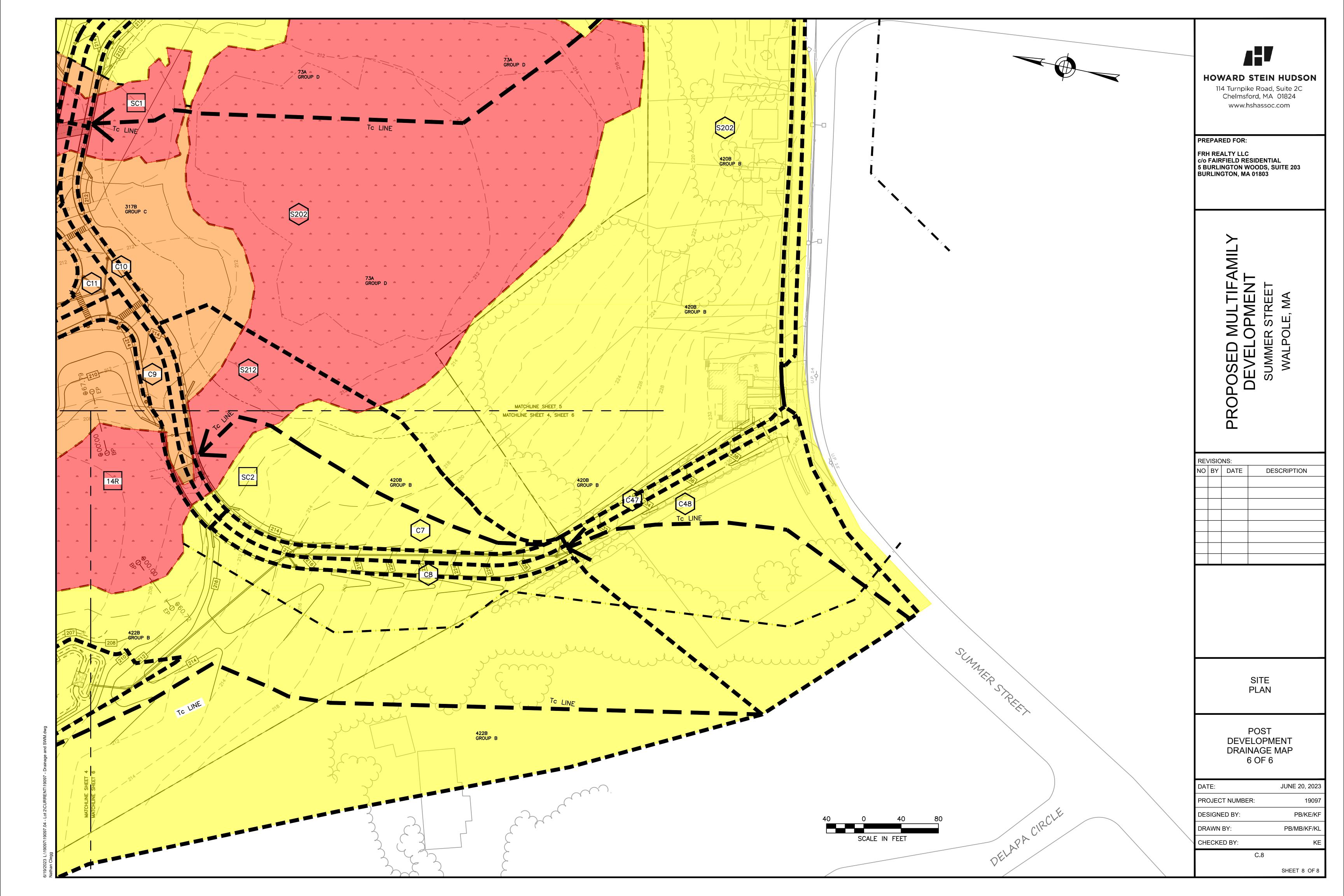






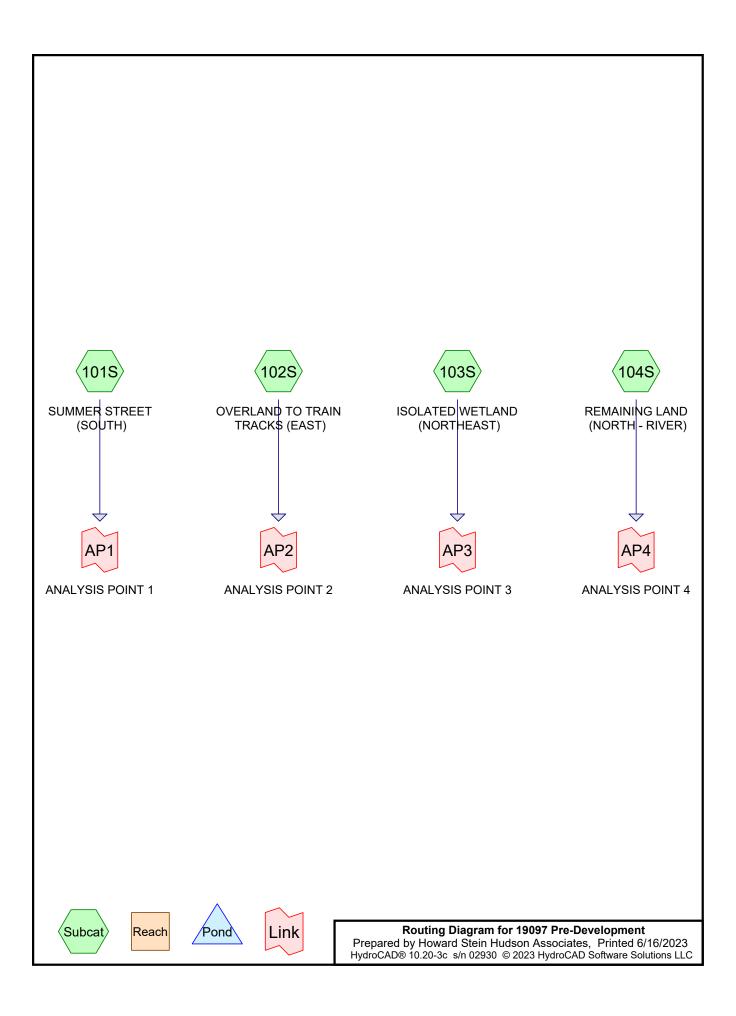








Appendix E: HydroCAD, Stage Storage, and Pocket Wetland Calculations



Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023 Page 2

Project Notes

Rainfall events imported from "19097 Post-Development.hcp"

19097 Pre-Development
Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023 Page 3

Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2YR	Type III 24-hr		Default	24.00	1	3.27	2
2	10YR	Type III 24-hr		Default	24.00	1	4.96	2
3	25YR	Type III 24-hr		Default	24.00	1	6.29	2
4	100YR	Type III 24-hr		Default	24.00	1	9.06	2

Printed 6/16/2023 Page 4

Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
143,648	68	1 acre lots, 20% imp, HSG B (102S, 104S)
549	79	1 acre lots, 20% imp, HSG C (104S)
179,555	61	>75% Grass cover, Good, HSG B (101S, 102S)
15,945	74	>75% Grass cover, Good, HSG C (102S)
3,192	80	>75% Grass cover, Good, HSG D (102S)
89,402	30	Brush, Good, HSG A (103S, 104S)
2,920	65	Brush, Good, HSG C (104S)
4,643	73	Brush, Good, HSG D (103S)
1,262	96	Gravel surface, HSG B (101S)
33,283	98	Paved parking, HSG B (101S, 102S)
448,007	98	Water Surface, 0% imp, HSG D (102S, 103S, 104S)
212,938	30	Woods, Good, HSG A (103S, 104S)
358,427	55	Woods, Good, HSG B (102S, 104S)
866,259	70	Woods, Good, HSG C (102S, 103S, 104S)
213,890	77	Woods, Good, HSG D (102S, 103S, 104S)
2,573,920	68	TOTAL AREA

19097 Pre-Development
Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023 Page 5

Soil Listing (all nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
302,340	HSG A	103S, 104S
716,175	HSG B	101S, 102S, 104S
885,673	HSG C	102S, 103S, 104S
669,732	HSG D	102S, 103S, 104S
0	Other	
2,573,920		TOTAL AREA

19097 Pre-Development
Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023 Page 6

Ground Covers (all nodes)

HSC	S-A	HSG-B	HSG-C	SG-C HSG-D		Total	Ground
(sc	ı-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover
	0	143,648	549	0	0	144,197	1 acre lots, 20% imp
	0	179,555	15,945	3,192	0	198,692	>75% Grass cover, Good
89,4	102	0	2,920	4,643	0	96,965	Brush, Good
	0	1,262	0	0	0	1,262	Gravel surface
	0	33,283	0	0	0	33,283	Paved parking
	0	0	0	448,007	0	448,007	Water Surface, 0% imp
212,9	938	358,427	866,259	213,890	0	1,651,514	Woods, Good
302,	340	716,175	885,673	669,732	0	2,573,920	TOTAL AREA

Su Nυ

Type III 24-hr 2YR Rainfall=3.27" Printed 6/16/2023

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 7

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment101S: SUMMER STREET Runoff Area=13,756 sf 64.57% Impervious Runoff Depth>2.06"

Tc=6.0 min CN=88 Runoff=0.74 cfs 2,360 cf

Subcatchment102S: OVERLANDTO Runoff Area=956,300 sf 3.58% Impervious Runoff Depth>1.07"

Flow Length=1,531' Tc=44.5 min CN=74 Runoff=12.30 cfs 85,349 cf

Subcatchment103S: ISOLATED Runoff Area=105,094 sf 0.00% Impervious Runoff Depth>0.97"

Tc=6.0 min CN=72 Runoff=2.52 cfs 8,514 cf

Subcatchment104S: REMAINING LAND Runoff Area=1,498,770 sf 1.27% Impervious Runoff Depth>0.59"

Flow Length=987' Tc=28.6 min CN=64 Runoff=10.77 cfs 73,247 cf

Link AP1: ANALYSISPOINT 1 Inflow=0.74 cfs 2,360 cf

Primary=0.74 cfs 2,360 cf

Link AP2: ANALYSISPOINT 2 Inflow=12.30 cfs 85,349 cf

Primary=12.30 cfs 85,349 cf

Link AP3: ANALYSISPOINT 3 Inflow=2.52 cfs 8,514 cf

Primary=2.52 cfs 8,514 cf

Link AP4: ANALYSISPOINT 4 Inflow=10.77 cfs 73,247 cf

Primary=10.77 cfs 73,247 cf

Total Runoff Area = 2,573,920 sf Runoff Volume = 169,469 cf Average Runoff Depth = 0.79" 97.59% Pervious = 2,511,798 sf 2.41% Impervious = 62,122 sf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023

Page 8

Summary for Subcatchment 101S: SUMMER STREET (SOUTH)

Runoff = 0.74 cfs @ 12.09 hrs, Volume= 2,360 cf, Depth> 2.06"

Routed to Link AP1: ANALYSIS POINT 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description							
	3,612	61	>75% Gras	s cover, Go	ood, HSG B					
	8,882	98	Paved park	ing, HSG E	В					
	1,262	96	Gravel surfa	ace, HSG E	В					
	13,756	88	Weighted A	verage						
	4,874		35.43% Pervious Area							
	8,882		64.57% Impervious Area							
Tc	Length	Slope	e Velocity	Capacity	Description					
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment 102S: OVERLAND TO TRAIN TRACKS (EAST)

Runoff = 12.30 cfs @ 12.66 hrs, Volume= 85,349 cf, Depth> 1.07"

Routed to Link AP2: ANALYSIS POINT 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Area (sf)	CN	Description
175,943	61	>75% Grass cover, Good, HSG B
184,742	55	Woods, Good, HSG B
24,401	98	Paved parking, HSG B
49,311	68	1 acre lots, 20% imp, HSG B
15,945	74	>75% Grass cover, Good, HSG C
235,274	70	Woods, Good, HSG C
3,192	80	>75% Grass cover, Good, HSG D
1,550	77	Woods, Good, HSG D
265,942	98	Water Surface, 0% imp, HSG D
956,300	74	Weighted Average
922,037		96.42% Pervious Area
34,263		3.58% Impervious Area

Prepared by Howard Stein Hudson Associates

Printed 6/16/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 9

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	3.2	50	0.0800	0.26	(0.0)	Sheet Flow,
	0.2	00	0.0000	0.20		Grass: Short n= 0.150 P2= 3.27"
	0.3	77	0.0780	4.50		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	41.0	1,404	0.0130	0.57		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	44.5	1,531	Total			

Summary for Subcatchment 103S: ISOLATED WETLAND (NORTHEAST)

Runoff = 2.52 cfs @ 12.10 hrs, Volume= 8

8,514 cf, Depth> 0.97"

Routed to Link AP3: ANALYSIS POINT 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Area (sf)	CN	Description	
5,813	30	Brush, Good, HSG A	
1,646	30	Woods, Good, HSG A	
48,198	70	Woods, Good, HSG C	
4,643	73	Brush, Good, HSG D	
35,989	77	Woods, Good, HSG D	
8,805	98	Water Surface, 0% imp, HSG D	
105,094	72	Weighted Average	
105,094		100.00% Pervious Area	
Tc Length	n Slo _l	pe Velocity Capacity Description	
(min) (feet)) (ft/	/ft) (ft/sec) (cfs)	
6.0		Direct Entry,	

Summary for Subcatchment 104S: REMAINING LAND (NORTH - RIVER)

Runoff = 10.77 cfs @ 12.50 hrs, Volume= 73,247 cf, Depth> 0.59"

Routed to Link AP4: ANALYSIS POINT 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates

Printed 6/16/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

<u>Page 10</u>

	Ar	rea (sf)	CN [Description		
		83,589	30 E	Brush, Goo	d, HSG A	
	2	11,292	30 \	Voods, Go	od, HSG A	
	1	73,685	55 \	Voods, Go	od, HSG B	
	!	94,337		,	20% imp, I	
		549	79 <i>´</i>	l acre lots,	20% imp, I	HSG C
		2,920		Brush, Goo	,	
		82,787		•	od, HSG C	
		73,260			ace, 0% imp	
	1	76,351	77 \	<u> Voods, Go</u>	od, HSG D	
	1,4	98,770	64 \	Veighted A	verage	
	1,4	79,793	ć	98.73% Pe	rvious Area	
		18,977	•	1.27% Impe	ervious Are	a
	_		01			B
,	Tc	Length	Slope			Description
	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
(9.2	50	0.0400	0.09		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.27"
19	9.4	937	0.0260	0.81		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
28	8.6	987	Total			

Summary for Link AP1: ANALYSIS POINT 1

Inflow Area = 13,756 sf, 64.57% Impervious, Inflow Depth > 2.06" for 2YR event

Inflow = 0.74 cfs @ 12.09 hrs, Volume= 2,360 cf

Primary = 0.74 cfs @ 12.09 hrs, Volume= 2,360 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP2: ANALYSIS POINT 2

Inflow Area = 956,300 sf, 3.58% Impervious, Inflow Depth > 1.07" for 2YR event

Inflow = 12.30 cfs @ 12.66 hrs, Volume= 85,349 cf

Primary = 12.30 cfs @ 12.66 hrs, Volume= 85,349 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP3: ANALYSIS POINT 3

Inflow Area = 105,094 sf, 0.00% Impervious, Inflow Depth > 0.97" for 2YR event

Inflow = 2.52 cfs @ 12.10 hrs, Volume= 8,514 cf

Primary = 2.52 cfs @ 12.10 hrs, Volume= 8,514 cf, Atten= 0%, Lag= 0.0 min

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023

Page 11

Summary for Link AP4: ANALYSIS POINT 4

Inflow Area = 1,498,770 sf, 1.27% Impervious, Inflow Depth > 0.59" for 2YR event

Inflow = 10.77 cfs @ 12.50 hrs, Volume= 73,247 cf

Primary = 10.77 cfs @ 12.50 hrs, Volume= 73,247 cf, Atten= 0%, Lag= 0.0 min

Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023 Page 12

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment101S: SUMMER STREET Runoff Area=13,756 sf 64.57% Impervious Runoff Depth>3.63"

Tc=6.0 min CN=88 Runoff=1.28 cfs 4,159 cf

Subcatchment102S: OVERLANDTO Runoff Area=956,300 sf 3.58% Impervious Runoff Depth>2.31"

Flow Length=1,531' Tc=44.5 min CN=74 Runoff=27.75 cfs 184,006 cf

Subcatchment103S: ISOLATED Runoff Area=105,094 sf 0.00% Impervious Runoff Depth>2.16"

Tc=6.0 min CN=72 Runoff=5.96 cfs 18,960 cf

Subcatchment104S: REMAINING LAND Runoff Area=1,498,770 sf 1.27% Impervious Runoff Depth>1.54" Flow Length=987' Tc=28.6 min CN=64 Runoff=33.90 cfs 192,708 cf

Link AP1: ANALYSISPOINT 1 Inflow=1.28 cfs 4,159 cf

Primary=1.28 cfs 4,159 cf

Link AP2: ANALYSISPOINT 2 Inflow=27.75 cfs 184,006 cf

Primary=27.75 cfs 184,006 cf

Link AP3: ANALYSISPOINT 3 Inflow=5.96 cfs 18,960 cf

Primary=5.96 cfs 18,960 cf

Link AP4: ANALYSISPOINT 4 Inflow=33.90 cfs 192,708 cf

Primary=33.90 cfs 192,708 cf

Total Runoff Area = 2,573,920 sf Runoff Volume = 399,833 cf Average Runoff Depth = 1.86" 97.59% Pervious = 2,511,798 sf 2.41% Impervious = 62,122 sf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023

Page 13

Summary for Subcatchment 101S: SUMMER STREET (SOUTH)

Runoff = 1.28 cfs @ 12.09 hrs, Volume= 4,159 cf, Depth> 3.63"

Routed to Link AP1: ANALYSIS POINT 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

	\rea (sf)	CN	Description							
	3,612	61	>75% Gras	s cover, Go	ood, HSG B					
	8,882	98	Paved park	ing, HSG E	В					
	1,262	96	Gravel surfa	ace, HSG E	В					
	13,756	88	88 Weighted Average							
	4,874		35.43% Pervious Area							
	8,882		64.57% lm	pervious Ar	rea					
_										
Tc	3	Slope	,	Capacity	·					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment 102S: OVERLAND TO TRAIN TRACKS (EAST)

Runoff = 27.75 cfs @ 12.62 hrs, Volume= 184,006 cf, Depth> 2.31"

Routed to Link AP2: ANALYSIS POINT 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Area (sf)	CN	Description
175,943	61	>75% Grass cover, Good, HSG B
184,742	55	Woods, Good, HSG B
24,401	98	Paved parking, HSG B
49,311	68	1 acre lots, 20% imp, HSG B
15,945	74	>75% Grass cover, Good, HSG C
235,274	70	Woods, Good, HSG C
3,192	80	>75% Grass cover, Good, HSG D
1,550	77	Woods, Good, HSG D
265,942	98	Water Surface, 0% imp, HSG D
956,300	74	Weighted Average
922,037		96.42% Pervious Area
34,263		3.58% Impervious Area

Prepared by Howard Stein Hudson Associates

Printed 6/16/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 14

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	3.2	50	0.0800	0.26	, ,	Sheet Flow,
						Grass: Short n= 0.150 P2= 3.27"
	0.3	77	0.0780	4.50		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	41.0	1,404	0.0130	0.57		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	44.5	1,531	Total			

Summary for Subcatchment 103S: ISOLATED WETLAND (NORTHEAST)

Runoff = 5.96 cfs @ 12.10 hrs, Volume=

18,960 cf, Depth> 2.16"

Routed to Link AP3: ANALYSIS POINT 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Area (sf)	CN	Description				
5,813	30	Brush, Good, HSG A				
1,646	30	Woods, Good, HSG A				
48,198	70	Woods, Good, HSG C				
4,643	73	Brush, Good, HSG D				
35,989	77	Woods, Good, HSG D				
8,805	98	98 Water Surface, 0% imp, HSG D				
105,094	72	Weighted Average				
105,094		100.00% Pervious Area				
Tc Length	n Slo _l	pe Velocity Capacity Description				
(min) (feet)) (ft/	/ft) (ft/sec) (cfs)				
6.0		Direct Entry,				

Summary for Subcatchment 104S: REMAINING LAND (NORTH - RIVER)

Runoff = 33.90 cfs @ 12.44 hrs, Volume= 192,708 cf, Depth> 1.54"

Routed to Link AP4: ANALYSIS POINT 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates

Printed 6/16/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 15

A	rea (sf)	CN	Description		
	83,589	30	Brush, Goo	d, HSG A	
2	11,292	30	Woods, Go	od, HSG A	
1	73,685	55	Woods, Go	od, HSG B	
	94,337		1 acre lots,		
	549	79	1 acre lots,	20% imp, I	HSG C
	2,920	65	Brush, Goo	d, HSG C	
5	82,787	70	Woods, Go	od, HSG C	
1	73,260	98	Water Surfa	ace, 0% imp	p, HSG D
1	76,351	77	Woods, Go	<u>od, HSG D</u>	
1,4	98,770	64	Weighted A	verage	
1,4	79,793		98.73% Pei	rvious Area	
	18,977		1.27% Impe	ervious Are	a
Tc	Length	Slope	•	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.2	50	0.0400	0.09		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.27"
19.4	937	0.0260	0.81		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
28.6	987	Total			

Summary for Link AP1: ANALYSIS POINT 1

Inflow Area = 13,756 sf, 64.57% Impervious, Inflow Depth > 3.63" for 10YR event

Inflow = 1.28 cfs @ 12.09 hrs, Volume= 4,159 cf

Primary = 1.28 cfs @ 12.09 hrs, Volume= 4,159 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP2: ANALYSIS POINT 2

Inflow Area = 956,300 sf, 3.58% Impervious, Inflow Depth > 2.31" for 10YR event

Inflow = 27.75 cfs @ 12.62 hrs, Volume= 184,006 cf

Primary = 27.75 cfs @ 12.62 hrs, Volume= 184,006 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP3: ANALYSIS POINT 3

Inflow Area = 105,094 sf, 0.00% Impervious, Inflow Depth > 2.16" for 10YR event

Inflow = 5.96 cfs @ 12.10 hrs, Volume= 18,960 cf

Primary = 5.96 cfs @ 12.10 hrs, Volume= 18,960 cf, Atten= 0%, Lag= 0.0 min

Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023 Page 16

Summary for Link AP4: ANALYSIS POINT 4

Inflow Area = 1,498,770 sf, 1.27% Impervious, Inflow Depth > 1.54" for 10YR event

Inflow = 33.90 cfs @ 12.44 hrs, Volume= 192,708 cf

Primary = 33.90 cfs @ 12.44 hrs, Volume= 192,708 cf, Atten= 0%, Lag= 0.0 min

Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023 Page 17

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment101S: SUMMER STREET Runoff Area=13,756 sf 64.57% Impervious Runoff Depth>4.90"

Tc=6.0 min CN=88 Runoff=1.71 cfs 5,619 cf

Subcatchment102S: OVERLANDTO Runoff Area=956,300 sf 3.58% Impervious Runoff Depth>3.40"

Flow Length=1,531' Tc=44.5 min CN=74 Runoff=41.12 cfs 270,829 cf

Subcatchment103S: ISOLATED Runoff Area=105,094 sf 0.00% Impervious Runoff Depth>3.23"

Tc=6.0 min CN=72 Runoff=8.96 cfs 28,279 cf

Subcatchment104S: REMAINING LAND Runoff Area=1,498,770 sf 1.27% Impervious Runoff Depth>2.46" Flow Length=987' Tc=28.6 min CN=64 Runoff=56.04 cfs 306,701 cf

Link AP1: ANALYSISPOINT 1 Inflow=1.71 cfs 5,619 cf

Primary=1.71 cfs 5,619 cf

Link AP2: ANALYSISPOINT 2 Inflow=41.12 cfs 270,829 cf

Primary=41.12 cfs 270,829 cf

Link AP3: ANALYSISPOINT 3 Inflow=8.96 cfs 28,279 cf

Primary=8.96 cfs 28,279 cf

Link AP4: ANALYSISPOINT 4 Inflow=56.04 cfs 306,701 cf

Primary=56.04 cfs 306,701 cf

Total Runoff Area = 2,573,920 sf Runoff Volume = 611,428 cf Average Runoff Depth = 2.85" 97.59% Pervious = 2,511,798 sf 2.41% Impervious = 62,122 sf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023

Page 18

Summary for Subcatchment 101S: SUMMER STREET (SOUTH)

Runoff = 1.71 cfs @ 12.09 hrs, Volume= 5,619 cf, Depth> 4.90"

Routed to Link AP1: ANALYSIS POINT 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description				
	3,612	61	>75% Gras	s cover, Go	ood, HSG B		
	8,882	98	Paved park	ing, HSG E	В		
	1,262	96	Gravel surfa	ace, HSG E	В		
	13,756	88	88 Weighted Average				
	4,874		35.43% Pervious Area				
	8,882		64.57% Impervious Area				
Tc	Length	Slope	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment 102S: OVERLAND TO TRAIN TRACKS (EAST)

Runoff = 41.12 cfs @ 12.61 hrs, Volume= 270,829 cf, Depth> 3.40"

Routed to Link AP2: ANALYSIS POINT 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Area (sf)	CN	Description
175,943	61	>75% Grass cover, Good, HSG B
184,742	55	Woods, Good, HSG B
24,401	98	Paved parking, HSG B
49,311	68	1 acre lots, 20% imp, HSG B
15,945	74	>75% Grass cover, Good, HSG C
235,274	70	Woods, Good, HSG C
3,192	80	>75% Grass cover, Good, HSG D
1,550	77	Woods, Good, HSG D
265,942	98	Water Surface, 0% imp, HSG D
956,300	74	Weighted Average
922,037		96.42% Pervious Area
34,263		3.58% Impervious Area

Prepared by Howard Stein Hudson Associates

Printed 6/16/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

<u>Page 19</u>

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	3.2	50	0.0800	0.26		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.27"
	0.3	77	0.0780	4.50		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	41.0	1,404	0.0130	0.57		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
_	44.5	1,531	Total			

Summary for Subcatchment 103S: ISOLATED WETLAND (NORTHEAST)

Runoff = 8.96 cfs @ 12.09 hrs, Volume= 28

28,279 cf, Depth> 3.23"

Routed to Link AP3: ANALYSIS POINT 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Area (sf)	CN	Description				
5,813	30	Brush, Good, HSG A				
1,646	30	Woods, Good, HSG A				
48,198	70	Woods, Good, HSG C				
4,643	73	Brush, Good, HSG D				
35,989	77	Woods, Good, HSG D				
8,805	98	8 Water Surface, 0% imp, HSG D				
105,094	72	Weighted Average				
105,094		100.00% Pervious Area				
Tc Lengtl	h Slo _l					
(min) (feet	(ft/	/ft) (ft/sec) (cfs)				
6.0		Direct Entry,				

Summary for Subcatchment 104S: REMAINING LAND (NORTH - RIVER)

Runoff = 56.04 cfs @ 12.42 hrs, Volume= 306,701 cf, Depth> 2.46" Routed to Link AP4 : ANALYSIS POINT 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates

Printed 6/16/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 20

A	rea (sf)	CN [Description		
	83,589	30 E	Brush, Goo	d, HSG A	
2	211,292	30 V	Voods, Go	od, HSG A	
1	73,685	55 V	Voods, Go	od, HSG B	
	94,337			20% imp, I	
	549	79 1	acre lots,	20% imp, I	HSG C
	2,920		Brush, Goo	•	
	82,787			od, HSG C	
	73,260			ace, 0% imp	
1	76,351	77 \	<u>Voods, Go</u>	od, HSG D	
1,4	198,770	64 V	Veighted A	verage	
1,4	179,793	ξ	98.73% Pei	rvious Area	
	18,977	1	.27% Impe	ervious Are	a
_		01			B
Tc	Length	Slope	•		Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.2	50	0.0400	0.09		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.27"
19.4	937	0.0260	0.81		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
28.6	987	Total			

Summary for Link AP1: ANALYSIS POINT 1

Inflow Area = 13,756 sf, 64.57% Impervious, Inflow Depth > 4.90" for 25YR event

Inflow = 1.71 cfs @ 12.09 hrs, Volume= 5.619 cf

Primary = 1.71 cfs @ 12.09 hrs, Volume= 5,619 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP2: ANALYSIS POINT 2

Inflow Area = 956,300 sf, 3.58% Impervious, Inflow Depth > 3.40" for 25YR event

Inflow = 41.12 cfs @ 12.61 hrs, Volume= 270,829 cf

Primary = 41.12 cfs @ 12.61 hrs, Volume= 270,829 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP3: ANALYSIS POINT 3

Inflow Area = 105,094 sf, 0.00% Impervious, Inflow Depth > 3.23" for 25YR event

Inflow = 8.96 cfs @ 12.09 hrs, Volume= 28,279 cf

Primary = 8.96 cfs @ 12.09 hrs, Volume= 28,279 cf, Atten= 0%, Lag= 0.0 min

Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023 Page 21

Summary for Link AP4: ANALYSIS POINT 4

Inflow Area = 1,498,770 sf, 1.27% Impervious, Inflow Depth > 2.46" for 25YR event

Inflow = 56.04 cfs @ 12.42 hrs, Volume= 306,701 cf

Primary = 56.04 cfs @ 12.42 hrs, Volume= 306,701 cf, Atten= 0%, Lag= 0.0 min

Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023 Page 22

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment101S: SUMMER STREET Runoff Area=13,756 sf 64.57% Impervious Runoff Depth>7.60"

Tc=6.0 min CN=88 Runoff=2.59 cfs 8,714 cf

Subcatchment102S: OVERLANDTO Runoff Area=956,300 sf 3.58% Impervious Runoff Depth>5.83"

Flow Length=1,531' Tc=44.5 min CN=74 Runoff=70.33 cfs 464,971 cf

Subcatchment103S: ISOLATED Runoff Area=105,094 sf 0.00% Impervious Runoff Depth>5.63"

Tc=6.0 min CN=72 Runoff=15.56 cfs 49,317 cf

Subcatchment104S: REMAINING LAND Runoff Area=1,498,770 sf 1.27% Impervious Runoff Depth>4.62"

Flow Length=987' Tc=28.6 min CN=64 Runoff=107.72 cfs 576,512 cf

Link AP1: ANALYSISPOINT 1 Inflow=2.59 cfs 8,714 cf

Primary=2.59 cfs 8,714 cf

Link AP2: ANALYSISPOINT 2 Inflow=70.33 cfs 464,971 cf

Primary=70.33 cfs 464,971 cf

Link AP3: ANALYSISPOINT 3 Inflow=15.56 cfs 49,317 cf

Primary=15.56 cfs 49,317 cf

Link AP4: ANALYSISPOINT 4 Inflow=107.72 cfs 576,512 cf

Primary=107.72 cfs 576,512 cf

Total Runoff Area = 2,573,920 sf Runoff Volume = 1,099,514 cf Average Runoff Depth = 5.13" 97.59% Pervious = 2,511,798 sf 2.41% Impervious = 62,122 sf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/16/2023

Page 23

Summary for Subcatchment 101S: SUMMER STREET (SOUTH)

Runoff = 2.59 cfs @ 12.09 hrs, Volume= 8,714 cf, Depth> 7.60"

Routed to Link AP1: ANALYSIS POINT 1

Type III 24-hr 100YR Rainfall=9.06"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Area (sf) CN Description 3,612 61 >75% Grass cover, Good, HSG B 8,882 98 Paved parking, HSG B 1,262 96 Gravel surface, HSG B 13,756 88 Weighted Average 4,874 35.43% Pervious Area 8,882 64.57% Impervious Area Slope Velocity Capacity Tc Length Description (feet) (ft/ft) (ft/sec) (cfs) (min)

6.0 Direct Entry,

Summary for Subcatchment 102S: OVERLAND TO TRAIN TRACKS (EAST)

Runoff = 70.33 cfs @ 12.60 hrs, Volume= 464,971 cf, Depth> 5.83"

Routed to Link AP2: ANALYSIS POINT 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Area (sf)	CN	Description
175,943	61	>75% Grass cover, Good, HSG B
184,742	55	Woods, Good, HSG B
24,401	98	Paved parking, HSG B
49,311	68	1 acre lots, 20% imp, HSG B
15,945	74	>75% Grass cover, Good, HSG C
235,274	70	Woods, Good, HSG C
3,192	80	>75% Grass cover, Good, HSG D
1,550	77	Woods, Good, HSG D
265,942	98	Water Surface, 0% imp, HSG D
956,300	74	Weighted Average
922,037		96.42% Pervious Area
34,263		3.58% Impervious Area

Prepared by Howard Stein Hudson Associates

Printed 6/16/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 24

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	3.2	50	0.0800	0.26	(0.0)	Sheet Flow,
	0.2	00	0.0000	0.20		Grass: Short n= 0.150 P2= 3.27"
	0.3	77	0.0780	4.50		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	41.0	1,404	0.0130	0.57		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	44.5	1,531	Total			

Summary for Subcatchment 103S: ISOLATED WETLAND (NORTHEAST)

Runoff = 15.56 cfs @ 12.09 hrs, Volume=

49,317 cf, Depth> 5.63"

Routed to Link AP3: ANALYSIS POINT 3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Area (sf)	CN	Description				
5,813	30	Brush, Good, HSG A				
1,646	30	Woods, Good, HSG A				
48,198	70	Woods, Good, HSG C				
4,643	73	Brush, Good, HSG D				
35,989	77	Woods, Good, HSG D				
8,805	98	98 Water Surface, 0% imp, HSG D				
105,094	72	Weighted Average				
105,094		100.00% Pervious Area				
Tc Length (min) (feet)		pe Velocity Capacity Description /ft) (ft/sec) (cfs)				
6.0		Direct Entry,				

Summary for Subcatchment 104S: REMAINING LAND (NORTH - RIVER)

Runoff = 107.72 cfs @ 12.40 hrs, Volume= 576,512 cf, Depth> 4.62"

Routed to Link AP4: ANALYSIS POINT 4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates

Printed 6/16/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 25

A	rea (sf)	CN I	Description		
	83,589	30 I	Brush, Goo	d, HSG A	
2	11,292	30 \	Noods, Go	od, HSG A	
1	73,685	55 \	Noods, Go	od, HSG B	
	94,337	68	1 acre lots,	20% imp, I	HSG B
	549	79 ·	1 acre lots,	20% imp, I	HSG C
	2,920	65 I	Brush, Goo	d, HSG C	
5	82,787	70 \	Woods, Go	od, HSG C	
1	73,260	98 \	Nater Surfa	ace, 0% imp	p, HSG D
1	76,351	77 \	Noods, Go	<u>od, HSG D</u>	
1,4	98,770	64 \	Neighted A	verage	
1,4	79,793	(98.73% Pei	rvious Area	
	18,977	•	1.27% Impe	ervious Are	a
Tc	Length	Slope	•		Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.2	50	0.0400	0.09		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.27"
19.4	937	0.0260	0.81		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
28.6	987	Total			

Summary for Link AP1: ANALYSIS POINT 1

Inflow Area = 13,756 sf, 64.57% Impervious, Inflow Depth > 7.60" for 100YR event

Inflow = 2.59 cfs @ 12.09 hrs, Volume= 8,714 cf

Primary = 2.59 cfs @ 12.09 hrs, Volume= 8,714 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP2: ANALYSIS POINT 2

Inflow Area = 956,300 sf, 3.58% Impervious, Inflow Depth > 5.83" for 100YR event

Inflow = 70.33 cfs @ 12.60 hrs, Volume= 464,971 cf

Primary = 70.33 cfs @ 12.60 hrs, Volume= 464,971 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP3: ANALYSIS POINT 3

Inflow Area = 105,094 sf, 0.00% Impervious, Inflow Depth > 5.63" for 100YR event

Inflow = 15.56 cfs @ 12.09 hrs, Volume= 49,317 cf

Primary = 15.56 cfs @ 12.09 hrs, Volume= 49,317 cf, Atten= 0%, Lag= 0.0 min

Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

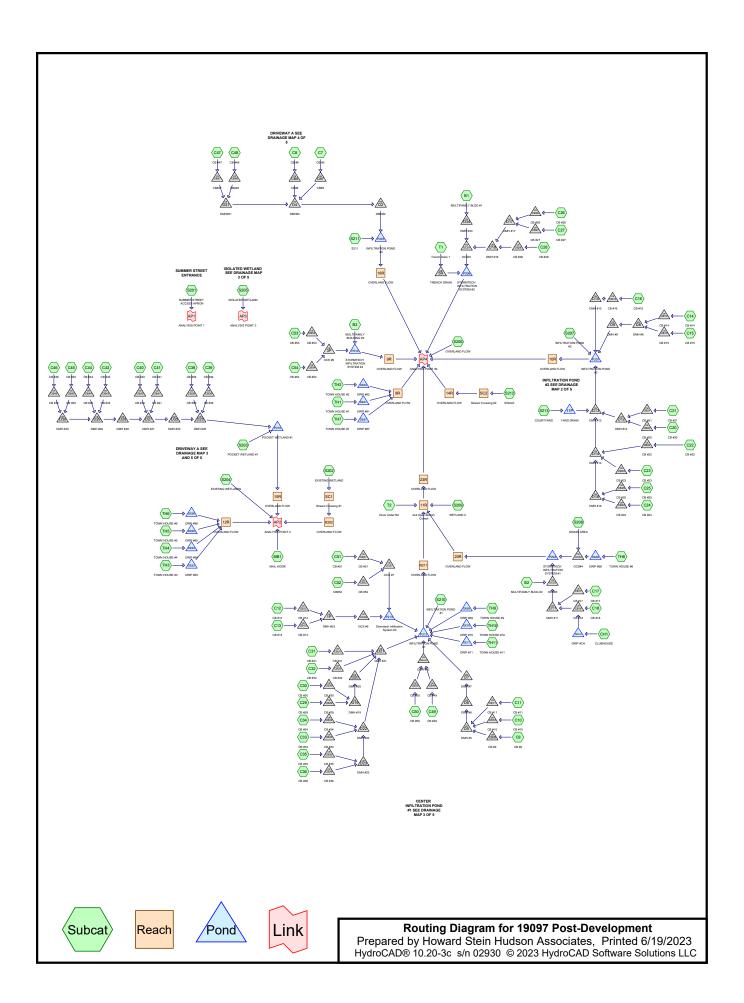
Printed 6/16/2023 Page 26

Summary for Link AP4: ANALYSIS POINT 4

Inflow Area = 1,498,770 sf, 1.27% Impervious, Inflow Depth > 4.62" for 100YR event

Inflow 107.72 cfs @ 12.40 hrs, Volume= 576,512 cf

107.72 cfs @ 12.40 hrs, Volume= Primary 576,512 cf, Atten= 0%, Lag= 0.0 min



19097 Post-Development

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 2

Project Notes

Rainfall events imported from "19097 PreDevelopment.hcp" Rainfall events imported from "19097 PostDevelopment-prelim.hcp"

19097 Post-Development
Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 3

Rainfall Events Listing

Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	2YR	Type III 24-hr		Default	24.00	1	3.27	2
2	10YR	Type III 24-hr		Default	24.00	1	4.96	2
3	25YR	Type III 24-hr		Default	24.00	1	6.29	2
4	100YR	Type III 24-hr		Default	24.00	1	9.06	2

Printed 6/19/2023 Page 4

Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
17,568	51	1 acre lots, 20% imp, HSG A (S206)
147,453	68	1 acre lots, 20% imp, HSG B (C48, S206)
40,742	39	>75% Grass cover, Good, HSG A (C14, C16, C21, C23, C25, C27, C53, C54,
		S206, S208, S209, S213, T2)
258,351	61	>75% Grass cover, Good, HSG B (C43, C44, C45, C46, C8, C9, S201, S202,
		S203, S204, S206, S211, S212)
271,812	74	>75% Grass cover, Good, HSG C (C13, C14, C16, C17, C18, C20, C22, C23,
		C25, C28, C29, C30, C31, C32, C33, C34, C49, C50, C52, C53, C54, C9, CH1, S202, S203, S204, S205, S206, S207, S208, S209, S210, S212, S213, T1, T2,
		TH1, TH10, TH11, TH2, TH3, TH4, TH5, TH6, TH7, TH8, TH9)
22,374	80	>75% Grass cover, Good, HSG D (C22, C23, C24, C25, C26, C27, C28, C35,
22,014	00	C9, CH1, S202, S205, S206, S212, S213, T1)
41,148	30	Brush, Good, HSG A (S206)
107,179	48	Brush, Good, HSG B (S202, S204, S206, S212)
66,752	65	Brush, Good, HSG C (S202, S204, S205, S206, S210, S212)
6,281	73	Brush, Good, HSG D (S204, S205)
46,900	98	Paved parking, HSG A (C14, C15, C16, C20, C21, C22, C23, C25, C27, C53,
,		C54, S206, S208, S209, S213, T2)
39,770	98	Paved parking, HSG B (C38, C39, C40, C41, C43, C44, C45, C46, S201, S202,
		S212)
141,055	98	Paved parking, HSG C (C13, C14, C15, C16, C17, C18, C20, C21, C22, C23,
		C25, C28, C29, C30, C31, C32, C33, C34, C35, C36, C38, C39, C51, C52, C53,
		C54, C9, S202, S204, S205, S206, S208, S209, S212, S213, T1, T2)
41,121	98	Paved parking, HSG D (C22, C23, C24, C25, C26, C27, C28, C35, C38, C39,
		C9, S202, S204, S205, S212, S213, T1)
17,148	98	Paved roads w/curbs & sewers, HSG B (C10, C47, C48, C7, C8, C9)
34,156	98	Paved roads w/curbs & sewers, HSG C (C10, C11, C12, C49, C50)
919	98	Paved roads w/curbs & sewers, HSG D (C10)
8,649	98	Roofs, HSG A (B2, B3, S213)
14,884	98	Roofs, HSG B (MB1, S202)
110,093	98	Roofs, HSG C (B1, B2, B3, C13, C29, C30, C31, C32, C33, C34, C51, C9,
		CH1, S213, TH1, TH10, TH11, TH2, TH3, TH4, TH5, TH6, TH7, TH8, TH9)
3,102	98	Roofs, HSG D (B1, CH1)
1,371	98	Water Surface, 0% imp, HSG A (S207, S213)
1,060	98	Water Surface, 0% imp, HSG B (S203)
29,784	98	Water Surface, 0% imp, HSG C (S203, S207, S210, S213)
171,979	98	Water Surface, 0% imp, HSG D (S206, S209, S212)
7,328	98	Water Surface, HSG B (S211)
271,794	98	Water Surface, HSG D (S202, S204, S205)
145,962	30	Woods, Good, HSG A (S205, S206, S209)
123,002	55	Woods, Good, HSG B (S202, S204, S206, S212)

19097 Post-Development
Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 5

Area Listing (all nodes) (continued)

Area	CN	Description
 (sq-ft)		(subcatchment-numbers)
232,021	70	Woods, Good, HSG C (S202, S204, S205, S206, S209, S212)
152,162	77	Woods, Good, HSG D (S204, S205, S206)
2,573,920	75	TOTAL AREA

19097 Post-Development
Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 6

Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
302,340	HSG A	B2, B3, C14, C15, C16, C20, C21, C22, C23, C25, C27, C53, C54, S205, S206, S207, S208, S209, S213, T2
716,175	HSG B	C10, C38, C39, C40, C41, C43, C44, C45, C46, C47, C48, C7, C8, C9, MB1, S201, S202, S203, S204, S206, S211, S212
885,673	HSG C	B1, B2, B3, C10, C11, C12, C13, C14, C15, C16, C17, C18, C20, C21, C22, C23, C25, C28, C29, C30, C31, C32, C33, C34, C35, C36, C38, C39, C49, C50, C51, C52, C53, C54, C9, CH1, S202, S203, S204, S205, S206, S207, S208, S209, S210, S212, S213, T1, T2, TH1, TH10, TH11, TH2, TH3, TH4, TH5, TH6, TH7, TH8, TH9
669,732	HSG D	B1, C10, C22, C23, C24, C25, C26, C27, C28, C35, C38, C39, C9, CH1, S202, S204, S205, S206, S209, S212, S213, T1
0 2,573,920	Other	TOTAL AREA

Printed 6/19/2023 Page 7

Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)	Node Name
1	11R	194.00	193.55	30.0	0.0150	0.069	48.0	48.0	0.0	
2	SC1	206.37	205.51	43.1	0.0200	0.030	192.0	60.0	0.0	
3	SC2	208.52	207.64	36.5	0.0241	0.030	192.0	60.0	0.0	
4	1P	205.50	204.33	46.7	0.0251	0.013	0.0	12.0	0.0	
5	5R	197.22	196.50	36.0	0.0200	0.012	0.0	8.0	0.0	
6	11P	203.25	202.94	61.0	0.0051	0.012	0.0	12.0	0.0	
7	CB10	209.76	209.59	33.8	0.0050	0.013	0.0	12.0	0.0	
8	CB11	209.94	209.67	26.3	0.0103	0.013	0.0	12.0	0.0	
9	CB12	206.68	205.65	41.3	0.0249	0.013	0.0	12.0	0.0	
10	CB13	206.70	205.61	43.7	0.0249	0.013	0.0	12.0	0.0	
11	CB14	200.79	200.67	23.2	0.0052	0.013	0.0	12.0	0.0	
12	CB15	200.79	200.71	15.6	0.0051	0.013	0.0	12.0	0.0	
13	CB16	203.47	203.33	20.9	0.0067	0.013	0.0	12.0	0.0	
14	CB17	204.99	204.86	13.8	0.0094	0.013	0.0	12.0	0.0	
15	CB18	204.72	204.59	25.1	0.0052	0.013	0.0	15.0	0.0	
16	CB20	203.97	203.81	30.3	0.0053	0.013	0.0	12.0	0.0	
17	CB21	204.32	204.19	26.0	0.0050	0.013	0.0	12.0	0.0	
18	CB22	205.33	205.25	16.1	0.0050	0.012	0.0	12.0	0.0	
19	CB23	205.41	205.32	16.3	0.0055	0.012	0.0	12.0	0.0	
20	CB24	205.21	205.15	12.1	0.0050	0.012	0.0	12.0	0.0	
21	CB25	205.22	205.16	11.4	0.0053	0.012	0.0	15.0	0.0	
22	CB26	201.77	201.55	42.5	0.0052	0.013	0.0	12.0	0.0	
23	CB27	201.00	200.90	18.0	0.0056	0.013	0.0	12.0	0.0	
24	CB28	197.75	197.69	13.7	0.0044	0.013	0.0	12.0	0.0	
25	CB29	205.38	205.31	13.5	0.0052	0.013	0.0	12.0	0.0	
26	CB30	205.38	205.29	17.5	0.0051	0.013	0.0	12.0	0.0	
27	CB31	204.19	204.11	16.4	0.0049	0.013	0.0	12.0	0.0	
28	CB32	204.19	204.11	16.3	0.0049	0.013	0.0	12.0	0.0	
29	CB33	205.28	205.22	11.7	0.0051	0.013	0.0	12.0	0.0	
30	CB34	205.21	205.13	16.5	0.0048	0.013	0.0	12.0	0.0	
31	CB35	207.04	206.96	15.2	0.0053	0.013	0.0	12.0	0.0	
32	CB36	207.04	206.96	16.1	0.0050	0.013	0.0	12.0	0.0	
33	CB38	209.69	209.61	16.7	0.0048	0.012	0.0	12.0	0.0	
34	CB39	209.69	209.61	16.4	0.0049	0.013	0.0	12.0	0.0	
35	CB40	213.68	213.55	17.8	0.0073	0.013	0.0	12.0	0.0	
36	CB41	213.89	213.80	18.4	0.0049	0.013	0.0	12.0	0.0	
37	CB43	220.00	219.93	14.9	0.0047	0.013	0.0	12.0	0.0	
38	CB44	220.00	219.93	14.9	0.0047	0.013	0.0	12.0	0.0	
39	CB45	221.29	221.20	18.2	0.0049	0.013	0.0	12.0	0.0	
40	CB46	221.53	221.45	15.3	0.0052	0.013	0.0	12.0	0.0	
41	CB47	225.05	224.95	20.9	0.0048	0.012	0.0	12.0	0.0	

Printed 6/19/2023 Page 8

Pipe Listing (all nodes) (continued)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)	Node Name
40		` '	` '	, ,	. ,	0.040	,	, ,		Name
42	CB48	224.82	224.74	16.9	0.0047	0.012	0.0	15.0	0.0	
43 44	CB49 CB50	202.76	202.68	15.5	0.0052	0.013	0.0	12.0	0.0 0.0	
44 45	CB50 CB51	202.78 202.35	202.70 202.19	15.3 31.4	0.0052 0.0051	0.013 0.013	0.0 0.0	12.0 12.0	0.0	
46	CB51 CB52	202.33	202.19	25.5	0.0051	0.013	0.0	12.0	0.0	
47	CB52 CB53	202.08	202.53	32.0	0.0051	0.013	0.0	12.0	0.0	
48	CB53	202.76	202.02	36.7	0.0030	0.013	0.0	12.0	0.0	
49	CB34 CB7	212.60	212.45	15.1	0.0049	0.013	0.0	12.0	0.0	
50	CB8	213.79	213.64	15.1	0.0099	0.012	0.0	12.0	0.0	
51	CB9	210.10	209.71	19.9	0.0033	0.013	0.0	12.0	0.0	
52	D10	203.33	203.25	15.6	0.0051	0.013	0.0	12.0	0.0	
53	D11	204.25	204.03	44.6	0.0049	0.013	0.0	18.0	0.0	
54	D12	203.21	203.00	41.9	0.0050	0.013	0.0	12.0	0.0	
55	D13	201.95	201.65	60.1	0.0050	0.013	0.0	24.0	0.0	
56	D14	204.13	202.85	256.3	0.0050	0.012	0.0	18.0	0.0	
57	D16	204.90	204.38	103.5	0.0050	0.012	0.0	15.0	0.0	
58	D17	200.55	197.69	91.6	0.0312	0.013	0.0	12.0	0.0	
59	D18	197.44	196.98	46.3	0.0099	0.013	0.0	15.0	0.0	
60	D19	205.19	204.43	82.5	0.0092	0.013	0.0	12.0	0.0	
61	D2	206.90	206.52	38.2	0.0099	0.013	0.0	15.0	0.0	
62	D20	204.19	203.87	63.5	0.0050	0.013	0.0	15.0	0.0	
63	D21	203.02	202.66	72.4	0.0050	0.013	0.0	24.0	0.0	
64	D22	204.87	203.92	134.2	0.0071	0.013	0.0	15.0	0.0	
65	D23	206.70	204.97	173.3	0.0100	0.013	0.0	15.0	0.0	
66	D25	209.36	208.17	237.6	0.0050	0.012	0.0	15.0	0.0	
67	D27	213.34	212.38	63.9	0.0150	0.012	0.0	15.0	0.0	
68	D28	217.46	214.29	158.3	0.0200	0.013	0.0	12.0	0.0	
69	D29	219.83	217.55	150.9	0.0151	0.013	0.0	12.0	0.0	
70	D30	220.92	220.00	184.2	0.0050	0.013	0.0	12.0	0.0	
71	D31	224.63	213.09	288.5	0.0400	0.012	0.0	15.0	0.0	
72	D34	198.07	197.03	52.0	0.0200	0.012	0.0	12.0	0.0	
73	D35	212.28	209.71	171.5	0.0150	0.012	0.0	15.0	0.0	
74	D4	210.34	207.01	222.3	0.0150	0.012	0.0	15.0	0.0	
75	D5	209.09	208.17	183.0	0.0050	0.013	0.0	18.0	0.0	
76	D6	208.07	206.57	299.7	0.0050	0.013	0.0	18.0	0.0	
77	D7	206.47	204.04	44.2	0.0550	0.013	0.0	18.0	0.0	
78	D8	200.57	200.13	87.7	0.0050	0.013	0.0	12.0	0.0	
79	D9	200.03	199.97	11.9	0.0050	0.013	0.0	12.0	0.0	
80	DE61	212.70	212.65	10.0	0.0050	0.013	0.0	6.0	0.0	
81	DE62	212.70	212.65	10.0	0.0050	0.013	0.0	6.0	0.0	
82	DE63	207.50	207.45	10.0	0.0050	0.013	0.0	6.0	0.0	

Printed 6/19/2023 Page 9

Pipe Listing (all nodes) (continued)

 Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)	Node Name
 83	DE64	205.50	205.45	10.0	0.0050	0.013	0.0	6.0	0.0	
84	DE65	206.50	206.45	10.0	0.0050	0.013	0.0	6.0	0.0	
85	DE66	208.30	208.25	10.0	0.0050	0.013	0.0	6.0	0.0	
86	DE67	208.50	208.45	10.0	0.0050	0.013	0.0	6.0	0.0	
87	DE68	207.50	206.00	20.0	0.0750	0.013	0.0	6.0	0.0	
88	DE69	206.00	205.95	10.0	0.0050	0.013	0.0	6.0	0.0	
89	DE70	206.40	206.35	10.0	0.0050	0.013	0.0	6.0	0.0	
90	DE71	207.00	206.95	10.0	0.0050	0.013	0.0	6.0	0.0	
91	DECH	208.50	205.10	80.0	0.0425	0.013	0.0	4.0	0.0	
92	DMH32	202.59	201.57	19.2	0.0531	0.013	0.0	12.0	0.0	
93	P204	202.75	201.00	35.0	0.0500	0.012	0.0	15.0	0.0	
94	P206	194.65	194.50	30.0	0.0050	0.013	0.0	18.0	0.0	
95	P207	194.75	194.55	40.0	0.0050	0.012	0.0	15.0	0.0	
96	P210	202.25	202.03	44.0	0.0050	0.013	0.0	12.0	0.0	
97	P213	202.30	202.00	60.0	0.0050	0.013	0.0	12.0	0.0	
98	P214	201.00	200.88	25.0	0.0048	0.013	0.0	12.0	0.0	

Type III 24-hr 2YR Rainfall=3.27" Printed 6/19/2023

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 10

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentB1: MULTIFAMILYBLDG	Runoff Area=23,255 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=1.66 cfs 5,882 cf
SubcatchmentB2: MULTIFAMILYBLDG	Runoff Area=17,561 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=1.25 cfs 4,442 cf
SubcatchmentB3: MULTIFAMILY	Runoff Area=19,981 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=1.42 cfs 5,054 cf
SubcatchmentC10: CB #10	Runoff Area=6,961 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.50 cfs 1,761 cf
SubcatchmentC11: CB #11	Runoff Area=7,173 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.51 cfs 1,814 cf
SubcatchmentC12: CB #12	Runoff Area=5,238 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.37 cfs 1,325 cf
SubcatchmentC13: CB #13	Runoff Area=10,873 sf 90.78% Impervious Runoff Depth>2.82" Tc=6.0 min CN=96 Runoff=0.75 cfs 2,552 cf
SubcatchmentC14: CB #14	Runoff Area=12,099 sf 86.22% Impervious Runoff Depth>2.32" Tc=6.0 min CN=91 Runoff=0.73 cfs 2,341 cf
SubcatchmentC15: CB #15	Runoff Area=6,666 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.47 cfs 1,686 cf
SubcatchmentC16: CB #16	Runoff Area=8,516 sf 64.88% Impervious Runoff Depth>1.39" Tc=6.0 min CN=79 Runoff=0.31 cfs 985 cf
SubcatchmentC17: CB #17	Runoff Area=11,836 sf 73.87% Impervious Runoff Depth>2.42" Tc=6.0 min CN=92 Runoff=0.73 cfs 2,382 cf
SubcatchmentC18: CB #18	Runoff Area=18,591 sf 66.35% Impervious Runoff Depth>2.23" Tc=6.0 min CN=90 Runoff=1.08 cfs 3,458 cf
SubcatchmentC20: CB #20	Runoff Area=11,939 sf 88.95% Impervious Runoff Depth>2.71" Tc=6.0 min CN=95 Runoff=0.80 cfs 2,698 cf
SubcatchmentC21: CB #21	Runoff Area=10,174 sf 87.04% Impervious Runoff Depth>2.23" Tc=6.0 min CN=90 Runoff=0.59 cfs 1,892 cf
SubcatchmentC22: CB #22	Runoff Area=12,001 sf 91.62% Impervious Runoff Depth>2.82" Tc=6.0 min CN=96 Runoff=0.83 cfs 2,817 cf
SubcatchmentC23: CB #23	Runoff Area=9,694 sf 61.00% Impervious Runoff Depth>2.14" Tc=6.0 min CN=89 Runoff=0.54 cfs 1,732 cf

Tc=6.0 min CN=98 Runoff=0.40 cfs 1,413 cf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 11

SubcatchmentC24: CB #24	Runoff Area=7,930 sf 72.16% Impervious Runoff Depth>2.51" Tc=6.0 min CN=93 Runoff=0.51 cfs 1,660 cf
SubcatchmentC25: CB #25	Runoff Area=8,487 sf 80.92% Impervious Runoff Depth>2.61" Tc=6.0 min CN=94 Runoff=0.56 cfs 1,846 cf
SubcatchmentC26: CB #26	Runoff Area=8,835 sf 63.75% Impervious Runoff Depth>2.32" Tc=6.0 min CN=91 Runoff=0.53 cfs 1,710 cf
SubcatchmentC27: CB #27	Runoff Area=6,111 sf 91.90% Impervious Runoff Depth>2.82" Tc=6.0 min CN=96 Runoff=0.42 cfs 1,434 cf
SubcatchmentC28: CB #28	Runoff Area=10,372 sf 51.33% Impervious Runoff Depth>2.06" Tc=6.0 min CN=88 Runoff=0.56 cfs 1,779 cf
SubcatchmentC29: CB #29	Runoff Area=8,495 sf 84.21% Impervious Runoff Depth>2.61" Tc=6.0 min CN=94 Runoff=0.56 cfs 1,848 cf
SubcatchmentC30: CB #30	Runoff Area=8,933 sf 82.40% Impervious Runoff Depth>2.61" Tc=6.0 min CN=94 Runoff=0.59 cfs 1,943 cf
SubcatchmentC31: CB #31	Runoff Area=16,365 sf 68.64% Impervious Runoff Depth>2.23" Tc=6.0 min CN=90 Runoff=0.95 cfs 3,044 cf
SubcatchmentC32: CB #32	Runoff Area=12,710 sf 70.47% Impervious Runoff Depth>2.32" Tc=6.0 min CN=91 Runoff=0.76 cfs 2,460 cf
SubcatchmentC33: CB #33	Runoff Area=5,421 sf 83.90% Impervious Runoff Depth>2.61" Tc=6.0 min CN=94 Runoff=0.36 cfs 1,179 cf
SubcatchmentC34: CB #34	Runoff Area=8,622 sf 80.51% Impervious Runoff Depth>2.51" Tc=6.0 min CN=93 Runoff=0.55 cfs 1,804 cf
SubcatchmentC35: CB #35	Runoff Area=4,149 sf 98.10% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.30 cfs 1,049 cf
SubcatchmentC36: CB #36	Runoff Area=6,622 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.47 cfs 1,675 cf
SubcatchmentC38: CB #38	Runoff Area=7,637 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.54 cfs 1,932 cf
SubcatchmentC39: CB #39	Runoff Area=7,612 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.54 cfs 1,925 cf
SubcatchmentC40: CB #40	Runoff Area=4,211 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.30 cfs 1,065 cf
SubcatchmentC41: CB #41	Runoff Area=5,586 sf 100.00% Impervious Runoff Depth>3.04"

SubcatchmentMB1: MAIL KIOSK

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 12

SubcatchmentC43: CB #43	Runoff Area=3,109 sf 75.36% Impervious Runoff Depth>2.14" Tc=6.0 min CN=89 Runoff=0.17 cfs 555 cf
SubcatchmentC44: CB #44	Runoff Area=1,978 sf 84.43% Impervious Runoff Depth>2.42" Tc=6.0 min CN=92 Runoff=0.12 cfs 398 cf
SubcatchmentC45: CB #45	Runoff Area=2,465 sf 50.30% Impervious Runoff Depth>1.45" Tc=6.0 min CN=80 Runoff=0.09 cfs 299 cf
SubcatchmentC46: CB #46	Runoff Area=4,397 sf 50.97% Impervious Runoff Depth>1.45" Tc=6.0 min CN=80 Runoff=0.17 cfs 533 cf
SubcatchmentC47: CB #47	Runoff Area=3,012 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.21 cfs 762 cf
SubcatchmentC48: CB #48	Runoff Area=60,128 sf 25.16% Impervious Runoff Depth>0.87" Flow Length=400' Tc=11.8 min CN=70 Runoff=1.03 cfs 4,342 cf
SubcatchmentC49: CB #49	Runoff Area=5,238 sf 84.59% Impervious Runoff Depth>2.61" Tc=6.0 min CN=94 Runoff=0.34 cfs 1,139 cf
SubcatchmentC50: CB #50	Runoff Area=15,040 sf 77.20% Impervious Runoff Depth>2.51" Tc=6.0 min CN=93 Runoff=0.96 cfs 3,147 cf
SubcatchmentC51: CB #51	Runoff Area=6,823 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.49 cfs 1,726 cf
SubcatchmentC52: CB#52	Runoff Area=9,052 sf 87.14% Impervious Runoff Depth>2.71" Tc=6.0 min CN=95 Runoff=0.61 cfs 2,045 cf
SubcatchmentC53: CB #53	Runoff Area=7,863 sf 86.52% Impervious Runoff Depth>2.51" Tc=6.0 min CN=93 Runoff=0.50 cfs 1,646 cf
SubcatchmentC54: CB #54	Runoff Area=4,821 sf 86.85% Impervious Runoff Depth>2.32" Tc=6.0 min CN=91 Runoff=0.29 cfs 933 cf
SubcatchmentC7: CB #5	Runoff Area=4,650 sf 100.00% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.33 cfs 1,176 cf
SubcatchmentC8: CB #8	Runoff Area=5,450 sf 88.75% Impervious Runoff Depth>2.61" Tc=6.0 min CN=94 Runoff=0.36 cfs 1,185 cf
SubcatchmentC9: CB #9	Runoff Area=16,307 sf 93.95% Impervious Runoff Depth>2.92" Tc=6.0 min CN=97 Runoff=1.14 cfs 3,974 cf
SubcatchmentCH1: CLUBHOUSE	Runoff Area=6,262 sf 92.70% Impervious Runoff Depth>2.82" Tc=6.0 min CN=96 Runoff=0.43 cfs 1,470 cf

Runoff Area=938 sf 100.00% Impervious Runoff Depth>3.04"

Tc=6.0 min CN=98 Runoff=0.07 cfs 237 cf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 13

SubcatchmentS201: SUMMER STREET Runoff Area=9,943 sf 92.79% Impervious Runoff Depth>2.71"

Tc=6.0 min CN=95 Runoff=0.67 cfs 2.247 cf

SubcatchmentS202: EXISTING WETLANDRunoff Area=432,269 sf 42.08% Impervious Runoff Depth>1.19" Flow Length=856' Tc=23.2 min CN=76 Runoff=8.51 cfs 43,021 cf

SubcatchmentS203: POCKET WETLAND#1 Runoff Area=25,587 sf 0.00% Impervious Runoff Depth>0.97" Tc=6.0 min CN=72 Runoff=0.61 cfs 2,073 cf

SubcatchmentS204: EXISTING

Runoff Area=308,203 sf 31.07% Impervious Runoff Depth>1.14"
Flow Length=632' Tc=22.6 min CN=75 Runoff=5.78 cfs 29,158 cf

SubcatchmentS205: ISOLATEDWETLAND Runoff Area=55,420 sf 16.57% Impervious Runoff Depth>0.87"

Tc=6.0 min CN=70 Runoff=1.15 cfs 4.009 cf

SubcatchmentS206: OVERLANDFLOW Runoff Area=891,295 sf 2.91% Impervious Runoff Depth>0.63" Flow Length=1,467' Tc=34.5 min CN=65 Runoff=6.52 cfs 46,602 cf

SubcatchmentS207: INFILTRATIONPOND Runoff Area=20,803 sf 0.00% Impervious Runoff Depth>2.06" Tc=6.0 min CN=88 Runoff=1.12 cfs 3,568 cf

SubcatchmentS208: GRASS AREA Runoff Area=13,760 sf 9.33% Impervious Runoff Depth>1.14"

Tc=6.0 min CN=75 Runoff=0.40 cfs 1,308 cf

SubcatchmentS209: WETLANDC Runoff Area=107,073 sf 0.38% Impervious Runoff Depth>1.02" Flow Length=607' Slope=0.0150 '/' Tc=28.9 min CN=73 Runoff=1.60 cfs 9,101 cf

SubcatchmentS210: INFILTRATIONPOND Runoff Area=75,890 sf 0.00% Impervious Runoff Depth>1.32" Flow Length=580' Slope=0.0150 '/' Tc=16.5 min CN=78 Runoff=1.92 cfs 8,348 cf

SubcatchmentS211: S211Runoff Area=15,436 sf 47.47% Impervious Runoff Depth>1.39"

Tc=6.0 min CN=79 Runoff=0.56 cfs 1,786 cf

SubcatchmentS212: SWALE

Runoff Area=52,768 sf 0.60% Impervious Runoff Depth>0.63"

Flow Length=418' Tc=23.1 min CN=65 Runoff=0.46 cfs 2,770 cf

SubcatchmentS213: COURTYARD

Runoff Area=21,407 sf 48.10% Impervious Runoff Depth>1.39"

Tc=6.0 min CN=79 Runoff=0.78 cfs 2,476 cf

SubcatchmentT1: Trench Drain 1Runoff Area=11,173 sf 75.10% Impervious Runoff Depth>2.51"
Tc=6.0 min CN=93 Runoff=0.71 cfs 2,338 cf

SubcatchmentT2: Drive Under B2 Runoff Area=4,445 sf 64.30% Impervious Runoff Depth>1.32"

Tc=6.0 min CN=78 Runoff=0.15 cfs 490 cf

SubcatchmentTH1: TOWN HOUSE #1 Runoff Area=4,247 sf 92.68% Impervious Runoff Depth>2.82"

Tc=6.0 min CN=96 Runoff=0.29 cfs 997 cf

SubcatchmentTH10: TOWN HOUSE #10 Runoff Area=3,476 sf 91.60% Impervious Runoff Depth>2.82"

Tc=6.0 min CN=96 Runoff=0.24 cfs 816 cf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 14

SubcatchmentTH11: TOWN HOUSE #11 Runoff Area=4,210 sf 92.61% Impervious Runoff Depth>2.82"

Tc=6.0 min CN=96 Runoff=0.29 cfs 988 cf

SubcatchmentTH2: TOWN HOUSE #2 Runoff Area=4,247 sf 92.68% Impervious Runoff Depth>2.82"

Tc=6.0 min CN=96 Runoff=0.29 cfs 997 cf

SubcatchmentTH3: TOWN HOUSE #3 Runoff Area=3,013 sf 88.68% Impervious Runoff Depth>2.71"

Tc=6.0 min CN=95 Runoff=0.20 cfs 681 cf

SubcatchmentTH4: TOWN HOUSE #4 Runoff Area=3,470 sf 91.59% Impervious Runoff Depth>2.82"

Tc=6.0 min CN=96 Runoff=0.24 cfs 814 cf

SubcatchmentTH5: TOWN HOUSE #5 Runoff Area=3,016 sf 88.69% Impervious Runoff Depth>2.71"

Tc=6.0 min CN=95 Runoff=0.20 cfs 682 cf

SubcatchmentTH6: TOWN HOUSE #6 Runoff Area=3,407 sf 91.46% Impervious Runoff Depth>2.82" Tc=6.0 min CN=96 Runoff=0.23 cfs 800 cf

SubcatchmentTH7: TOWN HOUSE #7 Runoff Area=3,481 sf 91.61% Impervious Runoff Depth>2.82"

Tc=6.0 min CN=96 Runoff=0.24 cfs 817 cf

SubcatchmentTH8: TOWN HOUSE #8 Runoff Area=4,212 sf 92.62% Impervious Runoff Depth>2.82"

Tc=6.0 min CN=96 Runoff=0.29 cfs 989 cf

SubcatchmentTH9: TOWN HOUSE #9 Runoff Area=3,480 sf 91.61% Impervious Runoff Depth>2.82" Tc=6.0 min CN=96 Runoff=0.24 cfs 817 cf

Reach 8R: OVERLAND FLOWAvg. Flow Depth=0.04' Max Vel=0.06 fps Inflow=0.69 cfs 1,940 cf n=0.400 L=563.0' S=0.0213 '/' Capacity=28.09 cfs Outflow=0.11 cfs 1,734 cf

Reach 9R: OVERLAND FLOWAvg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0 cf n=0.400 L=211.0' S=0.0652'/' Capacity=23.45 cfs Outflow=0.00 cfs 0 cf

Reach 10R: OVERLAND FLOW Avg. Flow Depth=0.02' Max Vel=0.05 fps Inflow=0.04 cfs 101 cf n=0.400 L=164.0' S=0.0366'/ Capacity=17.57 cfs Outflow=0.02 cfs 101 cf

Reach 11R: 4x4 Open Bottom Culvert Avg. Flow Depth=0.36' Max Vel=1.19 fps Inflow=1.70 cfs 14,965 cf 48.0" x 48.0" Box Pipe n=0.069 L=30.0' S=0.0150 '/' Capacity=42.20 cfs Outflow=1.70 cfs 14,959 cf

Reach 12R: OVERLAND FLOWAvg. Flow Depth=0.06' Max Vel=0.09 fps Inflow=0.77 cfs 2,134 cf n=0.400 L=250.0' S=0.0240 '/' Capacity=29.80 cfs Outflow=0.30 cfs 2,085 cf

Reach 14R: OVERLAND FLOWAvg. Flow Depth=0.03' Max Vel=0.05 fps Inflow=0.46 cfs 2,770 cf n=0.400 L=852.0' S=0.0246 '/' Capacity=31.55 cfs Outflow=0.07 cfs 2,124 cf

Reach 15R: OVERLAND FLOWAvg. Flow Depth=0.04' Max Vel=0.06 fps Inflow=0.12 cfs 4,736 cf n=0.400 L=300.0' S=0.0200 '/' Capacity=27.21 cfs Outflow=0.12 cfs 4,228 cf

Reach 18R: OVERLAND FLOWAvg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0 cf n=0.400 L=609.0' S=0.0279 '/' Capacity=38.42 cfs Outflow=0.00 cfs 0 cf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 15

Reach 20R: OVERLAND FLOWAvg. Flow Depth=0.09' Max Vel=0.07 fps Inflow=0.93 cfs 5,781 cf n=0.400 L=560.0' S=0.0093 '/' Capacity=18.54 cfs Outflow=0.34 cfs 5,373 cf

Reach 23R: OVERLAND FLOWAvg. Flow Depth=0.16' Max Vel=0.15 fps Inflow=1.70 cfs 14,959 cf n=0.400 L=237.0' S=0.0211 '/' Capacity=31.93 cfs Outflow=1.24 cfs 14,613 cf

Reach R202: OVERLAND FLOWAvg. Flow Depth=0.21' Max Vel=0.13 fps Inflow=8.50 cfs 43,012 cf n=0.400 L=700.0' S=0.0107'/' Capacity=42.56 cfs Outflow=2.85 cfs 39,436 cf

Reach R211: OVERLAND FLOWAvg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0 cf n=0.400 L=600.0' S=0.0087 '/' Capacity=14.51 cfs Outflow=0.00 cfs 0 cf

Reach SC1: Stream Crossing #1 Avg. Flow Depth=0.21' Max Vel=2.48 fps Inflow=8.51 cfs 43,021 cf 192.0" x 60.0", R=207.0" Arch Pipe n=0.030 L=43.1' S=0.0200'/ Capacity=722.91 cfs Outflow=8.50 cfs 43,012 cf

Reach SC2: Stream Crossing #2Avg. Flow Depth=0.03' Max Vel=1.04 fps Inflow=0.46 cfs 2,770 cf 192.0" x 60.0", R=180.0" Arch Pipe n=0.030 L=36.5' S=0.0241 '/' Capacity=768.96 cfs Outflow=0.46 cfs 2,770 cf

Pond 1P: DMH #33 Peak Elev=206.05' Inflow=1.12 cfs 3,877 cf

12.0" Round Culvert n=0.013 L=46.7' S=0.0251'/' Outflow=1.12 cfs 3,877 cf

Pond 3P: OCS #8 Peak Elev=201.07' Inflow=0.79 cfs 2,578 cf

Outflow=0.79 cfs 2,578 cf

Pond 5R: TRENCH DRAIN

Peak Elev=197.74' Inflow=0.71 cfs 2,338 cf

8.0" Round Culvert n=0.012 L=36.0' S=0.0200 '/' Outflow=0.71 cfs 2,338 cf

Pond 11P: YARD DRAIN Peak Elev=207.32' Storage=383 cf Inflow=0.78 cfs 2,476 cf

Outflow=0.51 cfs 2,448 cf

Pond CB10: CB #10 Peak Elev=210.18' Inflow=0.50 cfs 1,761 cf

12.0" Round Culvert n=0.013 L=33.8' S=0.0050 '/' Outflow=0.50 cfs 1,761 cf

Pond CB11: CB #11 Peak Elev=210.32' Inflow=0.51 cfs 1,814 cf

12.0" Round Culvert n=0.013 L=26.3' S=0.0103'/' Outflow=0.51 cfs 1,814 cf

12.0" Round Culvert n=0.013 L=41.3' S=0.0249 '/' Outflow=0.37 cfs 1.325 cf

Pond CB12: CB #12 Peak Elev=206.98' Inflow=0.37 cfs 1,325 cf

Pond CB13: CB #13 Peak Elev=207.14' Inflow=0.75 cfs 2,552 cf 12.0" Round Culvert n=0.013 L=43.7' S=0.0249 '/' Outflow=0.75 cfs 2,552 cf

Pond CB14: CB #14 Peak Elev=201.41' Inflow=0.73 cfs 2,341 cf

12.0" Round Culvert n=0.013 L=23.2' S=0.0052 '/' Outflow=0.73 cfs 2,341 cf

Pond CB15: CB #15 Peak Elev=201.34' Inflow=0.47 cfs 1,686 cf 12.0" Round Culvert n=0.013 L=15.6' S=0.0051'/' Outflow=0.47 cfs 1.686 cf

Pond CB16: CB #16 Peak Elev=203.82' Inflow=0.31 cfs 985 cf 12.0" Round Culvert n=0.013 L=20.9' S=0.0067 '/' Outflow=0.31 cfs 985 cf

19097 Post-DevelopmentPrepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 16

Pond CB17: CB #17	Peak Elev=205.48' Inflow=0.73 cfs 2,382 cf 12.0" Round Culvert n=0.013 L=13.8' S=0.0094 '/' Outflow=0.73 cfs 2,382 cf
Pond CB18: CB #18	Peak Elev=205.37' Inflow=1.28 cfs 3,873 cf 15.0" Round Culvert n=0.013 L=25.1' S=0.0052 '/' Outflow=1.28 cfs 3,873 cf
Pond CB20: CB #20	Peak Elev=204.51' Inflow=0.80 cfs 2,698 cf 12.0" Round Culvert n=0.013 L=30.3' S=0.0053'/' Outflow=0.80 cfs 2,698 cf
Pond CB21: CB #21	Peak Elev=204.78' Inflow=0.59 cfs 1,892 cf 12.0" Round Culvert n=0.013 L=26.0' S=0.0050 '/' Outflow=0.59 cfs 1,892 cf
Pond CB22: CB #22	Peak Elev=205.89' Inflow=0.83 cfs 2,817 cf 12.0" Round Culvert n=0.012 L=16.1' S=0.0050 '/' Outflow=0.83 cfs 2,817 cf
Pond CB23: CB #23	Peak Elev=205.85' Inflow=0.54 cfs 1,732 cf 12.0" Round Culvert n=0.012 L=16.3' S=0.0055 '/' Outflow=0.54 cfs 1,732 cf
Pond CB24: CB #24	Peak Elev=205.64' Inflow=0.51 cfs 1,660 cf 12.0" Round Culvert n=0.012 L=12.1' S=0.0050 '/' Outflow=0.51 cfs 1,660 cf
Pond CB25: CB #25	Peak Elev=205.64' Inflow=0.56 cfs 1,846 cf 15.0" Round Culvert n=0.012 L=11.4' S=0.0053 '/' Outflow=0.56 cfs 1,846 cf
Pond CB26: CB #26	Peak Elev=202.20' Inflow=0.53 cfs 1,710 cf 12.0" Round Culvert n=0.013 L=42.5' S=0.0052 '/' Outflow=0.53 cfs 1,710 cf
Pond CB27: CB #27	Peak Elev=201.38' Inflow=0.42 cfs 1,434 cf 12.0" Round Culvert n=0.013 L=18.0' S=0.0056 '/' Outflow=0.42 cfs 1,434 cf
Pond CB28: CB #28	Peak Elev=198.22' Inflow=0.56 cfs 1,779 cf 12.0" Round Culvert n=0.013 L=13.7' S=0.0044 '/' Outflow=0.56 cfs 1,779 cf
Pond CB29: CB #29	Peak Elev=205.89' Inflow=0.56 cfs 1,848 cf 12.0" Round Culvert n=0.013 L=13.5' S=0.0052 '/' Outflow=0.56 cfs 1,848 cf
Pond CB30: CB #30	Peak Elev=205.90' Inflow=0.59 cfs 1,943 cf 12.0" Round Culvert n=0.013 L=17.5' S=0.0051 '/' Outflow=0.59 cfs 1,943 cf
Pond CB31: CB #31	Peak Elev=204.80' Inflow=0.95 cfs 3,044 cf 12.0" Round Culvert n=0.013 L=16.4' S=0.0049 '/' Outflow=0.95 cfs 3,044 cf
Pond CB32: CB #32	Peak Elev=204.73' Inflow=0.76 cfs 2,460 cf 12.0" Round Culvert n=0.013 L=16.3' S=0.0049'/ Outflow=0.76 cfs 2,460 cf
Pond CB33: CB #33	Peak Elev=205.66' Inflow=0.36 cfs 1,179 cf 12.0" Round Culvert n=0.013 L=11.7' S=0.0051 '/' Outflow=0.36 cfs 1,179 cf
Pond CB34: CB #34	Peak Elev=205.69' Inflow=0.55 cfs 1,804 cf 12.0" Round Culvert n=0.013 L=16.5' S=0.0048'/ Outflow=0.55 cfs 1,804 cf

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

<u>Page 17</u>

Peak Elev=207.36' Inflow=0.30 cfs 1,049 cf Pond CB35: CB #35 12.0" Round Culvert n=0.013 L=15.2' S=0.0053 '/' Outflow=0.30 cfs 1.049 cf Peak Elev=207.45' Inflow=0.47 cfs 1,675 cf Pond CB36: CB #36 12.0" Round Culvert n=0.013 L=16.1' S=0.0050 '/' Outflow=0.47 cfs 1,675 cf Pond CB38: CB #38 Peak Elev=210.28' Inflow=0.54 cfs 1,932 cf 12.0" Round Culvert n=0.012 L=16.7' S=0.0048 '/' Outflow=0.54 cfs 1.932 cf Peak Elev=210.28' Inflow=0.54 cfs 1,925 cf Pond CB39: CB #39 12.0" Round Culvert n=0.013 L=16.4' S=0.0049 '/' Outflow=0.54 cfs 1,925 cf Pond CB40: CB #40 Peak Elev=214.02' Inflow=0.30 cfs 1,065 cf 12.0" Round Culvert n=0.013 L=17.8' S=0.0073 '/' Outflow=0.30 cfs 1.065 cf Pond CB41: CB #41 Peak Elev=214.27' Inflow=0.40 cfs 1,413 cf 12.0" Round Culvert n=0.013 L=18.4' S=0.0049 '/' Outflow=0.40 cfs 1,413 cf Peak Elev=220.29' Inflow=0.17 cfs 555 cf Pond CB43: CB #43 12.0" Round Culvert n=0.013 L=14.9' S=0.0047 '/' Outflow=0.17 cfs 555 cf Peak Elev=220.26' Inflow=0.12 cfs 398 cf Pond CB44: CB #44 12.0" Round Culvert n=0.013 L=14.9' S=0.0047 '/' Outflow=0.12 cfs 398 cf Peak Elev=221.47' Inflow=0.09 cfs 299 cf Pond CB45: CB #45 12.0" Round Culvert n=0.013 L=18.2' S=0.0049 '/' Outflow=0.09 cfs 299 cf Peak Elev=221.77' Inflow=0.17 cfs 533 cf Pond CB46: CB #46 12.0" Round Culvert n=0.013 L=15.3' S=0.0052 '/' Outflow=0.17 cfs 533 cf Peak Elev=225.32' Inflow=0.21 cfs 762 cf **Pond CB47: CB#47** 12.0" Round Culvert n=0.012 L=20.9' S=0.0048 '/' Outflow=0.21 cfs 762 cf Peak Elev=225.40' Inflow=1.03 cfs 4,342 cf **Pond CB48: CB#48** 15.0" Round Culvert n=0.012 L=16.9' S=0.0047 '/' Outflow=1.03 cfs 4,342 cf Peak Elev=203.25' Inflow=0.34 cfs 1,139 cf Pond CB49: CB #49 12.0" Round Culvert n=0.013 L=15.5' S=0.0052'/' Outflow=0.34 cfs 1.139 cf Pond CB50: CB #50 Peak Elev=203.41' Inflow=0.96 cfs 3,147 cf 12.0" Round Culvert n=0.013 L=15.3' S=0.0052 '/' Outflow=0.96 cfs 3,147 cf Pond CB51: CB #51 Peak Elev=202.76' Inflow=0.49 cfs 1,726 cf 12.0" Round Culvert n=0.013 L=31.4' S=0.0051'/' Outflow=0.49 cfs 1,726 cf Pond CB52: CB #52 Peak Elev=203.15' Inflow=0.61 cfs 2,045 cf 12.0" Round Culvert n=0.013 L=25.5' S=0.0051 '/' Outflow=0.61 cfs 2.045 cf Pond CB53: CB #53 Peak Elev=203.20' Inflow=0.50 cfs 1.646 cf

12.0" Round Culvert n=0.013 L=32.0' S=0.0050 '/' Outflow=0.50 cfs 1,646 cf

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 18

Pond CB54: CB #54	Peak Elev=202.98' Inflow=0.29 cfs 933 cf 12.0" Round Culvert n=0.013 L=36.7' S=0.0049 '/' Outflow=0.29 cfs 933 cf
Pond CB7: CB#5	Peak Elev=212.90' Inflow=0.33 cfs 1,176 cf 12.0" Round Culvert n=0.012 L=15.1' S=0.0099'/' Outflow=0.33 cfs 1,176 cf

Pond CB8: CB#8 Peak Elev=214.11' Inflow=0.36 cfs 1,185 cf 12.0" Round Culvert n=0.013 L=15.1' S=0.0099 '/' Outflow=0.36 cfs 1,185 cf

Pond CB9: CB #9 Peak Elev=210.66' Inflow=1.14 cfs 3,974 cf 12.0" Round Culvert n=0.013 L=19.9' S=0.0196'/ Outflow=1.14 cfs 3,974 cf

Pond D10: DMH #10 Peak Elev=203.66' Inflow=0.31 cfs 985 cf 12.0" Round Culvert n=0.013 L=15.6' S=0.0051 '/' Outflow=0.31 cfs 985 cf

Pond D11: DMH #11 Peak Elev=205.02' Inflow=2.02 cfs 6,256 cf 18.0" Round Culvert n=0.013 L=44.6' S=0.0049 '/' Outflow=2.02 cfs 6,256 cf

Pond D12: DMH #12 Peak Elev=203.96' Inflow=1.40 cfs 4,590 cf 12.0" Round Culvert n=0.013 L=41.9' S=0.0050 '/' Outflow=1.40 cfs 4,590 cf

Pond D13: DMH #13 Peak Elev=202.97' Inflow=4.21 cfs 15,091 cf 24.0" Round Culvert n=0.013 L=60.1' S=0.0050 '/' Outflow=4.21 cfs 15,091 cf

Pond D14: DMH #14 Peak Elev=204.91' Inflow=2.43 cfs 8,054 cf 18.0" Round Culvert n=0.012 L=256.3' S=0.0050 '/' Outflow=2.43 cfs 8,054 cf

Pond D16: DMH #16 Peak Elev=205.47' Inflow=1.06 cfs 3,505 cf 15.0" Round Culvert n=0.012 L=103.5' S=0.0050 '/' Outflow=1.06 cfs 3,505 cf

Pond D17: DMH #17 Peak Elev=201.05' Inflow=0.95 cfs 3,144 cf 12.0" Round Culvert n=0.013 L=91.6' S=0.0312 '/' Outflow=0.95 cfs 3,144 cf

Pond D18: DMH #18 Peak Elev=198.06' Inflow=1.51 cfs 4,923 cf 15.0" Round Culvert n=0.013 L=46.3' S=0.0099 '/' Outflow=1.51 cfs 4,923 cf

Pond D19: DMH #19 Peak Elev=205.76' Inflow=1.15 cfs 3,791 cf 12.0" Round Culvert n=0.013 L=82.5' S=0.0092 '/' Outflow=1.15 cfs 3,791 cf

Pond D2: DMH#2 Peak Elev=207.57' Inflow=1.72 cfs 7,465 cf 15.0" Round Culvert n=0.013 L=38.2' S=0.0099 '/' Outflow=1.72 cfs 7,465 cf

Pond D20: DMH #20 Peak Elev=204.78' Inflow=1.15 cfs 3,791 cf 15.0" Round Culvert n=0.013 L=63.5' S=0.0050 '/' Outflow=1.15 cfs 3,791 cf

Pond D21: DMH #21 Peak Elev=204.08' Inflow=4.53 cfs 15,002 cf 24.0" Round Culvert n=0.013 L=72.4' S=0.0050 '/' Outflow=4.53 cfs 15.002 cf

Pond D22: DMH #22 Peak Elev=205.54' Inflow=1.67 cfs 5,708 cf 15.0" Round Culvert n=0.013 L=134.2' S=0.0071 '/' Outflow=1.67 cfs 5,708 cf

Pond DE62: DRIP #62

Peak Elev=213.08' Storage=238 cf Inflow=0.29 cfs 997 cf

Discarded=0.00 cfs 181 cf Primary=0.24 cfs 676 cf Outflow=0.24 cfs 857 cf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 19

Pond D23: DMH #23	Peak Elev=207.11' Inflow=0.77 cfs 2,724 cf 15.0" Round Culvert n=0.013 L=173.3' S=0.0100 '/' Outflow=0.77 cfs 2,724 cf
Pond D25: DMH #25	Peak Elev=210.19' Inflow=2.34 cfs 8,121 cf 15.0" Round Culvert n=0.012 L=237.6' S=0.0050 '/' Outflow=2.34 cfs 8,121 cf
Pond D27: DMH #27	Peak Elev=213.88' Inflow=1.26 cfs 4,263 cf 15.0" Round Culvert n=0.012 L=63.9' S=0.0150 '/' Outflow=1.26 cfs 4,263 cf
Pond D28: DMH #28	Peak Elev=217.83' Inflow=0.56 cfs 1,785 cf 12.0" Round Culvert n=0.013 L=158.3' S=0.0200 '/' Outflow=0.56 cfs 1,785 cf
Pond D29: DMH #29	Peak Elev=220.20' Inflow=0.56 cfs 1,785 cf 12.0" Round Culvert n=0.013 L=150.9' S=0.0151 '/' Outflow=0.56 cfs 1,785 cf
Pond D30: DMH #30	Peak Elev=221.21' Inflow=0.26 cfs 832 cf 12.0" Round Culvert n=0.013 L=184.2' S=0.0050 '/' Outflow=0.26 cfs 832 cf
Pond D31: DMH#31	Peak Elev=225.15' Inflow=1.18 cfs 5,104 cf 15.0" Round Culvert n=0.012 L=288.5' S=0.0400 '/' Outflow=1.18 cfs 5,104 cf
Pond D34: DMH #34	Peak Elev=198.77' Inflow=1.66 cfs 5,882 cf 12.0" Round Culvert n=0.012 L=52.0' S=0.0200 '/' Outflow=1.66 cfs 5,882 cf
Pond D35: DMH #35	Peak Elev=212.82' Inflow=1.26 cfs 4,263 cf 15.0" Round Culvert n=0.012 L=171.5' S=0.0150 '/' Outflow=1.26 cfs 4,263 cf
Pond D4: DMH#4	Peak Elev=210.98' Inflow=1.72 cfs 7,465 cf 15.0" Round Culvert n=0.012 L=222.3' S=0.0150 '/' Outflow=1.72 cfs 7,465 cf
Pond D5: DMH #5	Peak Elev=209.86' Inflow=2.15 cfs 7,549 cf 18.0" Round Culvert n=0.013 L=183.0' S=0.0050 '/' Outflow=2.15 cfs 7,549 cf
Pond D6: DMH #6	Peak Elev=208.82' Inflow=2.15 cfs 7,549 cf 18.0" Round Culvert n=0.013 L=299.7' S=0.0050 '/' Outflow=2.15 cfs 7,549 cf
Pond D7: DMH #7	Peak Elev=207.14' Inflow=2.15 cfs 7,549 cf 18.0" Round Culvert n=0.013 L=44.2' S=0.0550 '/' Outflow=2.15 cfs 7,549 cf
Pond D8: DMH #8	Peak Elev=201.27' Inflow=1.20 cfs 4,028 cf 12.0" Round Culvert n=0.013 L=87.7' S=0.0050 '/' Outflow=1.20 cfs 4,028 cf
Pond D9: DMH #9	Peak Elev=200.73' Inflow=1.20 cfs 4,028 cf 12.0" Round Culvert n=0.013 L=11.9' S=0.0050 '/' Outflow=1.20 cfs 4,028 cf
Pond DE61: DRIP #61	Peak Elev=213.08' Storage=238 cf Inflow=0.29 cfs 997 cf Discarded=0.00 cfs 181 cf Primary=0.24 cfs 676 cf Outflow=0.24 cfs 857 cf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 20

Pond DE63: DRIP #63		Peak	Elev=207.82	' Storage=135 cf	Inflow=0.20 cfs	681 cf

Discarded=0.00 cfs 107 cf Primary=0.18 cfs 488 cf Outflow=0.18 cfs 596 cf

Pond DE64: DRIP #64 Peak Elev=205.85' Storage=162 cf Inflow=0.24 cfs 814 cf

Discarded=0.00 cfs 129 cf Primary=0.21 cfs 586 cf Outflow=0.21 cfs 715 cf

Pond DE65: DRIP #65 Peak Elev=206.82' Storage=135 cf Inflow=0.20 cfs 682 cf

Discarded=0.00 cfs 107 cf Primary=0.18 cfs 489 cf Outflow=0.18 cfs 596 cf

Pond DE66: DRIP #66 Peak Elev=208.65' Storage=161 cf Inflow=0.23 cfs 800 cf Discarded=0.00 cfs 129 cf Primary=0.20 cfs 571 cf Outflow=0.21 cfs 700 cf

Pond DE67: DRIP #67 Peak Elev=208.85' Storage=162 cf Inflow=0.24 cfs 817 cf

Discarded=0.00 cfs 129 cf Primary=0.21 cfs 588 cf Outflow=0.21 cfs 718 cf

Pond DE68: DRIP #68 Peak Elev=207.82' Storage=220 cf Inflow=0.29 cfs 989 cf

Discarded=0.00 cfs 181 cf Primary=0.25 cfs 669 cf Outflow=0.25 cfs 850 cf

Pond DE69: DRIP #69 Peak Elev=206.35' Storage=162 cf Inflow=0.24 cfs 817 cf

Discarded=0.00 cfs 129 cf Primary=0.21 cfs 588 cf Outflow=0.21 cfs 717 cf

Pond DE70: DRIP #70 Peak Elev=206.75' Storage=162 cf Inflow=0.24 cfs 816 cf

Discarded=0.00 cfs 129 cf Primary=0.21 cfs 587 cf Outflow=0.21 cfs 716 cf

Pond DE71: DRIP #71 Peak Elev=207.38' Storage=237 cf Inflow=0.29 cfs 988 cf

Discarded=0.00 cfs 181 cf Primary=0.24 cfs 667 cf Outflow=0.24 cfs 848 cf

Pond DECH: DRIP #CH Peak Elev=209.01' Storage=260 cf Inflow=0.43 cfs 1,470 cf

Discarded=0.04 cfs 1,053 cf Primary=0.25 cfs 416 cf Outflow=0.28 cfs 1,469 cf

Pond DMH32: DMH #32 Peak Elev=203.19' Inflow=1.31 cfs 4,287 cf

12.0" Round Culvert n=0.013 L=19.2' S=0.0531 '/' Outflow=1.31 cfs 4,287 cf

Pond OCS1: OCS#1 Peak Elev=195.75' Inflow=3.17 cfs 10,805 cf

Outflow=3.17 cfs 10,805 cf

Pond OCS3: OCS#3 Peak Elev=203.95' Inflow=3.26 cfs 10,698 cf

Outflow=3.26 cfs 10.698 cf

Pond OCS4: OCS#4 Peak Elev=203.89' Inflow=0.64 cfs 1,977 cf

Outflow=0.64 cfs 1,977 cf

Pond OCS6: OCS #6 Peak Elev=201.93' Inflow=1.12 cfs 3,877 cf

Outflow=1.12 cfs 3,877 cf

Pond OCS7: OCS #7 Peak Elev=202.32' Inflow=1.10 cfs 3,771 cf

Outflow=1.10 cfs 3.771 cf

Pond P204: STORMTECHINFILTRATION Peak Elev=203.89' Storage=5,017 cf Inflow=3.90 cfs 12,675 cf Discarded=0.09 cfs 5,215 cf Primary=0.93 cfs 5,781 cf Outflow=1.02 cfs 10,996 cf

19097	Post-Dev	elopment
13031	I USL-DEV	CIODIIICIIL

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 21

Pond P205: INFILTRATIONPOND #3 Peak Elev=205.79' Storage=3,362 cf Inflow=2.27 cfs 9,251 cf

Discarded=0.26 cfs 9,244 cf Primary=0.00 cfs 0 cf Outflow=0.26 cfs 9,244 cf

Pond P206: STORMTECHINFILTRATION Peak Elev=195.67' Storage=4,118 cf Inflow=3.88 cfs 13,143 cf Discarded=0.49 cfs 13,142 cf Primary=0.00 cfs 0 cf Outflow=0.49 cfs 13,142 cf

Pond P207: INFILTRATIONPOND #2

Peak Elev=197.50' Storage=7,511 cf Inflow=6.85 cfs 23,672 cf

Discarded=0.96 cfs 23,563 cf Primary=0.04 cfs 101 cf Outflow=1.00 cfs 23,663 cf

Pond P210: POCKET WETLAND#1 Peak Elev=202.97' Storage=7,606 cf Inflow=2.95 cfs 10,193 cf

Outflow=0.12 cfs 4,736 cf

Pond P212: INFILTRATIONPOND #1 Peak Elev=201.74' Storage=14,075 cf Inflow=9.79 cfs 37,028 cf

Discarded=1.25 cfs 37,018 cf Primary=0.00 cfs 0 cf Outflow=1.25 cfs 37,018 cf

Pond P213: Stormtech Infiltration System #3 Peak Elev=201.93' Storage=1,973 cf Inflow=2.22 cfs 7,648 cf Discarded=0.39 cfs 7,647 cf Primary=0.00 cfs 0 cf Outflow=0.39 cfs 7,647 cf

Pond P214: STORMTECHINFILTRATION Peak Elev=200.78' Storage=1,357 cf Inflow=2.21 cfs 7,633 cf Discarded=0.61 cfs 7,642 cf Primary=0.00 cfs 0 cf Outflow=0.61 cfs 7,642 cf

Link AP1: ANALYSISPOINT 1 Inflow=0.67 cfs 2,247 cf

Primary=0.67 cfs 2,247 cf

Link AP2: ANALYSISPOINT 2 Inflow=7.31 cfs 75,144 cf

Primary=7.31 cfs 75,144 cf

Link AP3: ANALYSISPOINT 3 Inflow=1.15 cfs 4,009 cf

Primary=1.15 cfs 4,009 cf

Link AP4: ANALYSISPOINT #4 Inflow=7.79 cfs 65,174 cf

Primary=7.79 cfs 65,174 cf

Total Runoff Area = 2,573,920 sf Runoff Volume = 266,846 cf Average Runoff Depth = 1.24" 70.09% Pervious = 1,803,997 sf 29.91% Impervious = 769,923 sf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 22

Summary for Subcatchment B1: MULTIFAMILY BLDG #1

Runoff = 1.66 cfs @ 12.09 hrs, Volume= 5,882 cf, Depth> 3.04"

Routed to Pond D34: DMH #34

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description				
	20,156	98	Roofs, HSG C				
	3,099	98	Roofs, HSG	D D			
	23,255	98	Weighted A	verage			
	23,255		100.00% Im	pervious A	\rea		
Tc	Length	Slope	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment B2: MULTIFAMILY BLDG #2

Runoff = 1.25 cfs @ 12.09 hrs, Volume= 4,442 cf, Depth> 3.04"

Routed to Pond OCS3: OCS#3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description		
	7,873	98	Roofs, HSG	Α	
	9,688	98	Roofs, HSG	G C	
	17,561	98	Weighted A	verage	
	17,561		100.00% Im	npervious A	Area
Tc	Length	Slope	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
6.0					Direct Entry,

,

Summary for Subcatchment B3: MULTIFAMILY BUILDING #3

Runoff = 1.42 cfs @ 12.09 hrs, Volume= 5,054 cf, Depth> 3.04" Routed to Pond P214 : STORMTECH INFILTRATION SYSTEM #4

Area (sf)	CN	Description			
608	98	Roofs, HSG A			
19,373	98	Roofs, HSG C			
19,981	98	Weighted Average			
19,981		100.00% Impervious Area			

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 23

	Tc (min)	Length (feet)	•	Velocity (ft/sec)	Capacity (cfs)	Description	
_	6.0					Direct Entry,	

Summary for Subcatchment C10: CB #10

Runoff = 0.50 cfs @ 12.09 hrs, Volume= 1,761 cf, Depth> 3.04"

Routed to Pond CB10 : CB #10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description					
	339	98	Paved road	s w/curbs &	& sewers, HSG B			
	5,703	98	Paved road	s w/curbs &	& sewers, HSG C			
	919	98	Paved road	s w/curbs &	& sewers, HSG D			
	6,961	98	Weighted A	Weighted Average				
	6,961		100.00% In	npervious A	Area			
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment C11: CB #11

Runoff = 0.51 cfs @ 12.09 hrs, Volume= 1,814 cf, Depth> 3.04"

Routed to Pond CB11 : CB #11

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

_	Α	rea (sf)	CN I	Description						
		7,173	98 I	Paved roads w/curbs & sewers, HSG C						
		7,173		100.00% Impervious Area						
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	6.0			Direct Entry.						

Summary for Subcatchment C12: CB #12

Runoff = 0.37 cfs @ 12.09 hrs, Volume= 1,325 cf, Depth> 3.04"

Routed to Pond CB12 : CB #12

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 24

A	rea (sf)	CN E	Description						
	5,238	98 F	Paved roads w/curbs & sewers, HSG C						
	5,238	1	100.00% Impervious Area						
Tc	Length	Slone	pe Velocity Capacity Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment C13: CB #13

Runoff 0.75 cfs @ 12.09 hrs, Volume= 2,552 cf, Depth> 2.82"

Routed to Pond CB13: CB #13

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description						
	1,003	74	>75% Gras	s cover, Go	Good, HSG C				
	7,547	98	Paved park	ing, HSG C	C				
	2,323	98	Roofs, HSC	G C		_			
	10,873	96	Weighted A	verage					
	1,003		9.22% Perv	ious Area					
	9,870		90.78% Imp	pervious Ar	rea				
_				_					
Tc	3	Slope	,	Capacity	·				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)		_			
6.0					Direct Entry,				

Summary for Subcatchment C14: CB #14

0.73 cfs @ 12.09 hrs, Volume= 2,341 cf, Depth> 2.32" Runoff

Routed to Pond CB14: CB #14

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

 Α	rea (sf)	CN	Description								
	1,195	39	>75% Gras	>75% Grass cover, Good, HSG A							
	7,649	98	Paved park	ing, HSG A	١						
	472	74	>75% Gras	s cover, Go	ood, HSG C						
	2,783	98	Paved park	ing, HSG C)						
	12,099	91	Weighted Average								
	1,667		13.78% Per	vious Area	I						
	10,432		86.22% Imp	pervious Ar	ea						
т.	ما العرب ال	Clan	- \/alaaitu	Conneitu	December						
Tc	Length	Slop									
 (min)	(feet)	(ft/f	t) (ft/sec)	(cfs)							
~ ~					D: (= (

6.0 Direct Entry,

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 25

Summary for Subcatchment C15: CB #15

Runoff = 0.47 cfs @ 12.09 hrs, Volume= 1,686 cf, Depth> 3.04"

Routed to Pond CB15: CB #15

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description	Description					
	5,000	98	Paved parking, HSG A						
	1,666	98	Paved park	Paved parking, HSG C					
	6,666	98	Weighted A	Veighted Average					
	6,666		100.00% Impervious Area						
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment C16: CB #16

Runoff = 0.31 cfs @ 12.10 hrs, Volume= 985 cf, Depth> 1.39"

Routed to Pond CB16: CB #16

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description						
	2,467	39	>75% Gras	s cover, Go	ood, HSG A				
	4,380	98	Paved park	ing, HSG A	4				
	524	74	>75% Ġras	s cover, Go	ood, HSG C				
	1,145	98	Paved park	ing, HSG C					
	8,516	79	Weighted Average						
	2,991		35.12% Pei	vious Area	a				
	5,525		64.88% lmp	ervious Ar	rea				
Tc	Length	Slope	•	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment C17: CB #17

Runoff = 0.73 cfs @ 12.09 hrs, Volume= 2,382 cf, Depth> 2.42"

Routed to Pond CB17: CB #17

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 26

	Α	rea (sf)	CN	Description							
		3,093	74	>75% Grass cover, Good, HSG C							
		8,743	98	Paved park	ing, HSG C	C					
		11,836	92	Weighted Average							
		3,093		26.13% Pei	rvious Area	a					
		8,743	•	73.87% Imp	pervious Ar	rea					
	-		01		0 "	D					
	Tc	Length	Slope	e Velocity Capacity Description							
_	(min)	(feet)	(ft/ft)) (ft/sec) (cfs)							
•	6.0			Direct Entry							

6.0 Direct Entry,

Summary for Subcatchment C18: CB #18

Runoff = 1.08 cfs @ 12.09 hrs, Volume= 3,458 cf, Depth> 2.23"

Routed to Pond CB18: CB #18

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description					
	6,255	74	>75% Gras	s cover, Go	ood, HSG C			
	12,336	98	Paved park	ing, HSG C	2			
	18,591	90	Weighted Average					
	6,255		33.65% Pervious Area					
	12,336		66.35% Imp	pervious Ar	rea			
_		01		0 :				
Tc	Length	Slope	,	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C20: CB #20

Runoff = 0.80 cfs @ 12.09 hrs, Volume= 2,698 cf, Depth> 2.71"

Routed to Pond CB20 : CB #20

 Area (sf)	CN	Description				
3,319	98	Paved parking, HSG A				
1,319	74	>75% Grass cover, Good, HSG C				
 7,301	98	Paved parking, HSG C				
11,939	95	Weighted Average				
1,319		11.05% Pervious Area				
10,620		88.95% Impervious Area				

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 27

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment C21: CB #21

Runoff = 0.59 cfs @ 12.09 hrs, Volume= 1,892 cf, Depth> 2.23"

Routed to Pond CB21 : CB #21

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description					
	1,319	39	>75% Gras	s cover, Go	Good, HSG A			
	7,301	98	Paved park	ing, HSG A	A			
	1,554	98	Paved park	ing, HSG C	C			
	10,174	90	Weighted A	verage				
	1,319		12.96% Pervious Area					
	8,855		87.04% Imp	ervious Ar	rea			
Tc (min)	Length (feet)	Slop (ft/ft	•	Capacity (cfs)	•			
6.0					Direct Entry,			

Summary for Subcatchment C22: CB #22

Runoff = 0.83 cfs @ 12.09 hrs, Volume= 2,817 cf, Depth> 2.82"

Routed to Pond CB22 : CB #22

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Area (sf)	CN	Description					
2,946	98	Paved parking, HSG A					
177	74	>75% Grass cover, Good, HSG C					
2,641	98	Paved parking, HSG C					
829	80	>75% Grass cover, Good, HSG D					
5,408	98	Paved parking, HSG D					
12,001	96	Weighted Average					
1,006		8.38% Pervious Area					
10,995	10,995 91.62% Impervious Area						
Tc Length	n Sloj	pe Velocity Capacity Description					
(min) (feet) (ft/	(ft) (ft/sec) (cfs)					
		Diversit Fortune					

6.0 Direct Entry,

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 28

Summary for Subcatchment C23: CB #23

Runoff = 0.54 cfs @ 12.09 hrs, Volume= 1,732 cf, Depth> 2.14"

Routed to Pond CB23: CB #23

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description					
	242	39	>75% Gras	s cover, Go	ood, HSG A			
	3,016	98	Paved park	ing, HSG A	١			
	1,267	74	>75% Gras	s cover, Go	ood, HSG C			
	218	98	Paved park	ing, HSG C	;			
	2,272	80	>75% Gras	s cover, Go	ood, HSG D			
	2,679	98	Paved park	ing, HSG [)			
	9,694	89	Weighted Average					
	3,781		39.00% Pervious Area					
	5,913		61.00% Impervious Area					
Tc	Length	Slop	•	Capacity	Description			
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C24: CB #24

Runoff = 0.51 cfs @ 12.09 hrs, Volume= 1,660 cf, Depth> 2.51"

Routed to Pond CB24: CB #24

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

	rea (sf)	CN	Description					
	5,722	98	Paved park	ing, HSG D	D			
	2,208	80	>75% Ġras	s cover, Go	lood, HSG D			
	7,930	93	Weighted Average					
	2,208		27.84% Pe	vious Area	a			
	5,722		72.16% lm <mark>։</mark>	ervious Ar	rea			
_								
Тс	5	Slope	,	Capacity	·			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C25: CB #25

Runoff = 0.56 cfs @ 12.09 hrs, Volume= 1,846 cf, Depth> 2.61"

Routed to Pond CB25: CB #25

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 29

A	rea (sf)	CN	Description					
	211	39	>75% Gras	s cover, Go	od, HSG A			
	519	98	Paved park	ing, HSG A				
	15	74	>75% Gras	s cover, Go	od, HSG C			
	300	98	Paved park	ing, HSG C				
	1,393	80	>75% Gras	s cover, Go	od, HSG D			
	6,049	98	Paved park	ing, HSG D				
	8,487	94	Weighted Average					
	1,619		19.08% Per	vious Area				
	6,868		80.92% Impervious Area					
Tc	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C26: CB #26

Runoff = 0.53 cfs @ 12.09 hrs, Volume= 1,710 cf, Depth> 2.32"

Routed to Pond CB26: CB #26

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description					
	3,203	80	>75% Gras	s cover, Go	Good, HSG D			
	5,632	98	Paved park	ing, HSG D	D			
	8,835	91	Weighted Average					
	3,203	;	36.25% Pervious Area					
	5,632	(33.75% Imp	pervious Ar	rea			
Tc	Length	Slope	,	Capacity	·			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C27: CB #27

Runoff = 0.42 cfs @ 12.09 hrs, Volume= 1,434 cf, Depth> 2.82"

Routed to Pond CB27: CB #27

Area (sf)	CN	Description				
98	39	>75% Grass cover, Good, HSG A				
131	98	Paved parking, HSG A				
397	80	>75% Grass cover, Good, HSG D				
5,485	98	Paved parking, HSG D				
6,111	96	Weighted Average				
495)	8.10% Pervious Area				
5,616	;	91.90% Impervious Area				

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 30

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment C28: CB #28

Runoff = 0.56 cfs @ 12.09 hrs, Volume= 1,779 cf, Depth> 2.06"

Routed to Pond CB28: CB #28

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Aı	rea (sf)	CN	Description					
	2,751	74	>75% Grass	cover, Go	Good, HSG C			
	2,841	98	Paved parki	ng, HSG C	C			
	2,297	80	>75% Grass	cover, Go	Good, HSG D			
	2,483	98	Paved parki	ng, HSG D	D			
•	10,372	88	Weighted Average					
	5,048		48.67% Pervious Area					
	5,324		51.33% Impervious Area					
_								
Тс	Length	Slope	•	Capacity	•			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C29: CB #29

Runoff = 0.56 cfs @ 12.09 hrs, Volume= 1,848 cf, Depth> 2.61"

Routed to Pond CB29: CB #29

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

	Aı	rea (sf)	CN I	Description							
		1,341	74 :	>75% Grass cover, Good, HSG C							
		5,330	98 I	Paved park	ing, HSG C	C					
		1,824	98	Roofs, HSC	G C						
		8,495	94 \	Weighted Average							
		1,341		15.79% Pei	vious Area	a					
		7,154	;	84.21% Impervious Area							
	Tc	Length	Slope	,	Capacity	•					
<u>(n</u>	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	60					Direct Entry					

6.0 Direct Entry,

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 31

Summary for Subcatchment C30: CB #30

Runoff = 0.59 cfs @ 12.09 hrs, Volume= 1,943 cf, Depth> 2.61"

Routed to Pond CB30: CB #30

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description							
	1,572	74	>75% Grass cover, Good, HSG C							
	6,310	98	Paved park	ing, HSG C	C					
	1,051	98	Roofs, HSG C							
	8,933	94	94 Weighted Average							
	1,572		17.60% Pe	rvious Area	a					
	7,361		82.40% Imp	pervious Ar	rea					
_					-					
Тс	Length	Slope	,	Capacity	Description					
(min)_	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment C31: CB #31

Runoff = 0.95 cfs @ 12.09 hrs, Volume= 3,044 cf, Depth> 2.23"

Routed to Pond CB31: CB #31

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

	Area (sf)	CN	Description							
	5,132	74	>75% Grass cover, Good, HSG C							
	9,132	98	Paved park	ing, HSG C	C					
	2,101	98	Roofs, HSG C							
	16,365	90	0 Weighted Average							
	5,132		31.36% Pervious Area							
	11,233		68.64% lmp	pervious Ar	rea					
_										
Тс	Length	Slope	,	Capacity	·					
<u>(min)</u>	(feet)	(ft/ft	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment C32: CB #32

Runoff = 0.76 cfs @ 12.09 hrs, Volume= 2,460 cf, Depth> 2.32" Routed to Pond CB32 : CB #32

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 32

Area	(sf) CN	Description							
3,	753 74	>75% Grass cover, Good, HSG C							
7,0	068 98	Paved park	ing, HSG C	C					
1,8	389 98	Roofs, HSG	Roofs, HSG C						
12,	710 91	91 Weighted Average							
3,	753	29.53% Per	vious Area	a					
8,9	957	70.47% lmp	ervious Ar	rea					
- .				D					
	ngth Slo		Capacity	·					
(min)(feet) (ft/	ft) (ft/sec)	(cfs)						
6.0				Direct Entry,					

Summary for Subcatchment C33: CB #33

Runoff = 0.36 cfs @ 12.09 hrs, Volume= 1,179 cf, Depth> 2.61"

Routed to Pond CB33 : CB #33

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description								
	873	74	>75% Gras	>75% Grass cover, Good, HSG C							
	3,693	98	Paved park	ing, HSG C	C						
	855	98	Roofs, HSC	S C							
	5,421 873 4,548	94	Weighted A 16.10% Per 83.90% Imp	rvious Area							
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	·						
6.0					Direct Entry,						

Summary for Subcatchment C34: CB #34

Runoff = 0.55 cfs @ 12.09 hrs, Volume= 1,804 cf, Depth> 2.51"

Routed to Pond CB34: CB #34

Area (sf)	CN	Description			
1,680	74	>75% Grass cover, Good, HSG C			
5,115	98	Paved parking, HSG C			
1,827	98	Roofs, HSG C			
8,622	93	Weighted Average			
1,680		19.49% Pervious Area			
6,942		80.51% Impervious Area			

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 33

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•
6.0					Direct Entry

Direct Entry,

Summary for Subcatchment C35: CB #35

1,049 cf, Depth> 3.04" 0.30 cfs @ 12.09 hrs, Volume=

Routed to Pond CB35: CB #35

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

	Area (sf)	CN	Description						
	3,578	98	Paved park	ing, HSG C	С				
	79	80	>75% Gras	s cover, Go	Good, HSG D				
	492	98	Paved park	ing, HSG D	D				
	4,149	98	Weighted Average						
	79		1.90% Pervious Area						
	4,070		98.10% Impervious Area						
To	Length	Slope	e Velocity	Capacity	/ Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0)				Direct Entry.				

Summary for Subcatchment C36: CB #36

0.47 cfs @ 12.09 hrs, Volume= 1,675 cf, Depth> 3.04" Runoff

Routed to Pond CB36: CB #36

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

_	Α	rea (sf)	CN I	Description							
		6,622	98 I	Paved parking, HSG C							
		6,622	•	100.00% Impervious Area							
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
-	6.0	(1001)	(10,10)	(14000)	(0.0)	Direct Entry,					

Summary for Subcatchment C38: CB #38

0.54 cfs @ 12.09 hrs, Volume= 1,932 cf, Depth> 3.04" Runoff

Routed to Pond CB38: CB #38

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

<u>Page 34</u>

	rea (sf)	CN	Description						
	6,647	98	Paved parking, HSG B						
	392	98	Paved park	ing, HSG C	C				
	598	98	Paved park	Paved parking, HSG D					
	7,637	98	Weighted Average						
	7,637		100.00% Impervious Area						
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)						
6.0					Direct Entry				

.0 Direct Entry,

Summary for Subcatchment C39: CB #39

Runoff = 0.54 cfs @ 12.09 hrs, Volume= 1,925 cf, Depth> 3.04"

Routed to Pond CB39: CB #39

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Aı	rea (sf)	CN	Description				
	6,505	98	Paved park	ing, HSG B	В		
	519	98	Paved park	ing, HSG C	C		
	588	98	Paved park	ing, HSG D	D		
•	7,612	98	Weighted Average				
	7,612		100.00% Impervious Area				
Tc (min)	Length (feet)	Slop (ft/ft	,	Capacity (cfs)	•		
6.0					Direct Entry,		

Summary for Subcatchment C40: CB #40

Runoff = 0.30 cfs @ 12.09 hrs, Volume= 1,065 cf, Depth> 3.04"

Routed to Pond CB40 : CB #40

A	rea (sf)	CN E	CN Description								
	4,211	98 F	98 Paved parking, HSG B								
	4,211	1	100.00% Impervious Area								
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
6.0					Direct Entry,						

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 35

Summary for Subcatchment C41: CB #41

Runoff = 0.40 cfs @ 12.09 hrs, Volume= 1,413 cf, Depth> 3.04"

Routed to Pond CB41: CB #41

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN [Description						
	5,586	98 F	Paved parking, HSG B						
	5,586	1	100.00% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment C43: CB #43

Runoff = 0.17 cfs @ 12.09 hrs, Volume= 555 cf, Depth> 2.14"

Routed to Pond CB43: CB #43

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description						
	2,343	98	Paved parking, HSG B						
	766	61	>75% Gras	s cover, Go	lood, HSG B				
	3,109	89	Weighted Average						
	766		24.64% Per	rvious Area	a				
	2,343		75.36% lmp	pervious Ar	rea				
Tc (min)	Length (feet)	Slope (ft/ft	•	Capacity (cfs)	·				
6.0					Direct Entry,				

Summary for Subcatchment C44: CB #44

Runoff = 0.12 cfs @ 12.09 hrs, Volume= 398 cf, Depth> 2.42"

Routed to Pond CB44: CB #44

Area (sf)	CN	Description			
1,670	98	Paved parking, HSG B			
308	61	>75% Grass cover, Good, HSG B			
1,978	92	Weighted Average			
308		15.57% Pervious Area			
1,670		84.43% Impervious Area			

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 36

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment C45: CB #45

Runoff = 0.09 cfs @ 12.10 hrs, Volume= 299 cf, Depth> 1.45"

Routed to Pond CB45 : CB #45

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description						
	1,240	98	Paved parking, HSG B						
	1,225	61	>75% Gras	s cover, Go	ood, HSG B				
	2,465	80	Weighted Average						
	1,225		49.70% Pei	rvious Area	a				
	1,240		50.30% Imp	pervious Ar	rea				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description				
6.0	(.501)	(10,10)	(12000)	(0.0)	Direct Entry,				

Summary for Subcatchment C46: CB #46

Runoff = 0.17 cfs @ 12.10 hrs, Volume= 533 cf, Depth> 1.45"

Routed to Pond CB46: CB #46

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description						
	2,241	98	Paved parking, HSG B						
	2,156	61	>75% Grass cover, Good, HSG B						
	4,397	80	Weighted Average						
	2,156		49.03% Pei	rvious Area	a				
	2,241		50.97% lmp	pervious Ar	rea				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	·				
6.0	()	(1411)	()	()	Direct Entry,				

Summary for Subcatchment C47: CB #47

Runoff = 0.21 cfs @ 12.09 hrs, Volume= 762 cf, Depth> 3.04"

Routed to Pond CB47: CB#47

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 37

A	rea (sf)	CN E	Description						
	3,012	98 F	Paved roads w/curbs & sewers, HSG B						
	3,012	1	100.00% Impervious Area						
To	Longth	Slope	Velocity	Canacity	Description				
(min)	Length (feet)	(ft/ft)	(ft/sec)	(cfs)	Description				
6.0	•	•	•	, ,	Direct Entry,				

Summary for Subcatchment C48: CB #48

Runoff 1.03 cfs @ 12.19 hrs, Volume= 4,342 cf, Depth> 0.87"

Routed to Pond CB48: CB#48

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

	Area (sf)	CN D	escription		
	3,877	98 P	aved road	s w/curbs &	R sewers, HSG B
	56,251	68 1	acre lots,	20% imp, I	HSG B
	60,128	70 V	Veighted A	verage	
	45,001	7	4.84% Per	vious Area	
	15,127	2	5.16% Imp	ervious Ar	ea
To	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.0	50	0.0800	0.12		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.27"
4.8	350	0.0600	1.22		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
11.8	400	Total	•		

Summary for Subcatchment C49: CB #49

0.34 cfs @ 12.09 hrs, Volume= 1,139 cf, Depth> 2.61" Runoff

Routed to Pond CB49: CB #49

Area (sf)	CN	Description			
4,431	98	Paved roads w/curbs & sewers, HSG C			
807	74	>75% Grass cover, Good, HSG C			
5,238	94	Weighted Average			
807		15.41% Pervious Area			
001		10. 1 1701 CIVIOUS / (ICU			

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 38

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment C50: CB #50

0.96 cfs @ 12.09 hrs, Volume=

3,147 cf, Depth> 2.51"

Routed to Pond CB50: CB #50

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Ar	ea (sf)	CN	Description						
	3,429	74	>75% Grass cover, Good, HSG C						
	11,611	98	Paved road	s w/curbs &	& sewers, HSG C				
•	15,040	93	Weighted Average						
	3,429 22.80% Pervious Area								
•	11,611		77.20% lmp	ervious Ar	rea				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment C51: CB #51

0.49 cfs @ 12.09 hrs, Volume= 1,726 cf, Depth> 3.04" Runoff

Routed to Pond CB51: CB #51

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

	Aı	rea (sf)	CN	Description	Description					
-		3,147	98	Roofs, HSG	Roofs, HSG C					
		3,676	98	Paved parking, HSG C						
-		6,823	98	Weighted A	Weighted Average					
		6,823		100.00% Impervious Area						
	Тс	Length	Slope	,	Capacity	Description				
	(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
	6.0					Direct Entry,				

Summary for Subcatchment C52: CB#52

2,045 cf, Depth> 2.71" Runoff 0.61 cfs @ 12.09 hrs, Volume=

Routed to Pond CB52: CB #52

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 39

A	rea (sf)	CN	Description								
	1,164	74	74 >75% Grass cover, Good, HSG C								
	7,888	98	Paved park	ing, HSG C							
	9,052	95	95 Weighted Average								
	1,164		12.86% Pei	rvious Area							
	7,888		87.14% Impervious Area								
_											
Tc	Length	Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
6.0					Direct Entry						

Summary for Subcatchment C53: CB #53

Runoff = 0.50 cfs @ 12.09 hrs, Volume= 1,646 cf, Depth> 2.51"

Routed to Pond CB53: CB #53

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Aı	rea (sf)	CN	Description						
	287	39	>75% Gras	s cover, Go	Good, HSG A				
	3,287	98	Paved park	ing, HSG A	A				
	773	74	>75% Gras	s cover, Go	Good, HSG C				
	3,516	98	Paved park	ing, HSG C	C				
	7,863	93	93 Weighted Average						
	1,060		13.48% Pervious Area						
	6,803	33 86.52% Impervious Area							
Tc	Length	Slope		Capacity	/ Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment C54: CB #54

Runoff = 0.29 cfs @ 12.09 hrs, Volume= 933 cf, Depth> 2.32"

Routed to Pond CB54 : CB #54

_	Area (sf)	CN	Description
	550	39	>75% Grass cover, Good, HSG A
	4,176	98	Paved parking, HSG A
	84	74	>75% Grass cover, Good, HSG C
_	11	98	Paved parking, HSG C
	4,821	91	Weighted Average
	634		13.15% Pervious Area
	4,187		86.85% Impervious Area

6.0

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 40

Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•

Direct Entry,

Summary for Subcatchment C7: CB #5

Runoff = 0.33 cfs @ 12.09 hrs, Volume= 1,176 cf, Depth> 3.04"

Routed to Pond CB7: CB#5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN E	Description							
	4,650	98 F	98 Paved roads w/curbs & sewers, HSG B							
	4,650	1	00.00% In	npervious A	rea					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
6.0					Direct Entry,					

Summary for Subcatchment C8: CB #8

Runoff = 0.36 cfs @ 12.09 hrs, Volume= 1,185 cf, Depth> 2.61"

Routed to Pond CB8: CB#8

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description				
	4,837	98	Paved road	s w/curbs &	& sewers, HSG B		
	613	61	>75% Gras	s cover, Go	lood, HSG B		
	5,450	94	Weighted A	verage			
	613		11.25% Pervious Area				
	4,837		88.75% lmp	pervious Ar	rea		
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	·		
6.0					Direct Entry,		

Summary for Subcatchment C9: CB #9

Runoff = 1.14 cfs @ 12.09 hrs, Volume= 3,974 cf, Depth> 2.92"

Routed to Pond CB9: CB #9

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

<u>Page 41</u>

A	rea (sf)	CN	Description		
	31	61	>75% Gras	s cover, Go	ood, HSG B
	433	98	Paved road	ls w/curbs &	& sewers, HSG B
	904	74	>75% Gras	s cover, Go	ood, HSG C
	12,073	98	Paved park	ing, HSG C	
	2,305	98	Roofs, HSC	3 Č	
	52	80	>75% Gras	s cover, Go	ood, HSG D
	509	98	Paved park	ing, HSG D	
	16,307	97	Weighted A	verage	
	987		6.05% Perv	ious Area	
	15,320		93.95% Imp	pervious Ar	ea
			-		
Tc	Length	Slop	e Velocity	Capacity	Description
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment CH1: CLUBHOUSE

Runoff = 0.43 cfs @ 12.09 hrs, Volume= 1,470 cf, Depth> 2.82"

Routed to Pond DECH: DRIP #CH

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description						
	5,802	98	Roofs, HSC	G C					
	3	98	Roofs, HSG	G D					
	435	74	>75% Gras	s cover, Go	lood, HSG C				
	22	80	>75% Gras	s cover, Go	lood, HSG D				
	6,262	96	96 Weighted Average						
	457		7.30% Pervious Area						
	5,805	92.70% Impervious Area							
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment MB1: MAIL KIOSK

Runoff = 0.07 cfs @ 12.09 hrs, Volume= 237 cf, Depth> 3.04"

Routed to Link AP2: ANALYSIS POINT 2

 Area (sf)	CN	Description
938	98	Roofs, HSG B
938		100.00% Impervious Area

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 42

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment S201: SUMMER STREET ACCESS APRON

Runoff = 0.67 cfs @ 12.09 hrs, Volume= 2,247 cf, Depth> 2.71"

Routed to Link AP1: ANALYSIS POINT 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description							
	717	61	>75% Gras	75% Grass cover, Good, HSG B						
	9,226	98	Paved park	Paved parking, HSG B						
	9,943	95	Weighted A	Veighted Average						
	717		7.21% Pervious Area							
	9,226	!	92.79% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description					
6.0					Direct Entry,					

Summary for Subcatchment S202: EXISTING WETLAND

Runoff = 8.51 cfs @ 12.34 hrs, Volume= 43,021 cf, Depth> 1.19"

Routed to Reach SC1: Stream Crossing #1

Area (sf)	CN	Description
136,496	61	>75% Grass cover, Good, HSG B
83,935	55	Woods, Good, HSG B
29	98	Paved parking, HSG B
13,946	98	Roofs, HSG B
9,038	48	Brush, Good, HSG B
2,573	74	>75% Grass cover, Good, HSG C
17,121	70	Woods, Good, HSG C
98	98	Paved parking, HSG C
1,097	65	Brush, Good, HSG C
126	80	>75% Grass cover, Good, HSG D
132	98	Paved parking, HSG D
167,678	98	Water Surface, HSG D
432,269	76	Weighted Average
250,386		57.92% Pervious Area
181,883		42.08% Impervious Area

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

<u>Page 43</u>

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	50	0.0200	0.15		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.27"
1.4	118	0.0400	1.40		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
16.2	688	0.0200	0.71		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
23.2	856	Total			

Summary for Subcatchment S203: POCKET WETLAND #1

Runoff = 0.61 cfs @ 12.10 hrs, Volume=

2,073 cf, Depth> 0.97"

Routed to Pond p210: POCKET WETLAND #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Area (sf)	CN	Description					
12,682	61	>75% Grass cover, Good, HSG B					
1,060	98	Water Surface, 0% imp, HSG B					
7,785	74	>75% Grass cover, Good, HSG C					
4,060	98	Water Surface, 0% imp, HSG C					
25,587	72	Weighted Average					
25,587		100.00% Pervious Area					
Tc Length	Slop						
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)					
6.0		Direct Entry,					

Summary for Subcatchment S204: EXISTING WETLANDS

Runoff = 5.78 cfs @ 12.34 hrs, Volume= 29,158 cf, Depth> 1.14" Routed to Link ap2 : ANALYSIS POINT 2

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 44

A	rea (sf)	CN E	Description						
	53,739	61 >	61 >75% Grass cover, Good, HSG B						
	17,975	55 V	Voods, Go	od, HSG B					
	20,940	48 E	Brush, Goo	d, HSG B					
	41,421	74 >	75% Gras	s cover, Go	ood, HSG C				
	68,342		,	od, HSG C					
	116			ing, HSG C					
	1,904		Brush, Goo	•					
	1,528		Brush, Goo						
	2,508		,	od, HSG D					
	161			ing, HSG D)				
	4,073		Brush, Goo	•					
	95,496	98 V	Vater Surfa	ice, HSG D)				
3	08,203	75 V	5 5						
	12,430			vious Area					
	95,773	31.07% Impervious Are			ea				
Tc	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
3.2	50	0.2000	0.26		Sheet Flow,				
					Grass: Dense n= 0.240 P2= 3.27"				
19.4	582	0.0100	0.50		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
22.6	632	Total							

Summary for Subcatchment S205: ISOLATED WETLAND

noff = 1.15 cfs @ 12.10 hrs, Volume= Routed to Link AP3 : ANALYSIS POINT 3 Runoff

4,009 cf, Depth> 0.87"

Area (sf)	CN	Description
10,910	30	Woods, Good, HSG A
3,684	74	>75% Grass cover, Good, HSG C
2,275	70	Woods, Good, HSG C
171	98	Paved parking, HSG C
1,706	65	Brush, Good, HSG C
1,940	80	>75% Grass cover, Good, HSG D
23,513	77	Woods, Good, HSG D
393	98	Paved parking, HSG D
2,208	73	Brush, Good, HSG D
8,620	98	Water Surface, HSG D
55,420	70	Weighted Average
46,236		83.43% Pervious Area
9,184		16.57% Impervious Area

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 45

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment S206: OVERLAND FLOW

6.52 cfs @ 12.58 hrs, Volume=

46,602 cf, Depth> 0.63"

Routed to Link AP4: ANALYSIS POINT #4

A	rea (sf)	CN E	Description						
	16,514								
	18,226			od, HSG A					
	713		,	ing, HSG A					
	41,148		Brush, Goo						
	17,568			20% imp, H	HSG A				
	37,410				ood, HSG B				
	13,900	55 V	Voods, Go	od, HSG B					
	54,538	48 E	Brush, Goo	d, HSG B					
	91,202	68 1	acre lots,	20% imp, I	HSG B				
	77,444			,	ood, HSG C				
1	14,763		,	od, HSG C					
	3,493			ing, HSG C					
	57,740		Brush, Goo						
	5,763			i% Grass cover, Good, HSG D					
	26,141		Woods, Good, HSG D Water Surface, 0% imp, HSG D						
	14,732				p, HSG D				
	91,295		Veighted A						
	65,335	_		rvious Area					
	25,960	2	91% impe	ervious Are	a				
Tc	Length	Slope		Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
8.5	50	0.0500	0.10		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.27"				
5.0	334	0.0250	1.11		Shallow Concentrated Flow,				
					Short Grass Pasture Kv= 7.0 fps				
1.9	91	0.0250	0.79		Shallow Concentrated Flow,				
40.0	404	0.0400			Woodland Kv= 5.0 fps				
10.2	491	0.0400	0.80		Shallow Concentrated Flow, BRUSH				
8.9	501	0.0350	0.94		Kv= 4.0 fps Shallow Concentrated Flow,				
0.9	301	0.0330	0.54		Woodland Kv= 5.0 fps				
34.5	1 167	Total			vvoodiana itv- 0.0 ips				
34.3	1,467	Total							

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 46

Summary for Subcatchment S207: INFILTRATION POND #2

Runoff = 1.12 cfs @ 12.09 hrs, Volume=

3,568 cf, Depth> 2.06"

Routed to Pond P207: INFILTRATION POND #2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

	Area (sf)	CN	Description					
	839	98	Water Surfa	ace, 0% im	p, HSG A			
	8,802	74	>75% Gras	s cover, G	ood, HSG C			
	11,162	98	Water Surfa	ace, 0% im	p, HSG C			
	20,803 88 Weighted Average							
	20,803 100.00% Pervious Area				a			
_		01			5			
	C Length	Slop	,	Capacity	Description			
(mi	n) (feet)	(ft/f	t) (ft/sec)	(cfs)				
6	.0				Direct Entry,			

Summary for Subcatchment S208: GRASS AREA

Runoff = 0.40 cfs @ 12.10 hrs, Volume=

1,308 cf, Depth> 1.14"

Routed to Pond OCS4: OCS#4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Area	ı (sf)	CN	Description						
	476	39	>75% Gras	s cover, Go	Good, HSG A				
12	,000	74	>75% Gras	s cover, Go	Good, HSG C				
	168	98	Paved park	ing, HSG A	A				
1	,116	98	Paved park	ing, HSG C	C				
13	,760	75	Weighted Average						
12	,476	9	90.67% Per	vious Area	a				
1	,284	,	9.33% Impe	ervious Area	ea				
	ength	Slope	Velocity	Capacity	/ Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment S209: WETLAND C

Runoff = 1.60 cfs @ 12.44 hrs, Volume= Routed to Reach 11R : 4x4 Open Bottom Culvert 9,101 cf, Depth> 1.02"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 47

A	rea (sf)	CN	Description						
	10,826	39	>75% Grass cover, Good, HSG A						
	16,826	30	Noods, Go	od, HSG A					
	8,863	74	>75% Gras	s cover, Go	ood, HSG C				
	26,084	70	Noods, Go	od, HSG C					
	44,067	98	Nater Surfa	ace, 0% imp	p, HSG D				
	304			ing, HSG A					
	103	98	Paved park	ing, HSG C					
•	107,073	73	Neighted A	verage					
•	106,666	,	99.62% Pe	rvious Area					
	407	(0.38% Impe	ervious Are	a				
Tc	Length	Slope	•	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
13.7	50	0.0150	0.06		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.27"				
15.2	557	0.0150	0.61		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
28.9	607	Total							

Summary for Subcatchment S210: INFILTRATION POND #1

Runoff = 1.92 cfs @ 12.24 hrs, Volume=

8,348 cf, Depth> 1.32"

Routed to Pond P212: INFILTRATION POND #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN E	CN Description							
	13,844	98 V	Water Surface, 0% imp, HSG C							
	59,814	74 >	75% Gras	s cover, Go	ood, HSG C					
	2,232	65 E	Brush, Goo	d, HSG C						
	75,890	78 V	Veighted A	verage						
	75,890	1	00.00% Pe	ervious Are	a					
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.2	50	0.0150	0.13		Sheet Flow,					
					Grass: Short n= 0.150 P2= 3.27"					
10.3	530	0.0150	0.86		Shallow Concentrated Flow,					
					Short Grass Pasture Kv= 7.0 fps					
16.5	580	Total								

Summary for Subcatchment S211: S211

Runoff = 0.56 cfs @ 12.10 hrs, Volume= Routed to Pond P205 : INFILTRATION POND #3 1,786 cf, Depth> 1.39"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 48

A	rea (sf)	CN	Description							
	8,108	61	>75% Grass cover, Good, HSG B							
	7,328	98	Water Surfa	ace, HSG E	B					
	15,436	79	Weighted Average							
	8,108		52.53% Pervious Area							
	7,328		47.47% lmp	pervious Ar	rea					
_										
Tc	Length	Slope	,	Capacity	Description					
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment S212: SWALE

Runoff = 0.46 cfs @ 12.40 hrs, Volume= 2,770 cf, Depth> 0.63"

Routed to Reach SC2 : Stream Crossing #2

	Α	rea (sf)	CN I	CN Description								
		4,100	61 :	61 >75% Grass cover, Good, HSG B								
		7,192	55 \	Woods, Good, HSG B								
		1,180	74	>75% Gras	s cover, Go	ood, HSG C						
		3,436	70 \	Woods, Go	od, HSG C							
		13,180	98 \	Nater Surfa	ace, 0% im	p, HSG D						
		72	98 I	Paved park	ing, HSG E	3						
		22,663	48 I	Brush, Goo	d, HSG B							
		545		Brush, Goo	•							
		107		•	ing, HSG C							
		135			ing, HSG [
_		158	80 :	>75% Gras	s cover, Go	ood, HSG D						
		52,768	65 \	Neighted A	verage							
		52,454			rvious Area							
		314	(0.60% Impe	ervious Are	a						
	Тс	Length	Slope	•		Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	16.1	50	0.0400	0.05		Sheet Flow,						
						Woods: Dense underbrush n= 0.800 P2= 3.27"						
	3.9	232	0.0600	0.98		Shallow Concentrated Flow, BRUSH						
						Kv= 4.0 fps						
	3.1	136	0.0220	0.74		Shallow Concentrated Flow,						
_						Woodland Kv= 5.0 fps						
	23.1	418	Total									

Printed 6/19/2023

Page 49

Summary for Subcatchment S213: COURTYARD

Runoff = 0.78 cfs @ 12.10 hrs, Volume= 2,476 cf, Depth> 1.39"

Routed to Pond 11P: YARD DRAIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Aı	rea (sf)	CN	Description							
	5,047	39	>75% Grass cover, Good, HSG A							
	1,678	98	Paved parking, HSG A							
	168	98	Roofs, HSG A							
	532	98	Water Surface, 0% imp, HSG A							
	4,518	74	>75% Grass cover, Good, HSG C							
	7,080	98	Paved parking, HSG C							
	878	98	Roofs, HSG C							
	718	98	Water Surface, 0% imp, HSG C							
	296	80	>75% Grass cover, Good, HSG D							
	492	98	Paved parking, HSG D							
	21,407	79	Weighted Average							
	11,111		51.90% Pervious Area							
	10,296		48.10% Impervious Area							
_		01	VI " 0 " D ' "							
Tc	Length	Slop								
(min)	(feet)	(ft/f								
6.0			Direct Entry,							

Summary for Subcatchment T1: Trench Drain 1

Runoff = 0.71 cfs @ 12.09 hrs, Volume= 2,338 cf, Depth> 2.51"

Routed to Pond 5R: TRENCH DRAIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Area (sf)	CN	Description								
1,443	74	>75% Grass cover, Good, HSG C								
4,228	98	Paved parking, HSG C								
1,339	80	>75% Grass cover, Good, HSG D								
4,163	98	Paved parking, HSG D								
11,173	93	Weighted Average								
2,782		24.90% Pervious Area								
8,391		75.10% Impervious Area								
Tc Length	Slo	pe Velocity Capacity Description								
(min) (feet)	(ft/	(ft) (ft/sec) (cfs)) (ft/sec) (cfs)							
6.0		Direct Entry								

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 50

Summary for Subcatchment T2: Drive Under B2

Runoff = 0.15 cfs @ 12.10 hrs, Volume=

490 cf, Depth> 1.32"

Routed to Reach 11R: 4x4 Open Bottom Culvert

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description							
	1,510	39	>75% Grass	cover, Go	Good, HSG A					
	2,313	98	Paved parki	ng, HSG A	A					
	77	74	>75% Grass	cover, Go	Good, HSG C					
	545	98	Paved parki	ng, HSG C	C					
•	4,445	78	Weighted Average							
	1,587		35.70% Per	vious Area	a					
	2,858		64.30% Imp	ervious Ar	ırea					
Тс	Length	Slop	e Velocity	Capacity	Description					
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment TH1: TOWN HOUSE #1

Runoff = 0.29 cfs @ 12.09 hrs, Volume=

997 cf, Depth> 2.82"

Routed to Pond DE61: DRIP #61

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	<u>Description</u>							
	3,936	98	Roofs, HSG C							
	311	74	>75% Grass cover, Good, HSG C							
	4,247	96	Weighted Average							
	311		7.32% Pervious Area							
	3,936		92.68% lmp	pervious Ar	rea					
Tc	Length	Slope	,	Capacity	Description					
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment TH10: TOWN HOUSE #10

Runoff = 0.24 cfs @ 12.09 hrs, Volume= 816 cf, Depth> 2.82"

Routed to Pond DE70: DRIP #70

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 51

Α	rea (sf)	CN	Description								
	3,184	98	Roofs, HSG C								
	292	74	>75% Gras	s cover, Go	od, HSG C						
	3,476	96	Weighted Average								
	292		8.40% Pervious Area								
	3,184	!	91.60% Imp	pervious Ar	ea						
_		٠.									
Tc	Length	Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
6.0					Direct Entry						

Direct Entry,

Summary for Subcatchment TH11: TOWN HOUSE #11

Runoff = 0.29 cfs @ 12.09 hrs, Volume=

988 cf, Depth> 2.82"

Routed to Pond DE71: DRIP #71

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description						
	3,899	98	Roofs, HSG C						
	311	74	>75% Gras	s cover, Go	Good, HSG C				
	4,210	96	Weighted Average						
	311		7.39% Perv	vious Area					
	3,899		92.61% lm	pervious Ar	rea				
_									
Tc	Length	Slope	,	Capacity	·				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment TH2: TOWN HOUSE #2

Runoff = 0.29 cfs @ 12.09 hrs, Volume=

997 cf, Depth> 2.82"

Routed to Pond DE62: DRIP #62

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description							
	3,936	98	Roofs, HSG C							
	311	74	>75% Grass cover, Good, HSG C							
	4,247	96	Weighted A	verage						
	311		7.32% Pervious Area							
	3,936		92.68% lmp	pervious Ar	ea					
Tc	Length	Slope	,	Capacity	Description					
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)						

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 52

Summary for Subcatchment TH3: TOWN HOUSE #3

Runoff = 0.20 cfs @ 12.09 hrs, Volume= 681 cf, Depth> 2.71"

Routed to Pond DE63: DRIP #63

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description							
	2,672	98	Roofs, HSG C							
	341	74	>75% Gras	s cover, Go	Good, HSG C					
	3,013	95	Weighted Average							
	341		11.32% Pei	rvious Area	a					
	2,672		88.68% lmp	pervious Ar	rea					
Tc	Length	Slope	•	Capacity	•					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment TH4: TOWN HOUSE #4

Runoff = 0.24 cfs @ 12.09 hrs, Volume= 814 cf, Depth> 2.82"

Routed to Pond DE64: DRIP #64

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description								
	3,178	98	Roofs, HSG C								
	292	74	>75% Grass cover, Good, HSG C								
	3,470	96	Weighted Average								
	292		8.41% Pervious Area								
	3,178	!	91.59% Imp	pervious Ar	rea						
_		0.1			5						
Tc	Length	Slope	,	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
6.0					Direct Entry,						

Summary for Subcatchment TH5: TOWN HOUSE #5

Runoff = 0.20 cfs @ 12.09 hrs, Volume= 682 cf, Depth> 2.71"

Routed to Pond DE65: DRIP #65

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 53

Α	rea (sf)	CN	Description					
	2,675	98	Roofs, HSG C					
	341	74	>75% Grass cover, Good, HSG C					
	3,016	95	95 Weighted Average					
	341		11.31% Pervious Area					
	2,675	:	88.69% Impervious Area					
_		01		0 "	D			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry			

Direct Entry,

Summary for Subcatchment TH6: TOWN HOUSE #6

Runoff = 0.23 cfs @ 12.09 hrs, Volume=

800 cf, Depth> 2.82"

Routed to Pond DE66: DRIP #66

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

Aı	rea (sf)	CN	Description						
	3,116	98	Roofs, HSG C						
	291	74	>75% Gras	75% Grass cover, Good, HSG C					
	3,407	96	Weighted Average						
	291		8.54% Perv	ious Area					
	3,116		91.46% lmp	pervious Ar	rea				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	,	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment TH7: TOWN HOUSE #7

Runoff = 0.24 cfs @ 12.09 hrs, Volume= 817 cf, Depth> 2.82"

Routed to Pond DE67 : DRIP #67

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description					
	3,189	98	Roofs, HSG C					
	292	74	>75% Gras	75% Grass cover, Good, HSG C				
	3,481 292		96 Weighted Average 8.39% Pervious Area					
	3,189		91.61% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description			

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 54

Summary for Subcatchment TH8: TOWN HOUSE #8

Runoff 0.29 cfs @ 12.09 hrs, Volume= 989 cf, Depth> 2.82"

Routed to Pond DE68: DRIP #68

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description				
	3,901	98	Roofs, HSG C				
	311	74	>75% Grass cover, Good, HSG C				
	4,212	1,212 96 Weighted Average					
	311		7.38% Perv	ious Area			
	3,901	!	92.62% lmp	pervious Ar	rea		
_				_			
Tc	Length	Slope	,	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment TH9: TOWN HOUSE #9

817 cf, Depth> 2.82" 0.24 cfs @ 12.09 hrs, Volume=

Routed to Pond DE69: DRIP #69

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

A	rea (sf)	CN	Description					
	3,188	98	Roofs, HSG C					
	292	74	>75% Gras	>75% Grass cover, Good, HSG C				
	3,480	96	Weighted Average					
	292		8.39% Perv	ious Area				
	3,188		91.61% Impervious Area					
Тс	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft	,	(cfs)	Description			
6.0		•			Direct Entry,			

Direct Entry,

Summary for Reach 8R: OVERLAND FLOW

11,975 sf, 92.37% Impervious, Inflow Depth > 1.94" for 2YR event Inflow Area =

0.69 cfs @ 12.14 hrs, Volume= 1,940 cf Inflow

0.11 cfs @ 12.77 hrs, Volume= 1,734 cf, Atten= 84%, Lag= 37.7 min Outflow

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.06 fps, Min. Travel Time= 156.1 min

Avg. Velocity = 0.04 fps, Avg. Travel Time= 252.3 min

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 55

Peak Storage= 1,031 cf @ 12.77 hrs

Average Depth at Peak Storage= 0.04', Surface Width= 50.37' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 28.09 cfs

50.00' x 1.00' deep channel, n= 0.400 Side Slope Z-value= 5.0 '/' Top Width= 60.00' Length= 563.0' Slope= 0.0213 '/'

Inlet Invert= 208.00', Outlet Invert= 196.00'



Summary for Reach 9R: OVERLAND FLOW

Inflow Area = 32,665 sf, 94.81% Impervious, Inflow Depth = 0.00" for 2YR event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

Average Depth at Peak Storage= 0.00'

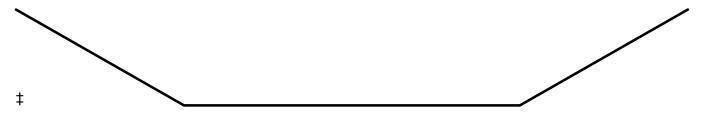
Bank-Full Depth= 1.00' Flow Area= 30.0 sf. Capacity= 23.45 cfs

20.00' x 1.00' deep channel, n= 0.400

Side Slope Z-value= 10.0 '/' Top Width= 40.00'

Length= 211.0' Slope= 0.0652 '/'

Inlet Invert= 201.75', Outlet Invert= 188.00'



Summary for Reach 10R: OVERLAND FLOW

Inflow Area = 129,716 sf, 63.13% Impervious, Inflow Depth = 0.01" for 2YR event

Inflow = 0.04 cfs @ 12.71 hrs, Volume= 101 cf

Outflow = 0.02 cfs @ 13.19 hrs, Volume= 101 cf, Atten= 53%, Lag= 29.2 min

Routed to Link AP4: ANALYSIS POINT #4

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 56

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Max. Velocity= 0.05 fps, Min. Travel Time= 54.7 min

Avg. Velocity = 0.03 fps, Avg. Travel Time= 80.0 min

Peak Storage= 60 cf @ 13.19 hrs

Average Depth at Peak Storage= 0.02', Surface Width= 20.36'

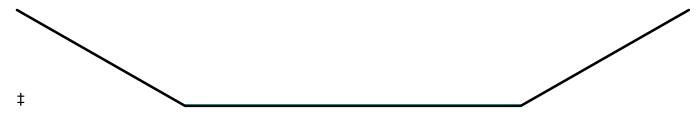
Bank-Full Depth= 1.00' Flow Area= 30.0 sf, Capacity= 17.57 cfs

20.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 40.00'

Length= 164.0' Slope= 0.0366 '/'

Inlet Invert= 192.00', Outlet Invert= 186.00'



Summary for Reach 11R: 4x4 Open Bottom Culvert

[52] Hint: Inlet/Outlet conditions not evaluated

424,818 sf, 45.99% Impervious, Inflow Depth > 0.42" for 2YR event Inflow Area =

Inflow 1.70 cfs @ 12.45 hrs, Volume= 14,965 cf

Outflow 1.70 cfs @ 12.45 hrs, Volume= 14,959 cf, Atten= 0%, Lag= 0.3 min

Routed to Reach 23R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 1.19 fps, Min. Travel Time= 0.4 min

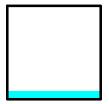
Avg. Velocity = 0.55 fps, Avg. Travel Time= 0.9 min

Peak Storage= 43 cf @ 12.45 hrs

Average Depth at Peak Storage= 0.36', Surface Width= 4.00' Bank-Full Depth= 4.00' Flow Area= 16.0 sf. Capacity= 42.20 cfs

48.0" W x 48.0" H Box Pipe n= 0.069 Riprap, 6-inch Length= 30.0' Slope= 0.0150 '/'

Inlet Invert= 194.00', Outlet Invert= 193.55'



Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 57

Summary for Reach 12R: OVERLAND FLOW

Inflow Area = 12,906 sf, 90.20% Impervious, Inflow Depth > 1.98" for 2YR event

Inflow = 0.77 cfs @ 12.13 hrs, Volume= 2,134 cf

Outflow = 0.30 cfs @ 12.43 hrs, Volume= 2,085 cf, Atten= 61%, Lag= 18.0 min

Routed to Link AP2: ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.09 fps, Min. Travel Time= 45.1 min

Avg. Velocity = 0.04 fps, Avg. Travel Time= 107.5 min

Peak Storage= 811 cf @ 12.43 hrs

Average Depth at Peak Storage= 0.06', Surface Width= 50.64' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 29.80 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value = 5.0 '/' Top Width = 60.00'

Length= 250.0' Slope= 0.0240 '/'

Inlet Invert= 202.00', Outlet Invert= 196.00'

‡

Summary for Reach 14R: OVERLAND FLOW

Inflow Area = 52,768 sf, 0.60% Impervious, Inflow Depth > 0.63" for 2YR event

Inflow = 0.46 cfs @ 12.41 hrs, Volume= 2,770 cf

Outflow = 0.07 cfs @ 14.79 hrs, Volume= 2,124 cf, Atten= 84%, Lag= 142.6 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.05 fps, Min. Travel Time= 266.3 min

Avg. Velocity = 0.04 fps, Avg. Travel Time= 316.0 min

Peak Storage= 1,163 cf @ 14.79 hrs

Average Depth at Peak Storage= 0.03', Surface Width= 50.54'

Bank-Full Depth= 1.00' Flow Area= 60.0 sf, Capacity= 31.55 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 70.00'

Length= 852.0' Slope= 0.0246 '/'

Inlet Invert= 207.00', Outlet Invert= 186.00'

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 58



Summary for Reach 15R: OVERLAND FLOW

Inflow Area = 62,582 sf, 52.00% Impervious, Inflow Depth > 0.91" for 2YR event

Inflow = 0.12 cfs @ 15.47 hrs, Volume= 4,736 cf

Outflow = 0.12 cfs @ 17.02 hrs, Volume= 4,228 cf, Atten= 2%, Lag= 92.9 min

Routed to Link AP2: ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.06 fps, Min. Travel Time= 82.1 min

Avg. Velocity = 0.06 fps, Avg. Travel Time= 90.4 min

Peak Storage= 595 cf @ 17.02 hrs

Average Depth at Peak Storage= 0.04', Surface Width= 50.40' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 27.21 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value = 5.0 '/' Top Width = 60.00'

Length= 300.0' Slope= 0.0200 '/'

Inlet Invert= 202.00', Outlet Invert= 196.00'

‡

Summary for Reach 18R: OVERLAND FLOW

Inflow Area = 88,676 sf, 39.42% Impervious, Inflow Depth = 0.00" for 2YR event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 1.00' Flow Area= 75.0 sf, Capacity= 38.42 cfs

19097 Post-Development

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

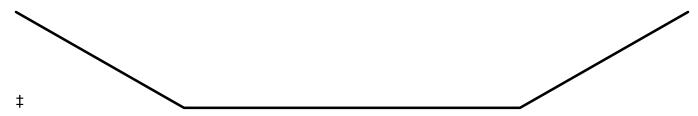
Page 59

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 100.00'

Length= 609.0' Slope= 0.0279 '/'

Inlet Invert= 203.00', Outlet Invert= 186.00'



Summary for Reach 20R: OVERLAND FLOW

Inflow Area = 72,222 sf, 68.72% Impervious, Inflow Depth = 0.96" for 2YR event

Inflow = 0.93 cfs @ 12.49 hrs, Volume= 5,781 cf

Outflow = 0.34 cfs @ 13.75 hrs, Volume= 5,373 cf, Atten= 64%, Lag= 75.2 min

Routed to Reach 11R: 4x4 Open Bottom Culvert

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.07 fps, Min. Travel Time= 128.2 min

Avg. Velocity = 0.04 fps, Avg. Travel Time= 210.1 min

Peak Storage= 2,612 cf @ 13.75 hrs

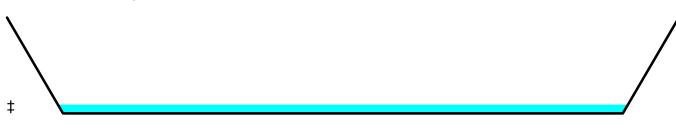
Average Depth at Peak Storage= 0.09', Surface Width= 50.92' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 18.54 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'

Length= 560.0' Slope= 0.0093 '/'

Inlet Invert= 200.00', Outlet Invert= 194.80'



Summary for Reach 23R: OVERLAND FLOW

Inflow Area = 424,818 sf, 45.99% Impervious, Inflow Depth > 0.42" for 2YR event

Inflow = 1.70 cfs @ 12.45 hrs, Volume= 14,959 cf

Outflow = 1.24 cfs @ 12.76 hrs, Volume= 14,613 cf, Atten= 27%, Lag= 18.2 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.15 fps, Min. Travel Time= 26.3 min

Avg. Velocity = 0.08 fps, Avg. Travel Time= 52.5 min

19097 Post-Development

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 60

Peak Storage= 1,960 cf @ 12.76 hrs

Average Depth at Peak Storage= 0.16', Surface Width= 56.23' Bank-Full Depth= 1.00' Flow Area= 70.0 sf, Capacity= 31.93 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 20.0 '/' Top Width= 90.00'

Length= 237.0' Slope= 0.0211 '/'

Inlet Invert= 193.00', Outlet Invert= 188.00'



Summary for Reach R202: OVERLAND FLOW

[62] Hint: Exceeded Reach SC1 OUTLET depth by 0.11' @ 13.30 hrs

Inflow Area = 432,269 sf, 42.08% Impervious, Inflow Depth > 1.19" for 2YR event

Inflow = 8.50 cfs @ 12.35 hrs, Volume= 43,012 cf

Outflow = 2.85 cfs @ 12.90 hrs, Volume= 39,436 cf, Atten= 66%, Lag= 33.4 min

Routed to Link AP2: ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.13 fps, Min. Travel Time= 89.4 min Avg. Velocity = 0.07 fps, Avg. Travel Time= 163.7 min

Peak Storage= 15,282 cf @ 12.90 hrs

Average Depth at Peak Storage= 0.21', Surface Width= 110.38' Bank-Full Depth= 1.00' Flow Area= 125.0 sf, Capacity= 42.56 cfs

100.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 150.00'

Length= 700.0' Slope= 0.0107 '/'

Inlet Invert= 205.50', Outlet Invert= 198.00'



Summary for Reach R211: OVERLAND FLOW

Inflow Area = 241,078 sf, 59.10% Impervious, Inflow Depth = 0.00" for 2YR event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routed to Reach 11R: 4x4 Open Bottom Culvert

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 61

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min

Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 1.00' Flow Area= 50.0 sf, Capacity= 14.51 cfs

35.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 15.0 '/' Top Width= 65.00'

Length= 600.0' Slope= 0.0087 '/'

Inlet Invert= 200.00', Outlet Invert= 194.80'



Summary for Reach SC1: Stream Crossing #1

[52] Hint: Inlet/Outlet conditions not evaluated

Inflow Area = 432,269 sf, 42.08% Impervious, Inflow Depth > 1.19" for 2YR event

Inflow 8.51 cfs @ 12.34 hrs, Volume= 43,021 cf

43,012 cf, Atten= 0%, Lag= 0.1 min Outflow 8.50 cfs @ 12.35 hrs, Volume=

Routed to Reach R202: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 2.48 fps, Min. Travel Time= 0.3 min

Avg. Velocity = 1.06 fps, Avg. Travel Time= 0.7 min

Peak Storage= 148 cf @ 12.35 hrs

Average Depth at Peak Storage= 0.21', Surface Width= 16.00'

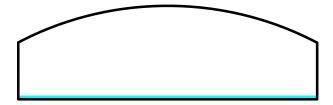
Bank-Full Depth= 5.00' Flow Area= 69.8 sf, Capacity= 722.91 cfs

192.0" W x 60.0" H, R=207.0" Arch Pipe

n= 0.030 Stream, clean & straight

Length= 43.1' Slope= 0.0200 '/'

Inlet Invert= 206.37', Outlet Invert= 205.51'



Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 62

Summary for Reach SC2: Stream Crossing #2

[52] Hint: Inlet/Outlet conditions not evaluated

Inflow Area = 52,768 sf, 0.60% Impervious, Inflow Depth > 0.63" for 2YR event

Inflow = 0.46 cfs @ 12.40 hrs, Volume= 2,770 cf

Outflow = 0.46 cfs @ 12.41 hrs, Volume= 2,770 cf, Atten= 0%, Lag= 0.6 min

Routed to Reach 14R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 1.04 fps, Min. Travel Time= 0.6 min Avg. Velocity = 1.04 fps, Avg. Travel Time= 0.6 min

Peak Storage= 16 cf @ 12.41 hrs

Average Depth at Peak Storage= 0.03', Surface Width= 16.00' Bank-Full Depth= 5.00' Flow Area= 68.1 sf, Capacity= 768.96 cfs

192.0" W x 60.0" H, R=180.0" Arch Pipe n= 0.030 Stream, clean & straight Length= 36.5' Slope= 0.0241 '/'

Inlet Invert= 208.52', Outlet Invert= 207.64'



Summary for Pond 1P: DMH #33

Inflow Area = 16,111 sf, 93.77% Impervious, Inflow Depth > 2.89" for 2YR event

Inflow = 1.12 cfs @ 12.09 hrs, Volume= 3,877 cf

Outflow = 1.12 cfs @ 12.09 hrs, Volume= 3,877 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.12 cfs @ 12.09 hrs, Volume= 3.877 cf

Routed to Pond OCS6: OCS #6

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.05' @ 12.09 hrs

Flood Elev= 209.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.50'	12.0" Round Culvert L= 46.7' Ke= 0.500
			Inlet / Outlet Invert= 205.50' / 204.33' S= 0.0251 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.09 cfs @ 12.09 hrs HW=206.04' TW=201.78' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.09 cfs @ 2.51 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 63

Summary for Pond 3P: OCS #8

12,684 sf, 86.64% Impervious, Inflow Depth > 2.44" for 2YR event Inflow Area =

Inflow 0.79 cfs @ 12.09 hrs, Volume= 2.578 cf

0.79 cfs @ 12.09 hrs, Volume= Outflow 2,578 cf, Atten= 0%, Lag= 0.0 min

0.79 cfs @ 12.09 hrs, Volume= Primary = 2.578 cf Routed to Pond P214: STORMTECH INFILTRATION SYSTEM #4

Routing by Dvn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 201.07' @ 12.09 hrs

Flood Elev= 206.36'

Device Routing Invert Outlet Devices

#1 200.62' 12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads Primary

Primary OutFlow Max=0.77 cfs @ 12.09 hrs HW=201.07' TW=200.49' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.77 cfs @ 2.28 fps)

Summary for Pond 5R: TRENCH DRAIN

Inflow Area = 11,173 sf, 75.10% Impervious, Inflow Depth > 2.51" for 2YR event

Inflow 0.71 cfs @ 12.09 hrs, Volume= 2,338 cf

0.71 cfs @ 12.09 hrs, Volume= 2,338 cf, Atten= 0%, Lag= 0.0 min Outflow

Primary 0.71 cfs @ 12.09 hrs, Volume= 2,338 cf Routed to Pond P206: STORMTECH INFILTRATION SYSTEM #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 197.74' @ 12.09 hrs

Flood Elev= 199.50'

Device Routing Invert Outlet Devices #1 197.22' **8.0" Round Culvert** L= 36.0' Ke= 0.500 Primary Inlet / Outlet Invert= 197.22' / 196.50' S= 0.0200 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=0.70 cfs @ 12.09 hrs HW=197.73' TW=195.23' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.70 cfs @ 2.43 fps)

Summary for Pond 11P: YARD DRAIN

21,407 sf, 48.10% Impervious, Inflow Depth > 1.39" for 2YR event Inflow Area =

Inflow 0.78 cfs @ 12.10 hrs, Volume= 2,476 cf

Outflow 2,448 cf, Atten= 34%, Lag= 6.4 min =

0.51 cfs @ 12.20 hrs, Volume= 0.51 cfs @ 12.20 hrs, Volume= Primary 2,448 cf

Routed to Pond D13: DMH #13

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.32' @ 12.20 hrs Surf.Area= 5,486 sf Storage= 383 cf

Plug-Flow detention time= 22.4 min calculated for 2,448 cf (99% of inflow)

Center-of-Mass det. time= 15.7 min (859.9 - 844.2)

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 64

Volume	Inve	rt Avail.Sto	rage Storage	Description	
#1	207.2	5' 5,4	75 cf Custom	n Stage Data (Pri	smatic)Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
207.2	25	5,050	0	0	
208.0	00	9,550	5,475	5,475	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	203.25'	12.0" Round	d Culvert L= 61.0	0' Ke= 0.500
#2	Device 1	207.25'	n= 0.012 Cor 4.0" x 4.0" H X 4 rows C= 0	rrugated PP, smooriz. Orifice/Grat	202.94' S= 0.0051 '/' Cc= 0.900 oth interior, Flow Area= 0.79 sf te X 4.00 columns 4.0" Grate (44% open area) ds

Primary OutFlow Max=0.51 cfs @ 12.20 hrs HW=207.32' TW=202.75' (Dynamic Tailwater) -1=Culvert (Passes 0.51 cfs of 6.55 cfs potential flow) **2=Orifice/Grate** (Weir Controls 0.51 cfs @ 0.88 fps)

Summary for Pond CB10: CB #10

Inflow Area = 6,961 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event

0.50 cfs @ 12.09 hrs, Volume= 1,761 cf Inflow =

Outflow 1,761 cf, Atten= 0%, Lag= 0.0 min

0.50 cfs @ 12.09 hrs, Volume= 0.50 cfs @ 12.09 hrs, Volume= Primary = 1.761 cf

Routed to Pond D5: DMH #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.18' @ 12.09 hrs Flood Elev= 212.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.76'	12.0" Round Culvert L= 33.8' Ke= 0.500
			Inlet / Outlet Invert= 209.76' / 209.59' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.48 cfs @ 12.09 hrs HW=210.17' TW=209.85' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.48 cfs @ 2.34 fps)

Summary for Pond CB11: CB #11

Inflow Area	a =	7,173 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event
Inflow	=	0.51 cfs @ 12.09 hrs, Volume= 1,814 cf
Outflow	=	0.51 cfs @ 12.09 hrs, Volume= 1,814 cf, Atten= 0%, Lag= 0.0 min
Primary	=	0.51 cfs @ 12.09 hrs, Volume= 1,814 cf

Routed to Pond D5 : DMH #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 65

Peak Elev= 210.32' @ 12.09 hrs Flood Elev= 213.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.94'	12.0" Round Culvert L= 26.3' Ke= 0.500 Inlet / Outlet Invert= 209.94' / 209.67' S= 0.0103 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.50 cfs @ 12.09 hrs HW=210.31' TW=209.85' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.50 cfs @ 2.79 fps)

Summary for Pond CB12: CB #12

Inflow Area = 5,238 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event

Inflow 0.37 cfs @ 12.09 hrs, Volume= 1,325 cf

0.37 cfs @ 12.09 hrs, Volume= Outflow = 1,325 cf, Atten= 0%, Lag= 0.0 min

1,325 cf Primary = 0.37 cfs @ 12.09 hrs, Volume=

Routed to Pond 1P: DMH #33

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.98' @ 12.09 hrs

Flood Elev= 209.84'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.68'	12.0" Round Culvert L= 41.3' Ke= 0.500 Inlet / Outlet Invert= 206.68' / 205.65' S= 0.0249 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior Flow Area= 0.79 sf

Primary OutFlow Max=0.36 cfs @ 12.09 hrs HW=206.98' TW=206.04' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.36 cfs @ 1.86 fps)

Summary for Pond CB13: CB #13

Inflow Area = 10,873 sf, 90.78% Impervious, Inflow Depth > 2.82" for 2YR event

0.75 cfs @ 12.09 hrs, Volume= 2,552 cf Inflow =

Outflow 0.75 cfs @ 12.09 hrs, Volume= 2,552 cf, Atten= 0%, Lag= 0.0 min

0.75 cfs @ 12.09 hrs, Volume= Primary = 2,552 cf

Routed to Pond 1P: DMH #33

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.14' @ 12.09 hrs

Flood Elev= 209.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.70'	12.0" Round Culvert L= 43.7' Ke= 0.500 Inlet / Outlet Invert= 206.70' / 205.61' S= 0.0249 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.73 cfs @ 12.09 hrs HW=207.13' TW=206.04' (Dynamic Tailwater) **1=Culvert** (Inlet Controls 0.73 cfs @ 2.24 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 66

Summary for Pond CB14: CB #14

Inflow Area = 12,099 sf, 86.22% Impervious, Inflow Depth > 2.32" for 2YR event

Inflow = 0.73 cfs @ 12.09 hrs, Volume= 2,341 cf

Outflow = 0.73 cfs @ 12.09 hrs, Volume= 2,341 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.73 cfs @ 12.09 hrs, Volume= 2,341 cf

Routed to Pond D8: DMH #8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 201.41' @ 12.09 hrs

Flood Elev= 203.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.79'	12.0" Round Culvert L= 23.2' Ke= 0.500 Inlet / Outlet Invert= 200.79' / 200.67' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.71 cfs @ 12.09 hrs HW=201.40' TW=201.26' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.71 cfs @ 2.02 fps)

Summary for Pond CB15: CB #15

Inflow Area = 6,666 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event

Inflow = 0.47 cfs @ 12.09 hrs, Volume= 1,686 cf

Outflow = 0.47 cfs @ 12.09 hrs, Volume= 1,686 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.47 cfs @ 12.09 hrs, Volume= 1,686 cf

Routed to Pond D8: DMH #8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 201.34' @ 12.09 hrs

Flood Elev= 203.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.79'	12.0" Round Culvert L= 15.6' Ke= 0.500
			Inlet / Outlet Invert= 200.79' / 200.71' S= 0.0051 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.46 cfs @ 12.09 hrs HW=201.33' TW=201.26' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.46 cfs @ 1.54 fps)

Summary for Pond CB16: CB #16

Inflow Area = 8,516 sf, 64.88% Impervious, Inflow Depth > 1.39" for 2YR event

Inflow = 0.31 cfs @ 12.10 hrs, Volume= 985 cf

Outflow = 0.31 cfs @ 12.10 hrs, Volume= 985 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.31 cfs @ 12.10 hrs, Volume= 985 cf

Routed to Pond D10: DMH #10

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 67

Peak Elev= 203.82' @ 12.10 hrs Flood Elev= 206.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.47'	12.0" Round Culvert L= 20.9' Ke= 0.500 Inlet / Outlet Invert= 203.47' / 203.33' S= 0.0067 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.31 cfs @ 12.10 hrs HW=203.81' TW=203.66' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.31 cfs @ 1.90 fps)

Summary for Pond CB17: CB #17

Inflow Area = 11,836 sf, 73.87% Impervious, Inflow Depth > 2.42" for 2YR event

Inflow 0.73 cfs @ 12.09 hrs, Volume= 2.382 cf

0.73 cfs @ 12.09 hrs, Volume= Outflow 2,382 cf, Atten= 0%, Lag= 0.0 min

2,382 cf 0.73 cfs @ 12.09 hrs, Volume= Primary =

Routed to Pond D11: DMH #11

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 205.48' @ 12.09 hrs

Flood Elev= 208.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.99'	12.0" Round Culvert L= 13.8' Ke= 0.500 Inlet / Outlet Invert= 204.99' / 204.86' S= 0.0094 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.72 cfs @ 12.09 hrs HW=205.47' TW=205.01' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.72 cfs @ 2.78 fps)

Summary for Pond CB18: CB #18

Inflow Area = 24,853 sf, 72.99% Impervious, Inflow Depth > 1.87" for 2YR event

1.28 cfs @ 12.10 hrs, Volume= 3,873 cf Inflow =

Outflow 1.28 cfs @ 12.10 hrs, Volume= 3,873 cf, Atten= 0%, Lag= 0.0 min

1.28 cfs @ 12.10 hrs, Volume= Primary = 3,873 cf

Routed to Pond D11: DMH #11

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.37' @ 12.10 hrs

Flood Elev= 208.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.72'	15.0" Round Culvert L= 25.1' Ke= 0.500 Inlet / Outlet Invert= 204.72' / 204.59' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.28 cfs @ 12.10 hrs HW=205.37' TW=205.02' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 1.28 cfs @ 2.90 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 68

Summary for Pond CB20: CB #20

Inflow Area = 11,939 sf, 88.95% Impervious, Inflow Depth > 2.71" for 2YR event

Inflow = 0.80 cfs @ 12.09 hrs, Volume= 2,698 cf

Outflow = 0.80 cfs @ 12.09 hrs, Volume= 2,698 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.80 cfs @ 12.09 hrs, Volume= 2,698 cf

Routed to Pond D12: DMH #12

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 204.51' @ 12.09 hrs

Flood Elev= 207.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.97'	12.0" Round Culvert L= 30.3' Ke= 0.500 Inlet / Outlet Invert= 203.97' / 203.81' S= 0.0053 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.78 cfs @ 12.09 hrs HW=204.50' TW=203.95' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.78 cfs @ 2.66 fps)

Summary for Pond CB21: CB #21

Inflow Area = 10,174 sf, 87.04% Impervious, Inflow Depth > 2.23" for 2YR event

Inflow = 0.59 cfs @ 12.09 hrs, Volume= 1,892 cf

Outflow = 0.59 cfs @ 12.09 hrs, Volume= 1,892 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.59 cfs @ 12.09 hrs, Volume= 1,892 cf

Routed to Pond D12: DMH #12

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.78' @ 12.09 hrs

Flood Elev= 208.02'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.32'	12.0" Round Culvert L= 26.0' Ke= 0.500
			Inlet / Outlet Invert= 204.32' / 204.19' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.58 cfs @ 12.09 hrs HW=204.78' TW=203.95' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.58 cfs @ 2.43 fps)

Summary for Pond CB22: CB #22

Inflow Area = 12,001 sf, 91.62% Impervious, Inflow Depth > 2.82" for 2YR event

Inflow = 0.83 cfs @ 12.09 hrs, Volume= 2,817 cf

Outflow = 0.83 cfs @ 12.09 hrs, Volume= 2,817 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.83 cfs @ 12.09 hrs, Volume= 2,817 cf

Routed to Pond D14: DMH #14

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 69

Peak Elev= 205.89' @ 12.09 hrs

Flood Elev= 208.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.33'	12.0" Round Culvert L= 16.1' Ke= 0.500 Inlet / Outlet Invert= 205.33' / 205.25' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.80 cfs @ 12.09 hrs HW=205.88' TW=204.90' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.80 cfs @ 2.64 fps)

Summary for Pond CB23: CB #23

Inflow Area = 9,694 sf, 61.00% Impervious, Inflow Depth > 2.14" for 2YR event

Inflow 0.54 cfs @ 12.09 hrs, Volume= 1.732 cf

0.54 cfs @ 12.09 hrs, Volume= Outflow = 1,732 cf, Atten= 0%, Lag= 0.0 min

1,732 ∪i, 1,732 cf Primary = 0.54 cfs @ 12.09 hrs, Volume=

Routed to Pond D14: DMH #14

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.85' @ 12.09 hrs

Flood Elev= 208.57'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.41'	12.0" Round Culvert L= 16.3' Ke= 0.500
			Inlet / Outlet Invert= 205.41' / 205.32' S= 0.0055 '/' Cc= 0.900
			n= 0.012 Corrugated PP_smooth interior_Flow Area= 0.79 sf

Primary OutFlow Max=0.53 cfs @ 12.09 hrs HW=205.84' TW=204.90' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.53 cfs @ 2.42 fps)

Summary for Pond CB24: CB #24

Inflow Area = 7,930 sf, 72.16% Impervious, Inflow Depth > 2.51" for 2YR event

0.51 cfs @ 12.09 hrs, Volume= 1,660 cf Inflow =

Outflow 0.51 cfs @ 12.09 hrs, Volume= 1,660 cf, Atten= 0%, Lag= 0.0 min

0.51 cfs @ 12.09 hrs, Volume= Primary = 1,660 cf

Routed to Pond D16: DMH #16

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.64' @ 12.09 hrs

Flood Elev= 209.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.21'	12.0" Round Culvert L= 12.1' Ke= 0.500 Inlet / Outlet Invert= 205.21' / 205.15' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.49 cfs @ 12.09 hrs HW=205.63' TW=205.46' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.49 cfs @ 2.31 fps)

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 70

Summary for Pond CB25: CB #25

Inflow Area = 8,487 sf, 80.92% Impervious, Inflow Depth > 2.61" for 2YR event

Inflow = 0.56 cfs @ 12.09 hrs, Volume= 1,846 cf

Outflow = 0.56 cfs @ 12.09 hrs, Volume= 1,846 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.56 cfs @ 12.09 hrs, Volume= 1,846 cf

Routed to Pond D16: DMH #16

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.64' @ 12.09 hrs

Flood Elev= 208.38'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.22'	15.0" Round Culvert L= 11.4' Ke= 0.500 Inlet / Outlet Invert= 205.22' / 205.16' S= 0.0053 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=0.54 cfs @ 12.09 hrs HW=205.63' TW=205.46' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.54 cfs @ 2.31 fps)

Summary for Pond CB26: CB #26

Inflow Area = 8,835 sf, 63.75% Impervious, Inflow Depth > 2.32" for 2YR event

Inflow = 0.53 cfs @ 12.09 hrs, Volume= 1,710 cf

Outflow = 0.53 cfs @ 12.09 hrs, Volume= 1,710 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.53 cfs @ 12.09 hrs, Volume= 1,710 cf

Routed to Pond D17: DMH #17

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 202.20' @ 12.09 hrs

Flood Elev= 204.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	201.77'	12.0" Round Culvert L= 42.5' Ke= 0.500 Inlet / Outlet Invert= 201.77' / 201.55' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.52 cfs @ 12.09 hrs HW=202.19' TW=201.05' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.52 cfs @ 2.42 fps)

Summary for Pond CB27: CB #27

Inflow Area = 6,111 sf, 91.90% Impervious, Inflow Depth > 2.82" for 2YR event

Inflow = 0.42 cfs @ 12.09 hrs, Volume= 1,434 cf

Outflow = 0.42 cfs @ 12.09 hrs, Volume= 1,434 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.42 cfs @ 12.09 hrs, Volume= 1,434 cf

Routed to Pond D17: DMH #17

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 71

Peak Elev= 201.38' @ 12.09 hrs Flood Elev= 204.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	201.00'	12.0" Round Culvert L= 18.0' Ke= 0.500 Inlet / Outlet Invert= 201.00' / 200.90' S= 0.0056 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.41 cfs @ 12.09 hrs HW=201.38' TW=201.04' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.41 cfs @ 2.24 fps)

Summary for Pond CB28: CB #28

Inflow Area = 10,372 sf, 51.33% Impervious, Inflow Depth > 2.06" for 2YR event

Inflow = 0.56 cfs @ 12.09 hrs, Volume= 1,779 cf

Outflow = 0.56 cfs @ 12.09 hrs, Volume= 1,779 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.56 cfs @ 12.09 hrs, Volume= 1,779 cf

Routed to Pond D18: DMH #18

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 198.22' @ 12.09 hrs

Flood Elev= 200.92'

Device	Routing	Invert	Outlet Devices
#1	Primary	197.75'	12.0" Round Culvert L= 13.7' Ke= 0.500
			Inlet / Outlet Invert= 197.75' / 197.69' S= 0.0044 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.55 cfs @ 12.09 hrs HW=198.21' TW=198.05' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.55 cfs @ 2.25 fps)

Summary for Pond CB29: CB #29

Inflow Area = 8,495 sf, 84.21% Impervious, Inflow Depth > 2.61" for 2YR event

Inflow = 0.56 cfs @ 12.09 hrs, Volume= 1.848 cf

Outflow = 0.56 cfs @ 12.09 hrs, Volume= 1,848 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.56 cfs @ 12.09 hrs, Volume= 1,848 cf

Routed to Pond D19: DMH #19

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.89' @ 12.09 hrs

Flood Elev= 208.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.38'	12.0" Round Culvert L= 13.5' Ke= 0.500 Inlet / Outlet Invert= 205.38' / 205.31' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.54 cfs @ 12.09 hrs HW=205.88' TW=205.75' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.54 cfs @ 2.02 fps)

19097 Post-Development Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 72

Summary for Pond CB30: CB #30

Inflow Area = 8,933 sf, 82.40% Impervious, Inflow Depth > 2.61" for 2YR event

Inflow = 0.59 cfs @ 12.09 hrs, Volume= 1,943 cf

Outflow = 0.59 cfs @ 12.09 hrs, Volume= 1,943 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.59 cfs @ 12.09 hrs, Volume= 1,943 cf

Routed to Pond D19: DMH #19

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.90' @ 12.09 hrs

Flood Elev= 208.54'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.38'	12.0" Round Culvert L= 17.5' Ke= 0.500 Inlet / Outlet Invert= 205.38' / 205.29' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.57 cfs @ 12.09 hrs HW=205.89' TW=205.75' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.57 cfs @ 2.05 fps)

Summary for Pond CB31: CB #31

Inflow Area = 16,365 sf, 68.64% Impervious, Inflow Depth > 2.23" for 2YR event

Inflow = 0.95 cfs @ 12.09 hrs, Volume= 3,044 cf

Outflow = 0.95 cfs @ 12.09 hrs, Volume= 3,044 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.95 cfs @ 12.09 hrs, Volume= 3,044 cf

Routed to Pond D21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.80' @ 12.09 hrs

Flood Elev= 207.36'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.19'	12.0" Round Culvert L= 16.4' Ke= 0.500
			Inlet / Outlet Invert= 204.19' / 204.11' S= 0.0049 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.93 cfs @ 12.09 hrs HW=204.79' TW=204.06' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.93 cfs @ 2.69 fps)

Summary for Pond CB32: CB #32

Inflow Area = 12,710 sf, 70.47% Impervious, Inflow Depth > 2.32" for 2YR event

Inflow = 0.76 cfs @ 12.09 hrs, Volume= 2,460 cf

Outflow = 0.76 cfs @ 12.09 hrs, Volume= 2,460 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.76 cfs @ 12.09 hrs, Volume= 2,460 cf

Routed to Pond D21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 73

Peak Elev= 204.73' @ 12.09 hrs Flood Elev= 207.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.19'	12.0" Round Culvert L= 16.3' Ke= 0.500 Inlet / Outlet Invert= 204.19' / 204.11' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.75 cfs @ 12.09 hrs HW=204.72' TW=204.06' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.75 cfs @ 2.54 fps)

Summary for Pond CB33: CB #33

Inflow Area = 5,421 sf, 83.90% Impervious, Inflow Depth > 2.61" for 2YR event

Inflow 0.36 cfs @ 12.09 hrs, Volume= 1,179 cf

0.36 cfs @ 12.09 hrs, Volume= Outflow = 1,179 cf, Atten= 0%, Lag= 0.0 min

1,179 cf Primary = 0.36 cfs @ 12.09 hrs, Volume=

Routed to Pond D22: DMH #22

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.66' @ 12.09 hrs

Flood Elev= 208.45'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.28'	12.0" Round Culvert L= 11.7' Ke= 0.500 Inlet / Outlet Invert= 205.28' / 205.22' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.35 cfs @ 12.09 hrs HW=205.65' TW=205.53' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.35 cfs @ 1.94 fps)

Summary for Pond CB34: CB #34

Inflow Area = 8,622 sf, 80.51% Impervious, Inflow Depth > 2.51" for 2YR event

0.55 cfs @ 12.09 hrs, Volume= 1,804 cf Inflow =

Outflow 0.55 cfs @ 12.09 hrs, Volume= 1,804 cf, Atten= 0%, Lag= 0.0 min

0.55 cfs @ 12.09 hrs, Volume= Primary = 1.804 cf

Routed to Pond D22: DMH #22

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.69' @ 12.09 hrs

Flood Elev= 208.38'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.21'	12.0" Round Culvert L= 16.5' Ke= 0.500 Inlet / Outlet Invert= 205.21' / 205.13' S= 0.0048 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.54 cfs @ 12.09 hrs HW=205.68' TW=205.53' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.54 cfs @ 2.15 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

<u>Page 74</u>

Summary for Pond CB35: CB #35

Inflow Area = 4,149 sf, 98.10% Impervious, Inflow Depth > 3.04" for 2YR event

Inflow = 0.30 cfs @ 12.09 hrs, Volume= 1,049 cf

Outflow = 0.30 cfs @ 12.09 hrs, Volume= 1,049 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.30 cfs @ 12.09 hrs, Volume= 1,049 cf

Routed to Pond D23: DMH #23

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 207.36' @ 12.09 hrs

Flood Elev= 210.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	207.04'	12.0" Round Culvert L= 15.2' Ke= 0.500 Inlet / Outlet Invert= 207.04' / 206.96' S= 0.0053 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.29 cfs @ 12.09 hrs HW=207.36' TW=207.11' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.29 cfs @ 2.01 fps)

Summary for Pond CB36: CB #36

Inflow Area = 6,622 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event

Inflow = 0.47 cfs @ 12.09 hrs, Volume= 1,675 cf

Outflow = 0.47 cfs @ 12.09 hrs, Volume= 1,675 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.47 cfs @ 12.09 hrs, Volume= 1,675 cf

Routed to Pond D23: DMH #23

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.45' @ 12.09 hrs

Flood Elev= 210.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	207.04'	12.0" Round Culvert L= 16.1' Ke= 0.500 Inlet / Outlet Invert= 207.04' / 206.96' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.46 cfs @ 12.09 hrs HW=207.45' TW=207.11' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.46 cfs @ 2.25 fps)

Summary for Pond CB38: CB #38

Inflow Area = 7,637 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event

Inflow = 0.54 cfs @ 12.09 hrs, Volume= 1,932 cf

Outflow = 0.54 cfs @ 12.09 hrs, Volume= 1,932 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.54 cfs @ 12.09 hrs, Volume= 1,932 cf

Routed to Pond D25: DMH #25

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 75

Peak Elev= 210.28' @ 12.09 hrs Flood Elev= 212.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.69'	12.0" Round Culvert L= 16.7' Ke= 0.500 Inlet / Outlet Invert= 209.69' / 209.61' S= 0.0048 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.53 cfs @ 12.09 hrs HW=210.26' TW=210.18' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.53 cfs @ 1.63 fps)

Summary for Pond CB39: CB #39

Inflow Area = 7,612 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event

Inflow 0.54 cfs @ 12.09 hrs, Volume= 1.925 cf

0.54 cfs @ 12.09 hrs, Volume= Outflow = 1,925 cf, Atten= 0%, Lag= 0.0 min

1,925 cf 0.54 cfs @ 12.09 hrs, Volume= Primary =

Routed to Pond D25: DMH #25

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.28' @ 12.09 hrs

Flood Elev= 212.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.69'	12.0" Round Culvert L= 16.4' Ke= 0.500 Inlet / Outlet Invert= 209.69' / 209.61' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.53 cfs @ 12.09 hrs HW=210.27' TW=210.18' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.53 cfs @ 1.62 fps)

Summary for Pond CB40: CB #40

Inflow Area = 4,211 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event

0.30 cfs @ 12.09 hrs, Volume= 1,065 cf Inflow =

Outflow 0.30 cfs @ 12.09 hrs, Volume= 1,065 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.30 cfs @ 12.09 hrs, Volume= 1,065 cf

Routed to Pond D27: DMH #27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 214.02' @ 12.09 hrs

Flood Elev= 217.04'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.68'	12.0" Round Culvert L= 17.8' Ke= 0.500 Inlet / Outlet Invert= 213.68' / 213.55' S= 0.0073 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.29 cfs @ 12.09 hrs HW=214.01' TW=213.87' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.29 cfs @ 1.89 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 76

Summary for Pond CB41: CB #41

Inflow Area = 5,586 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event

Inflow = 0.40 cfs @ 12.09 hrs, Volume= 1,413 cf

Outflow = 0.40 cfs @ 12.09 hrs, Volume= 1,413 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.40 cfs @ 12.09 hrs, Volume= 1,413 cf

Routed to Pond D27: DMH #27

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 214.27' @ 12.09 hrs

Flood Elev= 217.06'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.89'	12.0" Round Culvert L= 18.4' Ke= 0.500 Inlet / Outlet Invert= 213.89' / 213.80' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.39 cfs @ 12.09 hrs HW=214.26' TW=213.87' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.39 cfs @ 2.16 fps)

Summary for Pond CB43: CB #43

Inflow Area = 3,109 sf, 75.36% Impervious, Inflow Depth > 2.14" for 2YR event

Inflow = 0.17 cfs @ 12.09 hrs, Volume= 555 cf

Outflow = 0.17 cfs @ 12.09 hrs, Volume= 555 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.17 cfs @ 12.09 hrs, Volume= 555 cf

Routed to Pond D29: DMH #29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 220.29' @ 12.09 hrs

Flood Elev= 223.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	220.00'	12.0" Round Culvert L= 14.9' Ke= 0.500
	-		Inlet / Outlet Invert= 220.00' / 219.93' S= 0.0047 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.17 cfs @ 12.09 hrs HW=220.28' TW=220.20' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.17 cfs @ 1.41 fps)

Summary for Pond CB44: CB #44

Inflow Area = 1,978 sf, 84.43% Impervious, Inflow Depth > 2.42" for 2YR event

Inflow = 0.12 cfs @ 12.09 hrs, Volume= 398 cf

Outflow = 0.12 cfs @ 12.09 hrs, Volume= 398 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.12 cfs @ 12.09 hrs, Volume= 398 cf

Routed to Pond D29: DMH #29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 77

Peak Elev= 220.26' @ 12.09 hrs

Flood Elev= 223.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	220.00'	12.0" Round Culvert L= 14.9' Ke= 0.500 Inlet / Outlet Invert= 220.00' / 219.93' S= 0.0047 '/' Cc= 0.900 n= 0.013 Corrugated PE. smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.12 cfs @ 12.09 hrs HW=220.26' TW=220.20' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.12 cfs @ 1.14 fps)

Summary for Pond CB45: CB #45

Inflow Area = 2,465 sf, 50.30% Impervious, Inflow Depth > 1.45" for 2YR event

Inflow = 0.09 cfs @ 12.10 hrs, Volume= 299 cf

Outflow = 0.09 cfs @ 12.10 hrs, Volume= 299 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.09 cfs @ 12.10 hrs, Volume= 299 cf

Routed to Pond D30: DMH #30

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 221.47' @ 12.10 hrs

Flood Elev= 224.46'

Device	Routing	Invert	Outlet Devices
#1	Primary	221.29'	12.0" Round Culvert L= 18.2' Ke= 0.500 Inlet / Outlet Invert= 221.29' / 221.20' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.09 cfs @ 12.10 hrs HW=221.47' TW=221.21' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.09 cfs @ 1.48 fps)

Summary for Pond CB46: CB #46

Inflow Area = 4,397 sf, 50.97% Impervious, Inflow Depth > 1.45" for 2YR event

Inflow = 0.17 cfs @ 12.10 hrs, Volume= 533 cf

Outflow = 0.17 cfs @ 12.10 hrs, Volume= 533 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.17 cfs @ 12.10 hrs, Volume= 533 cf

Routed to Pond D30: DMH #30

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 221.77' @ 12.10 hrs

Flood Elev= 224.69'

Device	Routing	Invert	Outlet Devices
#1	Primary	221.53'	12.0" Round Culvert L= 15.3' Ke= 0.500 Inlet / Outlet Invert= 221.53' / 221.45' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.17 cfs @ 12.10 hrs HW=221.77' TW=221.21' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.17 cfs @ 1.74 fps)

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 78

Summary for Pond CB47: CB#47

Inflow Area = 3,012 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event

Inflow = 0.21 cfs @ 12.09 hrs, Volume= 762 cf

Outflow = 0.21 cfs @ 12.09 hrs, Volume= 762 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.21 cfs @ 12.09 hrs, Volume= 762 cf

Routed to Pond D31: DMH#31

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 225.32' @ 12.09 hrs

Flood Elev= 230.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	225.05'	12.0" Round Culvert L= 20.9' Ke= 0.500 Inlet / Outlet Invert= 225.05' / 224.95' S= 0.0048 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.21 cfs @ 12.09 hrs HW=225.31' TW=225.08' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.21 cfs @ 1.89 fps)

Summary for Pond CB48: CB#48

Inflow Area = 60,128 sf, 25.16% Impervious, Inflow Depth > 0.87" for 2YR event

Inflow = 1.03 cfs @ 12.19 hrs, Volume= 4,342 cf

Outflow = 1.03 cfs @ 12.19 hrs, Volume= 4,342 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.03 cfs @ 12.19 hrs, Volume= 4,342 cf

Routed to Pond D31: DMH#31

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 225.40' @ 12.19 hrs

Flood Elev= 230.25'

Device	Routing	Invert	Outlet Devices
#1	Primary	224.82'	15.0" Round Culvert L= 16.9' Ke= 0.500
			Inlet / Outlet Invert= 224.82' / 224.74' S= 0.0047 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.01 cfs @ 12.19 hrs HW=225.40' TW=225.14' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.01 cfs @ 2.70 fps)

Summary for Pond CB49: CB #49

Inflow Area = 5,238 sf, 84.59% Impervious, Inflow Depth > 2.61" for 2YR event

Inflow = 0.34 cfs @ 12.09 hrs, Volume= 1,139 cf

Outflow = 0.34 cfs @ 12.09 hrs, Volume= 1,139 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.34 cfs @ 12.09 hrs, Volume= 1,139 cf

Routed to Pond DMH32: DMH #32

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 79

Peak Elev= 203.25' @ 12.09 hrs

Flood Elev= 205.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.76'	12.0" Round Culvert L= 15.5' Ke= 0.500 Inlet / Outlet Invert= 202.76' / 202.68' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.34 cfs @ 12.09 hrs HW=203.24' TW=203.18' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.34 cfs @ 1.31 fps)

Summary for Pond CB50: CB #50

Inflow Area = 15,040 sf, 77.20% Impervious, Inflow Depth > 2.51" for 2YR event

Inflow 0.96 cfs @ 12.09 hrs, Volume= 3,147 cf

0.96 cfs @ 12.09 hrs, Volume= 3,147 cf, Atten= 0%, Lag= 0.0 min 3,147 cf Outflow =

Primary = 0.96 cfs @ 12.09 hrs, Volume=

Routed to Pond DMH32: DMH #32

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.41' @ 12.09 hrs

Flood Elev= 205.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.78'	12.0" Round Culvert L= 15.3' Ke= 0.500 Inlet / Outlet Invert= 202.78' / 202.70' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.94 cfs @ 12.09 hrs HW=203.40' TW=203.18' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.94 cfs @ 2.63 fps)

Summary for Pond CB51: CB #51

Inflow Area = 6,823 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event

0.49 cfs @ 12.09 hrs, Volume= 1,726 cf Inflow =

Outflow 0.49 cfs @ 12.09 hrs, Volume= 1,726 cf, Atten= 0%, Lag= 0.0 min

0.49 cfs @ 12.09 hrs, Volume= Primary = 1,726 cf

Routed to Pond OCS7: OCS #7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 202.76' @ 12.09 hrs

Flood Elev= 212.77'

Device F	Routing	Invert	Outlet Devices
#1 F	Primary	202.35'	12.0" Round Culvert L= 31.4' Ke= 0.500 Inlet / Outlet Invert= 202.35' / 202.19' S= 0.0051 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.47 cfs @ 12.09 hrs HW=202.76' TW=202.32' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.47 cfs @ 2.33 fps)

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 80

Summary for Pond CB52: CB #52

Inflow Area = 9,052 sf, 87.14% Impervious, Inflow Depth > 2.71" for 2YR event

Inflow = 0.61 cfs @ 12.09 hrs, Volume= 2,045 cf

Outflow = 0.61 cfs @ 12.09 hrs, Volume= 2,045 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.61 cfs @ 12.09 hrs, Volume= 2,045 cf

Routed to Pond OCS7: OCS #7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.15' @ 12.09 hrs

Flood Elev= 205.84'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.68'	12.0" Round Culvert L= 25.5' Ke= 0.500 Inlet / Outlet Invert= 202.68' / 202.55' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.59 cfs @ 12.09 hrs HW=203.14' TW=202.32' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.59 cfs @ 2.45 fps)

Summary for Pond CB53: CB #53

Inflow Area = 7,863 sf, 86.52% Impervious, Inflow Depth > 2.51" for 2YR event

Inflow = 0.50 cfs @ 12.09 hrs, Volume= 1,646 cf

Outflow = 0.50 cfs @ 12.09 hrs, Volume= 1,646 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.50 cfs @ 12.09 hrs, Volume= 1,646 cf

Routed to Pond 3P: OCS #8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.20' @ 12.09 hrs

Flood Elev= 205.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.78'	12.0" Round Culvert L= 32.0' Ke= 0.500 Inlet / Outlet Invert= 202.78' / 202.62' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.49 cfs @ 12.09 hrs HW=203.20' TW=201.07' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.49 cfs @ 2.34 fps)

Summary for Pond CB54: CB #54

Inflow Area = 4,821 sf, 86.85% Impervious, Inflow Depth > 2.32" for 2YR event

Inflow = 0.29 cfs @ 12.09 hrs, Volume= 933 cf

Outflow = 0.29 cfs @ 12.09 hrs, Volume= 933 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.29 cfs @ 12.09 hrs, Volume= 933 cf

Routed to Pond 3P: OCS #8

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 81

Peak Elev= 202.98' @ 12.09 hrs

Flood Elev= 205.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.66'	12.0" Round Culvert L= 36.7' Ke= 0.500 Inlet / Outlet Invert= 202.66' / 202.48' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.28 cfs @ 12.09 hrs HW=202.97' TW=201.07' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.28 cfs @ 2.03 fps)

Summary for Pond CB7: CB#5

Inflow Area = 4,650 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event

Inflow 0.33 cfs @ 12.09 hrs, Volume= 1.176 cf

0.33 cfs @ 12.09 hrs, Volume= Outflow = 1,176 cf, Atten= 0%, Lag= 0.0 min

1,176 cl, 1,176 cf Primary = 0.33 cfs @ 12.09 hrs, Volume=

Routed to Pond D4: DMH#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 212.90' @ 12.09 hrs

Flood Elev= 215.79'

Device	Routing	Invert	Outlet Devices
#1	Primary	212.60'	12.0" Round Culvert L= 15.1' Ke= 0.500 Inlet / Outlet Invert= 212.60' / 212.45' S= 0.0099 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.32 cfs @ 12.09 hrs HW=212.90' TW=210.95' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.32 cfs @ 2.44 fps)

Summary for Pond CB8: CB#8

Inflow Area = 5,450 sf, 88.75% Impervious, Inflow Depth > 2.61" for 2YR event

0.36 cfs @ 12.09 hrs, Volume= 1,185 cf Inflow =

Outflow 0.36 cfs @ 12.09 hrs, Volume= 1,185 cf, Atten= 0%, Lag= 0.0 min

0.36 cfs @ 12.09 hrs, Volume= Primary = 1,185 cf

Routed to Pond D4: DMH#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 214.11' @ 12.09 hrs

Flood Elev= 215.79'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.79'	12.0" Round Culvert L= 15.1' Ke= 0.500 Inlet / Outlet Invert= 213.79' / 213.64' S= 0.0099 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.35 cfs @ 12.09 hrs HW=214.11' TW=210.95' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.35 cfs @ 2.42 fps)

19097 Post-Development Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 82

Summary for Pond CB9: CB #9

Inflow Area = 16,307 sf, 93.95% Impervious, Inflow Depth > 2.92" for 2YR event

Inflow = 1.14 cfs @ 12.09 hrs, Volume= 3,974 cf

Outflow = 1.14 cfs @ 12.09 hrs, Volume= 3,974 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.14 cfs @ 12.09 hrs, Volume= 3,974 cf

Routed to Pond D5: DMH #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.66' @ 12.09 hrs

Flood Elev= 213.27'

Device	Routing	Invert	Outlet Devices
#1	Primary	210.10'	12.0" Round Culvert L= 19.9' Ke= 0.500 Inlet / Outlet Invert= 210.10' / 209.71' S= 0.0196 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.11 cfs @ 12.09 hrs HW=210.65' TW=209.85' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.11 cfs @ 2.52 fps)

Summary for Pond D10: DMH #10

Inflow Area = 8,516 sf, 64.88% Impervious, Inflow Depth > 1.39" for 2YR event

Inflow = 0.31 cfs @ 12.10 hrs, Volume= 985 cf

Outflow = 0.31 cfs @ 12.10 hrs, Volume= 985 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.31 cfs @ 12.10 hrs, Volume= 985 cf

Routed to Pond P207: INFILTRATION POND #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.66' @ 12.10 hrs

Flood Elev= 206.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.33'	12.0" Round Culvert L= 15.6' Ke= 0.500 Inlet / Outlet Invert= 203.33' / 203.25' S= 0.0051 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.31 cfs @ 12.10 hrs HW=203.66' TW=197.13' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.31 cfs @ 2.04 fps)

Summary for Pond D11: DMH #11

Inflow Area = 36,689 sf, 73.28% Impervious, Inflow Depth > 2.05" for 2YR event

Inflow = 2.02 cfs @ 12.10 hrs, Volume= 6,256 cf

Outflow = 2.02 cfs @ 12.10 hrs, Volume= 6,256 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.02 cfs @ 12.10 hrs, Volume= 6,256 cf

Routed to Pond OCS3: OCS#3

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

<u> Page 83</u>

Peak Elev= 205.02' @ 12.10 hrs

Flood Elev= 208.33'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.25'	18.0" Round Culvert L= 44.6' Ke= 0.500 Inlet / Outlet Invert= 204.25' / 204.03' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=1.99 cfs @ 12.10 hrs HW=205.01' TW=203.95' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.99 cfs @ 3.22 fps)

Summary for Pond D12: DMH #12

Inflow Area = 22,113 sf, 88.07% Impervious, Inflow Depth > 2.49" for 2YR event

Inflow = 1.40 cfs @ 12.09 hrs, Volume= 4,590 cf

Outflow = 1.40 cfs @ 12.09 hrs, Volume= 4,590 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.40 cfs @ 12.09 hrs, Volume= 4,590 cf

Routed to Pond D13: DMH #13

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.96' @ 12.09 hrs

Flood Elev= 207.78'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.21'	12.0" Round Culvert L= 41.9' Ke= 0.500
			Inlet / Outlet Invert= 203.21' / 203.00' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.36 cfs @ 12.09 hrs HW=203.95' TW=202.96' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.36 cfs @ 3.06 fps)

Summary for Pond D13: DMH #13

Inflow Area = 81,632 sf, 72.61% Impervious, Inflow Depth > 2.22" for 2YR event

Inflow = 4.21 cfs @ 12.09 hrs, Volume= 15,091 cf

Outflow = 4.21 cfs @ 12.09 hrs, Volume= 15,091 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.21 cfs @ 12.09 hrs, Volume= 15,091 cf

Routed to Pond P207: INFILTRATION POND #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 202.97' @ 12.09 hrs

Flood Elev= 208.12'

Device	Routing	Invert	Outlet Devices
#1	Primary	201.95'	24.0" Round Culvert L= 60.1' Ke= 0.500 Inlet / Outlet Invert= 201.95' / 201.65' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=4.15 cfs @ 12.09 hrs HW=202.96' TW=197.13' (Dynamic Tailwater) 1=Culvert (Barrel Controls 4.15 cfs @ 3.80 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 84

Summary for Pond D14: DMH #14

Inflow Area = 38,112 sf, 77.40% Impervious, Inflow Depth > 2.54" for 2YR event

Inflow = 2.43 cfs @ 12.09 hrs, Volume= 8,054 cf

Outflow = 2.43 cfs @ 12.09 hrs, Volume= 8,054 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.43 cfs @ 12.09 hrs, Volume= 8,054 cf

Routed to Pond d13: DMH #13

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.91' @ 12.09 hrs

Flood Elev= 208.78'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.13'	18.0" Round Culvert L= 256.3' Ke= 0.500 Inlet / Outlet Invert= 204.13' / 202.85' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=2.37 cfs @ 12.09 hrs HW=204.90' TW=202.96' (Dynamic Tailwater) 1=Culvert (Barrel Controls 2.37 cfs @ 3.79 fps)

Summary for Pond D16: DMH #16

Inflow Area = 16,417 sf, 76.69% Impervious, Inflow Depth > 2.56" for 2YR event

Inflow = 1.06 cfs @ 12.09 hrs, Volume= 3,505 cf

Outflow = 1.06 cfs @ 12.09 hrs, Volume= 3,505 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.06 cfs @ 12.09 hrs, Volume= 3,505 cf

Routed to Pond D14: DMH #14

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.47' @ 12.09 hrs

Flood Elev= 208.59'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.90'	15.0" Round Culvert L= 103.5' Ke= 0.500 Inlet / Outlet Invert= 204.90' / 204.38' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.04 cfs @ 12.09 hrs HW=205.46' TW=204.90' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.04 cfs @ 2.84 fps)

Summary for Pond D17: DMH #17

Inflow Area = 14,946 sf, 75.26% Impervious, Inflow Depth > 2.52" for 2YR event

Inflow = 0.95 cfs @ 12.09 hrs, Volume= 3,144 cf

Outflow = 0.95 cfs @ 12.09 hrs, Volume= 3,144 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.95 cfs @ 12.09 hrs, Volume= 3,144 cf

Routed to Pond D18: DMH #18

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 85

Peak Elev= 201.05' @ 12.09 hrs Flood Elev= 204.84'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.55'	12.0" Round Culvert L= 91.6' Ke= 0.500 Inlet / Outlet Invert= 200.55' / 197.69' S= 0.0312 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.93 cfs @ 12.09 hrs HW=201.04' TW=198.05' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.93 cfs @ 2.40 fps)

Summary for Pond D18: DMH #18

Inflow Area = 25,318 sf, 65.46% Impervious, Inflow Depth > 2.33" for 2YR event

Inflow = 1.51 cfs @ 12.09 hrs, Volume= 4,923 cf

Outflow = 1.51 cfs @ 12.09 hrs, Volume= 4,923 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.51 cfs @ 12.09 hrs, Volume= 4,923 cf

Routed to Pond OCS1: OCS#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 198.06' @ 12.09 hrs

Flood Elev= 201.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	197.44'	15.0" Round Culvert L= 46.3' Ke= 0.500 Inlet / Outlet Invert= 197.44' / 196.98' S= 0.0099 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 1.23 sf

Primary OutFlow Max=1.48 cfs @ 12.09 hrs HW=198.05' TW=195.74' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.48 cfs @ 3.65 fps)

Summary for Pond D19: DMH #19

Inflow Area = 17,428 sf, 83.29% Impervious, Inflow Depth > 2.61" for 2YR event

Inflow = 1.15 cfs @ 12.09 hrs, Volume= 3.791 cf

Outflow = 1.15 cfs @ 12.09 hrs, Volume= 3,791 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.15 cfs @ 12.09 hrs, Volume= 3,791 cf

Routed to Pond d20: DMH #20

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.76' @ 12.09 hrs

Flood Elev= 208.57'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.19'	12.0" Round Culvert L= 82.5' Ke= 0.500 Inlet / Outlet Invert= 205.19' / 204.43' S= 0.0092 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.12 cfs @ 12.09 hrs HW=205.75' TW=204.78' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.12 cfs @ 3.57 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 86

Summary for Pond D2: DMH#2

Inflow Area = 73,240 sf, 37.72% Impervious, Inflow Depth > 1.22" for 2YR event

Inflow = 1.72 cfs @ 12.13 hrs, Volume= 7,465 cf

Outflow = 1.72 cfs @ 12.13 hrs, Volume= 7,465 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.72 cfs @ 12.13 hrs, Volume= 7,465 cf

Routed to Pond P205: INFILTRATION POND #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.57' @ 12.13 hrs

Flood Elev= 212.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.90'	15.0" Round Culvert L= 38.2' Ke= 0.500 Inlet / Outlet Invert= 206.90' / 206.52' S= 0.0099 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.69 cfs @ 12.13 hrs HW=207.57' TW=205.31' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.69 cfs @ 3.69 fps)

Summary for Pond D20: DMH #20

Inflow Area = 17,428 sf, 83.29% Impervious, Inflow Depth > 2.61" for 2YR event

Inflow = 1.15 cfs @ 12.09 hrs, Volume= 3,791 cf

Outflow = 1.15 cfs @ 12.09 hrs, Volume= 3,791 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.15 cfs @ 12.09 hrs, Volume= 3,791 cf

Routed to Pond D21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.78' @ 12.09 hrs

Flood Elev= 207.68'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.19'	15.0" Round Culvert L= 63.5' Ke= 0.500
			Inlet / Outlet Invert= 204.19' / 203.87' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.12 cfs @ 12.09 hrs HW=204.78' TW=204.06' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.12 cfs @ 2.90 fps)

Summary for Pond D21: DMH #21

Inflow Area = 71,317 sf, 79.77% Impervious, Inflow Depth > 2.52" for 2YR event

Inflow = 4.53 cfs @ 12.09 hrs, Volume= 15,002 cf

Outflow = 4.53 cfs @ 12.09 hrs, Volume= 15,002 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.53 cfs @ 12.09 hrs, Volume= 15,002 cf

Routed to Pond p212: INFILTRATION POND #1

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 87

Peak Elev= 204.08' @ 12.09 hrs Flood Elev= 207.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.02'	24.0" Round Culvert L= 72.4' Ke= 0.500 Inlet / Outlet Invert= 203.02' / 202.66' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=4.42 cfs @ 12.09 hrs HW=204.06' TW=200.94' (Dynamic Tailwater) 1=Culvert (Barrel Controls 4.42 cfs @ 3.89 fps)

Summary for Pond D22: DMH #22

Inflow Area = 24,814 sf, 89.39% Impervious, Inflow Depth > 2.76" for 2YR event

Inflow 1.67 cfs @ 12.09 hrs, Volume= 5.708 cf

1.67 cfs @ 12.09 hrs, Volume= 5,708 cf, Atten= 0%, Lag= 0.0 min 5,708 cf Outflow

Primary = 1.67 cfs @ 12.09 hrs, Volume=

Routed to Pond d21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.54' @ 12.09 hrs

Flood Elev= 208.46'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.87'	15.0" Round Culvert L= 134.2' Ke= 0.500 Inlet / Outlet Invert= 204.87' / 203.92' S= 0.0071 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 1.23 sf

Primary OutFlow Max=1.63 cfs @ 12.09 hrs HW=205.53' TW=204.06' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.63 cfs @ 3.64 fps)

Summary for Pond D23: DMH #23

Inflow Area = 10,771 sf, 99.27% Impervious, Inflow Depth > 3.04" for 2YR event

0.77 cfs @ 12.09 hrs, Volume= 2,724 cf Inflow =

0.77 cfs @ 12.09 hrs, Volume= Outflow 2,724 cf, Atten= 0%, Lag= 0.0 min

0.77 cfs @ 12.09 hrs, Volume= Primary = 2,724 cf

Routed to Pond D22: DMH #22

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.11' @ 12.09 hrs

Flood Elev= 210.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.70'	15.0" Round Culvert L= 173.3' Ke= 0.500 Inlet / Outlet Invert= 206.70' / 204.97' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=0.75 cfs @ 12.09 hrs HW=207.11' TW=205.52' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.75 cfs @ 3.20 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 88

Summary for Pond D25: DMH #25

Inflow Area = 36,995 sf, 87.96% Impervious, Inflow Depth > 2.63" for 2YR event

Inflow = 2.34 cfs @ 12.09 hrs, Volume= 8,121 cf

Outflow = 2.34 cfs @ 12.09 hrs, Volume= 8,121 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.34 cfs @ 12.09 hrs, Volume= 8,121 cf

Routed to Pond P210: POCKET WETLAND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.19' @ 12.09 hrs

Flood Elev= 213.11'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.36'	15.0" Round Culvert L= 237.6' Ke= 0.500 Inlet / Outlet Invert= 209.36' / 208.17' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.28 cfs @ 12.09 hrs HW=210.18' TW=202.37' (Dynamic Tailwater) 1=Culvert (Barrel Controls 2.28 cfs @ 3.78 fps)

Summary for Pond D27: DMH #27

Inflow Area = 21,746 sf, 79.51% Impervious, Inflow Depth > 2.35" for 2YR event

Inflow = 1.26 cfs @ 12.09 hrs, Volume= 4,263 cf

Outflow = 1.26 cfs @ 12.09 hrs, Volume= 4,263 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.26 cfs @ 12.09 hrs, Volume= 4,263 cf

Routed to Pond D35: DMH #35

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 213.88' @ 12.09 hrs

Flood Elev= 217.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.34'	15.0" Round Culvert L= 63.9' Ke= 0.500
			Inlet / Outlet Invert= 213.34' / 212.38' S= 0.0150 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.23 cfs @ 12.09 hrs HW=213.87' TW=212.81' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.23 cfs @ 2.48 fps)

Summary for Pond D28: DMH #28

Inflow Area = 11,949 sf, 62.72% Impervious, Inflow Depth > 1.79" for 2YR event

Inflow = 0.56 cfs @ 12.09 hrs, Volume= 1,785 cf

Outflow = 0.56 cfs @ 12.09 hrs, Volume= 1,785 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.56 cfs @ 12.09 hrs, Volume= 1,785 cf

Routed to Pond D27: DMH #27

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 89

Peak Elev= 217.83' @ 12.09 hrs Flood Elev= 220.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	217.46'	12.0" Round Culvert L= 158.3' Ke= 0.500 Inlet / Outlet Invert= 217.46' / 214.29' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.55 cfs @ 12.09 hrs HW=217.83' TW=213.87' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.55 cfs @ 2.07 fps)

Summary for Pond D29: DMH #29

Inflow Area = 11,949 sf, 62.72% Impervious, Inflow Depth > 1.79" for 2YR event

Inflow 0.56 cfs @ 12.09 hrs, Volume= 1.785 cf

0.56 cfs @ 12.09 hrs, Volume= Outflow 1,785 cf, Atten= 0%, Lag= 0.0 min =

1,705 cl, 1,785 cf Primary = 0.56 cfs @ 12.09 hrs, Volume=

Routed to Pond D28: DMH #28

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 220.20' @ 12.09 hrs

Flood Elev= 223.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	219.83'	12.0" Round Culvert L= 150.9' Ke= 0.500
	-		Inlet / Outlet Invert= 219.83' / 217.55' S= 0.0151 '/' Cc= 0.900
			n= 0.013 Corrugated PE_smooth interior_Flow Area= 0.79 sf

Primary OutFlow Max=0.55 cfs @ 12.09 hrs HW=220.20' TW=217.83' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.55 cfs @ 2.07 fps)

Summary for Pond D30: DMH #30

Inflow Area = 6,862 sf, 50.73% Impervious, Inflow Depth > 1.45" for 2YR event

0.26 cfs @ 12.10 hrs, Volume= Inflow = 832 cf

Outflow 0.26 cfs @ 12.10 hrs, Volume= 832 cf, Atten= 0%, Lag= 0.0 min

0.26 cfs @ 12.10 hrs, Volume= Primary = 832 cf

Routed to Pond D29: DMH #29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 221.21' @ 12.10 hrs

Flood Elev= 224.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	220.92'	12.0" Round Culvert L= 184.2' Ke= 0.500 Inlet / Outlet Invert= 220.92' / 220.00' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.26 cfs @ 12.10 hrs HW=221.21' TW=220.20' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.26 cfs @ 2.04 fps)

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 90

Summary for Pond D31: DMH#31

Inflow Area = 63,140 sf, 28.73% Impervious, Inflow Depth > 0.97" for 2YR event

Inflow = 1.18 cfs @ 12.17 hrs, Volume= 5,104 cf

Outflow = 1.18 cfs @ 12.17 hrs, Volume= 5,104 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.18 cfs @ 12.17 hrs, Volume= 5,104 cf

Routed to Pond D4: DMH#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 225.15' @ 12.17 hrs

Flood Elev= 229.59'

Device	Routing	Invert	Outlet Devices
#1	Primary	224.63'	15.0" Round Culvert L= 288.5' Ke= 0.500 Inlet / Outlet Invert= 224.63' / 213.09' S= 0.0400 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.15 cfs @ 12.17 hrs HW=225.14' TW=210.96' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.15 cfs @ 2.44 fps)

Summary for Pond D34: DMH #34

Inflow Area = 23,255 sf,100.00% Impervious, Inflow Depth > 3.04" for 2YR event

Inflow = 1.66 cfs @ 12.09 hrs, Volume= 5,882 cf

Outflow = 1.66 cfs @ 12.09 hrs, Volume= 5,882 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.66 cfs @ 12.09 hrs, Volume= 5,882 cf

Routed to Pond OCS1: OCS#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 198.77' @ 12.09 hrs

Flood Elev= 202.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	198.07'	12.0" Round Culvert L= 52.0' Ke= 0.500 Inlet / Outlet Invert= 198.07' / 197.03' S= 0.0200' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.61 cfs @ 12.09 hrs HW=198.75' TW=195.74' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.61 cfs @ 2.82 fps)

Summary for Pond D35: DMH #35

Inflow Area = 21,746 sf, 79.51% Impervious, Inflow Depth > 2.35" for 2YR event

Inflow = 1.26 cfs @ 12.09 hrs, Volume= 4,263 cf

Outflow = 1.26 cfs @ 12.09 hrs, Volume= 4,263 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.26 cfs @ 12.09 hrs, Volume= 4,263 cf

Routed to Pond D25: DMH #25

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 91

Peak Elev= 212.82' @ 12.09 hrs

Flood Elev= 215.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	212.28'	15.0" Round Culvert L= 171.5' Ke= 0.500 Inlet / Outlet Invert= 212.28' / 209.71' S= 0.0150 '/' Cc= 0.900 n= 0.012. Flow Area= 1.23 sf

Primary OutFlow Max=1.23 cfs @ 12.09 hrs HW=212.81' TW=210.18' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.23 cfs @ 2.48 fps)

Summary for Pond D4: DMH#4

Inflow Area = 73,240 sf, 37.72% Impervious, Inflow Depth > 1.22" for 2YR event

Inflow = 1.72 cfs @ 12.13 hrs, Volume= 7,465 cf

Outflow = 1.72 cfs @ 12.13 hrs, Volume= 7,465 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.72 cfs @ 12.13 hrs, Volume= 7,465 cf

Routed to Pond D2: DMH#2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.98' @ 12.13 hrs

Flood Elev= 217.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	210.34'	15.0" Round Culvert L= 222.3' Ke= 0.500
			Inlet / Outlet Invert= 210.34' / 207.01' S= 0.0150 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior. Flow Area= 1.23 sf

Primary OutFlow Max=1.69 cfs @ 12.13 hrs HW=210.97' TW=207.57' (Dynamic Tailwater) —1=Culvert (Inlet Controls 1.69 cfs @ 2.71 fps)

Summary for Pond D5: DMH #5

Inflow Area = 30,441 sf, 96.76% Impervious, Inflow Depth > 2.98" for 2YR event

Inflow = 2.15 cfs @ 12.09 hrs, Volume= 7,549 cf

Outflow = 2.15 cfs @ 12.09 hrs, Volume= 7,549 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.15 cfs @ 12.09 hrs, Volume= 7,549 cf

Routed to Pond D6: DMH #6

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 209.86' @ 12.09 hrs

Flood Elev= 212.97'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.09'	18.0" Round Culvert L= 183.0' Ke= 0.500 Inlet / Outlet Invert= 209.09' / 208.17' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=209.85' TW=208.81' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.09 cfs @ 3.39 fps)

Type III 24-hr 2YR Rainfall=3.27"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 92

Summary for Pond D6: DMH #6

Inflow Area = 30,441 sf, 96.76% Impervious, Inflow Depth > 2.98" for 2YR event

Inflow = 2.15 cfs @ 12.09 hrs, Volume= 7,549 cf

Outflow = 2.15 cfs @ 12.09 hrs, Volume= 7,549 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.15 cfs @ 12.09 hrs, Volume= 7,549 cf

Routed to Pond D7: DMH #7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 208.82' @ 12.09 hrs

Flood Elev= 214.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	208.07'	18.0" Round Culvert L= 299.7' Ke= 0.500 Inlet / Outlet Invert= 208.07' / 206.57' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=208.81' TW=207.13' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.09 cfs @ 3.51 fps)

Summary for Pond D7: DMH #7

Inflow Area = 30,441 sf, 96.76% Impervious, Inflow Depth > 2.98" for 2YR event

Inflow = 2.15 cfs @ 12.09 hrs, Volume= 7,549 cf

Outflow = 2.15 cfs @ 12.09 hrs, Volume= 7,549 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.15 cfs @ 12.09 hrs, Volume= 7,549 cf

Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.14' @ 12.09 hrs

Flood Elev= 213.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.47'	18.0" Round Culvert L= 44.2' Ke= 0.500 Inlet / Outlet Invert= 206.47' / 204.04' S= 0.0550 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=207.13' TW=200.93' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.09 cfs @ 2.77 fps)

Summary for Pond D8: DMH #8

Inflow Area = 18,765 sf, 91.12% Impervious, Inflow Depth > 2.58" for 2YR event

Inflow = 1.20 cfs @ 12.09 hrs, Volume= 4,028 cf

Outflow = 1.20 cfs @ 12.09 hrs, Volume= 4,028 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.20 cfs @ 12.09 hrs, Volume= 4,028 cf

Routed to Pond D9: DMH #9

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 93

Peak Elev= 201.27' @ 12.09 hrs Flood Elev= 204.72'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.57'	12.0" Round Culvert L= 87.7' Ke= 0.500 Inlet / Outlet Invert= 200.57' / 200.13' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.17 cfs @ 12.09 hrs HW=201.26' TW=200.72' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.17 cfs @ 2.87 fps)

Summary for Pond D9: DMH #9

Inflow Area = 18,765 sf, 91.12% Impervious, Inflow Depth > 2.58" for 2YR event
Inflow = 1.20 cfs @ 12.09 hrs, Volume= 4,028 cf
Outflow = 1.20 cfs @ 12.09 hrs, Volume= 4,028 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.20 cfs @ 12.09 hrs, Volume= 4,028 cf
Routed to Pond P207 : INFILTRATION POND #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 200.73' @ 12.09 hrs Flood Elev= 204.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.03'	12.0" Round Culvert L= 11.9' Ke= 0.500
			Inlet / Outlet Invert= 200.03' / 199.97' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PF smooth interior Flow Area= 0.79 sf

Primary OutFlow Max=1.17 cfs @ 12.09 hrs HW=200.72' TW=197.12' (Dynamic Tailwater) —1=Culvert (Barrel Controls 1.17 cfs @ 2.84 fps)

Summary for Pond DE61: DRIP #61

Inflow Area =	4,247 sf, 92.68% Impervious,	Inflow Depth > 2.82" for 2YR event
Inflow =	0.29 cfs @ 12.09 hrs, Volume=	997 cf
Outflow =	0.24 cfs @ 12.15 hrs, Volume=	857 cf, Atten= 16%, Lag= 3.5 min
Discarded =	0.00 cfs @ 7.35 hrs, Volume=	181 cf
Primary =	0.24 cfs @ 12.15 hrs, Volume=	676 cf
Routed to Rea	ch 8R : OVERLAND FLOW	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 213.08' @ 12.15 hrs Surf.Area= 665 sf Storage= 238 cf

Plug-Flow detention time= 100.3 min calculated for 857 cf (86% of inflow) Center-of-Mass det. time= 39.1 min (812.1 - 773.0)

Volume	Invert	Avail.Storage	Storage Description
#1	212.19'	539 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 94

Elevation	Surf.Area	Voids	Inc.Store	Cum.Store
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)
212.19	665	0.0	0	0
212.20	665	40.0	3	3
214.19	665	40.0	529	532
214.20	665	100.0	7	539

Device	Routing	Invert	Outlet Devices
#1	Primary	214.10'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	212.70'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 212.70' / 212.65' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	212.19'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 7.35 hrs HW=212.21' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.24 cfs @ 12.15 hrs HW=213.08' TW=208.02' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.24 cfs @ 2.06 fps)

Summary for Pond DE62: DRIP #62

Inflow Area =	4,247 sf	, 92.68% Impervious,	Inflow Depth > 2.82" for 2YR event
Inflow =	0.29 cfs @	12.09 hrs, Volume=	997 cf
Outflow =	0.24 cfs @	12.15 hrs, Volume=	857 cf, Atten= 16%, Lag= 3.5 min
Discarded =	0.00 cfs @	7.35 hrs, Volume=	181 cf
Primary =	0.24 cfs @	12.15 hrs, Volume=	676 cf
Routed to Read	ch 8R : OVER	RLAND FLOW	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 213.08' @ 12.15 hrs Surf.Area= 665 sf Storage= 238 cf

Plug-Flow detention time= 100.3 min calculated for 857 cf (86% of inflow) Center-of-Mass det. time= 39.1 min (812.1 - 773.0)

volume	invert	Ava	II.Storage	Storage Descrip	tion		
#1	212.19'		539 cf	Custom Stage I	Data (Prismatic	Listed below (Recalc)	
Elevation (feet)	Surf. <i>i</i> (s	Area sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
212.19		665	0.0	0	0		
212.20		665	40.0	3	3		
214.19		665	40.0	529	532		
214.20		665	100.0	7	539		

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 95

Device	Routing	Invert	Outlet Devices
#1	Primary	214.10'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	212.70'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 212.70' / 212.65' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	212.19'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 7.35 hrs HW=212.21' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.24 cfs @ 12.15 hrs HW=213.08' TW=208.02' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.24 cfs @ 2.06 fps)

Summary for Pond DE63: DRIP #63

Inflow Area = 3,013 sf, 88.68% Impervious, Inflow Depth > 2.71" for 2YR event

Inflow = 0.20 cfs @ 12.09 hrs, Volume= 681 cf

Outflow = 0.18 cfs @ 12.13 hrs, Volume= 596 cf, Atten= 11%, Lag= 2.6 min

Discarded = 0.00 cfs @ 7.45 hrs, Volume= 107 cf Primary = 0.18 cfs @ 12.13 hrs, Volume= 488 cf

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.82' @ 12.13 hrs Surf.Area= 404 sf Storage= 135 cf

Plug-Flow detention time= 90.3 min calculated for 596 cf (87% of inflow)

Center-of-Mass det. time= 33.7 min (813.5 - 779.9)

Volume	Invert Av	/ail.Storage	Storage Descrip	otion	
#1	206.99'	327 cf	Custom Stage	Data (Prismatic	Listed below (Recalc)
Elevation	Surf.Are		Inc.Store	Cum.Store	
(feet)	(sq-fi	(%)	(cubic-feet)	(cubic-feet)	
206.99	40	4 0.0	0	0	
207.00	40	4 40.0	2	2	
208.99	40	4 40.0	322	323	
209.00	40	4 100.0	4	327	

Device	Routing	Invert	Outlet Devices
#1	Primary	208.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	207.50'	6.0" Round Culvert L= 10.0' Ke= 0.500
	•		Inlet / Outlet Invert= 207.50' / 207.45' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	206.99'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 96

Discarded OutFlow Max=0.00 cfs @ 7.45 hrs HW=207.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.18 cfs @ 12.13 hrs HW=207.82' TW=202.04' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.18 cfs @ 1.90 fps)

Summary for Pond DE64: DRIP #64

Inflow Area = 3,470 sf, 91.59% Impervious, Inflow Depth > 2.82" for 2YR event

Inflow = 0.24 cfs @ 12.09 hrs, Volume= 814 cf

Outflow = 0.21 cfs @ 12.13 hrs, Volume= 715 cf, Atten= 12%, Lag= 2.8 min

Discarded = 0.00 cfs @ 7.05 hrs, Volume= 129 cf Primary = 0.21 cfs @ 12.13 hrs, Volume= 586 cf

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.85' @ 12.13 hrs Surf.Area= 470 sf Storage= 162 cf

Plug-Flow detention time= 90.8 min calculated for 715 cf (88% of inflow)

Center-of-Mass det. time= 35.0 min (808.0 - 773.0)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	tion	
#1	204.9	99'	381 cf	Custom Stage	Data (Prismatic)L	isted below (Recalc)
Elevation		Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
204.9	99	470	0.0	0	0	
205.0	00	470	40.0	2	2	
206.9	99	470	40.0	374	376	
207.0	00	470	100.0	5	381	
Device	Routing	In	vert Out	let Devices		
#1	Primary	206	3.90' 180	.0' long x 0.5' br	eadth Broad-Cres	sted Rectangular Weir
			Hea	ad (feet) 0.20 0.4	0 0.60 0.80 1.00	<u>-</u>
				` '	2.92 3.08 3.30 3	
#2	Drimary	205		`	1 - 10 0' Ka- 0 4	

#1	Primary	206.90	180.0 long x 0.5 breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	205.50'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 205.50' / 205.45' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	204.99'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 7.05 hrs HW=205.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.20 cfs @ 12.13 hrs HW=205.85' TW=202.04' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.20 cfs @ 1.97 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 97

Summary for Pond DE65: DRIP #65

Inflow Area = 3,016 sf, 88.69% Impervious, Inflow Depth > 2.71" for 2YR event

Inflow = 0.20 cfs @ 12.09 hrs, Volume= 682 cf

Outflow = 0.18 cfs @ 12.13 hrs, Volume= 596 cf, Atten= 11%, Lag= 2.6 min

Discarded = 0.00 cfs @ 7.45 hrs, Volume= 107 cf Primary = 0.18 cfs @ 12.13 hrs, Volume= 489 cf

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.82' @ 12.13 hrs Surf.Area= 404 sf Storage= 135 cf

Plug-Flow detention time= 89.4 min calculated for 595 cf (87% of inflow)

Center-of-Mass det. time= 33.6 min (813.5 - 779.9)

Volume	Inve	ert Ava	il.Storage	Storage Descrip	otion	
#1	205.9	99'	327 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
		0 ()			0 01	
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
205.9	99	404	0.0	0	0	
206.0	00	404	40.0	2	2	
207.9	99	404	40.0	322	323	
208.0	00	404	100.0	4	327	
Device	Routing	In	vert Out	tlet Devices		
#1	Primary	207	7.90' 180	0.0' long x 0.5' bi	readth Broad-Cre	ested Rectangular Weir
	,			ad (feet) 0.20 0.4		
				ef. (English) 2.80		
#2	Primary	206		" Round Culvert		
	, ,					S= 0.0050 '/' Cc= 0.900
				· -		rior, Flow Area= 0.20 sf
#3	Discarde	ed 205				area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 7.45 hrs HW=206.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.18 cfs @ 12.13 hrs HW=206.82' TW=202.04' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.18 cfs @ 1.90 fps)

Summary for Pond DE66: DRIP #66

Inflow Area = 3,407 sf, 91.46% Impervious, Inflow Depth > 2.82" for 2YR event

Inflow = 0.23 cfs @ 12.09 hrs, Volume= 800 cf

Outflow = 0.21 cfs @ 12.13 hrs, Volume= 700 cf, Atten= 12%, Lag= 2.8 min

Discarded = 0.00 cfs @ 6.25 hrs, Volume= 129 cf Primary = 0.20 cfs @ 12.13 hrs, Volume= 571 cf

Routed to Reach 12R: OVERLAND FLOW

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 98

Peak Elev= 208.65' @ 12.13 hrs Surf.Area= 470 sf Storage= 161 cf

Plug-Flow detention time= 91.7 min calculated for 700 cf (88% of inflow)

Center-of-Mass det. time= 35.3 min (808.3 - 773.0)

Volume	Inve	ert Ava	il.Storage	e Storage Descr	iption	
#1	207.7	79'	381 c	f Custom Stage	e Data (Prismatio	c)Listed below (Recalc)
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
207.7	79	470	0.0	0	0	
207.8	30	470	40.0	2	2	
209.7	79	470	40.0	374	376	
209.8	30	470	100.0	5	381	
Device	Routing	In	vert Ou	ıtlet Devices		
#1	Primary	209	9.70' 18	0.0' long x 0.5' k	readth Broad-C	rested Rectangular Weir
	,			ead (feet) 0.20 0.		
				ef. (English) 2.80		
#2	Primary	208		" Round Culver		
						S= 0.0050 '/' Cc= 0.900
						erior, Flow Area= 0.20 sf
#3	Discarde	ed 207			•	e area Phase-In= 0.01'
#3	Discarde	ed 207	'.79' 0. '	i /U in/hr Extiltrat	tion over Surfac	e area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 6.25 hrs HW=207.80' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.20 cfs @ 12.13 hrs HW=208.64' TW=202.04' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.20 cfs @ 1.96 fps)

Summary for Pond DE67: DRIP #67

Inflow Area =	3,481 sf, 91.61% Impervious,	Inflow Depth > 2.82" for 2YR event
Inflow =	0.24 cfs @ 12.09 hrs, Volume=	817 cf
Outflow =	0.21 cfs @ 12.13 hrs, Volume=	718 cf, Atten= 12%, Lag= 2.8 min
Discarded =	0.00 cfs @ 7.00 hrs, Volume=	129 cf
Primary =	0.21 cfs @ 12.13 hrs, Volume=	588 cf

Routed to Reach 8R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 208.85' @ 12.13 hrs Surf.Area= 470 sf Storage= 162 cf

Plug-Flow detention time= 89.9 min calculated for 716 cf (88% of inflow) Center-of-Mass det. time= 35.0 min (807.9 - 773.0)

Volume	Invert	Avail.Storage	Storage Description
#1	207.99'	381 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 99

Elevation	Surf.Area	Voids	Inc.Store	Cum.Store
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)
207.99	470	0.0	0	0
208.00	470	40.0	2	2
209.99	470	40.0	374	376
210.00	470	100.0	5	381

Device	Routing	Invert	Outlet Devices
#1	Primary	209.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	208.50'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 208.50' / 208.45' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	207.99'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 7.00 hrs HW=208.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.21 cfs @ 12.13 hrs HW=208.85' TW=208.02' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.21 cfs @ 1.98 fps)

Summary for Pond DE68: DRIP #68

Inflow Area =	4,212 sf	, 92.62% Impervious,	Inflow Depth > 2.82" for 2YR event				
Inflow =	0.29 cfs @	12.09 hrs, Volume=	989 cf				
Outflow =	0.25 cfs @	12.14 hrs, Volume=	850 cf, Atten= 13%, Lag= 3.0 min				
Discarded =	0.00 cfs @	7.35 hrs, Volume=	181 cf				
Primary =	0.25 cfs @	12.14 hrs, Volume=	669 cf				
Routed to Pond OCS4 : OCS#4							

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.82' @ 12.14 hrs Surf.Area= 665 sf Storage= 220 cf

Plug-Flow detention time= 97.1 min calculated for 848 cf (86% of inflow) Center-of-Mass det. time= 37.0 min (810.0 - 773.0)

Volume	Invert Ava	ail.Storage	Storage Descrip	otion	
#1	206.99'	539 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
Elevation	Surf.Area	Voids	Inc.Store	Cum.Store	
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
206.99	665	0.0	0	0	
207.00	665	40.0	3	3	
208.99	665	40.0	529	532	
209.00	665	100.0	7	539	

#3

Discarded

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 100

Device	Routing	Invert	Outlet Devices
#1	Primary	208.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	207.50'	6.0" Round Culvert L= 20.0' Ke= 0.500
			Inlet / Outlet Invert= 207.50' / 206.00' S= 0.0750 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	206.99'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 7.35 hrs HW=207.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.25 cfs @ 12.14 hrs HW=207.81' TW=203.64' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Inlet Controls 0.25 cfs @ 1.90 fps)

Summary for Pond DE69: DRIP #69

Inflow Area = 3,480 sf, 91.61% Impervious, Inflow Depth > 2.82" for 2YR event
Inflow = 0.24 cfs @ 12.09 hrs, Volume= 817 cf
Outflow = 0.21 cfs @ 12.13 hrs, Volume= 717 cf, Atten= 12%, Lag= 2.8 min
Discarded = 0.00 cfs @ 7.00 hrs, Volume= 129 cf
Primary = 0.21 cfs @ 12.13 hrs, Volume= 588 cf
Routed to Pond P212 : INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 206.35' @ 12.13 hrs Surf.Area= 470 sf Storage= 162 cf

Plug-Flow detention time= 89.9 min calculated for 716 cf (88% of inflow) Center-of-Mass det. time= 35.0 min (807.9 - 773.0)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion		
#1	205.	49'	381 cf	Custom Stage	Data (Prismatic) Listed below (I	Recalc)
Elevatio		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
205.4		470	0.0	0	0		
205.5	. •	470	40.0	2	2		
207.4	19	470	40.0	374	376		
207.5	50	470	100.0	5	381		
Device	Routing	In	vert Ou	tlet Devices			
#1	Primary	207	7.40' 180	0.0' long x 0.5' br	eadth Broad-Cr	ested Rectang	gular Weir
				ad (feet) 0.20 0.4			
#2	Primary	206	6. 00' 6.0	ef. (English) 2.80 " Round Culvert et / Outlet Invert= 2	L= 10.0' Ke= 0).500	Cc= 0.900

n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

205.49' **0.170** in/hr Exfiltration over Surface area Phase-In= 0.01'

207.90

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 101

Discarded OutFlow Max=0.00 cfs @ 7.00 hrs HW=205.51' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.21 cfs @ 12.13 hrs HW=206.35' TW=201.08' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.21 cfs @ 1.98 fps)

Summary for Pond DE70: DRIP #70

Inflow Area = 3,476 sf, 91.60% Impervious, Inflow Depth > 2.82" for 2YR event Inflow 0.24 cfs @ 12.09 hrs, Volume= 816 cf Outflow 0.21 cfs @ 12.13 hrs, Volume= 716 cf. Atten= 12%, Lag= 2.8 min Discarded = 6.20 hrs, Volume= 0.00 cfs @ 129 cf 0.21 cfs @ 12.13 hrs, Volume= 587 cf Primary Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 206.75' @ 12.13 hrs Surf.Area= 470 sf Storage= 162 cf

Plug-Flow detention time= 90.7 min calculated for 716 cf (88% of inflow) Center-of-Mass det. time= 35.0 min (808.0 - 773.0)

Volume Avail.Storage Storage Description Invert #1 Custom Stage Data (Prismatic)Listed below (Recalc) 205.89' Surf.Area Voids Inc.Store Elevation Cum.Store (feet) (sq-ft) (%)(cubic-feet) (cubic-feet) 205.89 470 0.0 0 0 2 205.90 470 40.0 2 207.89 376 470 40.0 374

5

381

Device	Routing	Invert	Outlet Devices
#1	Primary	207.80'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	206.40'	6.0" Round Culvert L= 10.0' Ke= 0.500
	-		Inlet / Outlet Invert= 206.40' / 206.35' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	205.89'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 6.20 hrs HW=205.90' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.21 cfs @ 12.13 hrs HW=206.75' TW=201.08' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.21 cfs @ 1.98 fps)

470

100.0

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 102

Summary for Pond DE71: DRIP #71

Inflow Area = 4,210 sf, 92.61% Impervious, Inflow Depth > 2.82" for 2YR event

Inflow 0.29 cfs @ 12.09 hrs, Volume= 988 cf

Outflow 0.24 cfs @ 12.15 hrs, Volume= 848 cf, Atten= 16%, Lag= 3.5 min

Discarded = 0.00 cfs @ 7.75 hrs, Volume= 181 cf 0.24 cfs @ 12.15 hrs, Volume= 667 cf Primary =

Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.38' @ 12.15 hrs Surf.Area= 665 sf Storage= 237 cf

Plug-Flow detention time= 100.7 min calculated for 848 cf (86% of inflow)

Center-of-Mass det. time= 39.3 min (812.3 - 773.0)

Volume	Inve	ert Avai	il.Storage	Storage Descrip	tion		
#1	206.4	.9'	805 cf	Custom Stage	Data (Prismatic) Lis	sted below (Recalc)	
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)		
206.4	19	665	0.0	0	0		
206.5	50	665	40.0	3	3		
209.4	19	665	40.0	795	798		
209.5	50	665	100.0	7	805		
Device	Routing	In	vert Out	et Devices			
#1	Primary	209	9.40' 180	.0' long x 0.5' br	eadth Broad-Crest	ted Rectangular Weir	_
	•			•	0 0.60 0.80 1.00	•	
			Coe	f. (English) 2.80	2.92 3.08 3.30 3.3	32	
#2	Primary	207	7.00' 6.0'	Round Culvert	L= 10.0' Ke= 0.50)0	
			Inle	t / Outlet Invert= 2	07.00' / 206.95' S=	= 0.0050 '/' Cc= 0.900	
				0	•	r, Flow Area= 0.20 sf	
#3	Discarde	d 206	6.49' 0.17	'0 in/hr Exfiltration	on over Surface ar	rea Phase-In= 0.01'	

Discarded OutFlow Max=0.00 cfs @ 7.75 hrs HW=206.52' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.24 cfs @ 12.15 hrs HW=207.38' TW=201.12' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.24 cfs @ 2.06 fps)

Summary for Pond DECH: DRIP #CH

Inflow Area = 6,262 sf, 92.70% Impervious, Inflow Depth > 2.82" for 2YR event

Inflow 0.43 cfs @ 12.09 hrs, Volume= 1,470 cf

0.28 cfs @ 12.19 hrs, Volume= 1,469 cf, Atten= 35%, Lag= 5.9 min Outflow

0.04 cfs @ 11.50 hrs, Volume= Discarded = 1,053 cf Primary = 0.25 cfs @ 12.19 hrs, Volume= 416 cf

Routed to Pond CB18: CB #18

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 103

Peak Elev= 209.01' @ 12.19 hrs Surf.Area= 636 sf Storage= 260 cf

Plug-Flow detention time= 19.9 min calculated for 1,469 cf (100% of inflow)

Center-of-Mass det. time= 19.7 min (792.7 - 773.0)

Volume	Inve	ert Ava	il.Storage	Storage Descrip	otion	
#1	207.9	99'	770 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
-		0 ()		. 0	0 01	
Elevation		Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
207.9	99	636	0.0	0	0	
208.0	00	636	40.0	3	3	
210.9	99	636	40.0	761	763	
211.0	00	636	100.0	6	770	
				-		
Device	Routing	In	vert Out	let Devices		
#1	Primary	210).90' 160	.0' long x 0.5' bi	readth Broad-Cre	ested Rectangular Weir
	,		Hea	ad (feet) 0.20 0.4	10 0.60 0.80 1.00	0
				` ,	2.92 3.08 3.30	
#2	Primary	208		` ' '	t L= 80.0' Ke= 0	
	,		Inle	t / Outlet Invert= 2	208.50' / 205.10'	S= 0.0425 '/' Cc= 0.900
				• -		ior, Flow Area= 0.09 sf
#3	Discarde	ed 207				area Phase-In= 0.01'

Discarded OutFlow Max=0.04 cfs @ 11.50 hrs HW=208.02' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.25 cfs @ 12.19 hrs HW=209.01' TW=205.27' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Inlet Controls 0.25 cfs @ 2.81 fps)

Summary for Pond DMH32: DMH #32

Inflow Area = 20,278 sf, 79.11% Impervious, Inflow Depth > 2.54" for 2YR event

Inflow = 1.31 cfs @ 12.09 hrs, Volume= 4,287 cf

Outflow = 1.31 cfs @ 12.09 hrs, Volume= 4,287 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.31 cfs @ 12.09 hrs, Volume= 4,287 cf

Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.19' @ 12.09 hrs

Flood Elev= 206.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.59'	12.0" Round Culvert L= 19.2' Ke= 0.500 Inlet / Outlet Invert= 202.59' / 201.57' S= 0.0531 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.27 cfs @ 12.09 hrs HW=203.18' TW=200.94' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.27 cfs @ 2.62 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 104

Summary for Pond OCS1: OCS#1

Inflow Area = 48,573 sf, 81.99% Impervious, Inflow Depth > 2.67" for 2YR event

Inflow = 3.17 cfs @ 12.09 hrs, Volume= 10,805 cf

Outflow = 3.17 cfs @ 12.09 hrs, Volume= 10,805 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.17 cfs @ 12.09 hrs, Volume= 10,805 cf Routed to Pond P206 : STORMTECH INFILTRATION SYSTEM #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 195.75' @ 12.09 hrs

Flood Elev= 201.48'

Device Routing Invert Outlet Devices

#1 Primary 195.00' **24.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=3.09 cfs @ 12.09 hrs HW=195.74' TW=195.23' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 3.09 cfs @ 2.93 fps)

Summary for Pond OCS3: OCS#3

Inflow Area = 54,250 sf, 81.93% Impervious, Inflow Depth > 2.37" for 2YR event

Inflow = 3.26 cfs @ 12.09 hrs, Volume= 10,698 cf

Outflow = 3.26 cfs @ 12.09 hrs, Volume= 10,698 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.26 cfs @ 12.09 hrs, Volume= 10,698 cf Routed to Pond p204 : STORMTECH INFILTRATION SYSTEM #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.95' @ 12.11 hrs

Flood Elev= 209.00'

Device Routing Invert Outlet Devices

#1 Primary 203.10' **18.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=3.20 cfs @ 12.09 hrs HW=203.94' TW=203.47' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 3.20 cfs @ 3.13 fps)

Summary for Pond OCS4: OCS#4

Inflow Area = 17,972 sf, 28.85% Impervious, Inflow Depth > 1.32" for 2YR event

Inflow = 0.64 cfs @ 12.11 hrs, Volume= 1,977 cf

Outflow = 0.64 cfs @ 12.11 hrs, Volume= 1,977 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.64 cfs @ 12.11 hrs, Volume= 1,977 cf Routed to Pond P204 : STORMTECH INFILTRATION SYSTEM #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.89' @ 12.49 hrs

Flood Elev= 208.00'

Device	Routing	Invert	Outlet Devices		
#1	Primary	203.10'	18.0" Vert. Orifice/Grate	C= 0.600	Limited to weir flow at low heads

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 105

Primary OutFlow Max=0.64 cfs @ 12.11 hrs HW=203.59' TW=203.52' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 0.64 cfs @ 1.29 fps)

Summary for Pond OCS6: OCS #6

Inflow Area = 16,111 sf, 93.77% Impervious, Inflow Depth > 2.89" for 2YR event

Inflow = 1.12 cfs @ 12.09 hrs, Volume= 3,877 cf

Outflow = 1.12 cfs @ 12.09 hrs, Volume= 3,877 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.12 cfs @ 12.09 hrs. Volume = 3,877 cf

Routed to Pond P213: Stormtech Infiltration System #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 201.93' @ 12.52 hrs

Flood Elev= 206.96'

Device Routing Invert Outlet Devices

#1 Primary 201.20' **12.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.10 cfs @ 12.09 hrs HW=201.78' TW=201.55' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 1.10 cfs @ 2.30 fps)

Summary for Pond OCS7: OCS #7

Inflow Area = 15,875 sf, 92.67% Impervious, Inflow Depth > 2.85" for 2YR event

Inflow = 1.10 cfs @ 12.09 hrs, Volume= 3,771 cf

Outflow = 1.10 cfs @ 12.09 hrs, Volume= 3,771 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.10 cfs @ 12.09 hrs, Volume= 3,771 cf

Routed to Pond P213: Stormtech Infiltration System #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 202.32' @ 12.09 hrs

Flood Elev= 206.47'

Device Routing Invert Outlet Devices

#1 Primary 201.78' **12.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.07 cfs @ 12.09 hrs HW=202.32' TW=201.55' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 1.07 cfs @ 2.49 fps)

Summary for Pond P204: STORMTECH INFILTRATION SYSTEM #1

Inflow Area = 72,222 sf, 68.72% Impervious, Inflow Depth > 2.11" for 2YR event

Inflow = 3.90 cfs @ 12.09 hrs, Volume= 12,675 cf

Outflow = 1.02 cfs @ 12.49 hrs, Volume= 10,996 cf, Atten= 74%, Lag= 23.8 min

Discarded = 0.09 cfs @ 10.60 hrs, Volume= 5,215 cf Primary = 0.93 cfs @ 12.49 hrs, Volume= 5,781 cf

Routed to Reach 20r: OVERLAND FLOW

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 106

Peak Elev= 203.89' @ 12.49 hrs Surf.Area= 5,670 sf Storage= 5,017 cf Flood Elev= 208.75' Surf.Area= 5,670 sf Storage= 13,379 cf

Plug-Flow detention time= 142.3 min calculated for 10,973 cf (87% of inflow)

Center-of-Mass det. time= 84.7 min (873.6 - 788.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	202.50'	5,923 cf	77.50'W x 67.70'L x 4.08'H STORMTECH SC-740
			21,423 cf Overall - 6,615 cf Embedded = 14,808 cf x 40.0% Voids
#2A	203.08'	6,615 cf	ADS_StormTech SC-740 +Capx 144 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			144 Chambers in 16 Rows
#3B	202.50'	427 cf	6.25'W x 67.70'L x 3.50'H ISOLATOR ROW
			1,481 cf Overall - 413 cf Embedded = 1,067 cf x 40.0% Voids
#4B	203.00'	413 cf	ADS_StormTech SC-740 +Cap x 9 Inside #3
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

13,379 cf Total Available Storage

Storage Group A created with Chamber Wizard Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	202.75'	15.0" Round Culvert L= 35.0' Ke= 0.500
	•		Inlet / Outlet Invert= 202.75' / 201.00' S= 0.0500 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf
#2	Device 1	204.75'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Device 1	203.25'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	202.50'	0.660 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.09 cfs @ 10.60 hrs HW=202.57' (Free Discharge) **4=Exfiltration** (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=0.93 cfs @ 12.49 hrs HW=203.88' TW=200.04' (Dynamic Tailwater)

-1=Culvert (Passes 0.93 cfs of 4.24 cfs potential flow)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.93 cfs @ 2.71 fps)

Summary for Pond P205: INFILTRATION POND #3

Inflow Area = 88,676 sf, 39.42% Impervious, Inflow Depth > 1.25" for 2YR event 2.27 cfs @ 12.12 hrs, Volume= Inflow 9.251 cf

Outflow 0.26 cfs @ 13.49 hrs, Volume= 9,244 cf, Atten= 89%, Lag= 82.2 min

Discarded = 0.26 cfs @ 13.49 hrs, Volume= 9.244 cf 0.00 cfs @ 0.00 hrs, Volume= Primary 0 cf

Routed to Reach 18R: OVERLAND FLOW

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

<u>Page 107</u>

Peak Elev= 205.79' @ 13.49 hrs Surf.Area= 4,584 sf Storage= 3,362 cf

Plug-Flow detention time= 120.6 min calculated for 9,244 cf (100% of inflow)

Center-of-Mass det. time= 120.1 min (954.0 - 833.9)

Volume	Inve	rt Avail.Sto	rage Storage	Description	
#1	205.0	0' 15,6	30 cf Custon	n Stage Data (P	rismatic)Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
205.0	00	3,939	0	0	
206.0	00	4,756	4,348	4,348	
207.0	00	5,627	5,192	9,539	
208.0	00	6,555	6,091	15,630	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	206.65'	15.0' long x	15.0' breadth B	road-Crested Rectangular Weir
			Head (feet) (0.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60
			Coef. (Englis	h) 2.68 2.70 2.	70 2.64 2.63 2.64 2.64 2.63
#2	Discarde	d 205.00'	2.410 in/hr E	xfiltration over	Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.26 cfs @ 13.49 hrs HW=205.79' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.26 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=205.00' TW=203.00' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond P206: STORMTECH INFILTRATION SYSTEM #2

Inflow Area = 59,746 sf, 80.70% Impervious, Inflow Depth > 2.64" for 2YR event Inflow = 3.88 cfs @ 12.09 hrs, Volume= 13,143 cf

Outflow = 0.49 cfs @ 11.75 hrs, Volume= 13,142 cf, Atten= 87%, Lag= 0.0 min Discarded = 0.49 cfs @ 11.75 hrs, Volume= 13,142 cf

Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routed to Link AP4 : ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 195.67' @ 12.66 hrs Surf.Area= 6,072 sf Storage= 4,118 cf

Plug-Flow detention time= 54.5 min calculated for 13,142 cf (100% of inflow) Center-of-Mass det. time= 54.4 min (832.0 - 777.6)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 108

Volume	Invert	Avail.Storage	Storage Description
#1A	194.67'	1,786 cf	39.50'W x 53.46'L x 3.33'H FIELD A
			7,038 cf Overall - 2,573 cf Embedded = 4,466 cf x 40.0% Voids
#2A	195.00'	2,573 cf	ADS_StormTech SC-740 +Cap x 56 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			56 Chambers in 8 Rows
#3B	194.67'	3,296 cf	58.50'W x 67.70'L x 3.33'H FIELD B
			13,201 cf Overall - 4,962 cf Embedded = 8,239 cf x 40.0% Voids
#4B	195.00'	4,962 cf	ADS_StormTech SC-740 +Cap x 108 Inside #3
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			108 Chambers in 12 Rows

12,616 cf Total Available Storage

Storage Group A created with Chamber Wizard Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	194.65'	18.0" Round Culvert L= 30.0' Ke= 0.200
	·		Inlet / Outlet Invert= 194.65' / 194.50' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf
#2	Device 1	195.85'	
#3	Discarded	194.67'	3.500 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.49 cfs @ 11.75 hrs HW=194.73' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.49 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=194.67' TW=0.00' (Dynamic Tailwater) 1=Culvert (Passes 0.00 cfs of 0.00 cfs potential flow)

2=Sharp-Crested Rectangular Weir(Controls 0.00 cfs)

Summary for Pond P207: INFILTRATION POND #2

Inflow Area =	129,716 sf, 63.13% Impervious,	Inflow Depth > 2.19" for 2YR event				
Inflow =	6.85 cfs @ 12.09 hrs, Volume=	23,672 cf				
Outflow =	1.00 cfs @ 12.71 hrs, Volume=	23,663 cf, Atten= 85%, Lag= 36.9 min				
Discarded =	0.96 cfs @ 12.71 hrs, Volume=	23,563 cf				
Primary =	0.04 cfs @ 12.71 hrs, Volume=	101 cf				
Routed to Reach 10R : OVERLAND FLOW						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 197.50' @ 12.71 hrs Surf.Area= 11,216 sf Storage= 7,511 cf

Plug-Flow detention time= 55.8 min calculated for 23,663 cf (100% of inflow) Center-of-Mass det. time= 55.6 min (856.6 - 801.0)

Volume	Invert	Avail.Storage	Storage Description
#1	196.80'	40,260 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 109

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
196.80	10,100	0	0
198.00	12,000	13,260	13,260
200.00	15,000	27,000	40,260
Device Routing	g Invert	Outlet Devices	

Device	Routing	Invert	Outlet Devices
#1	Primary	198.80'	20.0' long x 21.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Primary	194.75'	15.0" Round Culvert L= 40.0' Ke= 0.500
			Inlet / Outlet Invert= 194.75' / 194.55' S= 0.0050 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf
#3	Device 2	198.80'	6.0" x 6.0" Horiz. Orifice/Grate X 6.00 columns
			X 6 rows C= 0.600 in 48.0" x 48.0" Grate (56% open area)
			Limited to weir flow at low heads
#4	Device 2	197.40'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Discarded	196.80'	3.690 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.96 cfs @ 12.71 hrs HW=197.50' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 0.96 cfs)

Primary OutFlow Max=0.04 cfs @ 12.71 hrs HW=197.50' TW=192.01' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Passes 0.04 cfs of 8.49 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.10 fps)

Summary for Pond P210: POCKET WETLAND #1

Inflow Area = 62,582 sf, 52.00% Impervious, Inflow Depth > 1.95" for 2YR event

Inflow = 2.95 cfs @ 12.09 hrs, Volume= 10,193 cf

Outflow = 0.12 cfs @ 15.47 hrs, Volume= 4,736 cf, Atten= 96%, Lag= 203.0 min

Primary = 0.12 cfs @ 15.47 hrs, Volume= 4,736 cf

Routed to Reach 15R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Starting Elev= 201.00' Surf.Area= 376 sf Storage= 591 cf

Peak Elev= 202.97' @ 15.47 hrs Surf.Area= 5,971 sf Storage= 7,606 cf (7,015 cf above start)

Plug-Flow detention time= 467.1 min calculated for 4,136 cf (41% of inflow)

Center-of-Mass det. time= 282.9 min (1,072.6 - 789.6)

Volume	Invert	Avail.Storage	Storage Description
#1	199.00'	43,190 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 110

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
199.00	218	0	0
200.00	294	256	256
201.00	376	335	591
202.00	3,991	2,184	2,775
204.00	8,073	12,064	14,839
206.00	13,272	21,345	36,184
206.50	14,753	7,006	43,190

Device	Routing	Invert	Outlet Devices
#1	Primary	205.10'	20.0' long x 15.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Primary	202.25'	12.0" Round Culvert L= 44.0' Ke= 0.500
			Inlet / Outlet Invert= 202.25' / 202.03' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	202.30'	2.5" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	204.50'	6.0" x 6.0" Horiz. Orifice/Grate X 6.00 columns
			X 6 rows C= 0.600 in 48.0" x 48.0" Grate (56% open area)
			Limited to weir flow at low heads

Primary OutFlow Max=0.12 cfs @ 15.47 hrs HW=202.97' TW=202.04' (Dynamic Tailwater)

—1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Passes 0.12 cfs of 1.31 cfs potential flow)

-3=Orifice/Grate (Orifice Controls 0.12 cfs @ 3.62 fps)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond P212: INFILTRATION POND #1

Inflow Area = 241,078 sf, 59.10% Impervious, Inflow Depth > 1.84" for 2YR event
Inflow = 9.79 cfs @ 12.10 hrs, Volume= 37,028 cf
Outflow = 1.25 cfs @ 12.92 hrs, Volume= 37,018 cf, Atten= 87%, Lag= 49.6 min
Discarded = 1.25 cfs @ 12.92 hrs, Volume= 37,018 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Reach R211 : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 201.74' @ 12.92 hrs Surf.Area= 10,558 sf Storage= 14,075 cf

Plug-Flow detention time= 97.9 min calculated for 36,941 cf (100% of inflow) Center-of-Mass det. time= 97.5 min (895.7 - 798.2)

Volume	Invert	Avail.Storage	Storage Description
#1	200.00'	41,774 cf	Custom Stage Data (Conic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

<u>Page 111</u>

Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
200.00	4,354	0	0	4,354
201.00	9,360	6,699	6,699	9,368
202.00	10,993	10,166	16,865	11,040
204.00	13,976	24,909	41,774	14,126

Device	Routing	Invert	Outlet Devices
#1	Primary	202.50'	25.0' long x 20.0' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Discarded	200.00'	5.130 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=1.25 cfs @ 12.92 hrs HW=201.74' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 1.25 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=200.00' TW=200.00' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond P213: Stormtech Infiltration System #3

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

Inflow Area =	31,986 sf, 93.23% Impervious,	Inflow Depth > 2.87" for 2YR event
Inflow =	2.22 cfs @ 12.09 hrs, Volume=	7,648 cf
Outflow =	0.39 cfs @ 11.80 hrs, Volume=	7,647 cf, Atten= 82%, Lag= 0.0 min
Discarded =	0.39 cfs @ 11.80 hrs, Volume=	7,647 cf
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf
Routed to Pond	d P212 : INFILTRATION POND #1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 201.93' @ 12.54 hrs Surf.Area= 3,317 sf Storage= 1,973 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 27.1 min (795.0 - 767.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	200.95'	2,990 cf	49.00'W x 67.70'L x 3.50'H Field A
			11,610 cf Overall - 4,135 cf Embedded = 7,475 cf x 40.0% Voids
#2A	201.45'	4,135 cf	ADS_StormTech SC-740 +Cap x 90 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			90 Chambers in 10 Rows

7,125 cf Total Available Storage

Storage Group A created with Chamber Wizard

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 112

Device	Routing	Invert	Outlet Devices
#1	Primary	202.30'	12.0" Round Culvert L= 60.0' Ke= 0.500
	•		Inlet / Outlet Invert= 202.30' / 202.00' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	200.95'	5.130 in/hr Exfiltration over Surface area Phase-In= 0.01'
#3	Device 1	204.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Device 1	203.35'	6.0" W x 4.0" H Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.39 cfs @ 11.80 hrs HW=201.02' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.39 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=200.95' TW=200.00' (Dynamic Tailwater)

-1=Culvert (Controls 0.00 cfs)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond P214: STORMTECH INFILTRATION SYSTEM #4

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=2)

32,665 sf, 94.81% Impervious, Inflow Depth > 2.80" for 2YR event Inflow Area = Inflow 2.21 cfs @ 12.09 hrs, Volume= 7.633 cf

Outflow 0.61 cfs @ 11.90 hrs, Volume= 7,642 cf, Atten= 72%, Lag= 0.0 min

0.61 cfs @ 11.90 hrs, Volume= Discarded = 7.642 cf 0.00 cfs @ 0.00 hrs, Volume= 0 cf Primary =

Routed to Reach 9R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 200.78' @ 12.43 hrs Surf.Area= 3,201 sf Storage= 1,357 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 10.0 min (778.8 - 768.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	200.00'	1,851 cf	30.00'W x 67.70'L x 3.50'H Field A
			7,108 cf Overall - 2,481 cf Embedded = 4,627 cf x 40.0% Voids
#2A	200.50'	2,481 cf	ADS_StormTech SC-740 +Cap x 54 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			54 Chambers in 6 Rows
#3B	200.00'	1,087 cf	25.25'W x 46.34'L x 3.50'H Field B
			4,095 cf Overall - 1,378 cf Embedded = 2,717 cf x 40.0% Voids
#4B	200.50'	1,378 cf	
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			30 Chambers in 5 Rows
		6 707 cf	Total Available Storage

6,797 cf Total Available Storage

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 113

Device	Routing	Invert	Outlet Devices
#1	Primary	201.00'	12.0" Round Culvert L= 25.0' Ke= 0.500
	•		Inlet / Outlet Invert= 201.00' / 200.88' S= 0.0048 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	200.00'	8.280 in/hr Exfiltration over Surface area Phase-In= 0.01'
#3	Device 1	203.40'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Device 1	202.40'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.61 cfs @ 11.90 hrs HW=200.06' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.61 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=200.00' TW=201.75' (Dynamic Tailwater)

-1=Culvert (Controls 0.00 cfs)

3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Link AP1: ANALYSIS POINT 1

9,943 sf, 92.79% Impervious, Inflow Depth > 2.71" for 2YR event Inflow Area =

0.67 cfs @ 12.09 hrs, Volume= 2,247 cf Inflow

0.67 cfs @ 12.09 hrs, Volume= 2.247 cf. Atten= 0%. Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP2: ANALYSIS POINT 2

Inflow Area = 816,898 sf, 39.51% Impervious, Inflow Depth > 1.10" for 2YR event

7.31 cfs @ 12.40 hrs, Volume= 75.144 cf Inflow

75,144 cf, Atten= 0%, Lag= 0.0 min 7.31 cfs @ 12.40 hrs, Volume= Primary

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP3: ANALYSIS POINT 3

Inflow Area = 55,420 sf, 16.57% Impervious, Inflow Depth > 0.87" for 2YR event

1.15 cfs @ 12.10 hrs, Volume= Inflow 4.009 cf

1.15 cfs @ 12.10 hrs, Volume= 4,009 cf, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP4: ANALYSIS POINT #4

Inflow Area = 1,691,659 sf, 25.34% Impervious, Inflow Depth > 0.46" for 2YR event

Inflow 7.79 cfs @ 12.60 hrs, Volume= 65,174 cf

7.79 cfs @ 12.60 hrs, Volume= 65,174 cf. Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Tc=6.0 min CN=89 Runoff=0.93 cfs 3,014 cf

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 114

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

· · · · · · · · · · · · · · · · · · ·	
SubcatchmentB1: MULTIFAMILYBLDG	Runoff Area=23,255 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=2.53 cfs 9,148 cf
SubcatchmentB2: MULTIFAMILYBLDG	Runoff Area=17,561 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=1.91 cfs 6,908 cf
SubcatchmentB3: MULTIFAMILY	Runoff Area=19,981 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=2.17 cfs 7,860 cf
SubcatchmentC10: CB #10	Runoff Area=6,961 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.76 cfs 2,738 cf
SubcatchmentC11: CB #11	Runoff Area=7,173 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.78 cfs 2,822 cf
SubcatchmentC12: CB #12	Runoff Area=5,238 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.57 cfs 2,060 cf
SubcatchmentC13: CB #13	Runoff Area=10,873 sf 90.78% Impervious Runoff Depth>4.49" Tc=6.0 min CN=96 Runoff=1.16 cfs 4,068 cf
SubcatchmentC14: CB #14	Runoff Area=12,099 sf 86.22% Impervious Runoff Depth>3.94" Tc=6.0 min CN=91 Runoff=1.20 cfs 3,973 cf
SubcatchmentC15: CB #15	Runoff Area=6,666 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.73 cfs 2,622 cf
SubcatchmentC16: CB #16	Runoff Area=8,516 sf 64.88% Impervious Runoff Depth>2.76" Tc=6.0 min CN=79 Runoff=0.62 cfs 1,962 cf
SubcatchmentC17: CB #17	Runoff Area=11,836 sf 73.87% Impervious Runoff Depth>4.05" Tc=6.0 min CN=92 Runoff=1.20 cfs 3,992 cf
SubcatchmentC18: CB #18	Runoff Area=18,591 sf 66.35% Impervious Runoff Depth>3.83" Tc=6.0 min CN=90 Runoff=1.81 cfs 5,941 cf
SubcatchmentC20: CB #20	Runoff Area=11,939 sf 88.95% Impervious Runoff Depth>4.38" Tc=6.0 min CN=95 Runoff=1.26 cfs 4,355 cf
SubcatchmentC21: CB #21	Runoff Area=10,174 sf 87.04% Impervious Runoff Depth>3.83" Tc=6.0 min CN=90 Runoff=0.99 cfs 3,251 cf
SubcatchmentC22: CB #22	Runoff Area=12,001 sf 91.62% Impervious Runoff Depth>4.49" Tc=6.0 min CN=96 Runoff=1.28 cfs 4,490 cf
SubcatchmentC23: CB #23	Runoff Area=9,694 sf 61.00% Impervious Runoff Depth>3.73"

Prepared by Howard Stein Hudson Associates	
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC	

Printed 6/19/2023 Page 115

SubcatchmentC24: CB #24	Runoff Area=7,930 sf 72.16% Impervious Runoff Depth>4.16" Tc=6.0 min CN=93 Runoff=0.82 cfs 2,746 cf
SubcatchmentC25: CB #25	Runoff Area=8,487 sf 80.92% Impervious Runoff Depth>4.27" Tc=6.0 min CN=94 Runoff=0.89 cfs 3,017 cf
SubcatchmentC26: CB #26	Runoff Area=8,835 sf 63.75% Impervious Runoff Depth>3.94" Tc=6.0 min CN=91 Runoff=0.88 cfs 2,901 cf
SubcatchmentC27: CB #27	Runoff Area=6,111 sf 91.90% Impervious Runoff Depth>4.49" Tc=6.0 min CN=96 Runoff=0.65 cfs 2,287 cf
SubcatchmentC28: CB #28	Runoff Area=10,372 sf 51.33% Impervious Runoff Depth>3.63" Tc=6.0 min CN=88 Runoff=0.97 cfs 3,136 cf
SubcatchmentC29: CB #29	Runoff Area=8,495 sf 84.21% Impervious Runoff Depth>4.27" Tc=6.0 min CN=94 Runoff=0.89 cfs 3,020 cf
SubcatchmentC30: CB #30	Runoff Area=8,933 sf 82.40% Impervious Runoff Depth>4.27" Tc=6.0 min CN=94 Runoff=0.93 cfs 3,175 cf
SubcatchmentC31: CB #31	Runoff Area=16,365 sf 68.64% Impervious Runoff Depth>3.83" Tc=6.0 min CN=90 Runoff=1.59 cfs 5,230 cf
SubcatchmentC32: CB #32	Runoff Area=12,710 sf 70.47% Impervious Runoff Depth>3.94" Tc=6.0 min CN=91 Runoff=1.26 cfs 4,174 cf
SubcatchmentC33: CB #33	Runoff Area=5,421 sf 83.90% Impervious Runoff Depth>4.27" Tc=6.0 min CN=94 Runoff=0.57 cfs 1,927 cf
SubcatchmentC34: CB #34	Runoff Area=8,622 sf 80.51% Impervious Runoff Depth>4.16" Tc=6.0 min CN=93 Runoff=0.89 cfs 2,986 cf
SubcatchmentC35: CB #35	Runoff Area=4,149 sf 98.10% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.45 cfs 1,632 cf
SubcatchmentC36: CB #36	Runoff Area=6,622 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.72 cfs 2,605 cf
SubcatchmentC38: CB #38	Runoff Area=7,637 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.83 cfs 3,004 cf
SubcatchmentC39: CB #39	Runoff Area=7,612 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.83 cfs 2,994 cf
SubcatchmentC40: CB #40	Runoff Area=4,211 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.46 cfs 1,656 cf
SubcatchmentC41: CB #41	Runoff Area=5,586 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.61 cfs 2,197 cf

Prepared by Howard Stein H	ludson Associates
HydroCAD® 10.20-3c s/n 02930	© 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 116

SubcatchmentC43: CB #43	Runoff Area=3,109 sf 75.36% Impervious Runoff Depth>3.73" Tc=6.0 min CN=89 Runoff=0.30 cfs 967 cf
SubcatchmentC44: CB #44	Runoff Area=1,978 sf 84.43% Impervious Runoff Depth>4.05" Tc=6.0 min CN=92 Runoff=0.20 cfs 667 cf
SubcatchmentC45: CB #45	Runoff Area=2,465 sf 50.30% Impervious Runoff Depth>2.86" Tc=6.0 min CN=80 Runoff=0.19 cfs 587 cf
SubcatchmentC46: CB #46	Runoff Area=4,397 sf 50.97% Impervious Runoff Depth>2.86" Tc=6.0 min CN=80 Runoff=0.33 cfs 1,046 cf
SubcatchmentC47: CB #47	Runoff Area=3,012 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.33 cfs 1,185 cf
SubcatchmentC48: CB #48	Runoff Area=60,128 sf 25.16% Impervious Runoff Depth>2.00" Flow Length=400' Tc=11.8 min CN=70 Runoff=2.61 cfs 10,030 cf
SubcatchmentC49: CB #49	Runoff Area=5,238 sf 84.59% Impervious Runoff Depth>4.27" Tc=6.0 min CN=94 Runoff=0.55 cfs 1,862 cf
SubcatchmentC50: CB #50	Runoff Area=15,040 sf 77.20% Impervious Runoff Depth>4.16" Tc=6.0 min CN=93 Runoff=1.55 cfs 5,209 cf
SubcatchmentC51: CB #51	Runoff Area=6,823 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.74 cfs 2,684 cf
SubcatchmentC52: CB#52	Runoff Area=9,052 sf 87.14% Impervious Runoff Depth>4.38" Tc=6.0 min CN=95 Runoff=0.96 cfs 3,302 cf
SubcatchmentC53: CB #53	Runoff Area=7,863 sf 86.52% Impervious Runoff Depth>4.16" Tc=6.0 min CN=93 Runoff=0.81 cfs 2,723 cf
SubcatchmentC54: CB #54	Runoff Area=4,821 sf 86.85% Impervious Runoff Depth>3.94" Tc=6.0 min CN=91 Runoff=0.48 cfs 1,583 cf
SubcatchmentC7: CB #5	Runoff Area=4,650 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.51 cfs 1,829 cf
SubcatchmentC8: CB #8	Runoff Area=5,450 sf 88.75% Impervious Runoff Depth>4.27" Tc=6.0 min CN=94 Runoff=0.57 cfs 1,937 cf
SubcatchmentC9: CB #9	Runoff Area=16,307 sf 93.95% Impervious Runoff Depth>4.60" Tc=6.0 min CN=97 Runoff=1.76 cfs 6,257 cf
SubcatchmentCH1: CLUBHOUSE	Runoff Area=6,262 sf 92.70% Impervious Runoff Depth>4.49" Tc=6.0 min CN=96 Runoff=0.67 cfs 2,343 cf
SubcatchmentMB1: MAIL KIOSK	Runoff Area=938 sf 100.00% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.10 cfs 369 cf

19097 Post-Development	Type III 24-hr 10YR Rainfall=4.96
------------------------	-----------------------------------

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 117

SubcatchmentS201: SUMMER STREETRunoff Area=9,943 sf 92.79% Impervious Runoff Depth>4.38"
Tc=6.0 min CN=95 Runoff=1.05 cfs 3,627 cf

SubcatchmentS202: EXISTING WETLANDRunoff Area=432,269 sf 42.08% Impervious Runoff Depth>2.49" Flow Length=856' Tc=23.2 min CN=76 Runoff=18.32 cfs 89,722 cf

SubcatchmentS203: POCKET WETLAND#1 Runoff Area=25,587 sf 0.00% Impervious Runoff Depth>2.16"
Tc=6.0 min CN=72 Runoff=1.45 cfs 4,616 cf

SubcatchmentS204: EXISTING

Runoff Area=308,203 sf 31.07% Impervious Runoff Depth>2.41"
Flow Length=632' Tc=22.6 min CN=75 Runoff=12.75 cfs 61,782 cf

SubcatchmentS205: ISOLATEDWETLAND Runoff Area=55,420 sf 16.57% Impervious Runoff Depth>2.00"

Tc=6.0 min CN=70 Runoff=2.89 cfs 9.258 cf

SubcatchmentS206: OVERLANDFLOW Runoff Area=891,295 sf 2.91% Impervious Runoff Depth>1.61" Flow Length=1,467' Tc=34.5 min CN=65 Runoff=19.55 cfs 119,746 cf

SubcatchmentS207: INFILTRATIONPOND Runoff Area=20,803 sf 0.00% Impervious Runoff Depth>3.63" Tc=6.0 min CN=88 Runoff=1.94 cfs 6,290 cf

SubcatchmentS208: GRASS AREA Runoff Area=13,760 sf 9.33% Impervious Runoff Depth>2.41"

Tc=6.0 min CN=75 Runoff=0.88 cfs 2,769 cf

SubcatchmentS209: WETLANDC Runoff Area=107,073 sf 0.38% Impervious Runoff Depth>2.23" Flow Length=607' Slope=0.0150 '/' Tc=28.9 min CN=73 Runoff=3.68 cfs 19,940 cf

SubcatchmentS210: INFILTRATIONPOND Runoff Area=75,890 sf 0.00% Impervious Runoff Depth>2.67" Flow Length=580' Slope=0.0150 '/' Tc=16.5 min CN=78 Runoff=3.96 cfs 16,882 cf

SubcatchmentS211: S211

Runoff Area=15,436 sf 47.47% Impervious Runoff Depth>2.76"

Tc=6.0 min CN=79 Runoff=1.13 cfs 3,556 cf

SubcatchmentS212: SWALE

Runoff Area=52,768 sf 0.60% Impervious Runoff Depth>1.62"

Flow Length=418' Tc=23.1 min CN=65 Runoff=1.38 cfs 7,112 cf

SubcatchmentS213: COURTYARD

Runoff Area=21,407 sf 48.10% Impervious Runoff Depth>2.76"

Tc=6.0 min CN=79 Runoff=1.56 cfs 4,932 cf

SubcatchmentT1: Trench Drain 1 Runoff Area=11,173 sf 75.10% Impervious Runoff Depth>4.16"
Tc=6.0 min CN=93 Runoff=1.15 cfs 3.869 cf

SubcatchmentT2: Drive Under B2 Runoff Area=4,445 sf 64.30% Impervious Runoff Depth>2.68"

Tc=6.0 min CN=78 Runoff=0.31 cfs 991 cf

SubcatchmentTH1: TOWN HOUSE #1 Runoff Area=4,247 sf 92.68% Impervious Runoff Depth>4.49"
Tc=6.0 min CN=96 Runoff=0.45 cfs 1.589 cf

SubcatchmentTH10: TOWN HOUSE #10 Runoff Area=3,476 sf 91.60% Impervious Runoff Depth>4.49"

Tc=6.0 min CN=96 Runoff=0.37 cfs 1,301 cf

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 118

SubcatchmentTH11: TOWN HOUSE #11 Runoff Area=4,210 sf 92.61% Impervious Runoff Depth>4.49"
Tc=6.0 min CN=96 Runoff=0.45 cfs 1.575 cf

SubcatchmentTH2: TOWN HOUSE #2 Runoff Area=4,247 sf 92.68% Impervious Runoff Depth>4.49"
Tc=6.0 min CN=96 Runoff=0.45 cfs 1.589 cf

SubcatchmentTH3: TOWN HOUSE #3 Runoff Area=3,013 sf 88.68% Impervious Runoff Depth>4.38" Tc=6.0 min CN=95 Runoff=0.32 cfs 1.099 cf

SubcatchmentTH4: TOWN HOUSE #4 Runoff Area=3,470 sf 91.59% Impervious Runoff Depth>4.49"

Tc=6.0 min CN=96 Runoff=0.37 cfs 1,298 cf

SubcatchmentTH5: TOWN HOUSE #5 Runoff Area=3,016 sf 88.69% Impervious Runoff Depth>4.38" Tc=6.0 min CN=95 Runoff=0.32 cfs 1.100 cf

SubcatchmentTH6: TOWN HOUSE #6 Runoff Area=3,407 sf 91.46% Impervious Runoff Depth>4.49"

Tc=6.0 min CN=96 Runoff=0.36 cfs 1,275 cf

SubcatchmentTH7: TOWN HOUSE #7 Runoff Area=3,481 sf 91.61% Impervious Runoff Depth>4.49"

Tc=6.0 min CN=96 Runoff=0.37 cfs 1,302 cf

SubcatchmentTH8: TOWN HOUSE #8 Runoff Area=4,212 sf 92.62% Impervious Runoff Depth>4.49"

Tc=6.0 min CN=96 Runoff=0.45 cfs 1,576 cf

SubcatchmentTH9: TOWN HOUSE #9 Runoff Area=3,480 sf 91.61% Impervious Runoff Depth>4.49"

Tc=6.0 min CN=96 Runoff=0.37 cfs 1,302 cf

Reach 8R: OVERLAND FLOWAvg. Flow Depth=0.06' Max Vel=0.08 fps Inflow=1.10 cfs 3,558 cf n=0.400 L=563.0' S=0.0213 '/' Capacity=28.09 cfs Outflow=0.26 cfs 3,276 cf

Reach 9R: OVERLAND FLOWAvg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0 cf n=0.400 L=211.0' S=0.0652'/ Capacity=23.45 cfs Outflow=0.00 cfs 0 cf

Reach 10R: OVERLAND FLOWAvg. Flow Depth=0.19' Max Vel=0.22 fps Inflow=0.96 cfs 5,876 cf n=0.400 L=164.0' S=0.0366 '/' Capacity=17.57 cfs Outflow=0.92 cfs 5,876 cf

Reach 11R: 4x4 Open Bottom Culvert Avg. Flow Depth=0.63' Max Vel=1.62 fps Inflow=4.11 cfs 40,067 cf 48.0" x 48.0" Box Pipe n=0.069 L=30.0' S=0.0150 '/' Capacity=42.20 cfs Outflow=4.11 cfs 40,059 cf

Reach 12R: OVERLAND FLOWAvg. Flow Depth=0.10' Max Vel=0.12 fps Inflow=1.23 cfs 3,881 cf n=0.400 L=250.0' S=0.0240 '/' Capacity=29.80 cfs Outflow=0.61 cfs 3,787 cf

Reach 14R: OVERLAND FLOWAvg. Flow Depth=0.07' Max Vel=0.09 fps Inflow=1.38 cfs 7,111 cf n=0.400 L=852.0' S=0.0246 '/' Capacity=31.55 cfs Outflow=0.31 cfs 6,155 cf

Reach 15R: OVERLAND FLOWAvg. Flow Depth=0.05' Max Vel=0.07 fps Inflow=0.19 cfs 7,883 cf n=0.400 L=300.0' S=0.0200 '/' Capacity=27.21 cfs Outflow=0.19 cfs 7,158 cf

Reach 18R: OVERLAND FLOWAvg. Flow Depth=0.03' Max Vel=0.06 fps Inflow=0.40 cfs 1,449 cf n=0.400 L=609.0' S=0.0279 '/' Capacity=38.42 cfs Outflow=0.09 cfs 1,312 cf

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 119

Reach 20R: OVERLAND FLOW

Avg. Flow Depth=0.18' Max Vel=0.11 fps Inflow=1.79 cfs 13,822 cf

 $n = 0.400 \quad L = 560.0' \quad S = 0.0093 \; \text{'/'} \quad Capacity = 18.54 \; \text{cfs} \quad Outflow = 1.03 \; \text{cfs} \quad 13,230 \; \text{cf}$

Reach 23R: OVERLAND FLOWAvg. Flow Depth=0.29' Max Vel=0.22 fps Inflow=4.11 cfs 40,059 cf n=0.400 L=237.0' S=0.0211 '/' Capacity=31.93 cfs Outflow=3.63 cfs 39,543 cf

Reach R202: OVERLAND FLOW

Avg. Flow Depth=0.39' Max Vel=0.19 fps Inflow=18.32 cfs 89,705 cf

 $n = 0.400 \quad L = 700.0' \quad S = 0.0107 \; \text{$^{\prime\prime}$} \quad \text{Capacity} = 42.56 \; \text{cfs} \quad \text{Outflow} = 8.31 \; \text{cfs} \quad 84,798 \; \text{cf} \quad \text{$^{\prime\prime}$} = 1.000 \; \text{$^{$

Reach R211: OVERLAND FLOW Avg. Flow Depth=0.19' Max Vel=0.11 fps Inflow=3.94 cfs 6,192 cf

n=0.400 L=600.0' S=0.0087 '/' Capacity=14.51 cfs Outflow=0.78 cfs 5,906 cf

Reach SC1: Stream Crossing #1 Avg. Flow Depth=0.34' Max Vel=3.33 fps Inflow=18.32 cfs 89,722 cf 192.0" x 60.0", R=207.0" Arch Pipe n=0.030 L=43.1' S=0.0200 '/' Capacity=722.91 cfs Outflow=18.32 cfs 89,705 cf

Reach SC2: Stream Crossing #2Avg. Flow Depth=0.07' Max Vel=1.32 fps Inflow=1.38 cfs 7,112 cf 192.0" x 60.0", R=180.0" Arch Pipe n=0.030 L=36.5' S=0.0241 '/' Capacity=768.96 cfs Outflow=1.38 cfs 7,111 cf

Pond 1P: DMH #33 Peak Elev=206.22' Inflow=1.73 cfs 6,129 cf

12.0" Round Culvert n=0.013 L=46.7' S=0.0251 '/' Outflow=1.73 cfs 6,129 cf

Pond 3P: OCS #8 Peak Elev=201.47' Inflow=1.29 cfs 4,306 cf

Outflow=1.29 cfs 4,306 cf

Pond 5R: TRENCH DRAIN Peak Elev=198.02' Inflow=1.15 cfs 3,869 cf

8.0" Round Culvert n=0.012 L=36.0' S=0.0200 '/' Outflow=1.15 cfs 3,869 cf

Pond 11P: YARD DRAIN Peak Elev=207.37' Storage=671 cf Inflow=1.56 cfs 4,932 cf

Outflow=1.14 cfs 4,891 cf

Pond CB10: CB #10 Peak Elev=210.33' Inflow=0.76 cfs 2,738 cf

12.0" Round Culvert n=0.013 L=33.8' S=0.0050 '/' Outflow=0.76 cfs 2,738 cf

Pond CB11: CB #11 Peak Elev=210.43' Inflow=0.78 cfs 2,822 cf

12.0" Round Culvert n=0.013 L=26.3' S=0.0103 '/' Outflow=0.78 cfs 2,822 cf

Pond CB12: CB #12 Peak Elev=207.06' Inflow=0.57 cfs 2,060 cf 12.0" Round Culvert n=0.013 L=41.3' S=0.0249 '/' Outflow=0.57 cfs 2,060 cf

Pond CB13: CB #13 Peak Elev=207.26' Inflow=1.16 cfs 4,068 cf

12.0" Round Culvert n=0.013 L=43.7' S=0.0249 '/' Outflow=1.16 cfs 4,068 cf

Pond CB14: CB #14 Peak Elev=201.69' Inflow=1.20 cfs 3,973 cf

12.0" Round Culvert n=0.013 L=23.2' S=0.0052 '/' Outflow=1.20 cfs 3,973 cf

Pond CB15: CB #15 Peak Elev=201.60' Inflow=0.73 cfs 2,622 cf 12.0" Round Culvert n=0.013 L=15.6' S=0.0051 '/' Outflow=0.73 cfs 2.622 cf

Pond CB16: CB #16 Peak Elev=203.99' Inflow=0.62 cfs 1,962 cf

12.0" Round Culvert n=0.013 L=20.9' S=0.0067 '/' Outflow=0.62 cfs 1,962 cf

19097 Post-DevelopmentPrepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 120

11yare 67 10:20 00 0/11 02000	2020 Hydrody ib Collimate Colditions 220	120
Pond CB17: CB #17	Peak Elev=205.64' Inflow=1.20 cfs 3,992 12.0" Round Culvert n=0.013 L=13.8' S=0.0094'/' Outflow=1.20 cfs 3,992	
Pond CB18: CB #18	Peak Elev=205.61' Inflow=2.14 cfs 6,857 15.0" Round Culvert n=0.013 L=25.1' S=0.0052 '/' Outflow=2.14 cfs 6,857	
Pond CB20: CB #20	Peak Elev=204.67' Inflow=1.26 cfs 4,355 12.0" Round Culvert n=0.013 L=30.3' S=0.0053 '/' Outflow=1.26 cfs 4,355	
Pond CB21: CB #21	Peak Elev=204.94' Inflow=0.99 cfs 3,25' 12.0" Round Culvert n=0.013 L=26.0' S=0.0050'/' Outflow=0.99 cfs 3,25'	
Pond CB22: CB #22	Peak Elev=206.05' Inflow=1.28 cfs 4,490 12.0" Round Culvert n=0.012 L=16.1' S=0.0050 '/' Outflow=1.28 cfs 4,490	
Pond CB23: CB #23	Peak Elev=206.00' Inflow=0.93 cfs 3,014	
Pond CB24: CB #24	Peak Elev=205.83' Inflow=0.82 cfs 2,746 12.0" Round Culvert n=0.012 L=12.1' S=0.0050 '/' Outflow=0.82 cfs 2,746	
Pond CB25: CB #25	Peak Elev=205.82' Inflow=0.89 cfs 3,017 15.0" Round Culvert n=0.012 L=11.4' S=0.0053 '/' Outflow=0.89 cfs 3,017	
Pond CB26: CB #26	Peak Elev=202.34' Inflow=0.88 cfs 2,90' 12.0" Round Culvert n=0.013 L=42.5' S=0.0052'/' Outflow=0.88 cfs 2,90'	
Pond CB27: CB #27	Peak Elev=201.49' Inflow=0.65 cfs 2,28' 12.0" Round Culvert n=0.013 L=18.0' S=0.0056 '/' Outflow=0.65 cfs 2,28'	
Pond CB28: CB #28	Peak Elev=198.44' Inflow=0.97 cfs 3,136 12.0" Round Culvert n=0.013 L=13.7' S=0.0044 '/' Outflow=0.97 cfs 3,136	
Pond CB29: CB #29	Peak Elev=206.08' Inflow=0.89 cfs 3,020 12.0" Round Culvert n=0.013 L=13.5' S=0.0052 '/' Outflow=0.89 cfs 3,020	
Pond CB30: CB #30	Peak Elev=206.09' Inflow=0.93 cfs 3,175 12.0" Round Culvert n=0.013 L=17.5' S=0.0051 '/' Outflow=0.93 cfs 3,175	
Pond CB31: CB #31	Peak Elev=205.02' Inflow=1.59 cfs 5,230	
Pond CB32: CB #32	Peak Elev=204.91' Inflow=1.26 cfs 4,174 12.0" Round Culvert n=0.013 L=16.3' S=0.0049'/' Outflow=1.26 cfs 4,174	
Pond CB33: CB #33	Peak Elev=205.84' Inflow=0.57 cfs 1,927	
Pond CB34: CB #34	Peak Elev=205.89' Inflow=0.89 cfs 2,986 12.0" Round Culvert n=0.013 L=16.5' S=0.0048 '/' Outflow=0.89 cfs 2,986	

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Н	a	q	е	ΙZ	ı	
		_			_	

Peak Elev=207.44' Inflow=0.45 cfs 1,632 cf Pond CB35: CB #35 12.0" Round Culvert n=0.013 L=15.2' S=0.0053 '/' Outflow=0.45 cfs 1.632 cf Peak Elev=207.56' Inflow=0.72 cfs 2,605 cf Pond CB36: CB #36 12.0" Round Culvert n=0.013 L=16.1' S=0.0050 '/' Outflow=0.72 cfs 2,605 cf Pond CB38: CB #38 Peak Elev=210.56' Inflow=0.83 cfs 3,004 cf 12.0" Round Culvert n=0.012 L=16.7' S=0.0048 '/' Outflow=0.83 cfs 3.004 cf Peak Elev=210.56' Inflow=0.83 cfs 2,994 cf Pond CB39: CB #39 12.0" Round Culvert n=0.013 L=16.4' S=0.0049 '/' Outflow=0.83 cfs 2,994 cf Pond CB40: CB #40 Peak Elev=214.16' Inflow=0.46 cfs 1.656 cf 12.0" Round Culvert n=0.013 L=17.8' S=0.0073 '/' Outflow=0.46 cfs 1.656 cf Pond CB41: CB #41 Peak Elev=214.37' Inflow=0.61 cfs 2,197 cf 12.0" Round Culvert n=0.013 L=18.4' S=0.0049 '/' Outflow=0.61 cfs 2,197 cf Peak Elev=220.42' Inflow=0.30 cfs 967 cf Pond CB43: CB #43 12.0" Round Culvert n=0.013 L=14.9' S=0.0047 '/' Outflow=0.30 cfs 967 cf Peak Elev=220.39' Inflow=0.20 cfs 667 cf Pond CB44: CB #44 12.0" Round Culvert n=0.013 L=14.9' S=0.0047 '/' Outflow=0.20 cfs 667 cf Peak Elev=221.54' Inflow=0.19 cfs 587 cf Pond CB45: CB #45 12.0" Round Culvert n=0.013 L=18.2' S=0.0049 '/' Outflow=0.19 cfs 587 cf Peak Elev=221.87' Inflow=0.33 cfs 1,046 cf Pond CB46: CB #46 12.0" Round Culvert n=0.013 L=15.3' S=0.0052'/' Outflow=0.33 cfs 1,046 cf Peak Elev=225.52' Inflow=0.33 cfs 1,185 cf **Pond CB47: CB#47** 12.0" Round Culvert n=0.012 L=20.9' S=0.0048 '/' Outflow=0.33 cfs 1,185 cf Peak Elev=225.81' Inflow=2.61 cfs 10,030 cf **Pond CB48: CB#48** 15.0" Round Culvert n=0.012 L=16.9' S=0.0047 '/' Outflow=2.61 cfs 10,030 cf Peak Elev=203.45' Inflow=0.55 cfs 1,862 cf Pond CB49: CB #49 12.0" Round Culvert n=0.013 L=15.5' S=0.0052'/' Outflow=0.55 cfs 1,862 cf

Pond CB50: CB #50 Peak Elev=203.64' Inflow=1.55 cfs 5,209 cf 12.0" Round Culvert n=0.013 L=15.3' S=0.0052'/' Outflow=1.55 cfs 5,209 cf

Pond CB51: CB #51 Peak Elev=202.87' Inflow=0.74 cfs 2,684 cf 12.0" Round Culvert n=0.013 L=31.4' S=0.0051 '/' Outflow=0.74 cfs 2,684 cf

Pond CB52: CB #52 Peak Elev=203.29' Inflow=0.96 cfs 3,302 cf

12.0" Round Culvert n=0.013 L=25.5' S=0.0051'/' Outflow=0.96 cfs 3.302 cf

Pond CB53: CB #53 Peak Elev=203.33' Inflow=0.81 cfs 2,723 cf 12.0" Round Culvert n=0.013 L=32.0' S=0.0050 '/' Outflow=0.81 cfs 2,723 cf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 122

Pond CB54: CB #54	Peak Elev=203.07' Inflow=0.48	cfs 1,583 cf
	12.0" Round Culvert n=0.013 L=36.7' S=0.0049 '/' Outflow=0.48	cfs 1,583 cf

Pond CB7: CB#5 Peak Elev=212.99' Inflow=0.51 cfs 1,829 cf 12.0" Round Culvert n=0.012 L=15.1' S=0.0099 '/' Outflow=0.51 cfs 1,829 cf

Pond CB8: CB#8 Peak Elev=214.21' Inflow=0.57 cfs 1,937 cf 12.0" Round Culvert n=0.013 L=15.1' S=0.0099'/' Outflow=0.57 cfs 1.937 cf

Pond CB9: CB #9 Peak Elev=210.82' Inflow=1.76 cfs 6,257 cf 12.0" Round Culvert n=0.013 L=19.9' S=0.0196 '/' Outflow=1.76 cfs 6,257 cf

Pond D10: DMH #10 Peak Elev=203.81' Inflow=0.62 cfs 1,962 cf 12.0" Round Culvert n=0.013 L=15.6' S=0.0051'/' Outflow=0.62 cfs 1.962 cf

Pond D11: DMH #11 Peak Elev=205.28' Inflow=3.34 cfs 10,849 cf 18.0" Round Culvert n=0.013 L=44.6' S=0.0049 '/' Outflow=3.34 cfs 10,849 cf

Pond D12: DMH #12 Peak Elev=204.24' Inflow=2.25 cfs 7,606 cf 12.0" Round Culvert n=0.013 L=41.9' S=0.0050 '/' Outflow=2.25 cfs 7,606 cf

Pond D13: DMH #13 Peak Elev=203.33' Inflow=7.09 cfs 25,765 cf 24.0" Round Culvert n=0.013 L=60.1' S=0.0050 '/' Outflow=7.09 cfs 25,765 cf

Pond D14: DMH #14 Peak Elev=205.16' Inflow=3.91 cfs 13,267 cf 18.0" Round Culvert n=0.012 L=256.3' S=0.0050 '/' Outflow=3.91 cfs 13,267 cf

Pond D16: DMH #16 Peak Elev=205.68' Inflow=1.70 cfs 5,763 cf 15.0" Round Culvert n=0.012 L=103.5' S=0.0050 '/' Outflow=1.70 cfs 5,763 cf

Pond D17: DMH #17 Peak Elev=201.21' Inflow=1.53 cfs 5,188 cf 12.0" Round Culvert n=0.013 L=91.6' S=0.0312 '/' Outflow=1.53 cfs 5,188 cf

Pond D18: DMH #18 Peak Elev=198.27' Inflow=2.50 cfs 8,324 cf 15.0" Round Culvert n=0.013 L=46.3' S=0.0099 '/' Outflow=2.50 cfs 8,324 cf

Pond D19: DMH #19 Peak Elev=205.94' Inflow=1.82 cfs 6,195 cf 12.0" Round Culvert n=0.013 L=82.5' S=0.0092 '/' Outflow=1.82 cfs 6,195 cf

Pond D2: DMH#2 Peak Elev=207.99' Inflow=3.67 cfs 14,981 cf 15.0" Round Culvert n=0.013 L=38.2' S=0.0099 '/' Outflow=3.67 cfs 14,981 cf

Pond D20: DMH #20 Peak Elev=204.96' Inflow=1.82 cfs 6,195 cf 15.0" Round Culvert n=0.013 L=63.5' S=0.0050 '/' Outflow=1.82 cfs 6,195 cf

Pond D21: DMH #21 Peak Elev=204.42' Inflow=7.30 cfs 24,749 cf 24.0" Round Culvert n=0.013 L=72.4' S=0.0050 '/' Outflow=7.30 cfs 24.749 cf

Pond D22: DMH #22 Peak Elev=205.74' Inflow=2.62 cfs 9,150 cf 15.0" Round Culvert n=0.013 L=134.2' S=0.0071 '/' Outflow=2.62 cfs 9,150 cf

Pond DE62: DRIP #62

Peak Elev=213.22' Storage=273 cf Inflow=0.45 cfs 1,589 cf

Discarded=0.00 cfs 196 cf Primary=0.38 cfs 1,249 cf Outflow=0.39 cfs 1,445 cf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 123

Pond D23: DMH #23	Peak Elev=207.23' Inflow=1.17 cfs 4,237 cf 15.0" Round Culvert n=0.013 L=173.3' S=0.0100 '/' Outflow=1.17 cfs 4,237 cf
Pond D25: DMH #25	Peak Elev=210.49' Inflow=3.74 cfs 13,119 cf 15.0" Round Culvert n=0.012 L=237.6' S=0.0050 '/' Outflow=3.74 cfs 13,119 cf
Pond D27: DMH #27	Peak Elev=214.05' Inflow=2.08 cfs 7,120 cf 15.0" Round Culvert n=0.012 L=63.9' S=0.0150 '/' Outflow=2.08 cfs 7,120 cf
Pond D28: DMH #28	Peak Elev=217.98' Inflow=1.01 cfs 3,267 cf 12.0" Round Culvert n=0.013 L=158.3' S=0.0200 '/' Outflow=1.01 cfs 3,267 cf
Pond D29: DMH #29	Peak Elev=220.35' Inflow=1.01 cfs 3,267 cf 12.0" Round Culvert n=0.013 L=150.9' S=0.0151 '/' Outflow=1.01 cfs 3,267 cf
Pond D30: DMH #30	Peak Elev=221.34' Inflow=0.52 cfs 1,633 cf 12.0" Round Culvert n=0.013 L=184.2' S=0.0050 '/' Outflow=0.52 cfs 1,633 cf
Pond D31: DMH#31	Peak Elev=225.49' Inflow=2.84 cfs 11,215 cf 15.0" Round Culvert n=0.012 L=288.5' S=0.0400 '/' Outflow=2.84 cfs 11,215 cf
Pond D34: DMH #34	Peak Elev=199.01' Inflow=2.53 cfs 9,148 cf 12.0" Round Culvert n=0.012 L=52.0' S=0.0200 '/' Outflow=2.53 cfs 9,148 cf
Pond D35: DMH #35	Peak Elev=212.99' Inflow=2.08 cfs 7,120 cf 15.0" Round Culvert n=0.012 L=171.5' S=0.0150 '/' Outflow=2.08 cfs 7,120 cf
Pond D4: DMH#4	Peak Elev=211.36' Inflow=3.67 cfs 14,981 cf 15.0" Round Culvert n=0.012 L=222.3' S=0.0150 '/' Outflow=3.67 cfs 14,981 cf
Pond D5: DMH #5	Peak Elev=210.09' Inflow=3.30 cfs 11,817 cf 18.0" Round Culvert n=0.013 L=183.0' S=0.0050 '/' Outflow=3.30 cfs 11,817 cf
Pond D6: DMH #6	Peak Elev=209.03' Inflow=3.30 cfs 11,817 cf 18.0" Round Culvert n=0.013 L=299.7' S=0.0050 '/' Outflow=3.30 cfs 11,817 cf
Pond D7: DMH #7	Peak Elev=207.33' Inflow=3.30 cfs 11,817 cf 18.0" Round Culvert n=0.013 L=44.2' S=0.0550 '/' Outflow=3.30 cfs 11,817 cf
Pond D8: DMH #8	Peak Elev=201.54' Inflow=1.93 cfs 6,595 cf 12.0" Round Culvert n=0.013 L=87.7' S=0.0050 '/' Outflow=1.93 cfs 6,595 cf
Pond D9: DMH #9	Peak Elev=200.97' Inflow=1.93 cfs 6,595 cf 12.0" Round Culvert n=0.013 L=11.9' S=0.0050 '/' Outflow=1.93 cfs 6,595 cf
Pond DE61: DRIP #61	Peak Elev=213.22' Storage=273 cf Inflow=0.45 cfs 1,589 cf Discarded=0.00 cfs 196 cf Primary=0.38 cfs 1,249 cf Outflow=0.39 cfs 1,445 cf

Pond OCS7: OCS #7

Peak Elev=202.65' Inflow=1.70 cfs 5,986 cf

Outflow=1.70 cfs 5,986 cf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 124

•	· · · · · · · · · · · · · · · · · · ·
Pond DE63: DRIP #63	Peak Elev=207.93' Storage=152 cf Inflow=0.32 cfs 1,099 cf Discarded=0.00 cfs 117 cf Primary=0.29 cfs 895 cf Outflow=0.29 cfs 1,012 cf
Pond DE64: DRIP #64	Peak Elev=205.97' Storage=184 cf Inflow=0.37 cfs 1,298 cf Discarded=0.00 cfs 140 cf Primary=0.33 cfs 1,057 cf Outflow=0.33 cfs 1,197 cf
Pond DE65: DRIP #65	Peak Elev=206.93' Storage=152 cf Inflow=0.32 cfs 1,100 cf Discarded=0.00 cfs 117 cf Primary=0.29 cfs 896 cf Outflow=0.29 cfs 1,013 cf
Pond DE66: DRIP #66	Peak Elev=208.76' Storage=183 cf Inflow=0.36 cfs 1,275 cf Discarded=0.00 cfs 140 cf Primary=0.32 cfs 1,033 cf Outflow=0.33 cfs 1,173 cf
Pond DE67: DRIP #67	Peak Elev=208.97' Storage=184 cf Inflow=0.37 cfs 1,302 cf Discarded=0.00 cfs 140 cf Primary=0.33 cfs 1,061 cf Outflow=0.33 cfs 1,201 cf
Pond DE68: DRIP #68	Peak Elev=207.92' Storage=248 cf Inflow=0.45 cfs 1,576 cf Discarded=0.00 cfs 196 cf Primary=0.39 cfs 1,238 cf Outflow=0.40 cfs 1,434 cf
Pond DE69: DRIP #69	Peak Elev=206.47' Storage=184 cf Inflow=0.37 cfs 1,302 cf Discarded=0.00 cfs 140 cf Primary=0.33 cfs 1,061 cf Outflow=0.33 cfs 1,200 cf
Pond DE70: DRIP #70	Peak Elev=206.87' Storage=184 cf Inflow=0.37 cfs 1,301 cf Discarded=0.00 cfs 140 cf Primary=0.33 cfs 1,059 cf Outflow=0.33 cfs 1,199 cf
Pond DE71: DRIP #71	Peak Elev=207.51' Storage=273 cf Inflow=0.45 cfs 1,575 cf Discarded=0.00 cfs 196 cf Primary=0.38 cfs 1,235 cf Outflow=0.38 cfs 1,431 cf
Pond DECH: DRIP #CH	Peak Elev=209.49' Storage=382 cf Inflow=0.67 cfs 2,343 cf Discarded=0.04 cfs 1,427 cf Primary=0.38 cfs 916 cf Outflow=0.42 cfs 2,343 cf
Pond DMH32: DMH #32	Peak Elev=203.40' Inflow=2.09 cfs 7,071 cf 12.0" Round Culvert n=0.013 L=19.2' S=0.0531 '/' Outflow=2.09 cfs 7,071 cf
Pond OCS1: OCS#1	Peak Elev=196.14' Inflow=5.03 cfs 17,471 cf Outflow=5.03 cfs 17,471 cf
Pond OCS3: OCS#3	Peak Elev=204.75' Inflow=5.25 cfs 17,757 cf Outflow=5.25 cfs 17,757 cf
Pond OCS4: OCS#4	Peak Elev=204.72' Inflow=1.25 cfs 4,007 cf Outflow=1.25 cfs 4,007 cf
Pond OCS6: OCS #6	Peak Elev=202.65' Inflow=1.73 cfs 6,129 cf Outflow=1.73 cfs 6,129 cf

Pond P204: STORMTECHINFILTRATION Peak Elev=204.72' Storage=8,487 cf Inflow=6.49 cfs 21,764 cf Discarded=0.09 cfs 5,743 cf Primary=1.79 cfs 13,822 cf Outflow=1.88 cfs 19,565 cf

19097	Post-Dev	elopment
13031	I USL-DEV	CIODIIICIIL

Type III 24-hr 10YR Rainfall=4.96"

Page 125

Prepared by Howard Stein Hudson Associates Printed 6/19/2023 HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Pond P205: INFILTRATIONPOND #3 Peak Elev=206.70' Storage=7,871 cf Inflow=4.73 cfs 18,538 cf Discarded=0.30 cfs 14,188 cf Primary=0.40 cfs 1,449 cf Outflow=0.70 cfs 15,637 cf

Peak Elev=196.10' Storage=6,143 cf Inflow=6.18 cfs 21,341 cf Pond P206: STORMTECHINFILTRATION Discarded=0.49 cfs 18,384 cf Primary=1.63 cfs 2,954 cf Outflow=2.12 cfs 21,338 cf

Pond P207: INFILTRATIONPOND #2 Peak Elev=198.06' Storage=13,926 cf Inflow=11.58 cfs 40,612 cf Discarded=1.03 cfs 34,722 cf Primary=0.96 cfs 5,876 cf Outflow=1.99 cfs 40,597 cf

Peak Elev=203.77' Storage=13,036 cf Inflow=5.18 cfs 17,735 cf Pond P210: POCKET WETLAND#1 Outflow=0.19 cfs 7,883 cf

Pond P212: INFILTRATIONPOND #1 Peak Elev=202.65' Storage=24,325 cf Inflow=16.29 cfs 63,872 cf Discarded=1.42 cfs 57,664 cf Primary=3.94 cfs 6,192 cf Outflow=5.35 cfs 63,856 cf

Pond P213: Stormtech Infiltration System Peak Elev=202.65' Storage=3,828 cf Inflow=3.43 cfs 12,114 cf Discarded=0.39 cfs 12,113 cf Primary=0.00 cfs 0 cf Outflow=0.39 cfs 12,113 cf

Pond P214: STORMTECHINFILTRATION Peak Elev=201.46' Storage=3,080 cf Inflow=3.46 cfs 12,166 cf Discarded=0.61 cfs 12,168 cf Primary=0.00 cfs 0 cf Outflow=0.61 cfs 12,168 cf

Link AP1: ANALYSISPOINT 1 Inflow=1.05 cfs 3.627 cf Primary=1.05 cfs 3,627 cf

Link AP2: ANALYSISPOINT 2 Inflow=18.24 cfs 157.893 cf Primary=18.24 cfs 157,893 cf

Link AP3: ANALYSISPOINT 3 Inflow=2.89 cfs 9,258 cf Primary=2.89 cfs 9,258 cf

Link AP4: ANALYSISPOINT #4 Inflow=25.19 cfs 178,860 cf Primary=25.19 cfs 178,860 cf

Total Runoff Area = 2,573,920 sf Runoff Volume = 532,571 cf Average Runoff Depth = 2.48" 70.09% Pervious = 1,803,997 sf 29.91% Impervious = 769,923 sf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 126

Summary for Subcatchment B1: MULTIFAMILY BLDG #1

Runoff = 2.53 cfs @ 12.09 hrs, Volume= 9,148 cf, Depth> 4.72"

Routed to Pond D34: DMH #34

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description		
	20,156	98	Roofs, HSG	G C	
	3,099	98	Roofs, HSG	G D	
	23,255	98	Weighted A	verage	
	23,255		100.00% Im	pervious A	Area
Tc	Length	Slope	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment B2: MULTIFAMILY BLDG #2

Runoff = 1.91 cfs @ 12.09 hrs, Volume= 6,908 cf, Depth> 4.72"

Routed to Pond OCS3: OCS#3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description		
	7,873	98	Roofs, HSG	A A	
	9,688	98	Roofs, HSG	G C	
	17,561	98	Weighted A	verage	
	17,561		100.00% Im	npervious A	\rea
Tc	Length	Slope	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment B3: MULTIFAMILY BUILDING #3

Runoff = 2.17 cfs @ 12.09 hrs, Volume= 7,860 cf, Depth> 4.72" Routed to Pond P214 : STORMTECH INFILTRATION SYSTEM #4

 Area (sf)	CN	Description
608	98	Roofs, HSG A
 19,373	98	Roofs, HSG C
19,981	98	Weighted Average
19,981		100.00% Impervious Area

Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 127

	Тс	Length	Slope	Velocity	Capacity	Description
(m	in)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
(3.0					Direct Entry,

Summary for Subcatchment C10: CB #10

Runoff = 0.76 cfs @ 12.09 hrs, Volume= 2,738 cf,

2,738 cf, Depth> 4.72"

Routed to Pond CB10 : CB #10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description					
	339	98	Paved road	s w/curbs &	& sewers, HSG B			
	5,703	98	Paved road	s w/curbs &	& sewers, HSG C			
	919	98	Paved road	s w/curbs &	& sewers, HSG D			
	6,961	98	Weighted Average					
	6,961		100.00% In	npervious A	Area			
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment C11: CB #11

Runoff = 0.78 cfs @ 12.09 hrs, Volume= 2,822 cf, Depth> 4.72"

Routed to Pond CB11 : CB #11

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

_	Α	rea (sf)	CN I	Description							
		7,173	98 I	Paved roads w/curbs & sewers, HSG C							
		7,173		100.00% Impervious Area							
	Tc (min)	Length (feet)	Slope (ft/ft)	e Velocity Capacity Description t) (ft/sec) (cfs)							
	6.0			Direct Entry.							

Summary for Subcatchment C12: CB #12

Runoff = 0.57 cfs @ 12.09 hrs, Volume= 2,060 cf, Depth> 4.72"

Routed to Pond CB12 : CB #12

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 128

	A	rea (sf)	CN [Description						
		5,238	98 F	Paved roads w/curbs & sewers, HSG C						
		5,238	•	100.00% Impervious Area						
(n	Tc nin)	Length (feet)	Slope (ft/ft)							
	6.0			Direct Entry,						

Summary for Subcatchment C13: CB #13

Runoff 1.16 cfs @ 12.09 hrs, Volume= 4,068 cf, Depth> 4.49"

Routed to Pond CB13: CB #13

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description					
	1,003	74	>75% Gras	s cover, Go	Good, HSG C			
	7,547	98	Paved park	ing, HSG C	C			
	2,323	98	Roofs, HSC	S C				
	10,873	96	Weighted A	verage				
	1,003		9.22% Perv	rious Area				
	9,870		90.78% Imp	pervious Ar	rea			
_				_				
Tc	Length	Slope	,	Capacity	•			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C14: CB #14

1.20 cfs @ 12.09 hrs, Volume= Runoff

3,973 cf, Depth> 3.94"

Routed to Pond CB14: CB #14

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

 Α	rea (sf)	CN	Description						
	1,195	39	>75% Gras	s cover, Go	ood, HSG A				
	7,649	98	Paved park	ing, HSG A	١				
	472	74	>75% Gras	s cover, Go	ood, HSG C				
	2,783	98	Paved park	ing, HSG C)				
	12,099	91	Weighted A	verage					
	1,667		13.78% Per	vious Area	I				
	10,432		86.22% Imp	pervious Ar	ea				
т.	ما العرب ال	Clan	- \/alaaitu	Conneitu	Decemination				
Tc	Length	Slop							
 (min)	(feet)	(ft/f	t) (ft/sec)	(cfs)					
~ ~					D: (= (

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 129

Summary for Subcatchment C15: CB #15

Runoff = 0.73 cfs @ 12.09 hrs, Volume= 2.6

2,622 cf, Depth> 4.72"

Routed to Pond CB15: CB #15

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description		
	5,000	98	Paved park	ing, HSG A	4
	1,666	98	Paved park	ing, HSG C	
	6,666	98	Weighted A	verage	
	6,666		100.00% Im	npervious A	Area
Tc	Length	Slope	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment C16: CB #16

Runoff = 0.62 cfs @ 12.09 hrs, Volume=

1,962 cf, Depth> 2.76"

Routed to Pond CB16: CB #16

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description							
	2,467	39	>75% Gras	s cover, Go	Good, HSG A					
	4,380	98	Paved park	ing, HSG A	A					
	524	74	>75% Gras	s cover, Go	Good, HSG C					
	1,145	98	Paved park	ing, HSG C	C					
	8,516	79	Weighted A	verage						
	2,991		35.12% Pe	vious Area	a					
	5,525		64.88% Imp	ervious Ar	ırea					
Tc	Length	Slope	e Velocity Capacity Description							
(min)	(feet)	(ft/ft	t) (ft/sec) (cfs)							
6.0			Direct Entry,							

Summary for Subcatchment C17: CB #17

Runoff = 1.20 cfs @ 12.09 hrs, Volume= 3,992 cf, Depth> 4.05"

Routed to Pond CB17: CB #17

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 130

A	rea (sf)	CN	Description						
	3,093	74	>75% Gras	s cover, Go	Good, HSG C				
	8,743	98	Paved park	ing, HSG C	C				
	11,836	92	Weighted A	verage					
	3,093		26.13% Pei	rvious Area	a				
	8,743		73.87% Imp	pervious Ar	ırea				
_		01			B				
Тс	Length	Slope	e Velocity Capacity Description						
(min)	(feet)	(ft/ft)	r) (ft/sec) (cfs)						
6.0			Direct Entry.						

Summary for Subcatchment C18: CB #18

Runoff 1.81 cfs @ 12.09 hrs, Volume= 5,941 cf, Depth> 3.83"

Routed to Pond CB18: CB #18

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description		
	6,255				lood, HSG C
	12,336	98	Paved park	<u>ing, HSG C</u>	<u>C</u>
•	18,591	90	Weighted A	verage	
	6,255		33.65% Pe	rvious Area	a
	12,336		66.35% Imp	pervious Ar	rea
Tc	Length	Slope	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)	•
6.0					Direct Entry,

Summary for Subcatchment C20: CB #20

Runoff 1.26 cfs @ 12.09 hrs, Volume= 4,355 cf, Depth> 4.38"

Routed to Pond CB20: CB #20

 Area (sf)	CN	Description			
3,319	98	Paved parking, HSG A			
1,319	74	>75% Grass cover, Good, HSG C			
 7,301	98	Paved parking, HSG C			
11,939	95	Weighted Average			
1,319		11.05% Pervious Area			
10,620		88.95% Impervious Area			

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

<u>Page 131</u>

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

-

Summary for Subcatchment C21: CB #21

Runoff = 0.99 cfs @ 12.09 hrs, Volume= 3,251 cf, Depth> 3.83"

Routed to Pond CB21 : CB #21

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

	Area (sf)	CN	Description						
	1,319	39	>75% Gras	s cover, Go	Good, HSG A				
	7,301	98	Paved park	ing, HSG A	A				
	1,554	98	Paved park	ing, HSG C	C				
	10,174	90	Weighted A	verage					
	1,319		12.96% Per	rvious Area	a				
	8,855		87.04% Imp	pervious Ar	ırea				
To	9	Slope	•	Capacity	•				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry.				

Direct Entry,

Summary for Subcatchment C22: CB #22

Runoff = 1.28 cfs @ 12.09 hrs, Volume= 4,490 cf, Depth> 4.49"

Routed to Pond CB22 : CB #22

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Area (sf)	CN	Description						
2,946	98	Paved parkir	ng, HSG A					
177	74	>75% Grass	cover, Go	od, HSG C				
2,641	98	Paved parkir	ng, HSG C	,				
829	80	>75% Grass	cover, Go	od, HSG D				
5,408	98	Paved parkir	ng, HSG D)				
12,001	96	Weighted Av	erage					
1,006		8.38% Pervi	ous Area					
10,995		91.62% Impe	ervious Ar	ea				
		•						
Tc Length	Slo	pe Velocity	Capacity	Description				
(min) (feet)	(ft/	ft) (ft/sec)	(cfs)					
				D . 4 D . 4				

6.0 Direct Entry,

Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 132

Summary for Subcatchment C23: CB #23

Runoff = 0.93 cfs @ 12.09 hrs, Volume= 3,014 cf, Depth> 3.73"

Routed to Pond CB23: CB #23

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description			
	242	39	>75% Gras	s cover, Go	ood, HSG A	
	3,016	98	Paved park	ing, HSG A	١	
	1,267	74	>75% Gras	s cover, Go	ood, HSG C	
	218	98	Paved park	ing, HSG C		
	2,272	80	>75% Gras	s cover, Go	ood, HSG D	
	2,679	98	Paved park	ing, HSG [)	
	9,694	89	Weighted A	verage		
	3,781		39.00% Per	vious Area	1	
	5,913		61.00% Imp	ervious Ar	ea	
Tc	Length	Slope	,	Capacity	Description	
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment C24: CB #24

Runoff = 0.82 cfs @ 12.09 hrs, Volume= 2,746 cf, Depth> 4.16"

Routed to Pond CB24: CB #24

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

	rea (sf)	CN	Description		
	5,722	98	Paved park	ing, HSG D	D
	2,208	80	>75% Ġras	s cover, Go	lood, HSG D
	7,930	93	Weighted A	verage	
	2,208		27.84% Pe	vious Area	a
	5,722		72.16% lm <mark>։</mark>	ervious Ar	rea
_					
Тс	5	Slope	,	Capacity	·
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment C25: CB #25

Runoff = 0.89 cfs @ 12.09 hrs, Volume= 3,017 cf, Depth> 4.27"

Routed to Pond CB25: CB #25

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

Page 133

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Aı	rea (sf)	CN	Description			
	211	39	>75% Gras	s cover, Go	ood, HSG A	
	519	98	Paved park	ing, HSG A	1	
	15	74	>75% Gras	s cover, Go	ood, HSG C	
	300	98	Paved park	ing, HSG C	;	
	1,393	80	>75% Gras	s cover, Go	ood, HSG D	
	6,049	98	Paved park	ing, HSG D)	
	8,487	94	Weighted A	verage		
	1,619		19.08% Per	vious Area		
	6,868		80.92% Imp	ervious Ar	ea	
Tc	Length	Slop		Capacity	Description	
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment C26: CB #26

Runoff = 0.88 cfs @ 12.09 hrs, Volume= 2,901 cf, Depth> 3.94"

Routed to Pond CB26: CB #26

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description						
	3,203	80	>75% Gras	s cover, Go	Good, HSG D				
	5,632	98	Paved park	ing, HSG D	D				
	8,835	91	Weighted Average						
	3,203	;	36.25% Pei	rvious Area	a				
	5,632	(33.75% Imp	pervious Ar	rea				
Tc	Length	Slope	,	Capacity	·				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment C27: CB #27

Runoff = 0.65 cfs @ 12.09 hrs, Volume= 2,287 cf, Depth> 4.49"

Routed to Pond CB27: CB #27

Area (sf)	CN	Description			
98	39	>75% Grass cover, Good, HSG A			
131	98	Paved parking, HSG A			
397	80	>75% Grass cover, Good, HSG D			
5,485	98	Paved parking, HSG D			
6,111	96	Weighted Average			
495)	8.10% Pervious Area			
5,616	;	91.90% Impervious Area			

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 134

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment C28: CB #28

0.97 cfs @ 12.09 hrs, Volume= Runoff

3,136 cf, Depth> 3.63"

Routed to Pond CB28: CB #28

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Area	a (sf)	CN	Description						
2	2,751	74	>75% Gras	s cover, Go	ood, HSG C				
2	2,841	98	Paved park	ing, HSG C	;				
2	2,297	80	>75% Gras	s cover, Go	ood, HSG D				
2	2,483	98	Paved park	ing, HSG D)				
10),372	88	Weighted A	verage					
5	5,048		48.67% Per	vious Area	I				
5	5,324		51.33% Imp	ervious Ar	ea				
Tc L	.ength	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Direct Entry,

Summary for Subcatchment C29: CB #29

0.89 cfs @ 12.09 hrs, Volume= 3,020 cf, Depth> 4.27" Runoff

Routed to Pond CB29: CB #29

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

	Aı	rea (sf)	CN I	Description							
		1,341	74 :	>75% Gras	s cover, Go	Good, HSG C					
		5,330	98 I	Paved park	ing, HSG C	C					
		1,824	98	Roofs, HSC	G C						
		8,495	94 \	Weighted Average							
		1,341		15.79% Pei	vious Area	a					
		7,154	;	34.21% lmp	pervious Ar	ırea					
	Tc	Length	Slope	,	Capacity	•					
<u>(n</u>	nin)	(feet)	(ft/ft)	(ft/sec) (cfs)							
	60					Direct Entry					

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 135

Summary for Subcatchment C30: CB #30

Runoff = 0.93 cfs @ 12.09 hrs, Volume= 3,175 cf, Depth> 4.27"

Routed to Pond CB30: CB #30

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description							
	1,572	74	>75% Gras	s cover, Go	lood, HSG C					
	6,310	98	Paved park	ing, HSG C	C					
	1,051	98	Roofs, HSC	S C						
	8,933	94	Weighted A	verage						
	1,572		17.60% Pe	rvious Area	a					
	7,361		82.40% lmp	pervious Ar	rea					
Tc	Length	Slope	Velocity	Capacity	Description					
		•	,		·					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment C31: CB #31

Runoff = 1.59 cfs @ 12.09 hrs, Volume= 5,230 cf, Depth> 3.83"

Routed to Pond CB31: CB #31

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

	Area (sf)	CN	Description							
	5,132	74	>75% Gras	s cover, Go	Good, HSG C					
	9,132	98	Paved park	ing, HSG C	C					
	2,101	98	Roofs, HSC	S Č						
	16,365	90	Weighted A	verage						
	5,132		31.36% Per	rvious Area	a					
	11,233		68.64% lmp	pervious Ar	rea					
_										
Тс	Length	Slope	,	Capacity	·					
<u>(min)</u>	(feet)	(ft/ft	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment C32: CB #32

Runoff = 1.26 cfs @ 12.09 hrs, Volume= 4,174 cf, Depth> 3.94"

Routed to Pond CB32 : CB #32

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 136

A	rea (sf)	CN	Description							
	3,753	74	>75% Gras	s cover, Go	ood, HSG C					
	7,068	98	Paved park	ing, HSG C	C					
	1,889	98	Roofs, HSC	S C						
	12,710	91	Neighted A	verage						
	3,753		29.53% Pei	rvious Area	a					
	8,957	•	70.47% Imp	pervious Ar	rea					
Тс	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	•	(cfs)	•					
6.0		•			Direct Entry,					

Summary for Subcatchment C33: CB #33

Runoff = 0.57 cfs @ 12.09 hrs, Volume= 1,927 cf, Depth> 4.27"

Routed to Pond CB33: CB #33

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description						
	873	74	>75% Gras	s cover, Go	ood, HSG C				
	3,693	98	Paved park	ing, HSG C	C				
	855	98	Roofs, HSC	S C					
	5,421	94	Weighted A	verage					
	873		16.10% Pervious Area						
	4,548		83.90% Imp	pervious Ar	rea				
Tc	Length	Slope	,	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment C34: CB #34

Runoff = 0.89 cfs @ 12.09 hrs, Volume= 2,986 cf, Depth> 4.16"

Routed to Pond CB34: CB #34

 Area (sf)	CN	Description			
1,680	74	>75% Grass cover, Good, HSG C			
5,115	98	Paved parking, HSG C			
 1,827	98	Roofs, HSG C			
8,622	93	Weighted Average			
1,680		19.49% Pervious Area			
6,942		80.51% Impervious Area			

Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 137

	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.0					Direct Entry,

Summary for Subcatchment C35: CB #35

Runoff = 0.45 cfs @ 12.09 hrs, Volume=

1,632 cf, Depth> 4.72"

Routed to Pond CB35: CB #35

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description						
	3,578	98	Paved park	ing, HSG C					
	79	80	>75% Gras	s cover, Go	ood, HSG D				
	492	98	Paved park	ing, HSG D)				
	4,149	98	Weighted A	verage					
	79		1.90% Perv	ious Area					
	4,070		98.10% Imp	pervious Ar	ea				
Tc	Length	Slop	,	Capacity	Description				
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment C36: CB #36

Runoff = 0.72 cfs @ 12.09 hrs, Volume=

2,605 cf, Depth> 4.72"

Routed to Pond CB36: CB #36

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

_	Α	rea (sf)	CN I	Description							
		6,622	98 I	Paved parking, HSG C							
		6,622	•	100.00% Impervious Area							
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
-	6.0	(1001)	(10,10)	(14000)	(0.0)	Direct Entry,					

Summary for Subcatchment C38: CB #38

Runoff = 0.83 cfs @ 12.09 hrs, Volume= 3,004 cf, Depth> 4.72"

Routed to Pond CB38 : CB #38

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 138

	rea (sf)	CN	Description						
	6,647	98	Paved park	ing, HSG B	В				
	392	98	Paved park	ing, HSG C	C				
	598	98	Paved park	ing, HSG D	D				
	7,637	98	Weighted Average						
	7,637		100.00% In	npervious A	Area				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec) (cfs)						
6.0					Direct Entry				

5.0 Direct Entry,

Summary for Subcatchment C39: CB #39

Runoff = 0.83 cfs @ 12.09 hrs, Volume= 2

2,994 cf, Depth> 4.72"

Routed to Pond CB39: CB #39

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Aı	rea (sf)	CN	Description					
	6,505	98	Paved park	ing, HSG B	3			
	519	98	Paved park	ing, HSG C	${\tt C}$			
	588	98	Paved park	ing, HSG D)			
•	7,612	98	Weighted Average					
	7,612		100.00% Im	npervious A	Area			
Tc	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C40: CB #40

Runoff = 0.46 cfs @ 12.09 hrs, Volume= 1,656 cf, Depth> 4.72"

Routed to Pond CB40 : CB #40

A	rea (sf)	CN E	CN Description							
	4,211	98 F	98 Paved parking, HSG B							
	4,211	1	100.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
6.0					Direct Entry,					

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 139

Summary for Subcatchment C41: CB #41

Runoff = 0.61 cfs @ 12.09 hrs, Volume=

2,197 cf, Depth> 4.72"

Routed to Pond CB41: CB #41

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN [Description						
	5,586	98 F	Paved parking, HSG B						
	5,586	1	100.00% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment C43: CB #43

Runoff = 0.30 cfs @ 12.09 hrs, Volume=

967 cf, Depth> 3.73"

Routed to Pond CB43: CB #43

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description						
	2,343	98	Paved parking, HSG B						
	766	61	>75% Gras	>75% Grass cover, Good, HSG B					
	3,109	89	Weighted Average						
	766		24.64% Per	rvious Area	a				
	2,343		75.36% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft	•	Capacity (cfs)	·				
6.0					Direct Entry,				

Summary for Subcatchment C44: CB #44

Runoff = 0.20 cfs @ 12.09 hrs, Volume= 667 cf, Depth> 4.05"

Routed to Pond CB44: CB #44

Area (sf)	CN	Description			
1,670	98	Paved parking, HSG B			
308	61	>75% Grass cover, Good, HSG B			
1,978	92	Weighted Average			
308		15.57% Pervious Area			
1,670		84.43% Impervious Area			

Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 140

Тс	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•	

6.0 Direct Entry,

Summary for Subcatchment C45: CB #45

Runoff = 0.19 cfs @ 12.09 hrs, Volume= 587 cf, Depth> 2.86"

Routed to Pond CB45: CB #45

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description							
	1,240	98	Paved parking, HSG B							
	1,225	61	>75% Grass cover, Good, HSG B							
	2,465	80	Weighted Average							
	1,225		49.70% Pervious Area							
	1,240	:	50.30% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	·					
6.0	(ICCI)	(11/11)	(10/360)	(013)	Direct Entry,					
0.0					Direct Entry,					

Summary for Subcatchment C46: CB #46

Runoff = 0.33 cfs @ 12.09 hrs, Volume= 1,046 cf, Depth> 2.86"

Routed to Pond CB46: CB #46

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Aı	rea (sf)	CN	<u>Description</u>					
	2,241	98	Paved park	ing, HSG B	В			
	2,156	61	>75% Grass cover, Good, HSG B					
	4,397	80	Weighted A					
	2,156		49.03% Pervious Area					
	2,241		50.97% lmp	pervious Ar	rea			
т.	ما المراجعة	Clana	Valacity	Canacity	Description			
Tc	Length	Slope	,	Capacity	·			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C47: CB #47

Runoff = 0.33 cfs @ 12.09 hrs, Volume= 1,185 cf, Depth> 4.72"

Routed to Pond CB47: CB#47

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

<u>Page 141</u>

A	rea (sf)	CN E	Description						
	3,012	98 F	Paved roads w/curbs & sewers, HSG B						
	3,012	1	100.00% Impervious Area						
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment C48: CB #48

Runoff = 2.61 cfs @ 12.17 hrs, Volume=

10,030 cf, Depth> 2.00"

Routed to Pond CB48: CB#48

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN E							
	3,877	98 F	98 Paved roads w/curbs & sewers, HSG B						
	56,251	68 1	68 1 acre lots, 20% imp, HSG B						
	60,128	70 V	Veighted A	verage					
	45,001	7	4.84% Per	vious Area					
	15,127	2	5.16% lmp	ervious Ar	ea				
·									
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
7.0	50	0.0800	0.12		Sheet Flow,				
					Woods: Light underbrush n= 0.400 P2= 3.27"				
4.8	350	0.0600	1.22		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
11.8	400	Total	•						

Summary for Subcatchment C49: CB #49

Runoff = 0.55 cfs @ 12.09 hrs, Volume= 1,862 cf, Depth> 4.27"

Routed to Pond CB49: CB #49

 Area (sf)	CN	Description			
4,431	98	Paved roads w/curbs & sewers, HSG C			
 807	74	>75% Grass cover, Good, HSG C			
5 000		Weighted Average			
5,238	94	Weighted Average			
5,238 807	94	Weighted Average 15.41% Pervious Area			

Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 142

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment C50: CB #50

1.55 cfs @ 12.09 hrs, Volume=

5,209 cf, Depth> 4.16"

Routed to Pond CB50: CB #50

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Ar	ea (sf)	CN	Description						
	3,429	74	>75% Grass cover, Good, HSG C						
	11,611	98	Paved roads w/curbs & sewers, HSG C						
•	15,040	40 93 Weighted Average							
	3,429 22.80% Pervious Area				l e e e e e e e e e e e e e e e e e e e				
•	11,611		77.20% lmp	ervious Ar	rea				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment C51: CB #51

0.74 cfs @ 12.09 hrs, Volume= 2,684 cf, Depth> 4.72" Runoff

Routed to Pond CB51: CB #51

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

	Α	rea (sf)	CN	Description	Description					
		3,147	98	Roofs, HSG C						
		3,676	98	Paved park	Paved parking, HSG C					
		6,823	98	Weighted Average						
		6,823		100.00% Impervious Area						
	Tc	Length	Slop	e Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Summary for Subcatchment C52: CB#52

3,302 cf, Depth> 4.38" Runoff 0.96 cfs @ 12.09 hrs, Volume=

Routed to Pond CB52: CB #52

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 143

A	rea (sf)	CN	Description					
	1,164	74	>75% Gras	s cover, Go	od, HSG C			
	7,888	98	Paved parking, HSG C					
	9,052	95	Weighted A	verage				
	1,164		12.86% Pervious Area					
	7,888		87.14% Impervious Area					
_								
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry			

Summary for Subcatchment C53: CB #53

Runoff = 0.81 cfs @ 12.09 hrs, Volume= 2,723 cf, Depth> 4.16"

Routed to Pond CB53: CB #53

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description					
	287	39	>75% Gras	s cover, Go	ood, HSG A			
	3,287	98	Paved park	ing, HSG A	4			
	773	74	>75% Gras	s cover, Go	ood, HSG C			
	3,516	98	Paved parking, HSG C					
	7,863	93	Weighted A	verage				
	1,060		13.48% Per	rvious Area	l			
	6,803		86.52% Imp	pervious Ar	rea			
Tc	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C54: CB #54

Runoff = 0.48 cfs @ 12.09 hrs, Volume= 1,583 cf, Depth> 3.94"

Routed to Pond CB54: CB #54

_	Area (sf)	CN	Description				
	550	39	>75% Grass cover, Good, HSG A				
	4,176	98	Paved parking, HSG A				
	84	74	>75% Grass cover, Good, HSG C				
_	11	98	Paved parking, HSG C				
	4,821	91	Weighted Average				
	634		13.15% Pervious Area				
	4,187		86.85% Impervious Area				

Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 144

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
6.0					Direct Entry,		

Summary for Subcatchment C7: CB #5

Runoff = 0.51 cfs @ 12.09 hrs, Volume= 1,829 cf, Depth> 4.72"

Routed to Pond CB7: CB#5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN [Description		
	4,650	98 F	Paved road	s w/curbs &	& sewers, HSG B
	4,650	1	00.00% In	npervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment C8: CB #8

Runoff = 0.57 cfs @ 12.09 hrs, Volume= 1,937 cf, Depth> 4.27"

Routed to Pond CB8 : CB#8

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

_	Aı	rea (sf)	CN [Description					
		4,837	98 F	Paved road	s w/curbs &	& sewers, HSG B			
		613	61 >	>75% Grass cover, Good, HSG B					
		5,450	94 \	Veighted A	verage				
		613	•	11.25% Pervious Area					
		4,837	3	38.75% Imp	pervious Ar	rea			
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•			
	6.0					Direct Entry.			

Summary for Subcatchment C9: CB #9

Runoff = 1.76 cfs @ 12.09 hrs, Volume= 6,257 cf, Depth> 4.60"

Routed to Pond CB9: CB #9

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 145

A	rea (sf)	CN	CN Description				
	31	61	>75% Gras	s cover, Go	ood, HSG B		
	433	98	Paved road	s w/curbs &	& sewers, HSG B		
	904	74	>75% Gras	s cover, Go	ood, HSG C		
	12,073	98	Paved park	ing, HSG C			
	2,305	98	Roofs, HSC	GČ			
	52	80	>75% Gras	s cover, Go	ood, HSG D		
	509	98	Paved park	ing, HSG D			
•	16,307	97	Weighted A	verage			
	987		6.05% Perv	ious Area			
	15,320		93.95% Imp	ervious Ar	ea		
			•				
Tc	Length	Slop	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)	·		
6.0					Direct Entry,		

Summary for Subcatchment CH1: CLUBHOUSE

Runoff = 0.67 cfs @ 12.09 hrs, Volume= 2,343 cf, Depth> 4.49"

Routed to Pond DECH: DRIP #CH

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description		
	5,802	98	Roofs, HSC	G C	
	3	98	Roofs, HSG	G D	
	435	74	>75% Gras	s cover, Go	lood, HSG C
	22	80	>75% Gras	s cover, Go	lood, HSG D
	6,262	96	Weighted A	verage	
	457		7.30% Perv	ious Area	
	5,805		92.70% Imp	ervious Ar	rea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment MB1: MAIL KIOSK

Runoff = 0.10 cfs @ 12.09 hrs, Volume= 369 cf, Depth> 4.72"

Routed to Link AP2: ANALYSIS POINT 2

 Area (sf)	CN	Description
938	98	Roofs, HSG B
938		100.00% Impervious Area

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 146

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment S201: SUMMER STREET ACCESS APRON

Runoff = 1.05 cfs @ 12.09 hrs, Volume=

3,627 cf, Depth> 4.38"

Routed to Link AP1: ANALYSIS POINT 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description						
	717	61	>75% Gras	s cover, Go	ood, HSG B				
	9,226	98	Paved park	Paved parking, HSG B					
	9,943	95	Veighted Average						
	717		7.21% Pervious Area						
	9,226		92.79% Imp	pervious Ar	rea				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description				
6.0	(1001)	(14,11)	(1200)	(0.0)	Direct Entry,				

Direct Entry,

Summary for Subcatchment S202: EXISTING WETLAND

Runoff = 18.32 cfs @ 12.33 hrs, Volume=

89,722 cf, Depth> 2.49"

Routed to Reach SC1: Stream Crossing #1

Area (sf)	CN	Description
136,496	61	>75% Grass cover, Good, HSG B
83,935	55	Woods, Good, HSG B
29	98	Paved parking, HSG B
13,946	98	Roofs, HSG B
9,038	48	Brush, Good, HSG B
2,573	74	>75% Grass cover, Good, HSG C
17,121	70	Woods, Good, HSG C
98	98	Paved parking, HSG C
1,097	65	Brush, Good, HSG C
126	80	>75% Grass cover, Good, HSG D
132	98	Paved parking, HSG D
167,678	98	Water Surface, HSG D
432,269	76	Weighted Average
250,386		57.92% Pervious Area
181,883		42.08% Impervious Area

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 147

	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	5.6	50	0.0200	0.15		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.27"
	1.4	118	0.0400	1.40		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	16.2	688	0.0200	0.71		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
_	23.2	856	Total			

Summary for Subcatchment S203: POCKET WETLAND #1

1.45 cfs @ 12.10 hrs, Volume= Runoff

4,616 cf, Depth> 2.16"

Routed to Pond p210: POCKET WETLAND #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Area (sf)	CN	Description					
12,682	61	>75% Grass cover, Good, HSG B					
1,060	98	Water Surface, 0% imp, HSG B					
7,785	74	>75% Grass cover, Good, HSG C					
4,060	98	Water Surface, 0% imp, HSG C					
25,587	72	Weighted Average					
25,587		100.00% Pervious Area					
Tc Length							
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)	_				
6.0		Direct Entry,					

Summary for Subcatchment S204: EXISTING WETLANDS

Runoff 12.75 cfs @ 12.32 hrs, Volume= 61,782 cf, Depth> 2.41"

Routed to Link ap2: ANALYSIS POINT 2

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 148

A	rea (sf)	CN D	escription			
	53,739	61 >	ood, HSG B			
	17,975	55 V	Voods, Go	od, HSG B		
	20,940	48 E	Brush, Goo	d, HSG B		
	41,421	74 >	75% Gras	s cover, Go	ood, HSG C	
	68,342	70 V	Voods, Go	od, HSG C		
	116		•	ing, HSG C		
	1,904		Brush, Goo	•		
	1,528		Brush, Goo	,		
	2,508		,	od, HSG D		
161 98 Paved parking, HSG D						
4,073 73 Brush, Good, HSG D						
	95,496			ace, HSG D)	
308,203 75 Weighted Average						
212,430 68.93% Pervious Area						
95,773 31.07% Impervious Are					ea	
-	1	01	17.1	0: 1	December	
Tc	Length	Slope	Velocity		Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)		
3.2	50	0.2000	0.26		Sheet Flow,	
40.4	500	0.0400	0.50		Grass: Dense n= 0.240 P2= 3.27"	
19.4	582	0.0100	0.50		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
22.6	632	Total				

Summary for Subcatchment S205: ISOLATED WETLAND

noff = 2.89 cfs @ 12.10 hrs, Volume=
Routed to Link AP3 : ANALYSIS POINT 3 Runoff

9,258 cf, Depth> 2.00"

Area (sf)	CN	Description			
10,910	30	Woods, Good, HSG A			
3,684	74	>75% Grass cover, Good, HSG C			
2,275	70	Woods, Good, HSG C			
171	98	Paved parking, HSG C			
1,706	65	Brush, Good, HSG C			
1,940	80	>75% Grass cover, Good, HSG D			
23,513	77	Woods, Good, HSG D			
393	98	Paved parking, HSG D			
2,208	73	Brush, Good, HSG D			
8,620	8,620 98 Water Surface, HSG D				
55,420	70	Weighted Average			
46,236	46,236 83.43% Pervious Area				
9,184		16.57% Impervious Area			

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 149

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment S206: OVERLAND FLOW

Runoff = 19.55 cfs @ 12.52 hrs, Volume=

119,746 cf, Depth> 1.61"

Routed to Link AP4: ANALYSIS POINT #4

Aı	rea (sf)	CN [Description					
	16,514	39 >						
	18,226			od, HSG A				
	713		,	ing, HSG A				
	41,148		Brush, Good, HSG A					
	17,568		1 acre lots, 20% imp, HSG A					
	37,410		>75% Grass cover, Good, HSG B					
	13,900	55 V	, ,					
	54,538	48 E	Brush, Goo	d, HSG B				
	91,202	68 1	l acre lots,	20% imp, ł	HSG B			
	77,444	74 >	75% Gras	s cover, Go	ood, HSG C			
1	14,763		,	od, HSG C				
	3,493			ing, HSG C				
	57,740		Brush, Goo					
	5,763				ood, HSG D			
	26,141		,	od, HSG D				
114,732 98 Water Surface, 0%					o, HSG D			
	91,295		Veighted A					
	65,335	_		rvious Area				
	25,960	2	2.91% Impe	ervious Are	a			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·			
8.5	50	0.0500	0.10		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.27"			
5.0	334	0.0250	1.11		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
1.9 91 0.0250 0.79			Shallow Concentrated Flow,					
					Woodland Kv= 5.0 fps			
10.2	491	0.0400	0.80		Shallow Concentrated Flow, BRUSH			
					Kv= 4.0 fps			
8.9	501	0.0350	0.94		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
34.5	1,467	Total						

Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 150

Summary for Subcatchment S207: INFILTRATION POND #2

Runoff 1.94 cfs @ 12.09 hrs, Volume=

6,290 cf, Depth> 3.63"

Routed to Pond P207: INFILTRATION POND #2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Area (:	sf) CN	Description			
8	39 98	Water Surfa	ace, 0% im	np, HSG A	
8,8	02 74	>75% Gras	s cover, Go	Good, HSG C	
11,1	62 98	Water Surfa	ace, 0% imp	np, HSG C	
20,8	03 88	Weighted A	verage		
20,8	03	100.00% P	ervious Are	ea	
			_		
Tc Len		,	Capacity	•	
(min) (fe	eet) (ft/	ft) (ft/sec)	(cfs)		
6.0				Direct Entry,	

Summary for Subcatchment S208: GRASS AREA

2,769 cf, Depth> 2.41"

0.88 cfs @ 12.09 hrs, Volume=

Routed to Pond OCS4: OCS#4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Area	ı (sf)	CN	Description							
	476	39	>75% Gras	s cover, Go	Good, HSG A					
12	,000	74	>75% Gras	s cover, Go	Good, HSG C					
	168	98	Paved park	ing, HSG A	A					
1	,116	98	Paved park	ing, HSG C	C					
13	,760	75	Neighted A	verage						
12	,476	9	90.67% Per	vious Area	a					
1	,284	,	9.33% Impe	ervious Area	ea					
	ength	Slope	Velocity	Capacity	/ Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0			Direct Entry,							

Summary for Subcatchment S209: WETLAND C

19,940 cf, Depth> 2.23" Runoff 3.68 cfs @ 12.42 hrs, Volume=

Routed to Reach 11R: 4x4 Open Bottom Culvert

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 151

A	rea (sf)	CN	Description		
	10,826	39	>75% Gras	s cover, Go	ood, HSG A
	16,826	30	Woods, Go	od, HSG A	
	8,863	74	>75% Gras	s cover, Go	ood, HSG C
	26,084	70	Woods, Go	od, HSG C	
	44,067	98	Water Surfa	ace, 0% imp	p, HSG D
	304	98	Paved park	ing, HSG A	
	103	98	Paved park	ing, HSG C	
1	107,073	73	Weighted A	verage	
1	106,666	!	99.62% Pe	rvious Area	
	407		0.38% Impe	ervious Are	a
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
13.7	50	0.0150	0.06		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.27"
15.2	557	0.0150	0.61		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
28.9	607	Total			

Summary for Subcatchment S210: INFILTRATION POND #1

Runoff = 3.96 cfs @ 12.23 hrs, Volume= 16

16,882 cf, Depth> 2.67"

Routed to Pond P212: INFILTRATION POND #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Α	rea (sf)	CN [Description		
	13,844	98 V	Vater Surfa	ace, 0% imp	o, HSG C
	59,814	74 >	75% Gras	s cover, Go	ood, HSG C
	2,232	65 E	Brush, Goo	d, HSG C	
	75,890	78 V	Veighted A	verage	
	75,890	1	00.00% Pe	ervious Are	a
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.2	50	0.0150	0.13		Sheet Flow,
					Grass: Short n= 0.150 P2= 3.27"
10.3	530	0.0150	0.86		Shallow Concentrated Flow,
					Short Grass Pasture Kv= 7.0 fps
16.5	580	Total			

Summary for Subcatchment S211: S211

Runoff = 1.13 cfs @ 12.09 hrs, Volume= Routed to Pond P205 : INFILTRATION POND #3 3,556 cf, Depth> 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 152

A	rea (sf)	CN	Description						
	8,108	61	>75% Gras	s cover, Go	ood, HSG B				
	7,328	98	Water Surfa	ace, HSG B	В				
	15,436	79	Weighted A	verage					
	8,108		52.53% Pei	rvious Area	a				
	7,328		47.47% lmp	pervious Ar	rea				
_									
Тс	Length	Slope	,	Capacity	·				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment S212: SWALE

Runoff = 1.38 cfs @ 12.35 hrs, Volume=

7,112 cf, Depth> 1.62"

Routed to Reach SC2 : Stream Crossing #2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

	Α	rea (sf)	CN I	Description							
		4,100	61 :	51 >75% Grass cover, Good, HSG B							
		7,192	55 \	Noods, Go	od, HSG B						
		1,180	74	>75% Gras	s cover, Go	ood, HSG C					
		3,436	70 \	Woods, Go	od, HSG C						
		13,180	98 \	Nater Surfa	ace, 0% im	p, HSG D					
		72	98 I	Paved park	ing, HSG E	3					
		22,663	48 I	Brush, Goo	d, HSG B						
		545		Brush, Goo	•						
		107		•	ing, HSG C						
		135			ing, HSG [
_		158	80 :	>75% Gras	s cover, Go	ood, HSG D					
		52,768	65 \	Neighted A	verage						
		52,454			rvious Area						
		314	(0.60% Impe	ervious Are	a					
	Тс	Length	Slope	•		Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	16.1	50	0.0400	0.05		Sheet Flow,					
						Woods: Dense underbrush n= 0.800 P2= 3.27"					
	3.9	232	0.0600	0.98		Shallow Concentrated Flow, BRUSH					
						Kv= 4.0 fps					
	3.1	136	0.0220	0.74		Shallow Concentrated Flow,					
_						Woodland Kv= 5.0 fps					
	23.1	418	Total								

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 153

Summary for Subcatchment S213: COURTYARD

Runoff = 1.56 cfs @ 12.09 hrs, Volume= 4,932 cf, Depth> 2.76"

Routed to Pond 11P: YARD DRAIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description
	5,047	39	>75% Grass cover, Good, HSG A
	1,678	98	Paved parking, HSG A
	168	98	Roofs, HSG A
	532	98	Water Surface, 0% imp, HSG A
	4,518	74	>75% Grass cover, Good, HSG C
	7,080	98	Paved parking, HSG C
	878	98	Roofs, HSG C
	718	98	Water Surface, 0% imp, HSG C
	296	80	>75% Grass cover, Good, HSG D
	492	98	Paved parking, HSG D
	21,407	79	Weighted Average
	11,111		51.90% Pervious Area
	10,296		48.10% Impervious Area
Tc	Length	Slop	• • •
(min)	(feet)	(ft/f	ft) (ft/sec) (cfs)
6.0			Direct Entry,

Summary for Subcatchment T1: Trench Drain 1

Runoff = 1.15 cfs @ 12.09 hrs, Volume= 3,869 cf, Depth> 4.16"

Routed to Pond 5R: TRENCH DRAIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

	Area (s	f) CN	Description	Description						
	1,44	3 74	>75% Gras	s cover, Go	Good, HSG C					
	4,22	8 98	Paved park	ing, HSG C	C					
	1,33	9 80	>75% Gras	s cover, Go	Good, HSG D					
_	4,16	3 98	Paved park	ing, HSG D	D					
	11,17	3 93	Weighted A	verage						
	2,78	2	24.90% Per	rvious Area	a					
	8,39	1	75.10% lmp	pervious Ar	rea					
	Tc Leng	gth Slo	pe Velocity	Capacity	Description					
	(min) (fee	et) (ft.	/ft) (ft/sec)	(cfs)						
	6.0				Direct Entry					

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 154

Summary for Subcatchment T2: Drive Under B2

Runoff = 0.31 cfs @ 12.09 hrs, Volume=

991 cf, Depth> 2.68"

Routed to Reach 11R: 4x4 Open Bottom Culvert

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description		
	1,510	39	>75% Gras	s cover, Go	ood, HSG A
	2,313	98	Paved park	ing, HSG A	A
	77	74	>75% Gras	s cover, Go	ood, HSG C
	545	98	Paved park	ing, HSG C	C
•	4,445	78	Weighted A	verage	
	1,587		35.70% Per	vious Area	a
	2,858		64.30% Imp	ervious Ar	rea
Tc	Length	Slope	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment TH1: TOWN HOUSE #1

Runoff = 0.45 cfs @ 12.09 hrs, Volume= 1,

1,589 cf, Depth> 4.49"

Routed to Pond DE61: DRIP #61

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description							
	3,936	98	Roofs, HSG	G C						
	311	74	>75% Gras	>75% Grass cover, Good, HSG C						
	4,247	96	Weighted A	verage						
	311		7.32% Perv	ious Area						
	3,936		92.68% Imp	ervious Ar	rea					
_				_						
	Length	Slope	,	Capacity	Description					
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment TH10: TOWN HOUSE #10

Runoff = 0.37 cfs @ 12.09 hrs, Volume= 1,301 cf, Depth> 4.49" Routed to Pond DE70 : DRIP #70

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 155

A	rea (sf)	CN	Description						
	3,184	98	Roofs, HSC	G C					
	292	74	>75% Gras	s cover, Go	ood, HSG C				
	3,476	96	Weighted A	verage					
	292		8.40% Perv	ious Area					
	3,184		91.60% lmp	pervious Ar	ea				
_									
Tc	9	Slope	,	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry				

Summary for Subcatchment TH11: TOWN HOUSE #11

Runoff = 0.45 cfs @ 12.09 hrs, Volume=

1,575 cf, Depth> 4.49"

Routed to Pond DE71: DRIP #71

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description							
	3,899	98	Roofs, HSC	G C						
	311	74	>75% Gras	>75% Grass cover, Good, HSG C						
	4,210 311 3,899		Weighted Average 7.39% Pervious Area 92.61% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	•					
6.0					Direct Entry,					

Summary for Subcatchment TH2: TOWN HOUSE #2

Runoff = 0.45 cfs @ 12.09 hrs, Volume=

1,589 cf, Depth> 4.49"

Routed to Pond DE62: DRIP #62

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description					
	3,936	98	Roofs, HSG	C C				
	311	74	>75% Grass cover, Good, HSG C					
	4,247	96	Weighted A	verage				
	311		7.32% Pervious Area					
	3,936		92.68% Imp	ervious Ar	ea			
Tc	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)) (ft/sec)	(cfs)				

6.0

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 156

Summary for Subcatchment TH3: TOWN HOUSE #3

Runoff = 0.32 cfs @ 12.09 hrs, Volume= 1,099 cf, Depth> 4.38"

Routed to Pond DE63: DRIP #63

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

	Area (sf)	CN	Description				
	2,672	98	Roofs, HSC	G C			
	341	74	>75% Grass cover, Good, HSG C				
	3,013	95	Weighted A	verage			
	341		11.32% Pe	rvious Area	a		
	2,672		88.68% Imp	pervious Ar	ırea		
_				_			
	c Length	Slope	,	Capacity	•		
(mir	n) (feet)	(ft/ft) (ft/sec)	(cfs)			
6.	.0				Direct Entry,		

Summary for Subcatchment TH4: TOWN HOUSE #4

Runoff = 0.37 cfs @ 12.09 hrs, Volume= 1,298 cf, Depth> 4.49"

Routed to Pond DE64: DRIP #64

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description				
	3,178	98	Roofs, HSC	G C			
	292	74	>75% Grass cover, Good, HSG C				
	3,470	96	Neighted A	verage			
	292		8.41% Pervious Area				
	3,178	!	91.59% Imp	pervious Ar	rea		
_		0.1			5		
Tc	Length	Slope	,	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment TH5: TOWN HOUSE #5

Runoff = 0.32 cfs @ 12.09 hrs, Volume= 1,100 cf, Depth> 4.38"

Routed to Pond DE65: DRIP #65

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 157

A	rea (sf)	CN	Description				
	2,675	98	Roofs, HSG C				
	341	74	>75% Grass cover, Good, HSG C				
	3,016	95	Neighted A	verage			
	341		11.31% Pei	vious Area	a		
	2,675	;	38.69% Imp	ervious Ar	rea		
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	2005p.101.		
6.0					Direct Entry,		

Summary for Subcatchment TH6: TOWN HOUSE #6

Runoff = 0.36 cfs @ 12.09 hrs, Volume=

1,275 cf, Depth> 4.49"

Routed to Pond DE66: DRIP #66

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Aı	rea (sf)	CN	Description				
•	3,116	98	Roofs, HSG C				
	291	74	>75% Gras	s cover, Go	Good, HSG C		
	3,407 291 3,116		Weighted A 8.54% Perv 91.46% Imp	∕ious Area			
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	•		
6.0					Direct Entry,		

Summary for Subcatchment TH7: TOWN HOUSE #7

Runoff = 0.37 cfs @ 12.09 hrs, Volume= 1,302 cf, Depth> 4.49"

Routed to Pond DE67 : DRIP #67

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

Are	ea (sf)	CN	Description				
	3,189	98	Roofs, HSG	G C			
	292	74	>75% Grass cover, Good, HSG C				
	3,481 292 3,189		Weighted A 8.39% Perv 91.61% Imp	ious Area	ea		
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description		

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 158

Summary for Subcatchment TH8: TOWN HOUSE #8

Runoff = 0.45 cfs @ 12.09 hrs, Volume= 1,576 cf, Depth> 4.49"

Routed to Pond DE68: DRIP #68

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

	Α	rea (sf)	CN	Description				
		3,901	98	Roofs, HSC	G C			
_		311	74	>75% Grass cover, Good, HSG C				
		4,212	96	Weighted A	verage			
		311		7.38% Pervious Area				
		3,901		92.62% lmp	pervious Ar	ea		
	т.	ما المحمد ا	Clana	Valacity	Consoitu	Decemention		
	Tc	Length	Slope	,	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.0					Direct Entry,		

Summary for Subcatchment TH9: TOWN HOUSE #9

Runoff = 0.37 cfs @ 12.09 hrs, Volume= 1,302 cf, Depth> 4.49"

Routed to Pond DE69: DRIP #69

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

A	rea (sf)	CN	Description				
	3,188	98	Roofs, HSC	G C			
	292	74	>75% Grass cover, Good, HSG C				
	3,480	96	Weighted A	verage			
	292		3.39% Perv	ious Area			
	3,188	!	91.61% Imp	pervious Ar	rea		
_		01		0 "			
Tc	Length	Slope	,	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Reach 8R: OVERLAND FLOW

Inflow Area = 11,975 sf, 92.37% Impervious, Inflow Depth > 3.57" for 10YR event

Inflow = 1.10 cfs @ 12.14 hrs, Volume= 3,558 cf

Outflow = 0.26 cfs @ 12.59 hrs, Volume= 3,276 cf, Atten= 76%, Lag= 27.1 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.08 fps, Min. Travel Time= 110.7 min

Avg. Velocity = 0.04 fps, Avg. Travel Time= 214.4 min

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

<u>Page 159</u>

Peak Storage= 1,755 cf @ 12.59 hrs

Average Depth at Peak Storage= 0.06', Surface Width= 50.62' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 28.09 cfs

50.00' x 1.00' deep channel, n= 0.400 Side Slope Z-value= 5.0 '/' Top Width= 60.00' Length= 563.0' Slope= 0.0213 '/'

Inlet Invert= 208.00', Outlet Invert= 196.00'



Summary for Reach 9R: OVERLAND FLOW

Inflow Area = 32,665 sf, 94.81% Impervious, Inflow Depth = 0.00" for 10YR event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

Average Depth at Peak Storage= 0.00'

Bank-Full Depth= 1.00' Flow Area= 30.0 sf. Capacity= 23.45 cfs

20.00' x 1.00' deep channel, n= 0.400

Side Slope Z-value= 10.0 '/' Top Width= 40.00'

Length= 211.0' Slope= 0.0652 '/'

Inlet Invert= 201.75', Outlet Invert= 188.00'



Summary for Reach 10R: OVERLAND FLOW

Inflow Area = 129,716 sf, 63.13% Impervious, Inflow Depth = 0.54" for 10YR event

Inflow = 0.96 cfs @ 12.60 hrs, Volume= 5,876 cf

Outflow = 0.92 cfs @ 12.86 hrs, Volume= 5,876 cf, Atten= 4%, Lag= 15.2 min

Routed to Link AP4: ANALYSIS POINT #4

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 160

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.22 fps, Min. Travel Time= 12.3 min

Avg. Velocity = 0.07 fps, Avg. Travel Time= 38.0 min

Peak Storage= 680 cf @ 12.86 hrs

Average Depth at Peak Storage= 0.19', Surface Width= 23.79'

Bank-Full Depth= 1.00' Flow Area= 30.0 sf, Capacity= 17.57 cfs

20.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 40.00'

Length= 164.0' Slope= 0.0366 '/'

Inlet Invert= 192.00', Outlet Invert= 186.00'



Summary for Reach 11R: 4x4 Open Bottom Culvert

[52] Hint: Inlet/Outlet conditions not evaluated

Inflow Area = 424,818 sf, 45.99% Impervious, Inflow Depth > 1.13" for 10YR event

Inflow = 4.11 cfs @ 12.47 hrs, Volume= 40,067 cf

Outflow = 4.11 cfs @ 12.47 hrs, Volume= 40,059 cf, Atten= 0%, Lag= 0.2 min

Routed to Reach 23R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 1.62 fps, Min. Travel Time= 0.3 min

Avg. Velocity = 0.72 fps, Avg. Travel Time= 0.7 min

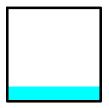
Peak Storage= 76 cf @ 12.47 hrs

Average Depth at Peak Storage= 0.63', Surface Width= 4.00'

Bank-Full Depth= 4.00' Flow Area= 16.0 sf, Capacity= 42.20 cfs

48.0" W x 48.0" H Box Pipe n= 0.069 Riprap, 6-inch Length= 30.0' Slope= 0.0150 '/'

Inlet Invert= 194.00', Outlet Invert= 193.55'



Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

Page 161

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Summary for Reach 12R: OVERLAND FLOW

Inflow Area = 12,906 sf, 90.20% Impervious, Inflow Depth > 3.61" for 10YR event

Inflow = 1.23 cfs @ 12.13 hrs, Volume= 3,881 cf

Outflow = 0.61 cfs @ 12.33 hrs, Volume= 3,787 cf, Atten= 51%, Lag= 12.2 min

Routed to Link AP2: ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.12 fps, Min. Travel Time= 34.1 min

Avg. Velocity = 0.04 fps, Avg. Travel Time= 93.2 min

Peak Storage= 1,246 cf @ 12.33 hrs

Average Depth at Peak Storage= 0.10', Surface Width= 50.99' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 29.80 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'

Length= 250.0' Slope= 0.0240 '/'

Inlet Invert= 202.00', Outlet Invert= 196.00'



Summary for Reach 14R: OVERLAND FLOW

Inflow Area = 52,768 sf, 0.60% Impervious, Inflow Depth > 1.62" for 10YR event

Inflow = 1.38 cfs @ 12.35 hrs, Volume= 7,111 cf

Outflow = 0.31 cfs @ 13.19 hrs, Volume= 6,155 cf, Atten= 77%, Lag= 49.9 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.09 fps, Min. Travel Time= 150.5 min

Avg. Velocity = 0.06 fps, Avg. Travel Time= 226.9 min

Peak Storage= 2,832 cf @ 13.19 hrs

Average Depth at Peak Storage= 0.07', Surface Width= 51.31'

Bank-Full Depth= 1.00' Flow Area= 60.0 sf, Capacity= 31.55 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 70.00'

Length= 852.0' Slope= 0.0246 '/'

Inlet Invert= 207.00', Outlet Invert= 186.00'

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 162



Summary for Reach 15R: OVERLAND FLOW

Inflow Area = 62,582 sf, 52.00% Impervious, Inflow Depth > 1.51" for 10YR event

Inflow = 0.19 cfs @ 15.68 hrs, Volume= 7,883 cf

Outflow = 0.19 cfs @ 16.81 hrs, Volume= 7,158 cf, Atten= 1%, Lag= 67.8 min

Routed to Link AP2: ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.07 fps, Min. Travel Time= 68.4 min

Avg. Velocity = 0.07 fps, Avg. Travel Time= 74.8 min

Peak Storage= 783 cf @ 16.81 hrs

Average Depth at Peak Storage= 0.05', Surface Width= 50.52' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 27.21 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'

Length= 300.0' Slope= 0.0200 '/'

Inlet Invert= 202.00', Outlet Invert= 196.00'



Summary for Reach 18R: OVERLAND FLOW

Inflow Area = 88,676 sf, 39.42% Impervious, Inflow Depth = 0.20" for 10YR event

Inflow = 0.40 cfs @ 12.90 hrs, Volume= 1,449 cf

Outflow = 0.09 cfs @ 14.19 hrs, Volume= 1,312 cf, Atten= 76%, Lag= 77.7 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.06 fps, Min. Travel Time= 167.4 min

Avg. Velocity = 0.04 fps, Avg. Travel Time= 265.5 min

Peak Storage= 953 cf @ 14.19 hrs

Average Depth at Peak Storage= 0.03', Surface Width= 51.54'

Bank-Full Depth= 1.00' Flow Area= 75.0 sf, Capacity= 38.42 cfs

Type III 24-hr 10YR Rainfall=4.96"

19097 Post-Development

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 163

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 100.00'

Length= 609.0' Slope= 0.0279 '/'

Inlet Invert= 203.00', Outlet Invert= 186.00'



Summary for Reach 20R: OVERLAND FLOW

Inflow Area = 72,222 sf, 68.72% Impervious, Inflow Depth > 2.30" for 10YR event

Inflow = 1.79 cfs @ 12.47 hrs, Volume= 13,822 cf

Outflow = 1.03 cfs @ 13.55 hrs, Volume= 13,230 cf, Atten= 43%, Lag= 65.2 min

Routed to Reach 11R: 4x4 Open Bottom Culvert

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.11 fps, Min. Travel Time= 83.0 min

Avg. Velocity = 0.06 fps, Avg. Travel Time= 155.3 min

Peak Storage= 5,110 cf @ 13.55 hrs

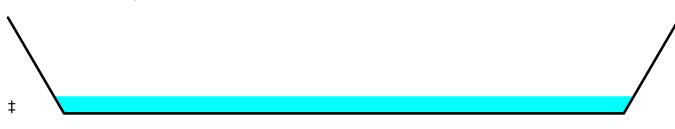
Average Depth at Peak Storage= 0.18', Surface Width= 51.79' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 18.54 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'

Length= 560.0' Slope= 0.0093 '/'

Inlet Invert= 200.00', Outlet Invert= 194.80'



Summary for Reach 23R: OVERLAND FLOW

Inflow Area = 424,818 sf, 45.99% Impervious, Inflow Depth > 1.13" for 10YR event

Inflow = 4.11 cfs @ 12.47 hrs, Volume= 40,059 cf

Outflow = 3.63 cfs @ 12.73 hrs, Volume= 39,543 cf, Atten= 12%, Lag= 15.7 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.22 fps, Min. Travel Time= 17.8 min

Avg. Velocity = 0.10 fps, Avg. Travel Time= 40.2 min

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 164

Peak Storage= 3,866 cf @ 12.73 hrs Average Depth at Peak Storage= 0.29', Surface Width= 61.69'

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Bank-Full Depth= 1.00' Flow Area= 70.0 sf, Capacity= 31.93 cfs

Side Slope Z-value= 20.0 '/' Top Width= 90.00'

Length= 237.0' Slope= 0.0211 '/'

Inlet Invert= 193.00', Outlet Invert= 188.00'



Summary for Reach R202: OVERLAND FLOW

[62] Hint: Exceeded Reach SC1 OUTLET depth by 0.21' @ 13.05 hrs

Inflow Area = 432,269 sf, 42.08% Impervious, Inflow Depth > 2.49" for 10YR event

Inflow = 18.32 cfs @ 12.33 hrs, Volume= 89,705 cf

Outflow = 8.31 cfs @ 12.74 hrs, Volume= 84,798 cf, Atten= 55%, Lag= 24.8 min

Routed to Link AP2: ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.19 fps, Min. Travel Time= 60.1 min Avg. Velocity = 0.09 fps, Avg. Travel Time= 135.3 min

Peak Storage= 29,976 cf @ 12.74 hrs

Average Depth at Peak Storage= 0.39', Surface Width= 119.51' Bank-Full Depth= 1.00' Flow Area= 125.0 sf, Capacity= 42.56 cfs

100.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 150.00'

Length= 700.0' Slope= 0.0107 '/'

Inlet Invert= 205.50', Outlet Invert= 198.00'



Summary for Reach R211: OVERLAND FLOW

Inflow Area = 241,078 sf, 59.10% Impervious, Inflow Depth = 0.31" for 10YR event

Inflow = 3.94 cfs @ 12.51 hrs, Volume= 6,192 cf

Outflow = 0.78 cfs @ 13.04 hrs, Volume= 5,906 cf, Atten= 80%, Lag= 31.9 min

Routed to Reach 11R: 4x4 Open Bottom Culvert

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023 Page 165

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.11 fps, Min. Travel Time= 92.1 min

Avg. Velocity = 0.05 fps, Avg. Travel Time= 217.1 min

Peak Storage= 4,295 cf @ 13.04 hrs

Average Depth at Peak Storage= 0.19', Surface Width= 40.68'

Bank-Full Depth= 1.00' Flow Area= 50.0 sf, Capacity= 14.51 cfs

35.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 15.0 '/' Top Width= 65.00'

Length= 600.0' Slope= 0.0087 '/'

Inlet Invert= 200.00', Outlet Invert= 194.80'



Summary for Reach SC1: Stream Crossing #1

[52] Hint: Inlet/Outlet conditions not evaluated

Inflow Area = 432,269 sf, 42.08% Impervious, Inflow Depth > 2.49" for 10YR event

Inflow = 18.32 cfs @ 12.33 hrs, Volume= 89,722 cf

Outflow = 18.32 cfs @ 12.33 hrs, Volume= 89,705 cf, Atten= 0%, Lag= 0.2 min

Routed to Reach R202: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 3.33 fps, Min. Travel Time= 0.2 min

Avg. Velocity = 1.19 fps, Avg. Travel Time= 0.6 min

Peak Storage= 237 cf @ 12.33 hrs

Average Depth at Peak Storage= 0.34', Surface Width= 16.00'

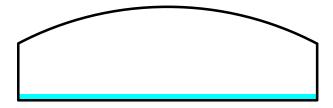
Bank-Full Depth= 5.00' Flow Area= 69.8 sf, Capacity= 722.91 cfs

192.0" W x 60.0" H, R=207.0" Arch Pipe

n= 0.030 Stream, clean & straight

Length= 43.1' Slope= 0.0200 '/'

Inlet Invert= 206.37', Outlet Invert= 205.51'



Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 166

Summary for Reach SC2: Stream Crossing #2

[52] Hint: Inlet/Outlet conditions not evaluated

52,768 sf, 0.60% Impervious, Inflow Depth > 1.62" for 10YR event Inflow Area =

Inflow 1.38 cfs @ 12.35 hrs, Volume= 7.112 cf

Outflow 1.38 cfs @ 12.35 hrs, Volume= 7,111 cf, Atten= 0%, Lag= 0.3 min

Routed to Reach 14R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 1.32 fps, Min. Travel Time= 0.5 min Avg. Velocity = 1.05 fps, Avg. Travel Time= 0.6 min

Peak Storage= 38 cf @ 12.35 hrs

Average Depth at Peak Storage= 0.07', Surface Width= 16.00' Bank-Full Depth= 5.00' Flow Area= 68.1 sf, Capacity= 768.96 cfs

192.0" W x 60.0" H, R=180.0" Arch Pipe n= 0.030 Stream, clean & straight Length= 36.5' Slope= 0.0241 '/'

Inlet Invert= 208.52', Outlet Invert= 207.64'



Summary for Pond 1P: DMH #33

Inflow Area = 16,111 sf, 93.77% Impervious, Inflow Depth > 4.56" for 10YR event

1.73 cfs @ 12.09 hrs, Volume= Inflow 6,129 cf

1.73 cfs @ 12.09 hrs, Volume= 1.73 cfs @ 12.09 hrs, Volume= 6,129 cf, Atten= 0%, Lag= 0.0 min Outflow

6,129 cf Primary

Routed to Pond OCS6: OCS #6

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.22' @ 12.09 hrs

Flood Elev= 209.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.50'	12.0" Round Culvert L= 46.7' Ke= 0.500
			Inlet / Outlet Invert= 205.50' / 204.33' S= 0.0251 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.69 cfs @ 12.09 hrs HW=206.20' TW=202.11' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.69 cfs @ 2.86 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 167

Summary for Pond 3P: OCS #8

Inflow Area = 12,684 sf, 86.64% Impervious, Inflow Depth > 4.07" for 10YR event

Inflow 1.29 cfs @ 12.09 hrs, Volume= 4.306 cf

1.29 cfs @ 12.09 hrs, Volume= Outflow 4,306 cf, Atten= 0%, Lag= 0.0 min

1.29 cfs @ 12.09 hrs, Volume= Primary 4.306 cf Routed to Pond P214: STORMTECH INFILTRATION SYSTEM #4

Routing by Dvn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 201.47' @ 12.53 hrs

Flood Elev= 206.36'

Device Routing Invert Outlet Devices

#1 200.62' 12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads Primary

Primary OutFlow Max=1.25 cfs @ 12.09 hrs HW=201.21' TW=200.83' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 1.25 cfs @ 2.61 fps)

Summary for Pond 5R: TRENCH DRAIN

Inflow Area = 11,173 sf, 75.10% Impervious, Inflow Depth > 4.16" for 10YR event

Inflow 1.15 cfs @ 12.09 hrs, Volume= 3,869 cf

1.15 cfs @ 12.09 hrs, Volume= 3,869 cf, Atten= 0%, Lag= 0.0 min Outflow

Primary 1.15 cfs @ 12.09 hrs, Volume= 3.869 cf Routed to Pond P206: STORMTECH INFILTRATION SYSTEM #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 198.02' @ 12.09 hrs

Flood Elev= 199.50'

Device Routing Invert Outlet Devices #1 197.22' **8.0" Round Culvert** L= 36.0' Ke= 0.500 Primary Inlet / Outlet Invert= 197.22' / 196.50' S= 0.0200 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=1.12 cfs @ 12.09 hrs HW=198.00' TW=195.61' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.12 cfs @ 3.21 fps)

Summary for Pond 11P: YARD DRAIN

21,407 sf, 48.10% Impervious, Inflow Depth > 2.76" for 10YR event Inflow Area =

Inflow 1.56 cfs @ 12.09 hrs, Volume= 4,932 cf

1.14 cfs @ 12.17 hrs, Volume= 1.14 cfs @ 12.17 hrs, Volume= Outflow 4,891 cf, Atten= 27%, Lag= 4.9 min =

Primary 4,891 cf

Routed to Pond D13: DMH #13

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.37' @ 12.17 hrs Surf.Area= 5,792 sf Storage= 671 cf

Plug-Flow detention time= 18.2 min calculated for 4,881 cf (99% of inflow)

Center-of-Mass det. time= 13.3 min (837.5 - 824.3)

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 168

Volume	Inve	rt Avail.Sto	rage Storage	ge Description	
#1	207.25	5' 5,47	75 cf Custor	om Stage Data (Prismatic)Listed below (Recalc)	
Elevatio (fee 207.2 208.0	et) 25	Surf.Area (sq-ft) 5,050 9,550	Inc.Store (cubic-feet) 0 5,475	Cum.Store (cubic-feet) 0 5,475	
Device	Routing	Invert	Outlet Devic	ces	
#1	Primary	203.25'		nd Culvert L= 61.0' Ke= 0.500	
#2	Device 1	207.25'	n= 0.012 Cc 4.0" x 4.0" F X 4 rows C=	t Invert= 203.25' / 202.94' S= 0.0051 '/' Cc= 0.900 corrugated PP, smooth interior, Flow Area= 0.79 sf Horiz. Orifice/Grate X 4.00 columns = 0.600 in 24.0" x 24.0" Grate (44% open area) veir flow at low heads	

Primary OutFlow Max=1.12 cfs @ 12.17 hrs HW=207.37' TW=203.12' (Dynamic Tailwater) -1=Culvert (Passes 1.12 cfs of 6.60 cfs potential flow) **2=Orifice/Grate** (Weir Controls 1.12 cfs @ 1.14 fps)

Summary for Pond CB10: CB #10

Inflow Area = 6,961 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event

Inflow = 0.76 cfs @ 12.09 hrs, Volume= 2,738 cf

2,738 cf, Atten= 0%, Lag= 0.0 min Outflow

0.76 cfs @ 12.09 hrs, Volume= 0.76 cfs @ 12.09 hrs, Volume= Primary = 2.738 cf

Routed to Pond D5: DMH #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.33' @ 12.09 hrs Flood Elev= 212.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.76'	12.0" Round Culvert L= 33.8' Ke= 0.500
			Inlet / Outlet Invert= 209.76' / 209.59' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.74 cfs @ 12.09 hrs HW=210.31' TW=210.07' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.74 cfs @ 2.38 fps)

Summary for Pond CB11: CB #11

7,173 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event Inflow Area =

Inflow = 0.78 cfs @ 12.09 hrs, Volume= 2,822 cf

Outflow 0.78 cfs @ 12.09 hrs, Volume= 2,822 cf, Atten= 0%, Lag= 0.0 min

0.78 cfs @ 12.09 hrs, Volume= 2,822 cf Primary =

Routed to Pond D5: DMH #5

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 169

Peak Elev= 210.43' @ 12.09 hrs Flood Elev= 213.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.94'	12.0" Round Culvert L= 26.3' Ke= 0.500 Inlet / Outlet Invert= 209.94' / 209.67' S= 0.0103 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.76 cfs @ 12.09 hrs HW=210.42' TW=210.07' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.76 cfs @ 2.95 fps)

Summary for Pond CB12: CB #12

Inflow Area = 5,238 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event

Inflow 0.57 cfs @ 12.09 hrs, Volume= 2.060 cf

0.57 cfs @ 12.09 hrs, Volume= 2,060 cf, Atten= 0%, Lag= 0.0 min 2,060 cf Outflow =

Primary = 0.57 cfs @ 12.09 hrs, Volume=

Routed to Pond 1P: DMH #33

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.06' @ 12.09 hrs

Flood Elev= 209.84'

Device Routing Invert Outlet Devices	
#1 Primary 206.68' 12.0" Round Culvert L= 41.3' Ke= 0.500 Inlet / Outlet Invert= 206.68' / 205.65' S= 0.0249 '/' (n= 0.013 Corrugated PE smooth interior Flow Areas	

Primary OutFlow Max=0.55 cfs @ 12.09 hrs HW=207.05' TW=206.20' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.55 cfs @ 2.08 fps)

Summary for Pond CB13: CB #13

Inflow Area = 10,873 sf, 90.78% Impervious, Inflow Depth > 4.49" for 10YR event

1.16 cfs @ 12.09 hrs, Volume= 4,068 cf Inflow =

Outflow 1.16 cfs @ 12.09 hrs, Volume= 4,068 cf, Atten= 0%, Lag= 0.0 min

1.16 cfs @ 12.09 hrs, Volume= Primary = 4,068 cf

Routed to Pond 1P: DMH #33

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.26' @ 12.09 hrs

Flood Elev= 209.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.70'	12.0" Round Culvert L= 43.7' Ke= 0.500 Inlet / Outlet Invert= 206.70' / 205.61' S= 0.0249 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.13 cfs @ 12.09 hrs HW=207.25' TW=206.20' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.13 cfs @ 2.53 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 170

Summary for Pond CB14: CB #14

Inflow Area = 12,099 sf, 86.22% Impervious, Inflow Depth > 3.94" for 10YR event

Inflow = 1.20 cfs @ 12.09 hrs, Volume= 3,973 cf

Outflow = 1.20 cfs @ 12.09 hrs, Volume= 3,973 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.20 cfs @ 12.09 hrs, Volume= 3,973 cf

Routed to Pond D8: DMH #8

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 201.69' @ 12.09 hrs

Flood Elev= 203.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.79'	12.0" Round Culvert L= 23.2' Ke= 0.500 Inlet / Outlet Invert= 200.79' / 200.67' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.17 cfs @ 12.09 hrs HW=201.67' TW=201.52' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.17 cfs @ 2.14 fps)

Summary for Pond CB15: CB #15

Inflow Area = 6,666 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event

Inflow = 0.73 cfs @ 12.09 hrs, Volume= 2,622 cf

Outflow = 0.73 cfs @ 12.09 hrs, Volume= 2,622 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.73 cfs @ 12.09 hrs, Volume= 2,622 cf

Routed to Pond D8: DMH #8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 201.60' @ 12.09 hrs

Flood Elev= 203.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.79'	12.0" Round Culvert L= 15.6' Ke= 0.500 Inlet / Outlet Invert= 200.79' / 200.71' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.71 cfs @ 12.09 hrs HW=201.58' TW=201.52' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.71 cfs @ 1.45 fps)

Summary for Pond CB16: CB #16

Inflow Area = 8,516 sf, 64.88% Impervious, Inflow Depth > 2.76" for 10YR event

Inflow = 0.62 cfs @ 12.09 hrs, Volume= 1,962 cf

Outflow = 0.62 cfs @ 12.09 hrs, Volume= 1,962 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.62 cfs @ 12.09 hrs, Volume= 1,962 cf

Routed to Pond D10: DMH #10

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

<u>Page 171</u>

Peak Elev= 203.99' @ 12.09 hrs Flood Elev= 206.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.47'	12.0" Round Culvert L= 20.9' Ke= 0.500 Inlet / Outlet Invert= 203.47' / 203.33' S= 0.0067 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.61 cfs @ 12.09 hrs HW=203.98' TW=203.81' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.61 cfs @ 2.20 fps)

Summary for Pond CB17: CB #17

Inflow Area = 11,836 sf, 73.87% Impervious, Inflow Depth > 4.05" for 10YR event

Inflow = 1.20 cfs @ 12.09 hrs, Volume= 3,992 cf

Outflow = 1.20 cfs @ 12.09 hrs, Volume= 3,992 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.20 cfs @ 12.09 hrs, Volume= 3.992 cf

Routed to Pond D11: DMH #11

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.64' @ 12.09 hrs

Flood Elev= 208.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.99'	12.0" Round Culvert L= 13.8' Ke= 0.500 Inlet / Outlet Invert= 204.99' / 204.86' S= 0.0094 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.17 cfs @ 12.09 hrs HW=205.63' TW=205.26' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.17 cfs @ 3.10 fps)

Summary for Pond CB18: CB #18

Inflow Area = 24,853 sf, 72.99% Impervious, Inflow Depth > 3.31" for 10YR event

Inflow = 2.14 cfs @ 12.09 hrs, Volume= 6,857 cf

Outflow = 2.14 cfs @ 12.09 hrs, Volume= 6,857 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.14 cfs @ 12.09 hrs, Volume= 6,857 cf

Routed to Pond D11: DMH #11

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.61' @ 12.09 hrs

Flood Elev= 208.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.72'	15.0" Round Culvert L= 25.1' Ke= 0.500 Inlet / Outlet Invert= 204.72' / 204.59' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.11 cfs @ 12.09 hrs HW=205.60' TW=205.27' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.11 cfs @ 3.22 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 172

Summary for Pond CB20: CB #20

Inflow Area = 11,939 sf, 88.95% Impervious, Inflow Depth > 4.38" for 10YR event

Inflow = 1.26 cfs @ 12.09 hrs, Volume= 4,355 cf

Outflow = 1.26 cfs @ 12.09 hrs, Volume= 4,355 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.26 cfs @ 12.09 hrs, Volume= 4,355 cf

Routed to Pond D12: DMH #12

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.67' @ 12.09 hrs

Flood Elev= 207.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.97'	12.0" Round Culvert L= 30.3' Ke= 0.500 Inlet / Outlet Invert= 203.97' / 203.81' S= 0.0053 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.23 cfs @ 12.09 hrs HW=204.66' TW=204.22' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.23 cfs @ 2.98 fps)

Summary for Pond CB21: CB #21

Inflow Area = 10,174 sf, 87.04% Impervious, Inflow Depth > 3.83" for 10YR event

Inflow = 0.99 cfs @ 12.09 hrs, Volume= 3,251 cf

Outflow = 0.99 cfs @ 12.09 hrs, Volume= 3,251 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.99 cfs @ 12.09 hrs, Volume= 3,251 cf

Routed to Pond D12: DMH #12

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.94' @ 12.09 hrs

Flood Elev= 208.02'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.32'	12.0" Round Culvert L= 26.0' Ke= 0.500 Inlet / Outlet Invert= 204.32' / 204.19' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.97 cfs @ 12.09 hrs HW=204.93' TW=204.22' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.97 cfs @ 2.76 fps)

Summary for Pond CB22: CB #22

Inflow Area = 12,001 sf, 91.62% Impervious, Inflow Depth > 4.49" for 10YR event

Inflow = 1.28 cfs @ 12.09 hrs, Volume= 4,490 cf

Outflow = 1.28 cfs @ 12.09 hrs, Volume= 4,490 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.28 cfs @ 12.09 hrs, Volume= 4,490 cf

Routed to Pond D14: DMH #14

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 173

Peak Elev= 206.05' @ 12.09 hrs

Flood Elev= 208.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.33'	12.0" Round Culvert L= 16.1' Ke= 0.500 Inlet / Outlet Invert= 205.33' / 205.25' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.25 cfs @ 12.09 hrs HW=206.04' TW=205.14' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.25 cfs @ 2.95 fps)

Summary for Pond CB23: CB #23

Inflow Area = 9,694 sf, 61.00% Impervious, Inflow Depth > 3.73" for 10YR event

Inflow 0.93 cfs @ 12.09 hrs, Volume= 3.014 cf

0.93 cfs @ 12.09 hrs, Volume= 3,014 or, 3,014 cf 3,014 cf, Atten= 0%, Lag= 0.0 min Outflow =

0.93 cfs @ 12.09 hrs, Volume= Primary =

Routed to Pond D14: DMH #14

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.00' @ 12.09 hrs

Flood Elev= 208.57'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.41'	12.0" Round Culvert L= 16.3' Ke= 0.500 Inlet / Outlet Invert= 205.41' / 205.32' S= 0.0055 '/' Cc= 0.900 n= 0.012 Corrugated PP smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.90 cfs @ 12.09 hrs HW=205.99' TW=205.14' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.90 cfs @ 2.75 fps)

Summary for Pond CB24: CB #24

Inflow Area = 7,930 sf, 72.16% Impervious, Inflow Depth > 4.16" for 10YR event

0.82 cfs @ 12.09 hrs, Volume= 2,746 cf Inflow =

0.82 cfs @ 12.09 hrs, Volume= Outflow 2,746 cf, Atten= 0%, Lag= 0.0 min

0.82 cfs @ 12.09 hrs, Volume= Primary = 2,746 cf

Routed to Pond D16: DMH #16

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.83' @ 12.09 hrs

Flood Elev= 209.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.21'	12.0" Round Culvert L= 12.1' Ke= 0.500 Inlet / Outlet Invert= 205.21' / 205.15' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.79 cfs @ 12.09 hrs HW=205.82' TW=205.67' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.79 cfs @ 2.29 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 174

Summary for Pond CB25: CB #25

Inflow Area = 8,487 sf, 80.92% Impervious, Inflow Depth > 4.27" for 10YR event

Inflow = 0.89 cfs @ 12.09 hrs, Volume= 3,017 cf

Outflow = 0.89 cfs @ 12.09 hrs, Volume= 3,017 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.89 cfs @ 12.09 hrs, Volume= 3,017 cf

Routed to Pond D16: DMH #16

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 205.82' @ 12.09 hrs

Flood Elev= 208.38'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.22'	15.0" Round Culvert L= 11.4' Ke= 0.500 Inlet / Outlet Invert= 205.22' / 205.16' S= 0.0053 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior. Flow Area= 1.23 sf

Primary OutFlow Max=0.86 cfs @ 12.09 hrs HW=205.81' TW=205.67' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.86 cfs @ 2.24 fps)

Summary for Pond CB26: CB #26

Inflow Area = 8,835 sf, 63.75% Impervious, Inflow Depth > 3.94" for 10YR event

Inflow = 0.88 cfs @ 12.09 hrs, Volume= 2,901 cf

Outflow = 0.88 cfs @ 12.09 hrs, Volume= 2,901 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.88 cfs @ 12.09 hrs, Volume= 2,901 cf

Routed to Pond D17: DMH #17

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 202.34' @ 12.09 hrs

Flood Elev= 204.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	201.77'	12.0" Round Culvert L= 42.5' Ke= 0.500
			Inlet / Outlet Invert= 201.77' / 201.55' S= 0.0052 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.86 cfs @ 12.09 hrs HW=202.33' TW=201.20' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.86 cfs @ 2.75 fps)

Summary for Pond CB27: CB #27

Inflow Area = 6,111 sf, 91.90% Impervious, Inflow Depth > 4.49" for 10YR event

Inflow = 0.65 cfs @ 12.09 hrs, Volume= 2,287 cf

Outflow = 0.65 cfs @ 12.09 hrs, Volume= 2,287 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.65 cfs @ 12.09 hrs, Volume= 2,287 cf

Routed to Pond D17: DMH #17

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 175

Peak Elev= 201.49' @ 12.09 hrs Flood Elev= 204.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	201.00'	12.0" Round Culvert L= 18.0' Ke= 0.500 Inlet / Outlet Invert= 201.00' / 200.90' S= 0.0056 '/' Cc= 0.900 n= 0.013 Corrugated PE. smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.64 cfs @ 12.09 hrs HW=201.48' TW=201.20' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.64 cfs @ 2.50 fps)

Summary for Pond CB28: CB #28

Inflow Area = 10,372 sf, 51.33% Impervious, Inflow Depth > 3.63" for 10YR event

Inflow = 0.97 cfs @ 12.09 hrs, Volume= 3,136 cf

Outflow = 0.97 cfs @ 12.09 hrs, Volume= 3,136 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.97 cfs @ 12.09 hrs, Volume= 3,136 cf

Routed to Pond D18: DMH #18

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 198.44' @ 12.09 hrs

Flood Elev= 200.92'

Device	Routing	Invert	Outlet Devices
#1	Primary	197.75'	12.0" Round Culvert L= 13.7' Ke= 0.500 Inlet / Outlet Invert= 197.75' / 197.69' S= 0.0044 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.95 cfs @ 12.09 hrs HW=198.43' TW=198.26' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.95 cfs @ 2.36 fps)

Summary for Pond CB29: CB #29

Inflow Area = 8,495 sf, 84.21% Impervious, Inflow Depth > 4.27" for 10YR event

Inflow = 0.89 cfs @ 12.09 hrs, Volume= 3.020 cf

Outflow = 0.89 cfs @ 12.09 hrs, Volume= 3,020 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.89 cfs @ 12.09 hrs, Volume= 3,020 cf

Routed to Pond D19: DMH #19

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.08' @ 12.09 hrs

Flood Elev= 208.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.38'	12.0" Round Culvert L= 13.5' Ke= 0.500 Inlet / Outlet Invert= 205.38' / 205.31' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.86 cfs @ 12.09 hrs HW=206.06' TW=205.93' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.86 cfs @ 2.13 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 176

Summary for Pond CB30: CB #30

Inflow Area = 8,933 sf, 82.40% Impervious, Inflow Depth > 4.27" for 10YR event

Inflow = 0.93 cfs @ 12.09 hrs, Volume= 3,175 cf

Outflow = 0.93 cfs @ 12.09 hrs, Volume= 3,175 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.93 cfs @ 12.09 hrs, Volume= 3,175 cf

Routed to Pond D19: DMH #19

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 206.09' @ 12.09 hrs

Flood Elev= 208.54'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.38'	12.0" Round Culvert L= 17.5' Ke= 0.500 Inlet / Outlet Invert= 205.38' / 205.29' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.91 cfs @ 12.09 hrs HW=206.08' TW=205.93' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.91 cfs @ 2.18 fps)

Summary for Pond CB31: CB #31

Inflow Area = 16,365 sf, 68.64% Impervious, Inflow Depth > 3.83" for 10YR event

Inflow = 1.59 cfs @ 12.09 hrs, Volume= 5,230 cf

Outflow = 1.59 cfs @ 12.09 hrs, Volume= 5,230 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.59 cfs @ 12.09 hrs, Volume= 5,230 cf

Routed to Pond D21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.02' @ 12.09 hrs

Flood Elev= 207.36'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.19'	12.0" Round Culvert L= 16.4' Ke= 0.500
			Inlet / Outlet Invert= 204.19' / 204.11' S= 0.0049 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.56 cfs @ 12.09 hrs HW=205.01' TW=204.39' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.56 cfs @ 3.07 fps)

Summary for Pond CB32: CB #32

Inflow Area = 12,710 sf, 70.47% Impervious, Inflow Depth > 3.94" for 10YR event

Inflow = 1.26 cfs @ 12.09 hrs, Volume= 4,174 cf

Outflow = 1.26 cfs @ 12.09 hrs, Volume= 4,174 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.26 cfs @ 12.09 hrs, Volume= 4,174 cf

Routed to Pond D21: DMH #21

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 177

Peak Elev= 204.91' @ 12.09 hrs Flood Elev= 207.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.19'	12.0" Round Culvert L= 16.3' Ke= 0.500 Inlet / Outlet Invert= 204.19' / 204.11' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.23 cfs @ 12.09 hrs HW=204.90' TW=204.39' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.23 cfs @ 2.89 fps)

Summary for Pond CB33: CB #33

Inflow Area = 5,421 sf, 83.90% Impervious, Inflow Depth > 4.27" for 10YR event

Inflow = 0.57 cfs @ 12.09 hrs, Volume= 1.927 cf

0.57 cfs @ 12.09 hrs, Volume= Outflow = 1,927 cf, Atten= 0%, Lag= 0.0 min

1,927 Gi, 1,927 cf Primary = 0.57 cfs @ 12.09 hrs, Volume=

Routed to Pond D22: DMH #22

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.84' @ 12.09 hrs

Flood Elev= 208.45'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.28'	12.0" Round Culvert L= 11.7' Ke= 0.500 Inlet / Outlet Invert= 205.28' / 205.22' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.55 cfs @ 12.09 hrs HW=205.82' TW=205.72' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.55 cfs @ 1.83 fps)

Summary for Pond CB34: CB #34

Inflow Area = 8,622 sf, 80.51% Impervious, Inflow Depth > 4.16" for 10YR event

0.89 cfs @ 12.09 hrs, Volume= 2.986 cf Inflow =

0.89 cfs @ 12.09 hrs, Volume= Outflow 2,986 cf, Atten= 0%, Lag= 0.0 min =

0.89 cfs @ 12.09 hrs, Volume= Primary = 2,986 cf

Routed to Pond D22: DMH #22

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.89' @ 12.09 hrs

Flood Elev= 208.38'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.21'	12.0" Round Culvert L= 16.5' Ke= 0.500 Inlet / Outlet Invert= 205.21' / 205.13' S= 0.0048 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.86 cfs @ 12.09 hrs HW=205.88' TW=205.72' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.86 cfs @ 2.21 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 178

Summary for Pond CB35: CB #35

Inflow Area = 4,149 sf, 98.10% Impervious, Inflow Depth > 4.72" for 10YR event

Inflow = 0.45 cfs @ 12.09 hrs, Volume= 1,632 cf

Outflow = 0.45 cfs @ 12.09 hrs, Volume= 1,632 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.45 cfs @ 12.09 hrs, Volume= 1,632 cf

Routed to Pond D23: DMH #23

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.44' @ 12.09 hrs

Flood Elev= 210.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	207.04'	12.0" Round Culvert L= 15.2' Ke= 0.500 Inlet / Outlet Invert= 207.04' / 206.96' S= 0.0053 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.44 cfs @ 12.09 hrs HW=207.44' TW=207.22' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.44 cfs @ 2.24 fps)

Summary for Pond CB36: CB #36

Inflow Area = 6,622 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event

Inflow = 0.72 cfs @ 12.09 hrs, Volume= 2,605 cf

Outflow = 0.72 cfs @ 12.09 hrs, Volume= 2,605 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.72 cfs @ 12.09 hrs, Volume= 2,605 cf

Routed to Pond D23: DMH #23

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.56' @ 12.09 hrs

Flood Elev= 210.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	207.04'	12.0" Round Culvert L= 16.1' Ke= 0.500 Inlet / Outlet Invert= 207.04' / 206.96' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.70 cfs @ 12.09 hrs HW=207.55' TW=207.22' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.70 cfs @ 2.50 fps)

Summary for Pond CB38: CB #38

Inflow Area = 7,637 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event

Inflow = 0.83 cfs @ 12.09 hrs, Volume= 3,004 cf

Outflow = 0.83 cfs @ 12.09 hrs, Volume= 3,004 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.83 cfs @ 12.09 hrs, Volume= 3,004 cf

Routed to Pond D25: DMH #25

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 179

Peak Elev= 210.56' @ 12.09 hrs

Flood Elev= 212.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.69'	12.0" Round Culvert L= 16.7' Ke= 0.500 Inlet / Outlet Invert= 209.69' / 209.61' S= 0.0048 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.81 cfs @ 12.09 hrs HW=210.54' TW=210.47' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.81 cfs @ 1.54 fps)

Summary for Pond CB39: CB #39

Inflow Area = 7,612 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event

Inflow 0.83 cfs @ 12.09 hrs, Volume= 2.994 cf

0.83 cfs @ 12.09 hrs, Volume= 2,994 cf, Atten= 0%, Lag= 0.0 min 2,994 cf Outflow =

0.83 cfs @ 12.09 hrs, Volume= Primary =

Routed to Pond D25: DMH #25

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.56' @ 12.09 hrs

Flood Elev= 212.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.69'	12.0" Round Culvert L= 16.4' Ke= 0.500 Inlet / Outlet Invert= 209.69' / 209.61' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior Flow Area= 0.79 sf

Primary OutFlow Max=0.81 cfs @ 12.09 hrs HW=210.54' TW=210.47' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.81 cfs @ 1.53 fps)

Summary for Pond CB40: CB #40

Inflow Area = 4,211 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event

0.46 cfs @ 12.09 hrs, Volume= 1,656 cf Inflow =

Outflow 0.46 cfs @ 12.09 hrs, Volume= 1,656 cf, Atten= 0%, Lag= 0.0 min

0.46 cfs @ 12.09 hrs, Volume= Primary = 1,656 cf

Routed to Pond D27: DMH #27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 214.16' @ 12.09 hrs

Flood Elev= 217.04'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.68'	12.0" Round Culvert L= 17.8' Ke= 0.500 Inlet / Outlet Invert= 213.68' / 213.55' S= 0.0073 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.45 cfs @ 12.09 hrs HW=214.15' TW=214.04' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.45 cfs @ 1.78 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 180

Summary for Pond CB41: CB #41

Inflow Area = 5,586 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event

Inflow = 0.61 cfs @ 12.09 hrs, Volume= 2,197 cf

Outflow = 0.61 cfs @ 12.09 hrs, Volume= 2,197 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.61 cfs @ 12.09 hrs, Volume= 2,197 cf

Routed to Pond D27: DMH #27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 214.37' @ 12.09 hrs

Flood Elev= 217.06'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.89'	12.0" Round Culvert L= 18.4' Ke= 0.500 Inlet / Outlet Invert= 213.89' / 213.80' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.59 cfs @ 12.09 hrs HW=214.36' TW=214.04' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.59 cfs @ 2.40 fps)

Summary for Pond CB43: CB #43

Inflow Area = 3,109 sf, 75.36% Impervious, Inflow Depth > 3.73" for 10YR event

Inflow = 0.30 cfs @ 12.09 hrs, Volume= 967 cf

Outflow = 0.30 cfs @ 12.09 hrs, Volume= 967 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.30 cfs @ 12.09 hrs, Volume= 967 cf

Routed to Pond D29: DMH #29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 220.42' @ 12.09 hrs

Flood Elev= 223.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	220.00'	12.0" Round Culvert L= 14.9' Ke= 0.500
			Inlet / Outlet Invert= 220.00' / 219.93' S= 0.0047 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.29 cfs @ 12.09 hrs HW=220.41' TW=220.34' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.29 cfs @ 1.40 fps)

Summary for Pond CB44: CB #44

Inflow Area = 1,978 sf, 84.43% Impervious, Inflow Depth > 4.05" for 10YR event

Inflow = 0.20 cfs @ 12.09 hrs, Volume= 667 cf

Outflow = 0.20 cfs @ 12.09 hrs, Volume= 667 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.20 cfs @ 12.09 hrs, Volume= 667 cf

Routed to Pond D29: DMH #29

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 181

Peak Elev= 220.39' @ 12.09 hrs

Flood Elev= 223.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	220.00'	12.0" Round Culvert L= 14.9' Ke= 0.500 Inlet / Outlet Invert= 220.00' / 219.93' S= 0.0047 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.20 cfs @ 12.09 hrs HW=220.38' TW=220.34' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.20 cfs @ 1.05 fps)

Summary for Pond CB45: CB #45

Inflow Area = 2,465 sf, 50.30% Impervious, Inflow Depth > 2.86" for 10YR event

Inflow = 0.19 cfs @ 12.09 hrs, Volume= 587 cf

Outflow = 0.19 cfs @ 12.09 hrs, Volume= 587 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.19 cfs @ 12.09 hrs, Volume= 587 cf

Routed to Pond D30: DMH #30

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 221.54' @ 12.09 hrs

Flood Elev= 224.46'

Device	Routing	Invert	Outlet Devices
#1	Primary	221.29'	12.0" Round Culvert L= 18.2' Ke= 0.500 Inlet / Outlet Invert= 221.29' / 221.20' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior Flow Area= 0.79 sf

Primary OutFlow Max=0.18 cfs @ 12.09 hrs HW=221.54' TW=221.33' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.18 cfs @ 1.78 fps)

Summary for Pond CB46: CB #46

Inflow Area = 4,397 sf, 50.97% Impervious, Inflow Depth > 2.86" for 10YR event

Inflow = 0.33 cfs @ 12.09 hrs, Volume= 1.046 cf

Outflow = 0.33 cfs @ 12.09 hrs, Volume= 1,046 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.33 cfs @ 12.09 hrs, Volume= 1,046 cf

Routed to Pond D30: DMH #30

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 221.87' @ 12.09 hrs

Flood Elev= 224.69'

Device	Routing	Invert	Outlet Devices
#1	Primary	221.53'	12.0" Round Culvert L= 15.3' Ke= 0.500 Inlet / Outlet Invert= 221.53' / 221.45' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.33 cfs @ 12.09 hrs HW=221.87' TW=221.33' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.33 cfs @ 2.07 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

Page 182

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Summary for Pond CB47: CB#47

Inflow Area = 3,012 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event

Inflow = 0.33 cfs @ 12.09 hrs, Volume= 1,185 cf

Outflow = 0.33 cfs @ 12.09 hrs, Volume= 1,185 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.33 cfs @ 12.09 hrs, Volume= 1,185 cf

Routed to Pond D31: DMH#31

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 225.52' @ 12.15 hrs

Flood Elev= 230.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	225.05'	12.0" Round Culvert L= 20.9' Ke= 0.500 Inlet / Outlet Invert= 225.05' / 224.95' S= 0.0048 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.33 cfs @ 12.09 hrs HW=225.47' TW=225.38' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.33 cfs @ 1.53 fps)

Summary for Pond CB48: CB#48

Inflow Area = 60,128 sf, 25.16% Impervious, Inflow Depth > 2.00" for 10YR event

Inflow = 2.61 cfs @ 12.17 hrs, Volume= 10,030 cf

Outflow = 2.61 cfs @ 12.17 hrs, Volume= 10,030 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.61 cfs @ 12.17 hrs, Volume= 10,030 cf

Routed to Pond D31: DMH#31

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 225.81' @ 12.17 hrs

Flood Elev= 230.25'

Device	Routing	Invert	Outlet Devices
#1	Primary	224.82'	15.0" Round Culvert L= 16.9' Ke= 0.500
			Inlet / Outlet Invert= 224.82' / 224.74' S= 0.0047 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.56 cfs @ 12.17 hrs HW=225.80' TW=225.48' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.56 cfs @ 3.40 fps)

Summary for Pond CB49: CB #49

Inflow Area = 5,238 sf, 84.59% Impervious, Inflow Depth > 4.27" for 10YR event

Inflow = 0.55 cfs @ 12.09 hrs, Volume= 1,862 cf

Outflow = 0.55 cfs @ 12.09 hrs, Volume= 1,862 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.55 cfs @ 12.09 hrs, Volume= 1,862 cf

Routed to Pond DMH32: DMH #32

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 183

Peak Elev= 203.45' @ 12.09 hrs Flood Elev= 205.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.76'	12.0" Round Culvert L= 15.5' Ke= 0.500 Inlet / Outlet Invert= 202.76' / 202.68' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.53 cfs @ 12.09 hrs HW=203.44' TW=203.39' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.53 cfs @ 1.32 fps)

Summary for Pond CB50: CB #50

Inflow Area = 15,040 sf, 77.20% Impervious, Inflow Depth > 4.16" for 10YR event

Inflow 1.55 cfs @ 12.09 hrs, Volume= 5,209 cf

5,209 ci, 5,209 cf 1.55 cfs @ 12.09 hrs, Volume= Outflow = 5,209 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.55 cfs @ 12.09 hrs, Volume=

Routed to Pond DMH32: DMH #32

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.64' @ 12.09 hrs

Flood Elev= 205.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.78'	12.0" Round Culvert L= 15.3' Ke= 0.500 Inlet / Outlet Invert= 202.78' / 202.70' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.51 cfs @ 12.09 hrs HW=203.63' TW=203.39' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.51 cfs @ 2.86 fps)

Summary for Pond CB51: CB #51

Inflow Area = 6,823 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event

0.74 cfs @ 12.09 hrs, Volume= 2,684 cf Inflow =

Outflow 0.74 cfs @ 12.09 hrs, Volume= 2,684 cf, Atten= 0%, Lag= 0.0 min

0.74 cfs @ 12.09 hrs, Volume= Primary = 2,684 cf

Routed to Pond OCS7: OCS #7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 202.87' @ 12.09 hrs

Flood Elev= 212.77'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.35'	12.0" Round Culvert L= 31.4' Ke= 0.500 Inlet / Outlet Invert= 202.35' / 202.19' S= 0.0051 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.72 cfs @ 12.09 hrs HW=202.86' TW=202.47' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.72 cfs @ 2.59 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 184

Summary for Pond CB52: CB #52

Inflow Area = 9,052 sf, 87.14% Impervious, Inflow Depth > 4.38" for 10YR event

Inflow = 0.96 cfs @ 12.09 hrs, Volume= 3,302 cf

Outflow = 0.96 cfs @ 12.09 hrs, Volume= 3,302 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.96 cfs @ 12.09 hrs, Volume= 3,302 cf

Routed to Pond OCS7: OCS #7

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 203.29' @ 12.09 hrs

Flood Elev= 205.84'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.68'	12.0" Round Culvert L= 25.5' Ke= 0.500 Inlet / Outlet Invert= 202.68' / 202.55' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.93 cfs @ 12.09 hrs HW=203.28' TW=202.48' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.93 cfs @ 2.74 fps)

Summary for Pond CB53: CB #53

Inflow Area = 7,863 sf, 86.52% Impervious, Inflow Depth > 4.16" for 10YR event

Inflow = 0.81 cfs @ 12.09 hrs, Volume= 2,723 cf

Outflow = 0.81 cfs @ 12.09 hrs, Volume= 2,723 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.81 cfs @ 12.09 hrs, Volume= 2,723 cf

Routed to Pond 3P: OCS #8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.33' @ 12.09 hrs

Flood Elev= 205.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.78'	12.0" Round Culvert L= 32.0' Ke= 0.500 Inlet / Outlet Invert= 202.78' / 202.62' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.79 cfs @ 12.09 hrs HW=203.32' TW=201.21' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.79 cfs @ 2.64 fps)

Summary for Pond CB54: CB #54

Inflow Area = 4,821 sf, 86.85% Impervious, Inflow Depth > 3.94" for 10YR event

Inflow = 0.48 cfs @ 12.09 hrs, Volume= 1,583 cf

Outflow = 0.48 cfs @ 12.09 hrs, Volume= 1,583 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.48 cfs @ 12.09 hrs, Volume= 1,583 cf

Routed to Pond 3P: OCS #8

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 185

Peak Elev= 203.07' @ 12.09 hrs

Flood Elev= 205.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.66'	12.0" Round Culvert L= 36.7' Ke= 0.500 Inlet / Outlet Invert= 202.66' / 202.48' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.47 cfs @ 12.09 hrs HW=203.07' TW=201.21' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.47 cfs @ 2.31 fps)

Summary for Pond CB7: CB#5

Inflow Area = 4,650 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event

Inflow 0.51 cfs @ 12.09 hrs, Volume= 1.829 cf

0.51 cfs @ 12.09 hrs, Volume= Outflow = 1,829 cf, Atten= 0%, Lag= 0.0 min

1,029 o., 1,829 cf Primary = 0.51 cfs @ 12.09 hrs, Volume=

Routed to Pond D4: DMH#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 212.99' @ 12.09 hrs

Flood Elev= 215.79'

Device	Routing	Invert	Outlet Devices
#1	Primary	212.60'	12.0" Round Culvert L= 15.1' Ke= 0.500 Inlet / Outlet Invert= 212.60' / 212.45' S= 0.0099 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.49 cfs @ 12.09 hrs HW=212.98' TW=211.29' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.49 cfs @ 2.67 fps)

Summary for Pond CB8: CB#8

Inflow Area = 5,450 sf, 88.75% Impervious, Inflow Depth > 4.27" for 10YR event

0.57 cfs @ 12.09 hrs, Volume= 1,937 cf Inflow =

Outflow 0.57 cfs @ 12.09 hrs, Volume= 1,937 cf, Atten= 0%, Lag= 0.0 min

0.57 cfs @ 12.09 hrs, Volume= Primary = 1,937 cf

Routed to Pond D4: DMH#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 214.21' @ 12.09 hrs

Flood Elev= 215.79'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.79'	12.0" Round Culvert L= 15.1' Ke= 0.500 Inlet / Outlet Invert= 213.79' / 213.64' S= 0.0099 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.55 cfs @ 12.09 hrs HW=214.20' TW=211.30' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.55 cfs @ 2.68 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 186

Summary for Pond CB9: CB #9

Inflow Area = 16,307 sf, 93.95% Impervious, Inflow Depth > 4.60" for 10YR event

Inflow = 1.76 cfs @ 12.09 hrs, Volume= 6,257 cf

Outflow = 1.76 cfs @ 12.09 hrs, Volume= 6,257 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.76 cfs @ 12.09 hrs, Volume= 6,257 cf

Routed to Pond D5: DMH #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.82' @ 12.09 hrs

Flood Elev= 213.27'

Device	Routing	Invert	Outlet Devices
#1	Primary	210.10'	12.0" Round Culvert L= 19.9' Ke= 0.500 Inlet / Outlet Invert= 210.10' / 209.71' S= 0.0196 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.71 cfs @ 12.09 hrs HW=210.81' TW=210.07' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.71 cfs @ 2.87 fps)

Summary for Pond D10: DMH #10

Inflow Area = 8,516 sf, 64.88% Impervious, Inflow Depth > 2.76" for 10YR event

Inflow = 0.62 cfs @ 12.09 hrs, Volume= 1,962 cf

Outflow = 0.62 cfs @ 12.09 hrs, Volume= 1,962 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.62 cfs @ 12.09 hrs, Volume= 1,962 cf

Routed to Pond P207: INFILTRATION POND #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.81' @ 12.09 hrs

Flood Elev= 206.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.33'	12.0" Round Culvert L= 15.6' Ke= 0.500
			Inlet / Outlet Invert= 203.33' / 203.25' S= 0.0051 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.61 cfs @ 12.09 hrs HW=203.81' TW=197.48' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.61 cfs @ 2.43 fps)

Summary for Pond D11: DMH #11

Inflow Area = 36,689 sf, 73.28% Impervious, Inflow Depth > 3.55" for 10YR event

Inflow = 3.34 cfs @ 12.09 hrs, Volume= 10,849 cf

Outflow = 3.34 cfs @ 12.09 hrs, Volume= 10,849 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.34 cfs @ 12.09 hrs, Volume= 10,849 cf

Routed to Pond OCS3: OCS#3

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 187

Peak Elev= 205.28' @ 12.09 hrs

Flood Elev= 208.33'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.25'	18.0" Round Culvert L= 44.6' Ke= 0.500 Inlet / Outlet Invert= 204.25' / 204.03' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior Flow Area= 1.77 sf

Primary OutFlow Max=3.28 cfs @ 12.09 hrs HW=205.27' TW=204.50' (Dynamic Tailwater) 1=Culvert (Barrel Controls 3.28 cfs @ 3.64 fps)

Summary for Pond D12: DMH #12

Inflow Area = 22,113 sf, 88.07% Impervious, Inflow Depth > 4.13" for 10YR event

Inflow = 2.25 cfs @ 12.09 hrs, Volume= 7,606 cf

Outflow = 2.25 cfs @ 12.09 hrs, Volume= 7,606 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.25 cfs @ 12.09 hrs, Volume= 7,606 cf

Routed to Pond D13: DMH #13

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.24' @ 12.09 hrs

Flood Elev= 207.78'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.21'	12.0" Round Culvert L= 41.9' Ke= 0.500 Inlet / Outlet Invert= 203.21' / 203.00' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=2.20 cfs @ 12.09 hrs HW=204.22' TW=203.31' (Dynamic Tailwater) 1=Culvert (Barrel Controls 2.20 cfs @ 3.44 fps)

Summary for Pond D13: DMH #13

Inflow Area = 81,632 sf, 72.61% Impervious, Inflow Depth > 3.79" for 10YR event

Inflow = 7.09 cfs @ 12.09 hrs, Volume= 25,765 cf

Outflow = 7.09 cfs @ 12.09 hrs, Volume= 25,765 cf, Atten= 0%, Lag= 0.0 min

Primary = 7.09 cfs @ 12.09 hrs, Volume= 25,765 cf

Routed to Pond P207: INFILTRATION POND #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.33' @ 12.09 hrs

Flood Elev= 208.12'

Device	Routing	Invert	Outlet Devices
#1	Primary	201.95'	24.0" Round Culvert L= 60.1' Ke= 0.500 Inlet / Outlet Invert= 201.95' / 201.65' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=6.99 cfs @ 12.09 hrs HW=203.32' TW=197.48' (Dynamic Tailwater) 1=Culvert (Barrel Controls 6.99 cfs @ 4.30 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 188

Summary for Pond D14: DMH #14

Inflow Area = 38,112 sf, 77.40% Impervious, Inflow Depth > 4.18" for 10YR event

Inflow = 3.91 cfs @ 12.09 hrs, Volume= 13,267 cf

Outflow = 3.91 cfs @ 12.09 hrs, Volume= 13,267 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.91 cfs @ 12.09 hrs, Volume= 13,267 cf

Routed to Pond d13: DMH #13

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.16' @ 12.09 hrs

Flood Elev= 208.78'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.13'	18.0" Round Culvert L= 256.3' Ke= 0.500 Inlet / Outlet Invert= 204.13' / 202.85' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=3.81 cfs @ 12.09 hrs HW=205.14' TW=203.31' (Dynamic Tailwater) 1=Culvert (Barrel Controls 3.81 cfs @ 4.25 fps)

Summary for Pond D16: DMH #16

Inflow Area = 16,417 sf, 76.69% Impervious, Inflow Depth > 4.21" for 10YR event

Inflow = 1.70 cfs @ 12.09 hrs, Volume= 5,763 cf

Outflow = 1.70 cfs @ 12.09 hrs, Volume= 5,763 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.70 cfs @ 12.09 hrs, Volume= 5,763 cf

Routed to Pond D14: DMH #14

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.68' @ 12.09 hrs

Flood Elev= 208.59'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.90'	15.0" Round Culvert L= 103.5' Ke= 0.500 Inlet / Outlet Invert= 204.90' / 204.38' S= 0.0050 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.66 cfs @ 12.09 hrs HW=205.67' TW=205.14' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.66 cfs @ 3.01 fps)

Summary for Pond D17: DMH #17

Inflow Area = 14,946 sf, 75.26% Impervious, Inflow Depth > 4.17" for 10YR event

Inflow = 1.53 cfs @ 12.09 hrs, Volume= 5,188 cf

Outflow = 1.53 cfs @ 12.09 hrs, Volume= 5,188 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.53 cfs @ 12.09 hrs, Volume= 5,188 cf

Routed to Pond D18: DMH #18

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 189

Peak Elev= 201.21' @ 12.09 hrs Flood Elev= 204.84'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.55'	12.0" Round Culvert L= 91.6' Ke= 0.500 Inlet / Outlet Invert= 200.55' / 197.69' S= 0.0312 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.49 cfs @ 12.09 hrs HW=201.20' TW=198.26' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.49 cfs @ 2.75 fps)

Summary for Pond D18: DMH #18

Inflow Area = 25,318 sf, 65.46% Impervious, Inflow Depth > 3.95" for 10YR event

Inflow = 2.50 cfs @ 12.09 hrs, Volume= 8,324 cf

Outflow = 2.50 cfs @ 12.09 hrs, Volume= 8,324 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.50 cfs @ 12.09 hrs, Volume= 8,324 cf

Routed to Pond OCS1: OCS#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 198.27' @ 12.09 hrs

Flood Elev= 201.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	197.44'	15.0" Round Culvert L= 46.3' Ke= 0.500 Inlet / Outlet Invert= 197.44' / 196.98' S= 0.0099 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 1.23 sf

Primary OutFlow Max=2.44 cfs @ 12.09 hrs HW=198.26' TW=196.02' (Dynamic Tailwater) 1=Culvert (Barrel Controls 2.44 cfs @ 4.06 fps)

Summary for Pond D19: DMH #19

Inflow Area = 17,428 sf, 83.29% Impervious, Inflow Depth > 4.27" for 10YR event

Inflow = 1.82 cfs @ 12.09 hrs, Volume= 6.195 cf

Outflow = 1.82 cfs @ 12.09 hrs, Volume= 6,195 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.82 cfs @ 12.09 hrs, Volume= 6,195 cf

Routed to Pond d20: DMH #20

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.94' @ 12.09 hrs

Flood Elev= 208.57'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.19'	12.0" Round Culvert L= 82.5' Ke= 0.500 Inlet / Outlet Invert= 205.19' / 204.43' S= 0.0092 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.77 cfs @ 12.09 hrs HW=205.93' TW=204.95' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.77 cfs @ 3.96 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 190

Summary for Pond D2: DMH#2

Inflow Area = 73,240 sf, 37.72% Impervious, Inflow Depth > 2.45" for 10YR event

Inflow = 3.67 cfs @ 12.14 hrs, Volume= 14,981 cf

Outflow = 3.67 cfs @ 12.14 hrs, Volume= 14,981 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.67 cfs @ 12.14 hrs, Volume= 14,981 cf

Routed to Pond P205: INFILTRATION POND #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.99' @ 12.14 hrs

Flood Elev= 212.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.90'	15.0" Round Culvert L= 38.2' Ke= 0.500 Inlet / Outlet Invert= 206.90' / 206.52' S= 0.0099 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.63 cfs @ 12.14 hrs HW=207.98' TW=205.83' (Dynamic Tailwater) 1=Culvert (Barrel Controls 3.63 cfs @ 4.32 fps)

Summary for Pond D20: DMH #20

Inflow Area = 17,428 sf, 83.29% Impervious, Inflow Depth > 4.27" for 10YR event

Inflow = 1.82 cfs @ 12.09 hrs, Volume= 6,195 cf

Outflow = 1.82 cfs @ 12.09 hrs, Volume= 6,195 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.82 cfs @ 12.09 hrs, Volume= 6,195 cf

Routed to Pond D21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.96' @ 12.09 hrs

Flood Elev= 207.68'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.19'	15.0" Round Culvert L= 63.5' Ke= 0.500 Inlet / Outlet Invert= 204.19' / 203.87' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.77 cfs @ 12.09 hrs HW=204.95' TW=204.39' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 1.77 cfs @ 3.25 fps)

Summary for Pond D21: DMH #21

Inflow Area = 71,317 sf, 79.77% Impervious, Inflow Depth > 4.16" for 10YR event

Inflow = 7.30 cfs @ 12.09 hrs, Volume= 24,749 cf

Outflow = 7.30 cfs @ 12.09 hrs, Volume= 24,749 cf, Atten= 0%, Lag= 0.0 min

Primary = 7.30 cfs @ 12.09 hrs, Volume= 24,749 cf

Routed to Pond p212: INFILTRATION POND #1

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 191

Peak Elev= 204.42' @ 12.09 hrs Flood Elev= 207.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.02'	24.0" Round Culvert L= 72.4' Ke= 0.500 Inlet / Outlet Invert= 203.02' / 202.66' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=7.11 cfs @ 12.09 hrs HW=204.39' TW=201.62' (Dynamic Tailwater) 1=Culvert (Barrel Controls 7.11 cfs @ 4.36 fps)

Summary for Pond D22: DMH #22

Inflow Area = 24,814 sf, 89.39% Impervious, Inflow Depth > 4.42" for 10YR event

Inflow = 2.62 cfs @ 12.09 hrs, Volume= 9,150 cf

Outflow = 2.62 cfs @ 12.09 hrs, Volume= 9,150 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.62 cfs @ 12.09 hrs, Volume= 9,150 cf

Routed to Pond d21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.74' @ 12.09 hrs

Flood Elev= 208.46'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.87'	15.0" Round Culvert L= 134.2' Ke= 0.500 Inlet / Outlet Invert= 204.87' / 203.92' S= 0.0071 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 1.23 sf

Primary OutFlow Max=2.55 cfs @ 12.09 hrs HW=205.72' TW=204.39' (Dynamic Tailwater) 1=Culvert (Barrel Controls 2.55 cfs @ 4.05 fps)

Summary for Pond D23: DMH #23

Inflow Area = 10,771 sf, 99.27% Impervious, Inflow Depth > 4.72" for 10YR event

Inflow = 1.17 cfs @ 12.09 hrs, Volume= 4,237 cf

Outflow = 1.17 cfs @ 12.09 hrs, Volume= 4,237 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.17 cfs @ 12.09 hrs, Volume= 4,237 cf

Routed to Pond D22: DMH #22

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.23' @ 12.09 hrs

Flood Elev= 210.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.70'	15.0" Round Culvert L= 173.3' Ke= 0.500 Inlet / Outlet Invert= 206.70' / 204.97' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.14 cfs @ 12.09 hrs HW=207.22' TW=205.72' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.14 cfs @ 3.49 fps)

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 192

Summary for Pond D25: DMH #25

Inflow Area = 36,995 sf, 87.96% Impervious, Inflow Depth > 4.26" for 10YR event

Inflow = 3.74 cfs @ 12.09 hrs, Volume= 13,119 cf

Outflow = 3.74 cfs @ 12.09 hrs, Volume= 13,119 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.74 cfs @ 12.09 hrs, Volume = 13,119 cf

Routed to Pond P210: POCKET WETLAND #1

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 210.49' @ 12.09 hrs

Flood Elev= 213.11'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.36'	15.0" Round Culvert L= 237.6' Ke= 0.500 Inlet / Outlet Invert= 209.36' / 208.17' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.64 cfs @ 12.09 hrs HW=210.47' TW=202.94' (Dynamic Tailwater) 1=Culvert (Barrel Controls 3.64 cfs @ 4.20 fps)

Summary for Pond D27: DMH #27

Inflow Area = 21,746 sf, 79.51% Impervious, Inflow Depth > 3.93" for 10YR event

Inflow = 2.08 cfs @ 12.09 hrs, Volume= 7,120 cf

Outflow = 2.08 cfs @ 12.09 hrs, Volume= 7,120 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.08 cfs @ 12.09 hrs, Volume= 7,120 cf

Routed to Pond D35: DMH #35

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 214.05' @ 12.09 hrs

Flood Elev= 217.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.34'	15.0" Round Culvert L= 63.9' Ke= 0.500 Inlet / Outlet Invert= 213.34' / 212.38' S= 0.0150 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.03 cfs @ 12.09 hrs HW=214.04' TW=212.98' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.03 cfs @ 2.85 fps)

Summary for Pond D28: DMH #28

Inflow Area = 11,949 sf, 62.72% Impervious, Inflow Depth > 3.28" for 10YR event

Inflow = 1.01 cfs @ 12.09 hrs, Volume= 3,267 cf

Outflow = 1.01 cfs @ 12.09 hrs, Volume= 3,267 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.01 cfs @ 12.09 hrs, Volume= 3,267 cf

Routed to Pond D27: DMH #27

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 193

Peak Elev= 217.98' @ 12.09 hrs Flood Elev= 220.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	217.46'	12.0" Round Culvert L= 158.3' Ke= 0.500 Inlet / Outlet Invert= 217.46' / 214.29' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.99 cfs @ 12.09 hrs HW=217.97' TW=214.04' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.99 cfs @ 2.44 fps)

Summary for Pond D29: DMH #29

Inflow Area = 11,949 sf, 62.72% Impervious, Inflow Depth > 3.28" for 10YR event

Inflow 1.01 cfs @ 12.09 hrs, Volume= 3,267 cf

1.01 cfs @ 12.09 hrs, Volume= Outflow 3,267 cf, Atten= 0%, Lag= 0.0 min

3,267 cf Primary = 1.01 cfs @ 12.09 hrs, Volume=

Routed to Pond D28: DMH #28

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 220.35' @ 12.09 hrs

Flood Elev= 223.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	219.83'	12.0" Round Culvert L= 150.9' Ke= 0.500 Inlet / Outlet Invert= 219.83' / 217.55' S= 0.0151 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.99 cfs @ 12.09 hrs HW=220.34' TW=217.97' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.99 cfs @ 2.44 fps)

Summary for Pond D30: DMH #30

Inflow Area = 6,862 sf, 50.73% Impervious, Inflow Depth > 2.86" for 10YR event

0.52 cfs @ 12.09 hrs, Volume= 1,633 cf Inflow =

Outflow 0.52 cfs @ 12.09 hrs, Volume= 1,633 cf, Atten= 0%, Lag= 0.0 min

0.52 cfs @ 12.09 hrs, Volume= Primary = 1,633 cf

Routed to Pond D29: DMH #29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 221.34' @ 12.09 hrs

Flood Elev= 224.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	220.92'	12.0" Round Culvert L= 184.2' Ke= 0.500 Inlet / Outlet Invert= 220.92' / 220.00' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.51 cfs @ 12.09 hrs HW=221.33' TW=220.34' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.51 cfs @ 2.44 fps)

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 194

Summary for Pond D31: DMH#31

Inflow Area = 63,140 sf, 28.73% Impervious, Inflow Depth > 2.13" for 10YR event

Inflow = 2.84 cfs @ 12.16 hrs, Volume= 11,215 cf

Outflow = 2.84 cfs @ 12.16 hrs, Volume= 11,215 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.84 cfs @ 12.16 hrs, Volume= 11,215 cf

Routed to Pond D4: DMH#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 225.49' @ 12.16 hrs

Flood Elev= 229.59'

Device	Routing	Invert	Outlet Devices
#1	Primary	224.63'	15.0" Round Culvert L= 288.5' Ke= 0.500 Inlet / Outlet Invert= 224.63' / 213.09' S= 0.0400 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.79 cfs @ 12.16 hrs HW=225.48' TW=211.34' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.79 cfs @ 3.14 fps)

Summary for Pond D34: DMH #34

Inflow Area = 23,255 sf,100.00% Impervious, Inflow Depth > 4.72" for 10YR event

Inflow = 2.53 cfs @ 12.09 hrs, Volume= 9,148 cf

Outflow = 2.53 cfs @ 12.09 hrs, Volume= 9,148 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.53 cfs @ 12.09 hrs, Volume= 9,148 cf

Routed to Pond OCS1: OCS#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 199.01' @ 12.09 hrs

Flood Elev= 202.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	198.07'	12.0" Round Culvert L= 52.0' Ke= 0.500
			Inlet / Outlet Invert= 198.07' / 197.03' S= 0.0200 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=2.46 cfs @ 12.09 hrs HW=198.99' TW=196.02' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.46 cfs @ 3.26 fps)

Summary for Pond D35: DMH #35

Inflow Area = 21,746 sf, 79.51% Impervious, Inflow Depth > 3.93" for 10YR event

Inflow = 2.08 cfs @ 12.09 hrs, Volume= 7,120 cf

Outflow = 2.08 cfs @ 12.09 hrs, Volume= 7,120 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.08 cfs @ 12.09 hrs, Volume= 7,120 cf

Routed to Pond D25: DMH #25

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 195

Peak Elev= 212.99' @ 12.09 hrs

Flood Elev= 215.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	212.28'	15.0" Round Culvert L= 171.5' Ke= 0.500 Inlet / Outlet Invert= 212.28' / 209.71' S= 0.0150 '/' Cc= 0.900 n= 0.012 Flow Area= 1.23 sf

Primary OutFlow Max=2.03 cfs @ 12.09 hrs HW=212.98' TW=210.47' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.03 cfs @ 2.85 fps)

Summary for Pond D4: DMH#4

Inflow Area = 73,240 sf, 37.72% Impervious, Inflow Depth > 2.45" for 10YR event

Inflow = 3.67 cfs @ 12.14 hrs, Volume= 14,981 cf

Outflow = 3.67 cfs @ 12.14 hrs, Volume= 14,981 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.67 cfs @ 12.14 hrs, Volume= 14,981 cf

Routed to Pond D2: DMH#2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 211.36' @ 12.14 hrs

Flood Elev= 217.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	210.34'	15.0" Round Culvert L= 222.3' Ke= 0.500
	-		Inlet / Outlet Invert= 210.34' / 207.01' S= 0.0150 '/' Cc= 0.900
			n= 0.012 Corrugated PP_smooth interior_Flow Area= 1.23 sf

Primary OutFlow Max=3.63 cfs @ 12.14 hrs HW=211.35' TW=207.98' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.63 cfs @ 3.42 fps)

Summary for Pond D5: DMH #5

Inflow Area = 30,441 sf, 96.76% Impervious, Inflow Depth > 4.66" for 10YR event

Inflow = 3.30 cfs @ 12.09 hrs, Volume= 11.817 cf

Outflow = 3.30 cfs @ 12.09 hrs, Volume= 11,817 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.30 cfs @ 12.09 hrs, Volume= 11,817 cf

Routed to Pond D6: DMH #6

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.09' @ 12.09 hrs

Flood Elev= 212.97'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.09'	18.0" Round Culvert L= 183.0' Ke= 0.500 Inlet / Outlet Invert= 209.09' / 208.17' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 1.77 sf

Primary OutFlow Max=3.21 cfs @ 12.09 hrs HW=210.07' TW=209.02' (Dynamic Tailwater) 1=Culvert (Outlet Controls 3.21 cfs @ 3.72 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 196

Summary for Pond D6: DMH #6

Inflow Area = 30,441 sf, 96.76% Impervious, Inflow Depth > 4.66" for 10YR event

Inflow = 3.30 cfs @ 12.09 hrs, Volume= 11,817 cf

Outflow = 3.30 cfs @ 12.09 hrs, Volume= 11,817 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.30 cfs @ 12.09 hrs, Volume= 11,817 cf

Routed to Pond D7: DMH #7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 209.03' @ 12.09 hrs

Flood Elev= 214.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	208.07'	18.0" Round Culvert L= 299.7' Ke= 0.500 Inlet / Outlet Invert= 208.07' / 206.57' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=3.21 cfs @ 12.09 hrs HW=209.02' TW=207.31' (Dynamic Tailwater) 1=Culvert (Outlet Controls 3.21 cfs @ 3.89 fps)

Summary for Pond D7: DMH #7

Inflow Area = 30,441 sf, 96.76% Impervious, Inflow Depth > 4.66" for 10YR event

Inflow = 3.30 cfs @ 12.09 hrs, Volume= 11,817 cf

Outflow = 3.30 cfs @ 12.09 hrs, Volume= 11,817 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.30 cfs @ 12.09 hrs, Volume= 11,817 cf

Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.33' @ 12.09 hrs

Flood Elev= 213.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.47'	18.0" Round Culvert L= 44.2' Ke= 0.500 Inlet / Outlet Invert= 206.47' / 204.04' S= 0.0550'/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=3.21 cfs @ 12.09 hrs HW=207.31' TW=201.62' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.21 cfs @ 3.13 fps)

Summary for Pond D8: DMH #8

Inflow Area = 18,765 sf, 91.12% Impervious, Inflow Depth > 4.22" for 10YR event

Inflow = 1.93 cfs @ 12.09 hrs, Volume= 6,595 cf

Outflow = 1.93 cfs @ 12.09 hrs, Volume= 6,595 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.93 cfs @ 12.09 hrs, Volume= 6,595 cf

Routed to Pond D9: DMH #9

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 197

Peak Elev= 201.54' @ 12.09 hrs Flood Elev= 204.72'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.57'	12.0" Round Culvert L= 87.7' Ke= 0.500 Inlet / Outlet Invert= 200.57' / 200.13' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.88 cfs @ 12.09 hrs HW=201.52' TW=200.95' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.88 cfs @ 3.14 fps)

Summary for Pond D9: DMH #9

Inflow Area = 18,765 sf, 91.12% Impervious, Inflow Depth > 4.22" for 10YR event
Inflow = 1.93 cfs @ 12.09 hrs, Volume= 6,595 cf
Outflow = 1.93 cfs @ 12.09 hrs, Volume= 6,595 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.93 cfs @ 12.09 hrs, Volume= 6,595 cf
Routed to Pond P207 : INFILTRATION POND #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 200.97' @ 12.09 hrs Flood Elev= 204.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.03'	12.0" Round Culvert L= 11.9' Ke= 0.500 Inlet / Outlet Invert= 200.03' / 199.97' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior Flow Area= 0.79 sf

Primary OutFlow Max=1.88 cfs @ 12.09 hrs HW=200.95' TW=197.46' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.88 cfs @ 3.23 fps)

Summary for Pond DE61: DRIP #61

Inflow Area =	4,247 sf, 92.68% Impervi	ous, Inflow Depth > 4.49" for 10YR event			
Inflow =	0.45 cfs @ 12.09 hrs, Volun	me= 1,589 cf			
Outflow =	0.39 cfs @ 12.14 hrs, Volun	me= 1,445 cf, Atten= 15%, Lag= 3.3 min			
Discarded =	0.00 cfs @ 5.25 hrs, Volun	me= 196 cf			
Primary =	0.38 cfs @ 12.14 hrs, Volun	me= 1,249 cf			
Routed to Reach 8R : OVERLAND FLOW					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 213.22' @ 12.14 hrs Surf.Area= 665 sf Storage= 273 cf

Plug-Flow detention time= 80.1 min calculated for 1,445 cf (91% of inflow) Center-of-Mass det. time= 34.3 min (796.4 - 762.1)

Volume	Invert	Avail.Storage	Storage Description
#1	212.19'	539 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 198

Elevation	Surf.Area	Voids	Inc.Store	Cum.Store
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)
212.19	665	0.0	0	0
212.20	665	40.0	3	3
214.19	665	40.0	529	532
214.20	665	100.0	7	539

Device	Routing	Invert	Outlet Devices
#1	Primary	214.10'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	,		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	212.70'	6.0" Round Culvert L= 10.0' Ke= 0.500
	•		Inlet / Outlet Invert= 212.70' / 212.65' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	212.19'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 5.25 hrs HW=212.21' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.38 cfs @ 12.14 hrs HW=213.21' TW=208.04' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Barrel Controls 0.38 cfs @ 2.33 fps)

Summary for Pond DE62: DRIP #62

Inflow Area =	4,247 sf, 92.68% Impervious,	Inflow Depth > 4.49" for 10YR event			
Inflow =	0.45 cfs @ 12.09 hrs, Volume=	1,589 cf			
Outflow =	0.39 cfs @ 12.14 hrs, Volume=	1,445 cf, Atten= 15%, Lag= 3.3 min			
Discarded =	0.00 cfs @ 5.25 hrs, Volume=	196 cf			
Primary =	0.38 cfs @ 12.14 hrs, Volume=	1,249 cf			
Routed to Reach 8R : OVERLAND FLOW					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 213.22' @ 12.14 hrs Surf.Area= 665 sf Storage= 273 cf

Plug-Flow detention time= 80.1 min calculated for 1,445 cf (91% of inflow) Center-of-Mass det. time= 34.3 min (796.4 - 762.1)

Volume	Invert Av	ail.Storage	Storage Descrip	otion	
#1	212.19'	539 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
Elevation (feet)	Surf.Area (sg-ft		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
212.19	66	5 0.0	Ó	0	
212.20	66	5 40.0	3	3	
214.19	66	40.0	529	532	
214.20	66	5 100.0	7	539	

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 199

Device	Routing	Invert	Outlet Devices
#1	Primary	214.10'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	212.70'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 212.70' / 212.65' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	212.19'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 5.25 hrs HW=212.21' (Free Discharge) **1 1 2 2 3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.38 cfs @ 12.14 hrs HW=213.21' TW=208.04' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.38 cfs @ 2.33 fps)

Summary for Pond DE63: DRIP #63

Inflow Area = 3,013 sf, 88.68% Impervious, Inflow Depth > 4.38" for 10YR event

Inflow = 0.32 cfs @ 12.09 hrs, Volume= 1,099 cf

1,012 cf, Atten= 9%, Lag= 2.2 min Outflow =

0.29 cfs @ 12.12 hrs, Volume= 0.00 cfs @ 5.45 hrs, Volume= 117 cf Discarded = 0.29 cfs @ 12.12 hrs, Volume= 895 cf Primary =

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.93' @ 12.12 hrs Surf.Area= 404 sf Storage= 152 cf

Plug-Flow detention time= 70.3 min calculated for 1,012 cf (92% of inflow)

Center-of-Mass det. time= 29.0 min (797.0 - 768.0)

Volume	Invert	Ava	il.Storage	Storage Descrip	otion		
#1	206.99'		327 cf	Custom Stage	Data (Prismatic	Listed below (Recalc)	
Elevation (fee		ırf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
206.9 207.0	00	404 404	0.0 40.0	0 2	0 2		
208.9 209.0	-	404 404	40.0 100.0	322 4	323 327		

Device	Routing	invert	Outlet Devices
#1	Primary	208.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	207.50'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 207.50' / 207.45' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	206.99'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 200

Discarded OutFlow Max=0.00 cfs @ 5.45 hrs HW=207.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.28 cfs @ 12.12 hrs HW=207.92' TW=202.08' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.28 cfs @ 2.15 fps)

Summary for Pond DE64: DRIP #64

Inflow Area = 3,470 sf, 91.59% Impervious, Inflow Depth > 4.49" for 10YR event

Inflow = 0.37 cfs @ 12.09 hrs, Volume= 1,298 cf

Outflow = 0.33 cfs @ 12.13 hrs, Volume= 1,197 cf, Atten= 11%, Lag= 2.5 min

Discarded = 0.00 cfs @ 4.85 hrs, Volume= 140 cf Primary = 0.33 cfs @ 12.13 hrs, Volume= 1,057 cf

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.97' @ 12.13 hrs Surf.Area= 470 sf Storage= 184 cf

Plug-Flow detention time= 71.7 min calculated for 1,197 cf (92% of inflow)

Center-of-Mass det. time= 30.6 min (792.8 - 762.1)

Volume	Inve	rt Ava	il.Storage	Storage Descrip	otion	
#1	204.99	9'	381 cf	Custom Stage	Data (Prismatio	Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
204.9	99	470	0.0	0	0	
205.0 206.9		470 470	40.0 40.0	2 374	2 376	
207.0	00	470	100.0	5	381	

Device	Routing	Invert	Outlet Devices
#1	Primary	206.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	205.50'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 205.50' / 205.45' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	204.99'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 4.85 hrs HW=205.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.32 cfs @ 12.13 hrs HW=205.96' TW=202.08' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.32 cfs @ 2.23 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 201

Summary for Pond DE65: DRIP #65

Inflow Area = 3,016 sf, 88.69% Impervious, Inflow Depth > 4.38" for 10YR event

Inflow 0.32 cfs @ 12.09 hrs, Volume= 1.100 cf

Outflow 0.29 cfs @ 12.12 hrs, Volume= 1,013 cf, Atten= 9%, Lag= 2.2 min

Discarded = 0.00 cfs @ 5.45 hrs, Volume= 117 cf 0.29 cfs @ 12.12 hrs, Volume= Primary = 896 cf

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.93' @ 12.12 hrs Surf.Area= 404 sf Storage= 152 cf

Plug-Flow detention time= 69.6 min calculated for 1,011 cf (92% of inflow)

Center-of-Mass det. time= 29.0 min (797.0 - 768.0)

Volume	Inve	ert Ava	il.Storage	Storage Descrip	otion	
#1	205.9	9'	327 cf	Custom Stage	Data (Prismatic)L	isted below (Recalc)
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
205.9	99	404	0.0	0	0	
206.0	00	404	40.0	2	2	
207.9	99	404	40.0	322	323	
208.0	00	404	100.0	4	327	
Device	Routing	In	vert Out	let Devices		
#1	Primary	207	7.90' 180	.0' long x 0.5' br	eadth Broad-Cres	sted Rectangular Weir
	,		Hea	nd (feet) 0.20 0.4	0 0.60 0.80 1.00	G
			Coe	f. (English) 2.80	2.92 3.08 3.30 3	3.32
#2	Primary	206	6.50' 6.0'	Round Culvert	L= 10.0' Ke= 0.5	500
			Inle	t / Outlet Invert= 2	206.50' / 206.45'	S= 0.0050 '/' Cc= 0.900
						or, Flow Area= 0.20 sf
#3	Discarde	d 205	5.99' 0.17	70 in/hr Exfiltratio	on over Surface a	irea Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 5.45 hrs HW=206.01' (Free Discharge) **T_3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.28 cfs @ 12.12 hrs HW=206.92' TW=202.08' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.28 cfs @ 2.15 fps)

Summary for Pond DE66: DRIP #66

3,407 sf, 91.46% Impervious, Inflow Depth > 4.49" for 10YR event Inflow Area =

Inflow 0.36 cfs @ 12.09 hrs, Volume= 1,275 cf

0.33 cfs @ 12.13 hrs, Volume= Outflow 1,173 cf, Atten= 11%, Lag= 2.5 min

0.00 cfs @ 4.10 hrs, Volume= Discarded = 140 cf Primary 0.32 cfs @ 12.13 hrs, Volume= 1.033 cf

Routed to Reach 12R: OVERLAND FLOW

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 202

Peak Elev= 208.76' @ 12.13 hrs Surf.Area= 470 sf Storage= 183 cf

Plug-Flow detention time= 71.9 min calculated for 1,171 cf (92% of inflow)

Center-of-Mass det. time= 30.9 min (793.0 - 762.1)

Volume	Inve	ert Ava	il.Storage	Storage Descrip	otion	
#1	207.7	79'	381 cf	Custom Stage	Data (Prismatic)	isted below (Recalc)
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
207.7	79	470	0.0	0	0	
207.8	30	470	40.0	2	2	
209.7	79	470	40.0	374	376	
209.8	30	470	100.0	5	381	
Device	Routing	In	vert Out	let Devices		
#1	Primary	209	9.70' 180	.0' long x 0.5' br	readth Broad-Cre	ested Rectangular Weir
	,				0.60 0.80 1.00	
				` ,	2.92 3.08 3.30	
#2	Primary	208		` ,	: L= 10.0' Ke= 0.	
						S= 0.0050 '/' Cc= 0.900
				.,		ior, Flow Area= 0.20 sf
#3	Discarde	d 207		•		area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 4.10 hrs HW=207.80' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.32 cfs @ 12.13 hrs HW=208.76' TW=202.08' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.32 cfs @ 2.22 fps)

Summary for Pond DE67: DRIP #67

Inflow Area	a =	3,481 sf	, 91.61% Impervious,	Inflow Depth > 4	.49" for 10YR event
Inflow	=	0.37 cfs @	12.09 hrs, Volume=	1,302 cf	
Outflow	=	0.33 cfs @	12.13 hrs, Volume=	1,201 cf,	Atten= 11%, Lag= 2.5 min
Discarded	=	0.00 cfs @	4.85 hrs, Volume=	140 cf	_
Primary	=	0.33 cfs @	12.13 hrs, Volume=	1,061 cf	

Routed to Reach 8R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 208.97' @ 12.13 hrs Surf.Area= 470 sf Storage= 184 cf

Plug-Flow detention time= 71.6 min calculated for 1,201 cf (92% of inflow) Center-of-Mass det. time= 30.6 min (792.7 - 762.1)

Volume	Invert	Avail.Storage	Storage Description
#1	207.99'	381 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 203

Elevation	Surf.Area	Voids	Inc.Store	Cum.Store
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)
207.99	470	0.0	0	0
208.00	470	40.0	2	2
209.99	470	40.0	374	376
210.00	470	100.0	5	381

Device	Routing	Invert	Outlet Devices
#1	Primary	209.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	208.50'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 208.50' / 208.45' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	207.99'	0.170 in/hr Exfiltration over Surface area Phase-ln= 0.01'

Discarded OutFlow Max=0.00 cfs @ 4.85 hrs HW=208.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.32 cfs @ 12.13 hrs HW=208.96' TW=208.04' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Barrel Controls 0.32 cfs @ 2.23 fps)

Summary for Pond DE68: DRIP #68

Inflow Area =	4,212 sf	, 92.62% Impervious,	Inflow Depth > 4.49" for 10YR event	
Inflow =	0.45 cfs @	12.09 hrs, Volume=	1,576 cf	
Outflow =	0.40 cfs @	12.13 hrs, Volume=	1,434 cf, Atten= 12%, Lag= 2.8 r	nin
Discarded =	0.00 cfs @	5.25 hrs, Volume=	196 cf	
Primary =	0.39 cfs @	12.13 hrs, Volume=	1,238 cf	
Routed to Pond	OCS4: OCS	S#4		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.92' @ 12.13 hrs Surf.Area= 665 sf Storage= 248 cf

Plug-Flow detention time= 77.4 min calculated for 1,431 cf (91% of inflow) Center-of-Mass det. time= 32.5 min (794.6 - 762.1)

Volume	Invert Ava	il.Storage	Storage Descript	tion	
#1	206.99'	539 cf	Custom Stage I	Data (Prismatic)	isted below (Recalc)
Elevation	Surf.Area	Voids	Inc.Store	Cum.Store	
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
206.99	665	0.0	0	0	
207.00	665	40.0	3	3	
208.99	665	40.0	529	532	
209.00	665	100.0	7	539	

#3

Discarded

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 204

Device	Routing	Invert	Outlet Devices
#1	Primary	208.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	207.50'	6.0" Round Culvert L= 20.0' Ke= 0.500
			Inlet / Outlet Invert= 207.50' / 206.00' S= 0.0750 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	206.99'	0.170 in/hr Exfiltration over Surface area Phase-ln= 0.01'

Discarded OutFlow Max=0.00 cfs @ 5.25 hrs HW=207.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.39 cfs @ 12.13 hrs HW=207.92' TW=204.29' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Inlet Controls 0.39 cfs @ 2.20 fps)

Summary for Pond DE69: DRIP #69

Inflow Area = 3,480 sf, 91.61% Impervious, Inflow Depth > 4.49" for 10YR event Inflow 0.37 cfs @ 12.09 hrs, Volume= 1,302 cf 0.33 cfs @ 12.13 hrs, Volume= Outflow 1,200 cf, Atten= 11%, Lag= 2.5 min 4.85 hrs, Volume= Discarded = 0.00 cfs @ 140 cf 0.33 cfs @ 12.13 hrs, Volume= 1.061 cf Primary Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 206.47' @ 12.13 hrs Surf.Area= 470 sf Storage= 184 cf

Plug-Flow detention time= 71.0 min calculated for 1,198 cf (92% of inflow) Center-of-Mass det. time= 30.6 min (792.7 - 762.1)

Volume	Inv	ert Ava	il.Storage	e Storage Descri	iption		
#1	#1 205.49' 381 cf		f Custom Stage Data (Prismatic)Listed below (Recalc)				
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)		
205.4	19	470	0.0	0	0		
205.5	50	470	40.0	2	2		
207.4	19	470	40.0	374	376		
207.5	50	470	100.0	5	381		
Device	Routing	Invert Out		ıtlet Devices			
#1	Primary	207.40' 180		0.0' long x 0.5' b	readth Broad-Cr	ested Rectangular Weir	
			He	ead (feet) 0.20 0.	40 0.60 0.80 1.0	00	
					2.92 3.08 3.30		
#2	Primary	206			rt L= 10.0' Ke= 0		
				let / Outlet Invert= 206.00' / 205.95' S= 0.0050 '/' Cc= 0.900			

n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 205

Discarded OutFlow Max=0.00 cfs @ 4.85 hrs HW=205.51' (Free Discharge) **T_3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.32 cfs @ 12.13 hrs HW=206.46' TW=201.82' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.32 cfs @ 2.23 fps)

Summary for Pond DE70: DRIP #70

3,476 sf, 91.60% Impervious, Inflow Depth > 4.49" for 10YR event Inflow Area = Inflow 0.37 cfs @ 12.09 hrs, Volume= 1,301 cf 0.33 cfs @ 12.13 hrs, Volume= 1,199 cf, Atten= 11%, Lag= 2.5 min Outflow 0.00 cfs @ 4.05 hrs, Volume= Discarded = 140 cf 0.33 cfs @ 12.13 hrs, Volume= 1,059 cf Primary Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 206.87' @ 12.13 hrs Surf.Area= 470 sf Storage= 184 cf

Plug-Flow detention time= 71.7 min calculated for 1,199 cf (92% of inflow) Center-of-Mass det. time= 30.6 min (792.7 - 762.1)

Volume	Invert Ava	ail.Storage	Storage Descrip	tion	
#1	205.89'	381 cf	Custom Stage	Data (Prismatic	Listed below (Recalc)
Elevation	Surf.Area	Voids	Inc.Store	Cum.Store	
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
205.89	470	0.0	0	0	
205.90	470	40.0	2	2	
207.89	470	40.0	374	376	
207.90	470	100.0	5	381	

Device	Routing	Invert	Outlet Devices
#1	Primary	207.80'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	206.40'	6.0" Round Culvert L= 10.0' Ke= 0.500
	•		Inlet / Outlet Invert= 206.40' / 206.35' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	205.89'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 4.05 hrs HW=205.90' (Free Discharge) **1 3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.32 cfs @ 12.13 hrs HW=206.86' TW=201.82' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.32 cfs @ 2.23 fps)

Invert

Volume

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 206

Summary for Pond DE71: DRIP #71

Inflow Area = 4,210 sf, 92.61% Impervious, Inflow Depth > 4.49" for 10YR event
Inflow = 0.45 cfs @ 12.09 hrs, Volume= 1,575 cf
Outflow = 0.38 cfs @ 12.14 hrs, Volume= 1,431 cf, Atten= 15%, Lag= 3.3 min
Discarded = 0.00 cfs @ 5.75 hrs, Volume= 196 cf
Primary = 0.38 cfs @ 12.14 hrs, Volume= 1,235 cf
Routed to Pond P212 : INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.51' @ 12.14 hrs Surf.Area= 665 sf Storage= 273 cf

Avail Storage Storage Description

Plug-Flow detention time= 79.8 min calculated for 1,428 cf (91% of inflow) Center-of-Mass det. time= 34.4 min (796.5 - 762.1)

volullie	IIIVE	eit Avai	ii.Storage	Sidiage Descrip	HOH	
#1	206.4	19'	805 cf	Custom Stage	Data (Prismatic) Lis	ted below (Recalc)
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
206.4	49	665	0.0	0	0	
206.5	50	665	40.0	3	3	
209.4	49	665	40.0	795	798	
209.5	50	665	100.0	7	805	
Device	Routing	In	vert Ou	tlet Devices		
#1	Primary	209	.40' 18 0	0.0' long x 0.5' br	eadth Broad-Crest	ed Rectangular Weir
			He	ad (feet) 0.20 0.4	0 0.60 0.80 1.00	
				` ' '	2.92 3.08 3.30 3.3	
#2	Primary	207			L= 10.0' Ke= 0.50	
						= 0.0050 '/' Cc= 0.900
110	D:			•	•	, Flow Area= 0.20 sf
#3	Discarde	ea 206	6.49' 0.1	/U in/nr Extiltration	on over Surface ar	∌a Pnase-in= 0.01′

Discarded OutFlow Max=0.00 cfs @ 5.75 hrs HW=206.52' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.38 cfs @ 12.14 hrs HW=207.51' TW=201.88' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Barrel Controls 0.38 cfs @ 2.33 fps)

Summary for Pond DECH: DRIP #CH

Inflow Area = 6,262 sf, 92.70% Impervious, Inflow Depth > 4.49" for 10YR event
Inflow = 0.67 cfs @ 12.09 hrs, Volume= 2,343 cf
Outflow = 0.42 cfs @ 12.19 hrs, Volume= 2,343 cf, Atten= 38%, Lag= 6.5 min
Discarded = 0.04 cfs @ 10.75 hrs, Volume= 1,427 cf
Primary = 0.38 cfs @ 12.19 hrs, Volume= 916 cf

Routed to Pond CB18: CB #18

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 207

Peak Elev= 209.49' @ 12.19 hrs Surf.Area= 636 sf Storage= 382 cf

Plug-Flow detention time= 20.3 min calculated for 2,343 cf (100% of inflow)

Center-of-Mass det. time= 20.2 min (782.3 - 762.1)

Volume	Inve	ert Ava	il.Storage	Storage Descrip	otion	
#1	207.9	99'	770 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
207.9	99	636	0.0	0	0	
208.0	00	636	40.0	3	3	
210.9	99	636	40.0	761	763	
211.0	00	636	100.0	6	770	
Device	Routing	In	vert Out	let Devices		
#1	Primary	210).90' 160	.0' long x 0.5' bi	readth Broad-Cro	ested Rectangular Weir
	,				10 0.60 0.80 1.0	
					2.92 3.08 3.30	
#2	Primary	208			t L= 80.0' Ke= 0	
<i>,,,</i>	1 minary	200				S= 0.0425 '/' Cc= 0.900
				· -		rior, Flow Area= 0.09 sf
4 2	Diagonda	-d 20-7				
#3	Discarde	eu 207	7.99' 2.4 '	iv in/nr Extiltrati	on over Surrace	area Phase-In= 0.01'

Discarded OutFlow Max=0.04 cfs @ 10.75 hrs HW=208.02' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.38 cfs @ 12.19 hrs HW=209.49' TW=205.42' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Inlet Controls 0.38 cfs @ 4.37 fps)

Summary for Pond DMH32: DMH #32

Inflow Area = 20,278 sf, 79.11% Impervious, Inflow Depth > 4.18" for 10YR event

Inflow = 2.09 cfs @ 12.09 hrs, Volume= 7,071 cf

Outflow = 2.09 cfs @ 12.09 hrs, Volume= 7,071 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.09 cfs @ 12.09 hrs, Volume= 7,071 cf

Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.40' @ 12.09 hrs

Flood Elev= 206.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.59'	12.0" Round Culvert L= 19.2' Ke= 0.500 Inlet / Outlet Invert= 202.59' / 201.57' S= 0.0531 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.04 cfs @ 12.09 hrs HW=203.39' TW=201.62' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.04 cfs @ 3.04 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 208

Summary for Pond OCS1: OCS#1

Inflow Area = 48,573 sf, 81.99% Impervious, Inflow Depth > 4.32" for 10YR event

Inflow = 5.03 cfs @ 12.09 hrs, Volume= 17,471 cf

Outflow = 5.03 cfs @ 12.09 hrs, Volume= 17,471 cf, Atten= 0%, Lag= 0.0 min

Primary = 5.03 cfs @ 12.09 hrs, Volume= 17,471 cf Routed to Pond P206 : STORMTECH INFILTRATION SYSTEM #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 196.14' @ 12.32 hrs

Flood Elev= 201.48'

Device Routing Invert Outlet Devices

#1 Primary 195.00' **24.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=4.92 cfs @ 12.09 hrs HW=196.02' TW=195.61' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 4.92 cfs @ 3.06 fps)

Summary for Pond OCS3: OCS#3

Inflow Area = 54,250 sf, 81.93% Impervious, Inflow Depth > 3.93" for 10YR event

Inflow = 5.25 cfs @ 12.09 hrs, Volume= 17,757 cf

Outflow = 5.25 cfs @ 12.09 hrs, Volume= 17,757 cf, Atten= 0%, Lag= 0.0 min

Primary = 5.25 cfs @ 12.09 hrs, Volume= 17,757 cf Routed to Pond p204 : STORMTECH INFILTRATION SYSTEM #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.75' @ 12.43 hrs

Flood Elev= 209.00'

Device Routing Invert Outlet Devices

#1 Primary 203.10' **18.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=5.16 cfs @ 12.09 hrs HW=204.49' TW=204.10' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 5.16 cfs @ 3.02 fps)

Summary for Pond OCS4: OCS#4

Inflow Area = 17,972 sf, 28.85% Impervious, Inflow Depth > 2.68" for 10YR event

Inflow = 1.25 cfs @ 12.10 hrs, Volume= 4,007 cf

Outflow = 1.25 cfs @ 12.10 hrs, Volume= 4,007 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.25 cfs @ 12.10 hrs, Volume= 4,007 cf Routed to Pond P204 : STORMTECH INFILTRATION SYSTEM #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.72' @ 12.46 hrs

Flood Elev= 208.00'

Device	Routing	Invert	Outlet Devices		
#1	Primary	203.10'	18.0" Vert. Orifice/Grate	C= 0.600	Limited to weir flow at low heads

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 209

Primary OutFlow Max=1.25 cfs @ 12.10 hrs HW=204.19' TW=204.15' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 1.25 cfs @ 0.91 fps)

Summary for Pond OCS6: OCS #6

Inflow Area = 16,111 sf, 93.77% Impervious, Inflow Depth > 4.56" for 10YR event

Inflow = 1.73 cfs @ 12.09 hrs, Volume= 6,129 cf

Outflow = 1.73 cfs @ 12.09 hrs, Volume= 6,129 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.73 cfs @ 12.09 hrs. Volume = 6.129 cf

Routed to Pond P213: Stormtech Infiltration System #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 202.65' @ 12.71 hrs

Flood Elev= 206.96'

Device Routing Invert Outlet Devices

#1 Primary 201.20' **12.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.71 cfs @ 12.09 hrs HW=202.11' TW=201.89' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 1.71 cfs @ 2.27 fps)

Summary for Pond OCS7: OCS #7

Inflow Area = 15,875 sf, 92.67% Impervious, Inflow Depth > 4.52" for 10YR event

Inflow = 1.70 cfs @ 12.09 hrs, Volume= 5,986 cf

Outflow = 1.70 cfs @ 12.09 hrs, Volume= 5,986 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.70 cfs @ 12.09 hrs, Volume= 5,986 cf

Routed to Pond P213: Stormtech Infiltration System #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 202.65' @ 12.71 hrs

Flood Elev= 206.47'

Device Routing Invert Outlet Devices

#1 Primary 201.78' **12.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.65 cfs @ 12.09 hrs HW=202.48' TW=201.89' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 1.65 cfs @ 2.84 fps)

Summary for Pond P204: STORMTECH INFILTRATION SYSTEM #1

Inflow Area = 72,222 sf, 68.72% Impervious, Inflow Depth > 3.62" for 10YR event

Inflow = 6.49 cfs @ 12.09 hrs, Volume= 21,764 cf

Outflow = 1.88 cfs @ 12.47 hrs, Volume= 19,565 cf, Atten= 71%, Lag= 22.4 min

Discarded = 0.09 cfs @ 9.15 hrs, Volume= 5,743 cf Primary = 1.79 cfs @ 12.47 hrs, Volume= 13,822 cf

Routed to Reach 20r: OVERLAND FLOW

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 210

Peak Elev= 204.72' @ 12.47 hrs Surf.Area= 5,670 sf Storage= 8,487 cf Flood Elev= 208.75' Surf.Area= 5,670 sf Storage= 13,379 cf

Plug-Flow detention time= 110.9 min calculated for 19,565 cf (90% of inflow)

Center-of-Mass det. time= 62.5 min (841.6 - 779.1)

Volume Invert Avail.Storage Storage Description	
#1A 202.50' 5,923 cf 77.50'W x 67.70'L x 4.08'H STORMTECH SC-740	
21,423 cf Overall - 6,615 cf Embedded = 14,808 cf x 4	10.0% Voids
#2A 203.08' 6,615 cf ADS_StormTech SC-740 +Cap x 144 Inside #1	
Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L =	45.9 cf
Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Ove	rlap
144 Chambers in 16 Rows	
#3B 202.50' 427 cf 6.25'W x 67.70'L x 3.50'H ISOLATOR ROW	
1,481 cf Overall - 413 cf Embedded = 1,067 cf x 40.09	% Voids
#4B 203.00' 413 cf ADS_StormTech SC-740 +Cap x 9 Inside #3	
Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L =	
Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Ove	rlap

13,379 cf Total Available Storage

Storage Group A created with Chamber Wizard Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	202.75'	15.0" Round Culvert L= 35.0' Ke= 0.500
	•		Inlet / Outlet Invert= 202.75' / 201.00' S= 0.0500 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf
#2	Device 1	204.75'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Device 1	203.25'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	202.50'	0.660 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.09 cfs @ 9.15 hrs HW=202.56' (Free Discharge) **4=Exfiltration** (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=1.79 cfs @ 12.47 hrs HW=204.71' TW=200.09' (Dynamic Tailwater)

-1=Culvert (Passes 1.79 cfs of 6.84 cfs potential flow)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 1.79 cfs @ 5.12 fps)

Summary for Pond P205: INFILTRATION POND #3

Inflow Area = 88,676 sf, 39.42% Impervious, Inflow Depth > 2.51" for 10YR event

4.73 cfs @ 12.12 hrs, Volume= Inflow 18,538 cf

Outflow 0.70 cfs @ 12.90 hrs, Volume= 15,637 cf, Atten= 85%, Lag= 46.7 min

Discarded = 0.30 cfs @ 12.90 hrs, Volume= 14,188 cf 0.40 cfs @ 12.90 hrs, Volume= Primary 1,449 cf

Routed to Reach 18R: OVERLAND FLOW

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 211

Peak Elev= 206.70' @ 12.90 hrs Surf.Area= 5,363 sf Storage= 7,871 cf

Plug-Flow detention time= 232.2 min calculated for 15,605 cf (84% of inflow)

Center-of-Mass det. time= 164.8 min (986.3 - 821.5)

Volume	Inve	rt Avail.Sto	rage Storage	Description	
#1	205.00	0' 15,6	30 cf Custom	Stage Data (Pi	rismatic)Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
205.0	00	3,939	0	0	
206.0	00	4,756	4,348	4,348	
207.0	00	5,627	5,192	9,539	
208.0	00	6,555	6,091	15,630	
Device	Routing	Invert	Outlet Device	s	
#1	Primary	206.65'	15.0' long x	15.0' breadth B	road-Crested Rectangular Weir
			` ,		0.80 1.00 1.20 1.40 1.60
			, ,	,	70 2.64 2.63 2.64 2.64 2.63
#2	Discarded	205.00'	2.410 in/hr E	xfiltration over	Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.30 cfs @ 12.90 hrs HW=206.70' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.30 cfs)

Primary OutFlow Max=0.40 cfs @ 12.90 hrs HW=206.70' TW=203.01' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 0.40 cfs @ 0.58 fps)

Summary for Pond P206: STORMTECH INFILTRATION SYSTEM #2

Inflow Area = 59,746 sf, 80.70% Impervious, Inflow Depth > 4.29" for 10YR event
Inflow = 6.18 cfs @ 12.09 hrs, Volume= 21,341 cf
Outflow = 2.12 cfs @ 12.37 hrs, Volume= 21,338 cf, Atten= 66%, Lag= 16.9 min
Discarded = 0.49 cfs @ 11.45 hrs, Volume= 18,384 cf
Primary = 1.63 cfs @ 12.37 hrs, Volume= 2,954 cf
Routed to Link AP4 : ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 196.10' @ 12.37 hrs Surf.Area= 6,072 sf Storage= 6,143 cf

Plug-Flow detention time= 65.3 min calculated for 21,338 cf (100% of inflow) Center-of-Mass det. time= 65.2 min (832.6 - 767.4)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 212

Volume	Invert	Avail.Storage	Storage Description
#1A	194.67'	1,786 cf	39.50'W x 53.46'L x 3.33'H FIELD A
			7,038 cf Overall - 2,573 cf Embedded = 4,466 cf x 40.0% Voids
#2A	195.00'	2,573 cf	ADS_StormTech SC-740 +Cap x 56 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			56 Chambers in 8 Rows
#3B	194.67'	3,296 cf	58.50'W x 67.70'L x 3.33'H FIELD B
			13,201 cf Overall - 4,962 cf Embedded = 8,239 cf x 40.0% Voids
#4B	195.00'	4,962 cf	ADS_StormTech SC-740 +Cap x 108 Inside #3
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			108 Chambers in 12 Rows

12,616 cf Total Available Storage

Storage Group A created with Chamber Wizard Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	194.65'	18.0" Round Culvert L= 30.0' Ke= 0.200
	·		Inlet / Outlet Invert= 194.65' / 194.50' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf
			11-0.013 Corrugated FL, Sillootti litterior, Trow Area-1.77 Si
#2	Device 1	195.85'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Discarded	194.67'	3.500 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.49 cfs @ 11.45 hrs HW=194.71' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.49 cfs)

Primary OutFlow Max=1.62 cfs @ 12.37 hrs HW=196.10' TW=0.00' (Dynamic Tailwater)

1=Culvert (Passes 1.62 cfs of 6.17 cfs potential flow)

2=Sharp-Crested Rectangular Weir (Weir Controls 1.62 cfs @ 1.64 fps)

Summary for Pond P207: INFILTRATION POND #2

Inflow Area =	129,716 sf, 63.13% Impervious,	Inflow Depth > 3.76" for 10YR event
Inflow =	11.58 cfs @ 12.09 hrs, Volume=	40,612 cf
Outflow =	1.99 cfs @ 12.60 hrs, Volume=	40,597 cf, Atten= 83%, Lag= 30.7 min
Discarded =	1.03 cfs @ 12.60 hrs, Volume=	34,722 cf
Primary =	0.96 cfs @ 12.60 hrs, Volume=	5,876 cf
Routed to Rea		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 198.06' @ 12.60 hrs Surf.Area= 12,083 sf Storage= 13,926 cf

Plug-Flow detention time= 75.9 min calculated for 40,513 cf (100% of inflow) Center-of-Mass det. time= 75.5 min (863.9 - 788.4)

Volume	Invert	Avail.Storage	Storage Description
#1	196.80'	40,260 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 213

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
196.80	10,100	0	0
198.00	12,000	13,260	13,260
200.00	15,000	27,000	40,260

Device	Routing	Invert	Outlet Devices
#1	Primary	198.80'	20.0' long x 21.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Primary	194.75'	15.0" Round Culvert L= 40.0' Ke= 0.500
	•		Inlet / Outlet Invert= 194.75' / 194.55' S= 0.0050 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf
#3	Device 2	198.80'	6.0" x 6.0" Horiz. Orifice/Grate X 6.00 columns
			X 6 rows C= 0.600 in 48.0" x 48.0" Grate (56% open area)
			Limited to weir flow at low heads
#4	Device 2	197.40'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Discarded	196.80'	3.690 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=1.03 cfs @ 12.60 hrs HW=198.06' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 1.03 cfs)

Primary OutFlow Max=0.96 cfs @ 12.60 hrs HW=198.06' TW=192.18' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir(Controls 0.00 cfs)

-2=Culvert (Passes 0.96 cfs of 9.67 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.96 cfs @ 2.76 fps)

Summary for Pond P210: POCKET WETLAND #1

Inflow Area = 62,582 sf, 52.00% Impervious, Inflow Depth > 3.40" for 10YR event

Inflow = 5.18 cfs @ 12.09 hrs, Volume= 17,735 cf

Outflow = 0.19 cfs @ 15.68 hrs, Volume= 7,883 cf, Atten= 96%, Lag= 215.2 min

Primary = 0.19 cfs @ 15.68 hrs, Volume= 7,883 cf

Routed to Reach 15R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Starting Elev= 201.00' Surf.Area= 376 sf Storage= 591 cf

Peak Elev= 203.77' @ 15.68 hrs Surf.Area= 7,604 sf Storage= 13,036 cf (12,445 cf above start)

Plug-Flow detention time= 459.2 min calculated for 7,277 cf (41% of inflow)

Center-of-Mass det. time= 292.2 min (1,075.2 - 783.0)

Volume	Invert	Avail.Storage	Storage Description
#1	199.00'	43,190 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 214

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
199.00	218	0	0
200.00	294	256	256
201.00	376	335	591
202.00	3,991	2,184	2,775
204.00	8,073	12,064	14,839
206.00	13,272	21,345	36,184
206.50	14,753	7,006	43,190

<u>Device</u>	Routing	Invert	Outlet Devices
#1	Primary	205.10'	20.0' long x 15.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Primary	202.25'	12.0" Round Culvert L= 44.0' Ke= 0.500
			Inlet / Outlet Invert= 202.25' / 202.03' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	202.30'	2.5" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	204.50'	6.0" x 6.0" Horiz. Orifice/Grate X 6.00 columns
			X 6 rows C= 0.600 in 48.0" x 48.0" Grate (56% open area)
			Limited to weir flow at low heads

Primary OutFlow Max=0.19 cfs @ 15.68 hrs HW=203.77' TW=202.05' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Passes 0.19 cfs of 3.19 cfs potential flow)

-3=Orifice/Grate (Orifice Controls 0.19 cfs @ 5.63 fps)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond P212: INFILTRATION POND #1

Inflow Area = 241,078 sf, 59.10% Impervious, Inflow Depth > 3.18" for 10YR event
Inflow = 16.29 cfs @ 12.10 hrs, Volume= 63,872 cf
Outflow = 5.35 cfs @ 12.51 hrs, Volume= 63,856 cf, Atten= 67%, Lag= 24.4 min
Discarded = 1.42 cfs @ 12.51 hrs, Volume= 57,664 cf
Primary = 3.94 cfs @ 12.51 hrs, Volume= 6,192 cf

Routed to Reach R211: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 202.65' @ 12.51 hrs Surf.Area= 11,925 sf Storage= 24,325 cf

Plug-Flow detention time= 142.3 min calculated for 63,856 cf (100% of inflow) Center-of-Mass det. time= 142.2 min (930.0 - 787.9)

Volume	Invert	Avail.Storage	Storage Description
#1	200.00'	41,774 cf	Custom Stage Data (Conic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 215

Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
200.00	4,354	0	0	4,354
201.00	9,360	6,699	6,699	9,368
202.00	10,993	10,166	16,865	11,040
204.00	13,976	24,909	41,774	14,126

Device	Routing	Invert	Outlet Devices
#1	Primary	202.50'	25.0' long x 20.0' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Discarded	200.00'	5.130 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=1.42 cfs @ 12.51 hrs HW=202.65' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 1.42 cfs)

Primary OutFlow Max=3.91 cfs @ 12.51 hrs HW=202.65' TW=200.07' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 3.91 cfs @ 1.04 fps)

Summary for Pond P213: Stormtech Infiltration System #3

Inflow Area =	31,986 sf, 93.23% Impervious,	Inflow Depth > 4.54" for 10YR event
Inflow =	3.43 cfs @ 12.09 hrs, Volume=	12,114 cf
Outflow =	0.39 cfs @ 11.65 hrs, Volume=	12,113 cf, Atten= 89%, Lag= 0.0 min
Discarded =	0.39 cfs @ 11.65 hrs, Volume=	12,113 cf
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf
Routed to Pond	P212 : INFILTRATION POND #1	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 202.65' @ 12.72 hrs Surf.Area= 3,317 sf Storage= 3,828 cf

Plug-Flow detention time= 62.1 min calculated for 12,113 cf (100% of inflow) Center-of-Mass det. time= 62.0 min (820.1 - 758.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	200.95'	2,990 cf	49.00'W x 67.70'L x 3.50'H Field A
			11,610 cf Overall - 4,135 cf Embedded = 7,475 cf x 40.0% Voids
#2A	201.45'	4,135 cf	ADS_StormTech SC-740 +Cap x 90 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			90 Chambers in 10 Rows
	•	7.405 (T 1 1 4 11 11 01

7,125 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	202.30'	12.0" Round Culvert L= 60.0' Ke= 0.500
	•		Inlet / Outlet Invert= 202.30' / 202.00' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	200.95'	5.130 in/hr Exfiltration over Surface area Phase-In= 0.01'
#3	Device 1	204.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 216

#4 Device 1 203.35' **6.0" W x 4.0" H Vert. Orifice/Grate** C= 0.600

Limited to weir flow at low heads

Discarded OutFlow Max=0.39 cfs @ 11.65 hrs HW=200.99' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.39 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=200.95' TW=200.00' (Dynamic Tailwater)

-1=Culvert (Controls 0.00 cfs)

3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond P214: STORMTECH INFILTRATION SYSTEM #4

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=3)

Inflow Area = 32,665 sf, 94.81% Impervious, Inflow Depth > 4.47" for 10YR event

Inflow = 3.46 cfs @ 12.09 hrs, Volume= 12,166 cf

Outflow = 0.61 cfs @ 11.75 hrs, Volume= 12,168 cf, Atten= 82%, Lag= 0.0 min

Discarded = 0.61 cfs @ 11.75 hrs, Volume= 12,168 cf Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routed to Reach 9R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 201.46' @ 12.54 hrs Surf.Area= 3,201 sf Storage= 3,080 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 26.6 min (786.2 - 759.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	200.00'	1,851 cf	30.00'W x 67.70'L x 3.50'H Field A
			7,108 cf Overall - 2,481 cf Embedded = 4,627 cf x 40.0% Voids
#2A	200.50'	2,481 cf	ADS_StormTech SC-740 +Cap x 54 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			54 Chambers in 6 Rows
#3B	200.00'	1,087 cf	25.25'W x 46.34'L x 3.50'H Field B
			4,095 cf Overall - 1,378 cf Embedded = 2,717 cf x 40.0% Voids
#4B	200.50'	1,378 cf	ADS_StormTech SC-740 +Cap x 30 Inside #3
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			30 Chambers in 5 Rows
		0.707 (T () A ())) O(

6,797 cf Total Available Storage

Storage Group A created with Chamber Wizard Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	201.00'	12.0" Round Culvert L= 25.0' Ke= 0.500
	•		Inlet / Outlet Invert= 201.00' / 200.88' S= 0.0048 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	200.00'	8.280 in/hr Exfiltration over Surface area Phase-In= 0.01'

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 217

#3 Device 1 203.40' 4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 44 Device 1 202.40' 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.61 cfs @ 11.75 hrs HW=200.05' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.61 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=200.00' TW=201.75' (Dynamic Tailwater)

-1=Culvert (Controls 0.00 cfs)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Link AP1: ANALYSIS POINT 1

Inflow Area = 9,943 sf, 92.79% Impervious, Inflow Depth > 4.38" for 10YR event

Inflow = 1.05 cfs @ 12.09 hrs, Volume= 3,627 cf

Primary = 1.05 cfs @ 12.09 hrs, Volume= 3,627 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP2: ANALYSIS POINT 2

Inflow Area = 816,898 sf, 39.51% Impervious, Inflow Depth > 2.32" for 10YR event

Inflow = 18.24 cfs @ 12.40 hrs, Volume= 157,893 cf

Primary = 18.24 cfs @ 12.40 hrs, Volume= 157,893 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP3: ANALYSIS POINT 3

Inflow Area = 55,420 sf, 16.57% Impervious, Inflow Depth > 2.00" for 10YR event

Inflow = 2.89 cfs @ 12.10 hrs, Volume= 9,258 cf

Primary = 2.89 cfs @ 12.10 hrs, Volume= 9,258 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP4: ANALYSIS POINT #4

Inflow Area = 1,691,659 sf, 25.34% Impervious, Inflow Depth > 1.27" for 10YR event

Inflow = 25.19 cfs @ 12.53 hrs, Volume= 178,860 cf

Primary = 25.19 cfs @ 12.53 hrs, Volume= 178,860 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 218

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

readiffeduling by byfi-didi-iii	a method - 1 ond rodding by byn-otor-ind method
SubcatchmentB1: MULTIFAMILYBLDG	Runoff Area=23,255 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=3.21 cfs 11,720 cf
SubcatchmentB2: MULTIFAMILYBLDG	Runoff Area=17,561 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=2.43 cfs 8,851 cf
SubcatchmentB3: MULTIFAMILY	Runoff Area=19,981 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=2.76 cfs 10,070 cf
SubcatchmentC10: CB #10	Runoff Area=6,961 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.96 cfs 3,508 cf
SubcatchmentC11: CB #11	Runoff Area=7,173 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.99 cfs 3,615 cf
SubcatchmentC12: CB #12	Runoff Area=5,238 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.72 cfs 2,640 cf
SubcatchmentC13: CB #13	Runoff Area=10,873 sf 90.78% Impervious Runoff Depth>5.81" Tc=6.0 min CN=96 Runoff=1.49 cfs 5,267 cf
SubcatchmentC14: CB #14	Runoff Area=12,099 sf 86.22% Impervious Runoff Depth>5.24" Tc=6.0 min CN=91 Runoff=1.57 cfs 5,281 cf
SubcatchmentC15: CB #15	Runoff Area=6,666 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.92 cfs 3,360 cf
SubcatchmentC16: CB #16	Runoff Area=8,516 sf 64.88% Impervious Runoff Depth>3.94" Tc=6.0 min CN=79 Runoff=0.88 cfs 2,794 cf
SubcatchmentC17: CB #17	Runoff Area=11,836 sf 73.87% Impervious Runoff Depth>5.35" Tc=6.0 min CN=92 Runoff=1.56 cfs 5,278 cf
SubcatchmentC18: CB #18	Runoff Area=18,591 sf 66.35% Impervious Runoff Depth>5.13" Tc=6.0 min CN=90 Runoff=2.38 cfs 7,940 cf
SubcatchmentC20: CB #20	Runoff Area=11,939 sf 88.95% Impervious Runoff Depth>5.70" Tc=6.0 min CN=95 Runoff=1.62 cfs 5,667 cf
SubcatchmentC21: CB #21	Runoff Area=10,174 sf 87.04% Impervious Runoff Depth>5.13" Tc=6.0 min CN=90 Runoff=1.30 cfs 4,345 cf
SubcatchmentC22: CB #22	Runoff Area=12,001 sf 91.62% Impervious Runoff Depth>5.81" Tc=6.0 min CN=96 Runoff=1.64 cfs 5,813 cf
SubcatchmentC23: CB #23	Runoff Area=9,694 sf 61.00% Impervious Runoff Depth>5.01" Tc=6.0 min CN=89 Runoff=1.22 cfs 4,050 cf

Prepared by Howard Stein Hudson Associates	Printed 6/19/2023
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC	Page 219

SubcatchmentC24: CB #24	Runoff Area=7,930 sf 72.16% Impervious Runoff Depth>5.47" Tc=6.0 min CN=93 Runoff=1.06 cfs 3,612 cf
SubcatchmentC25: CB #25	Runoff Area=8,487 sf 80.92% Impervious Runoff Depth>5.58" Tc=6.0 min CN=94 Runoff=1.14 cfs 3,947 cf
SubcatchmentC26: CB #26	Runoff Area=8,835 sf 63.75% Impervious Runoff Depth>5.24" Tc=6.0 min CN=91 Runoff=1.15 cfs 3,856 cf
SubcatchmentC27: CB #27	Runoff Area=6,111 sf 91.90% Impervious Runoff Depth>5.81" Tc=6.0 min CN=96 Runoff=0.84 cfs 2,960 cf
SubcatchmentC28: CB #28	Runoff Area=10,372 sf 51.33% Impervious Runoff Depth>4.90" Tc=6.0 min CN=88 Runoff=1.29 cfs 4,237 cf
SubcatchmentC29: CB #29	Runoff Area=8,495 sf 84.21% Impervious Runoff Depth>5.58" Tc=6.0 min CN=94 Runoff=1.14 cfs 3,950 cf
SubcatchmentC30: CB #30	Runoff Area=8,933 sf 82.40% Impervious Runoff Depth>5.58" Tc=6.0 min CN=94 Runoff=1.20 cfs 4,154 cf
SubcatchmentC31: CB #31	Runoff Area=16,365 sf 68.64% Impervious Runoff Depth>5.13" Tc=6.0 min CN=90 Runoff=2.10 cfs 6,989 cf
SubcatchmentC32: CB #32	Runoff Area=12,710 sf 70.47% Impervious Runoff Depth>5.24" Tc=6.0 min CN=91 Runoff=1.65 cfs 5,548 cf
SubcatchmentC33: CB #33	Runoff Area=5,421 sf 83.90% Impervious Runoff Depth>5.58" Tc=6.0 min CN=94 Runoff=0.73 cfs 2,521 cf
SubcatchmentC34: CB #34	Runoff Area=8,622 sf 80.51% Impervious Runoff Depth>5.47" Tc=6.0 min CN=93 Runoff=1.15 cfs 3,927 cf
SubcatchmentC35: CB #35	Runoff Area=4,149 sf 98.10% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.57 cfs 2,091 cf
SubcatchmentC36: CB #36	Runoff Area=6,622 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.92 cfs 3,337 cf
SubcatchmentC38: CB #38	Runoff Area=7,637 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=1.06 cfs 3,849 cf
SubcatchmentC39: CB #39	Runoff Area=7,612 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=1.05 cfs 3,836 cf
SubcatchmentC40: CB #40	Runoff Area=4,211 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.58 cfs 2,122 cf
SubcatchmentC41: CB #41	Runoff Area=5,586 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.77 cfs 2,815 cf

SubcatchmentC8: CB #8

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 220

SubcatchmentC43: CB #43	Runoff Area=3,109 sf 75.36% Impervious Runoff Depth>5.01" Tc=6.0 min CN=89 Runoff=0.39 cfs 1,299 cf
SubcatchmentC44: CB #44	Runoff Area=1,978 sf 84.43% Impervious Runoff Depth>5.35" Tc=6.0 min CN=92 Runoff=0.26 cfs 882 cf
SubcatchmentC45: CB #45	Runoff Area=2,465 sf 50.30% Impervious Runoff Depth>4.04" Tc=6.0 min CN=80 Runoff=0.26 cfs 830 cf
Subcatchment C46: CB #46	Runoff Area=4,397 sf 50.97% Impervious Runoff Depth>4.04" Tc=6.0 min CN=80 Runoff=0.47 cfs 1,481 cf
SubcatchmentC47: CB #47	Runoff Area=3,012 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.42 cfs 1,518 cf
SubcatchmentC48: CB #48	Runoff Area=60,128 sf 25.16% Impervious Runoff Depth>3.03" Flow Length=400' Tc=11.8 min CN=70 Runoff=4.02 cfs 15,183 cf
SubcatchmentC49: CB #49	Runoff Area=5,238 sf 84.59% Impervious Runoff Depth>5.58" Tc=6.0 min CN=94 Runoff=0.70 cfs 2,436 cf
SubcatchmentC50: CB #50	Runoff Area=15,040 sf 77.20% Impervious Runoff Depth>5.47" Tc=6.0 min CN=93 Runoff=2.00 cfs 6,850 cf
SubcatchmentC51: CB #51	Runoff Area=6,823 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.94 cfs 3,439 cf
SubcatchmentC52: CB#52	Runoff Area=9,052 sf 87.14% Impervious Runoff Depth>5.70" Tc=6.0 min CN=95 Runoff=1.23 cfs 4,297 cf
SubcatchmentC53: CB #53	Runoff Area=7,863 sf 86.52% Impervious Runoff Depth>5.47" Tc=6.0 min CN=93 Runoff=1.05 cfs 3,581 cf
SubcatchmentC54: CB #54	Runoff Area=4,821 sf 86.85% Impervious Runoff Depth>5.24" Tc=6.0 min CN=91 Runoff=0.63 cfs 2,104 cf
SubcatchmentC7: CB #5	Runoff Area=4,650 sf 100.00% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.64 cfs 2,344 cf

SubcatchmentC9: CB #9 Runoff Area=16,307 sf 93.95% Impervious Runoff Depth>5.93" Tc=6.0 min CN=97 Runoff=2.24 cfs 8,058 cf

Runoff Area=5,450 sf 88.75% Impervious Runoff Depth>5.58"

Tc=6.0 min CN=94 Runoff=0.73 cfs 2,534 cf

SubcatchmentCH1: CLUBHOUSE Runoff Area=6,262 sf 92.70% Impervious Runoff Depth>5.81" Tc=6.0 min CN=96 Runoff=0.86 cfs 3,033 cf

SubcatchmentMB1: MAIL KIOSK

Runoff Area=938 sf 100.00% Impervious Runoff Depth>6.05"

Tc=6.0 min CN=98 Runoff=0.13 cfs 473 cf

Tyne	III 24-hr	25VR	Rainfall=6.	20"
rype	III 24-III	201K	Railliaii-0.	.29

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 221

SubcatchmentS201: SUMMER STREET Runoff Area=9,943 sf 92.79% Impervious Runoff Depth>5.70" Tc=6.0 min CN=95 Runoff=1.35 cfs 4.720 cf

SubcatchmentS202: EXISTING WETLANDRunoff Area=432,269 sf 42.08% Impervious Runoff Depth>3.62" Flow Length=856' Tc=23.2 min CN=76 Runoff=26.72 cfs 130,245 cf

SubcatchmentS203: POCKET WETLAND#1 Runoff Area=25,587 sf 0.00% Impervious Runoff Depth>3.23"

Tc=6.0 min CN=72 Runoff=2.18 cfs 6,885 cf

SubcatchmentS204: EXISTING

Runoff Area=308,203 sf 31.07% Impervious Runoff Depth>3.52"
Flow Length=632' Tc=22.6 min CN=75 Runoff=18.73 cfs 90,283 cf

SubcatchmentS205: ISOLATEDWETLAND Runoff Area=55,420 sf 16.57% Impervious Runoff Depth>3.03"

Tc=6.0 min CN=70 Runoff=4.43 cfs 14.013 cf

SubcatchmentS206: OVERLANDFLOW Runoff Area=891,295 sf 2.91% Impervious Runoff Depth>2.54" Flow Length=1,467' Tc=34.5 min CN=65 Runoff=31.85 cfs 188,907 cf

SubcatchmentS207: INFILTRATIONPOND Runoff Area=20,803 sf 0.00% Impervious Runoff Depth>4.90" Tc=6.0 min CN=88 Runoff=2.59 cfs 8,498 cf

SubcatchmentS208: GRASS AREA Runoff Area=13,760 sf 9.33% Impervious Runoff Depth>3.53"

Tc=6.0 min CN=75 Runoff=1.28 cfs 4,045 cf

SubcatchmentS209: WETLANDC Runoff Area=107,073 sf 0.38% Impervious Runoff Depth>3.31" Flow Length=607' Slope=0.0150 '/' Tc=28.9 min CN=73 Runoff=5.50 cfs 29,544 cf

SubcatchmentS210: INFILTRATIONPOND Runoff Area=75,890 sf 0.00% Impervious Runoff Depth>3.83" Flow Length=580' Slope=0.0150 '/' Tc=16.5 min CN=78 Runoff=5.67 cfs 24,190 cf

SubcatchmentS211: S211

Runoff Area=15,436 sf 47.47% Impervious Runoff Depth>3.94"

Tc=6.0 min CN=79 Runoff=1.60 cfs 5,064 cf

SubcatchmentS212: SWALE

Runoff Area=52,768 sf 0.60% Impervious Runoff Depth>2.55"

Flow Length=418' Tc=23.1 min CN=65 Runoff=2.25 cfs 11,217 cf

SubcatchmentS213: COURTYARD

Runoff Area=21,407 sf 48.10% Impervious Runoff Depth>3.94"

Tc=6.0 min CN=79 Runoff=2.21 cfs 7,022 cf

SubcatchmentT1: Trench Drain 1Runoff Area=11,173 sf 75.10% Impervious Runoff Depth>5.47"
Tc=6.0 min CN=93 Runoff=1.49 cfs 5,089 cf

SubcatchmentT2: Drive Under B2 Runoff Area=4,445 sf 64.30% Impervious Runoff Depth>3.83" Tc=6.0 min CN=78 Runoff=0.45 cfs 1.420 cf

SubcatchmentTH1: TOWN HOUSE #1 Runoff Area=4,247 sf 92.68% Impervious Runoff Depth>5.81"

Tc=6.0 min CN=96 Runoff=0.58 cfs 2.057 cf

SubcatchmentTH10: TOWN HOUSE #10 Runoff Area=3,476 sf 91.60% Impervious Runoff Depth>5.81" Tc=6.0 min CN=96 Runoff=0.48 cfs 1,684 cf

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 222

SubcatchmentTH11: TOWN HOUSE #11 Runoff Area=4,210 sf 92.61% Impervious Runoff Depth>5.81" Tc=6.0 min CN=96 Runoff=0.58 cfs 2.039 cf

SubcatchmentTH2: TOWN HOUSE #2 Runoff Area=4,247 sf 92.68% Impervious Runoff Depth>5.81"

Tc=6.0 min CN=96 Runoff=0.58 cfs 2.057 cf

SubcatchmentTH3: TOWN HOUSE #3 Runoff Area=3,013 sf 88.68% Impervious Runoff Depth>5.70"
Tc=6.0 min CN=95 Runoff=0.41 cfs 1.430 cf

SubcatchmentTH4: TOWN HOUSE #4 Runoff Area=3,470 sf 91.59% Impervious Runoff Depth>5.81"

Tc=6.0 min CN=96 Runoff=0.47 cfs 1,681 cf

SubcatchmentTH5: TOWN HOUSE #5 Runoff Area=3,016 sf 88.69% Impervious Runoff Depth>5.70"

Tc=6.0 min CN=95 Runoff=0.41 cfs 1.432 cf

SubcatchmentTH6: TOWN HOUSE #6 Runoff Area=3,407 sf 91.46% Impervious Runoff Depth>5.81"

Tc=6.0 min CN=96 Runoff=0.47 cfs 1,650 cf

SubcatchmentTH7: TOWN HOUSE #7 Runoff Area=3,481 sf 91.61% Impervious Runoff Depth>5.81"

Tc=6.0 min CN=96 Runoff=0.48 cfs 1,686 cf

SubcatchmentTH8: TOWN HOUSE #8 Runoff Area=4,212 sf 92.62% Impervious Runoff Depth>5.81"

Tc=6.0 min CN=96 Runoff=0.58 cfs 2,040 cf

SubcatchmentTH9: TOWN HOUSE #9 Runoff Area=3,480 sf 91.61% Impervious Runoff Depth>5.81"

Tc=6.0 min CN=96 Runoff=0.48 cfs 1,686 cf

Reach 8R: OVERLAND FLOWAvg. Flow Depth=0.08' Max Vel=0.10 fps Inflow=1.38 cfs 4,854 cf n=0.400 L=563.0' S=0.0213 '/' Capacity=28.09 cfs Outflow=0.40 cfs 4,526 cf

Reach 9R: OVERLAND FLOWAvg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0 cf n=0.400 L=211.0' S=0.0652'/' Capacity=23.45 cfs Outflow=0.00 cfs 0 cf

Reach 10R: OVERLAND FLOWAvg. Flow Depth=0.25' Max Vel=0.26 fps Inflow=1.47 cfs 12,790 cf n=0.400 L=164.0' S=0.0366 '/' Capacity=17.57 cfs Outflow=1.45 cfs 12,790 cf

Reach 11R: 4x4 Open Bottom Culvert Avg. Flow Depth=1.12' Max Vel=2.11 fps Inflow=9.42 cfs 70,760 cf 48.0" x 48.0" Box Pipe n=0.069 L=30.0' S=0.0150 '/' Capacity=42.20 cfs Outflow=9.42 cfs 70,749 cf

Reach 12R: OVERLAND FLOWAvg. Flow Depth=0.12' Max Vel=0.14 fps Inflow=1.58 cfs 5,279 cf n=0.400 L=250.0' S=0.0240 '/' Capacity=29.80 cfs Outflow=0.85 cfs 5,158 cf

Reach 14R: OVERLAND FLOWAvg. Flow Depth=0.10' Max Vel=0.13 fps Inflow=2.25 cfs 11,215 cf n=0.400 L=852.0' S=0.0246 '/' Capacity=31.55 cfs Outflow=0.65 cfs 10,062 cf

Reach 15R: OVERLAND FLOWAvg. Flow Depth=0.06' Max Vel=0.08 fps Inflow=0.23 cfs 9,701 cf n=0.400 L=300.0' S=0.0200 '/' Capacity=27.21 cfs Outflow=0.23 cfs 8,871 cf

Reach 18R: OVERLAND FLOWAvg. Flow Depth=0.11' Max Vel=0.14 fps Inflow=3.39 cfs 7,468 cf n=0.400 L=609.0' S=0.0279 '/' Capacity=38.42 cfs Outflow=0.80 cfs 7,269 cf

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 223

Reach 20R: OVERLAND FLOWAvg. Flow Depth=0.23' Max Vel=0.13 fps Inflow=4.31 cfs 20,716 cf n=0.400 L=560.0' S=0.0093 '/' Capacity=18.54 cfs Outflow=1.57 cfs 19,942 cf

Reach 23R: OVERLAND FLOW Avg. Flow Depth=0.48' Max Vel=0.30 fps Inflow=9.42 cfs 70,749 cf

Reach R202: OVERLAND FLOW Avg. Flow Depth=0.52' Max Vel=0.23 fps Inflow=26.73 cfs 130,223 cf

n=0.400 L=700.0' S=0.0107 '/' Capacity=42.56 cfs Outflow=13.59 cfs 124,425 cf

n=0.400 L=237.0' S=0.0211 '/' Capacity=31.93 cfs Outflow=8.56 cfs 70,122 cf

Reach R211: OVERLAND FLOWAvg. Flow Depth=0.45' Max Vel=0.18 fps Inflow=11.59 cfs 20,184 cf n=0.400 L=600.0' S=0.0087 '/' Capacity=14.51 cfs Outflow=3.47 cfs 19,854 cf

Reach SC1: Stream Crossing #1 Avg. Flow Depth=0.43' Max Vel=3.86 fps Inflow=26.72 cfs 130,245 cf 192.0" x 60.0", R=207.0" Arch Pipe n=0.030 L=43.1' S=0.0200 '/' Capacity=722.91 cfs Outflow=26.73 cfs 130,223 cf

Reach SC2: Stream Crossing #2 Avg. Flow Depth=0.09' Max Vel=1.57 fps Inflow=2.25 cfs 11,217 cf 192.0" x 60.0", R=180.0" Arch Pipe n=0.030 L=36.5' S=0.0241 '/' Capacity=768.96 cfs Outflow=2.25 cfs 11,215 cf

Pond 1P: DMH #33 Peak Elev=206.34' Inflow=2.21 cfs 7,907 cf

12.0" Round Culvert n=0.013 L=46.7' S=0.0251 '/' Outflow=2.21 cfs 7,907 cf

Pond 3P: OCS #8 Peak Elev=202.09' Inflow=1.67 cfs 5,685 cf

Outflow=1.67 cfs 5,685 cf

Pond 5R: TRENCH DRAIN Peak Elev=198.33' Inflow=1.49 cfs 5,089 cf

8.0" Round Culvert n=0.012 L=36.0' S=0.0200 '/' Outflow=1.49 cfs 5,089 cf

Pond 11P: YARD DRAIN Peak Elev=207.41' Storage=883 cf Inflow=2.21 cfs 7,022 cf

Outflow=1.67 cfs 6,973 cf

Pond CB10: CB #10 Peak Elev=210.46' Inflow=0.96 cfs 3,508 cf

12.0" Round Culvert n=0.013 L=33.8' S=0.0050 '/' Outflow=0.96 cfs 3,508 cf

Pond CB11: CB #11 Peak Elev=210.55' Inflow=0.99 cfs 3,615 cf

12.0" Round Culvert n=0.013 L=26.3' S=0.0103 '/' Outflow=0.99 cfs 3,615 cf

Pond CB12: CB #12 Peak Elev=207.11' Inflow=0.72 cfs 2,640 cf 12.0" Round Culvert n=0.013 L=41.3' S=0.0249 '/' Outflow=0.72 cfs 2.640 cf

Pond CB13: CB #13 Peak Elev=207.35' Inflow=1.49 cfs 5,267 cf

12.0" Round Culvert n=0.013 L=43.7' S=0.0249 '/' Outflow=1.49 cfs 5,267 cf

Pond CB14: CB #14 Peak Elev=201.95' Inflow=1.57 cfs 5,281 cf

Pond CB15: CB #15 Peak Elev=201.84' Inflow=0.92 cfs 3,360 cf 12.0" Round Culvert n=0.013 L=15.6' S=0.0051 '/' Outflow=0.92 cfs 3,360 cf

Pond CB16: CB #16 Peak Elev=204.11' Inflow=0.88 cfs 2,794 cf

12.0" Round Culvert n=0.013 L=20.9' S=0.0067 '/' Outflow=0.88 cfs 2,794 cf

12.0" Round Culvert n=0.013 L=23.2' S=0.0052 '/' Outflow=1.57 cfs 5,281 cf

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 224

Pond CB17: CB #17	Peak Elev=205.78' Inflow=1.56 cfs 5,278 cfs 12.0" Round Culvert n=0.013 L=13.8' S=0.0094'/ Outflow=1.56 cfs 5,278 cfs 12.0"	
Pond CB18: CB #18	Peak Elev=205.80' Inflow=2.77 cfs 9,314 cfs 15.0" Round Culvert n=0.013 L=25.1' S=0.0052'/ Outflow=2.77 cfs 9,314 cfs 15.0"	
Pond CB20: CB #20	Peak Elev=204.88' Inflow=1.62 cfs 5,667 cfs 12.0" Round Culvert n=0.013 L=30.3' S=0.0053'/' Outflow=1.62 cfs 5,667 cfs 12.0"	
Pond CB21: CB #21	Peak Elev=205.05' Inflow=1.30 cfs 4,345 cfs 12.0" Round Culvert n=0.013 L=26.0' S=0.0050 '/' Outflow=1.30 cfs 4,345	
Pond CB22: CB #22	Peak Elev=206.17' Inflow=1.64 cfs 5,813 cfs 12.0" Round Culvert n=0.012 L=16.1' S=0.0050 '/' Outflow=1.64 cfs 5,813 cfs 12.0"	
Pond CB23: CB #23	Peak Elev=206.10' Inflow=1.22 cfs 4,050 cfs 12.0" Round Culvert n=0.012 L=16.3' S=0.0055 '/' Outflow=1.22 cfs 4,050 cfs 12.0"	
Pond CB24: CB #24	Peak Elev=205.98' Inflow=1.06 cfs 3,612 cfs 12.0" Round Culvert n=0.012 L=12.1' S=0.0050 '/' Outflow=1.06 cfs 3,612 cfs 12.0"	
Pond CB25: CB #25	Peak Elev=205.96' Inflow=1.14 cfs 3,947 cfs 15.0" Round Culvert n=0.012 L=11.4' S=0.0053'/' Outflow=1.14 cfs 3,947 cfs 15.0"	
Pond CB26: CB #26	Peak Elev=202.43' Inflow=1.15 cfs 3,856 cf 12.0" Round Culvert n=0.013 L=42.5' S=0.0052'/' Outflow=1.15 cfs 3,856 cf	
Pond CB27: CB #27	Peak Elev=201.56' Inflow=0.84 cfs 2,960 cfs 12.0" Round Culvert n=0.013 L=18.0' S=0.0056'/' Outflow=0.84 cfs 2,960 c	
Pond CB28: CB #28	Peak Elev=198.60' Inflow=1.29 cfs 4,237 cfs 12.0" Round Culvert n=0.013 L=13.7' S=0.0044 '/' Outflow=1.29 cfs 4,237 cfs 12.0"	
Pond CB29: CB #29	Peak Elev=206.22' Inflow=1.14 cfs 3,950 cf 12.0" Round Culvert n=0.013 L=13.5' S=0.0052'/' Outflow=1.14 cfs 3,950 cf	
Pond CB30: CB #30	Peak Elev=206.24' Inflow=1.20 cfs 4,154 cfs 12.0" Round Culvert n=0.013 L=17.5' S=0.0051 '/' Outflow=1.20 cfs 4,154 cfs 12.0"	
Pond CB31: CB #31	Peak Elev=205.18' Inflow=2.10 cfs 6,989 cf 12.0" Round Culvert n=0.013 L=16.4' S=0.0049'/' Outflow=2.10 cfs 6,989 cf	
Pond CB32: CB #32	Peak Elev=205.04' Inflow=1.65 cfs 5,548 cfs 12.0" Round Culvert n=0.013 L=16.3' S=0.0049'/' Outflow=1.65 cfs 5,548 cfs 12.0"	
Pond CB33: CB #33	Peak Elev=205.97' Inflow=0.73 cfs 2,521 cfs 12.0" Round Culvert n=0.013 L=11.7' S=0.0051 '/' Outflow=0.73 cfs 2,521 cfs 12.0"	
Pond CB34: CB #34	Peak Elev=206.04' Inflow=1.15 cfs 3,927 cfs 12.0" Round Culvert n=0.013 L=16.5' S=0.0048'/' Outflow=1.15 cfs 3,927 cfs 12.0"	

Pond CB53: CB #53

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 225

Pond CB35: CB #35	Peak Elev=207.50' Inflow=0.57 cfs 2,091 cf 12.0" Round Culvert n=0.013 L=15.2' S=0.0053 '/' Outflow=0.57 cfs 2,091 cf
Pond CB36: CB #36	Peak Elev=207.64' Inflow=0.92 cfs 3,337 cf 12.0" Round Culvert n=0.013 L=16.1' S=0.0050 '/' Outflow=0.92 cfs 3,337 cf
Pond CB38: CB #38	Peak Elev=210.83' Inflow=1.06 cfs 3,849 cf 12.0" Round Culvert n=0.012 L=16.7' S=0.0048 '/' Outflow=1.06 cfs 3,849 cf
Pond CB39: CB #39	Peak Elev=210.83' Inflow=1.05 cfs 3,836 cf 12.0" Round Culvert n=0.013 L=16.4' S=0.0049 '/' Outflow=1.05 cfs 3,836 cf
Pond CB40: CB #40	Peak Elev=214.28' Inflow=0.58 cfs 2,122 cf 12.0" Round Culvert n=0.013 L=17.8' S=0.0073 '/' Outflow=0.58 cfs 2,122 cf
Pond CB41: CB #41	Peak Elev=214.43' Inflow=0.77 cfs 2,815 cf 12.0" Round Culvert n=0.013 L=18.4' S=0.0049'/ Outflow=0.77 cfs 2,815 cf
Pond CB43: CB #43	Peak Elev=220.52' Inflow=0.39 cfs 1,299 cf 12.0" Round Culvert n=0.013 L=14.9' S=0.0047'/ Outflow=0.39 cfs 1,299 cf
Pond CB44: CB #44	Peak Elev=220.49' Inflow=0.26 cfs 882 cf 12.0" Round Culvert n=0.013 L=14.9' S=0.0047 '/' Outflow=0.26 cfs 882 cf
Pond CB45: CB #45	Peak Elev=221.59' Inflow=0.26 cfs 830 cf 12.0" Round Culvert n=0.013 L=18.2' S=0.0049 '/' Outflow=0.26 cfs 830 cf
Pond CB46: CB #46	Peak Elev=221.94' Inflow=0.47 cfs 1,481 cf 12.0" Round Culvert n=0.013 L=15.3' S=0.0052'/' Outflow=0.47 cfs 1,481 cf
Pond CB47: CB#47	Peak Elev=225.79' Inflow=0.42 cfs 1,518 cf 12.0" Round Culvert n=0.012 L=20.9' S=0.0048 '/' Outflow=0.42 cfs 1,518 cf
Pond CB48: CB#48	Peak Elev=226.24' Inflow=4.02 cfs 15,183 cf 15.0" Round Culvert n=0.012 L=16.9' S=0.0047 '/' Outflow=4.02 cfs 15,183 cf
Pond CB49: CB #49	Peak Elev=203.65' Inflow=0.70 cfs 2,436 cf 12.0" Round Culvert n=0.013 L=15.5' S=0.0052'/ Outflow=0.70 cfs 2,436 cf
Pond CB50: CB #50	Peak Elev=203.87' Inflow=2.00 cfs 6,850 cf 12.0" Round Culvert n=0.013 L=15.3' S=0.0052'/ Outflow=2.00 cfs 6,850 cf
Pond CB51: CB #51	Peak Elev=203.36' Inflow=0.94 cfs 3,439 cf 12.0" Round Culvert n=0.013 L=31.4' S=0.0051'/ Outflow=0.94 cfs 3,439 cf
Pond CB52: CB #52	Peak Elev=203.38' Inflow=1.23 cfs 4,297 cf

12.0" Round Culvert n=0.013 L=25.5' S=0.0051'/' Outflow=1.23 cfs 4,297 cf

12.0" Round Culvert n=0.013 L=32.0' S=0.0050 '/' Outflow=1.05 cfs 3,581 cf

Peak Elev=203.42' Inflow=1.05 cfs 3,581 cf

Printed 6/19/2023

Page 226

Pond CB54: CB #54 Peak Elev=203.14' Inflow=0.63 cfs 2,104 cf

12.0" Round Culvert n=0.013 L=36.7' S=0.0049 '/' Outflow=0.63 cfs 2,104 cf

Pond CB7: CB#5 Peak Elev=213.04' Inflow=0.64 cfs 2,344 cf

12.0" Round Culvert n=0.012 L=15.1' S=0.0099 '/' Outflow=0.64 cfs 2,344 cf

Pond CB8: CB#8 Peak Elev=214.27' Inflow=0.73 cfs 2,534 cf

12.0" Round Culvert n=0.013 L=15.1' S=0.0099 '/' Outflow=0.73 cfs 2,534 cf

Pond CB9: CB #9 Peak Elev=210.95' Inflow=2.24 cfs 8,058 cf

12.0" Round Culvert n=0.013 L=19.9' S=0.0196 '/' Outflow=2.24 cfs 8,058 cf

Pond D10: DMH #10 Peak Elev=203.91' Inflow=0.88 cfs 2,794 cf

12.0" Round Culvert n=0.013 L=15.6' S=0.0051'/' Outflow=0.88 cfs 2,794 cf

Pond D11: DMH #11 Peak Elev=205.64' Inflow=4.32 cfs 14,592 cf

18.0" Round Culvert n=0.013 L=44.6' S=0.0049 '/' Outflow=4.32 cfs 14,592 cf

Pond D12: DMH #12 Peak Elev=204.60' Inflow=2.92 cfs 10,012 cf

12.0" Round Culvert n=0.013 L=41.9' S=0.0050 '/' Outflow=2.92 cfs 10,012 cf

Pond D13: DMH #13 Peak Elev=203.59' Inflow=9.39 cfs 34,406 cf

24.0" Round Culvert n=0.013 L=60.1' S=0.0050 '/' Outflow=9.39 cfs 34,406 cf

Pond D14: DMH #14 Peak Elev=205.34' Inflow=5.06 cfs 17,421 cf

18.0" Round Culvert n=0.012 L=256.3' S=0.0050 '/' Outflow=5.06 cfs 17,421 cf

Pond D16: DMH #16 Peak Elev=205.84' Inflow=2.20 cfs 7,558 cf

15.0" Round Culvert n=0.012 L=103.5' S=0.0050 '/' Outflow=2.20 cfs 7,558 cf

Pond D17: DMH #17 Peak Elev=201.33' Inflow=1.98 cfs 6,816 cf

12.0" Round Culvert n=0.013 L=91.6' S=0.0312 '/' Outflow=1.98 cfs 6,816 cf

Pond D18: DMH #18 Peak Elev=198.43' Inflow=3.27 cfs 11,053 cf

15.0" Round Culvert n=0.013 L=46.3' S=0.0099 '/' Outflow=3.27 cfs 11,053 cf

Pond D19: DMH #19 Peak Elev=206.09' Inflow=2.34 cfs 8,105 cf

12.0" Round Culvert n=0.013 L=82.5' S=0.0092 '/' Outflow=2.34 cfs 8,105 cf

Pond D2: DMH#2 Peak Elev=208.37' Inflow=5.38 cfs 21,579 cf 15.0" Round Culvert n=0.013 L=38.2' S=0.0099 '/' Outflow=5.38 cfs 21,579 cf

Pond D20: DMH #20 Peak Elev=205.13' Inflow=2.34 cfs 8,105 cf 15.0" Round Culvert n=0.013 L=63.5' S=0.0050 '/' Outflow=2.34 cfs 8,105 cf

Pond D21: DMH #21 Peak Elev=204.66' Inflow=9.46 cfs 32,518 cf

24.0" Round Culvert n=0.013 L=72.4' S=0.0050 '/' Outflow=9.46 cfs 32.518 cf

Pond D22: DMH #22 Peak Elev=205.89' Inflow=3.37 cfs 11,876 cf

15.0" Round Culvert n=0.013 L=134.2' S=0.0071 '/' Outflow=3.37 cfs 11,876 cf

Printed 6/19/2023 Page 227

Pond D23: DMH #23		Peak Elev=207.31	I' Inflow=1.49 cfs 5,429 cf
	15.0" Round Culvert n=0.013	L=173.3' S=0.0100 '/'	Outflow=1.49 cfs 5,429 cf
D I DOE - DMII #05		DI040 701	I

Pond D25: DMH #25 Peak Elev=210.76' Inflow=4.84 cfs 17,114 cf 15.0" Round Culvert n=0.012 L=237.6' S=0.0050 '/' Outflow=4.84 cfs 17,114 cf

Pond D27: DMH #27 Peak Elev=214.18' Inflow=2.73 cfs 9,429 cf 15.0" Round Culvert n=0.012 L=63.9' S=0.0150 '/' Outflow=2.73 cfs 9,429 cf

Pond D28: DMH #28 Peak Elev=218.08' Inflow=1.38 cfs 4,491 cf 12.0" Round Culvert n=0.013 L=158.3' S=0.0200 '/' Outflow=1.38 cfs 4,491 cf

Pond D29: DMH #29 Peak Elev=220.45' Inflow=1.38 cfs 4,491 cf 12.0" Round Culvert n=0.013 L=150.9' S=0.0151 '/' Outflow=1.38 cfs 4.491 cf

Pond D30: DMH #30 Peak Elev=221.43' Inflow=0.73 cfs 2,311 cf 12.0" Round Culvert n=0.013 L=184.2' S=0.0050 '/' Outflow=0.73 cfs 2,311 cf

Pond D31: DMH#31 Peak Elev=225.78' Inflow=4.31 cfs 16,701 cf 15.0" Round Culvert n=0.012 L=288.5' S=0.0400 '/' Outflow=4.31 cfs 16,701 cf

Pond D34: DMH #34 Peak Elev=199.29' Inflow=3.21 cfs 11,720 cf 12.0" Round Culvert n=0.012 L=52.0' S=0.0200 '/' Outflow=3.21 cfs 11,720 cf

Pond D35: DMH #35 Peak Elev=213.12' Inflow=2.73 cfs 9,429 cf 15.0" Round Culvert n=0.012 L=171.5' S=0.0150 '/' Outflow=2.73 cfs 9,429 cf

Pond D4: DMH#4 Peak Elev=211.79' Inflow=5.38 cfs 21,579 cf 15.0" Round Culvert n=0.012 L=222.3' S=0.0150 '/' Outflow=5.38 cfs 21,579 cf

Pond D5: DMH #5

Peak Elev=210.26' Inflow=4.20 cfs 15,182 cf
18.0" Round Culvert n=0.013 L=183.0' S=0.0050 '/' Outflow=4.20 cfs 15,182 cf

Pond D6: DMH #6 Peak Elev=209.19' Inflow=4.20 cfs 15,182 cf 18.0" Round Culvert n=0.013 L=299.7' S=0.0050 '/' Outflow=4.20 cfs 15,182 cf

Pond D7: DMH #7 Peak Elev=207.46' Inflow=4.20 cfs 15,182 cf 18.0" Round Culvert n=0.013 L=44.2' S=0.0550 '/' Outflow=4.20 cfs 15,182 cf

Pond D8: DMH #8 Peak Elev=201.79' Inflow=2.49 cfs 8,641 cf 12.0" Round Culvert n=0.013 L=87.7' S=0.0050 '/' Outflow=2.49 cfs 8,641 cf

Pond D9: DMH #9 Peak Elev=201.16' Inflow=2.49 cfs 8,641 cf 12.0" Round Culvert n=0.013 L=11.9' S=0.0050 '/' Outflow=2.49 cfs 8,641 cf

Pond DE61: DRIP #61 Peak Elev=213.33' Storage=303 cf Inflow=0.58 cfs 2,057 cf Discarded=0.00 cfs 203 cf Primary=0.48 cfs 1,708 cf Outflow=0.48 cfs 1,911 cf

Pond DE62: DRIP #62 Peak Elev=213.33' Storage=303 cf Inflow=0.58 cfs 2,057 cf Discarded=0.00 cfs 203 cf Primary=0.48 cfs 1,708 cf Outflow=0.48 cfs 1,911 cf

Printed 6/19/2023

Page 228

Pond DE63: DRIP #63

Peak Elev=208.01' Storage=164 cf Inflow=0.41 cfs 1,430 cf

Discarded=0.00 cfs 121 cf Primary=0.37 cfs 1,221 cf Outflow=0.37 cfs 1,342 cf

Pond DE64: DRIP #64 Peak Elev=206.06' Storage=201 cf Inflow=0.47 cfs 1,681 cf

Discarded=0.00 cfs 144 cf Primary=0.42 cfs 1,433 cf Outflow=0.42 cfs 1,578 cf

Pond DE65: DRIP #65 Peak Elev=207.01' Storage=165 cf Inflow=0.41 cfs 1,432 cf

Discarded=0.00 cfs 121 cf Primary=0.37 cfs 1,222 cf Outflow=0.37 cfs 1,343 cf

Pond DE66: DRIP #66 Peak Elev=208.85' Storage=199 cf Inflow=0.47 cfs 1,650 cf

 $Discarded = 0.00 \ cfs \ 144 \ cf \ Primary = 0.41 \ cfs \ 1,403 \ cf \ Outflow = 0.42 \ cfs \ 1,547 \ cf$

Pond DE67: DRIP #67 Peak Elev=209.06' Storage=201 cf Inflow=0.48 cfs 1,686 cf

Discarded=0.00 cfs 144 cf Primary=0.42 cfs 1,439 cf Outflow=0.42 cfs 1,583 cf

Pond DE68: DRIP #68 Peak Elev=208.02' Storage=274 cf Inflow=0.58 cfs 2,040 cf

Discarded=0.00 cfs 203 cf Primary=0.49 cfs 1,694 cf Outflow=0.49 cfs 1,897 cf

Pond DE69: DRIP #69 Peak Elev=206.56' Storage=201 cf Inflow=0.48 cfs 1,686 cf

Discarded=0.00 cfs 144 cf Primary=0.42 cfs 1,438 cf Outflow=0.42 cfs 1,582 cf

Pond DE70: DRIP #70 Peak Elev=206.96' Storage=201 cf Inflow=0.48 cfs 1,684 cf

Discarded=0.00 cfs 144 cf Primary=0.42 cfs 1,436 cf Outflow=0.42 cfs 1,581 cf

Pond DE71: DRIP #71 Peak Elev=207.62' Storage=302 cf Inflow=0.58 cfs 2,039 cf

Discarded=0.00 cfs 203 cf Primary=0.48 cfs 1,690 cf Outflow=0.48 cfs 1,893 cf

Pond DECH: DRIP #CH Peak Elev=209.99' Storage=508 cf Inflow=0.86 cfs 3,033 cf

Discarded=0.04 cfs 1,659 cf Primary=0.42 cfs 1,373 cf Outflow=0.45 cfs 3,033 cf

Pond DMH32: DMH #32 Peak Elev=203.60' Inflow=2.71 cfs 9,286 cf

12.0" Round Culvert n=0.013 L=19.2' S=0.0531 '/' Outflow=2.71 cfs 9,286 cf

Pond OCS1: OCS#1 Peak Elev=196.41' Inflow=6.49 cfs 22,774 cf

Outflow=6.49 cfs 22,774 cf

Pond OCS3: OCS#3 Peak Elev=205.31' Inflow=6.75 cfs 23,442 cf

Outflow=6.75 cfs 23.442 cf

Pond OCS4: OCS#4 Peak Elev=205.10' Inflow=1.75 cfs 5,738 cf

Outflow=1.75 cfs 5,738 cf

Pond OCS6: OCS #6 Peak Elev=203.36' Inflow=2.21 cfs 7,907 cf

Outflow=2.21 cfs 7,907 cf

Pond OCS7: OCS #7 Peak Elev=203.36' Inflow=2.17 cfs 7,735 cf

Outflow=2.17 cfs 7.735 cf

Pond P204: STORMTECHINFILTRATION Peak Elev=205.09' Storage=9,827 cf Inflow=8.49 cfs 29,181 cf Discarded=0.09 cfs 6,064 cf Primary=4.31 cfs 20,716 cf Outflow=4.39 cfs 26,780 cf

19097	Post-Dev	elopment
13031	I USL-DEV	CIODIIICIIL

Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 229

Pond P205: INFILTRATIONPOND #3 Peak Elev=206.84' Storage=8,662 cf Inflow=6.85 cfs 26,643 cf Discarded=0.31 cfs 15,146 cf Primary=3.39 cfs 7,468 cf Outflow=3.69 cfs 22,614 cf

Pond P206: STORMTECHINFILTRATION Peak Elev=196.29' Storage=6,969 cf Inflow=7.97 cfs 27,862 cf Discarded=0.49 cfs 21,285 cf Primary=3.67 cfs 6,574 cf Outflow=4.16 cfs 27,859 cf

Pond P207: INFILTRATIONPOND #2 Peak Elev=198.50' Storage=19,452 cf Inflow=15.34 cfs 54,338 cf Discarded=1.09 cfs 41,530 cf Primary=1.47 cfs 12,790 cf Outflow=2.56 cfs 54,320 cf

Pond P210: POCKET WETLAND#1 Peak Elev=204.35' Storage=17,796 cf Inflow=7.02 cfs 23,999 cf Outflow=0.23 cfs 9,701 cf

Pond P212: INFILTRATIONPOND #1 Peak Elev=202.81' Storage=26,231 cf Inflow=21.45 cfs 85,741 cf Discarded=1.44 cfs 64,639 cf Primary=11.59 cfs 20,184 cf Outflow=13.03 cfs 84,823 cf

Pond P213: Stormtech Infiltration System Peak Elev=203.36' Storage=5,458 cf Inflow=4.38 cfs 15,642 cf Discarded=0.39 cfs 15,640 cf Primary=0.00 cfs 1 cf Outflow=0.40 cfs 15,641 cf

Pond P214: STORMTECHINFILTRATION Peak Elev=202.09' Storage=4,534 cf Inflow=4.44 cfs 15,756 cf Discarded=0.61 cfs 15,774 cf Primary=0.00 cfs 0 cf Outflow=0.61 cfs 15,774 cf

Link AP1: ANALYSISPOINT 1 Inflow=1.35 cfs 4,720 cf Primary=1.35 cfs 4,720 cf

Link AP2: ANALYSISPOINT 2Inflow=28.46 cfs 229,211 cf
Primary=28.46 cfs 229,211 cf

Link AP3: ANALYSISPOINT 3 Inflow=4.43 cfs 14,013 cf Primary=4.43 cfs 14,013 cf

Link AP4: ANALYSISPOINT #4Inflow=43.69 cfs 300,250 cf
Primary=43.69 cfs 300,250 cf

Total Runoff Area = 2,573,920 sf Runoff Volume = 764,876 cf Average Runoff Depth = 3.57" 70.09% Pervious = 1,803,997 sf 29.91% Impervious = 769,923 sf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 230

Summary for Subcatchment B1: MULTIFAMILY BLDG #1

Runoff = 3.21 cfs @ 12.09 hrs, Volume= 11,720 cf, Depth> 6.05"

Routed to Pond D34: DMH #34

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	rea (sf)	CN	Description			
	20,156	98	Roofs, HSG	C		
	3,099	98	Roofs, HSG	D D		
	23,255	98	Weighted A	verage		
	23,255		100.00% Impervious Area			
Tc	Length	Slope	e Velocity	Capacity	Description	
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment B2: MULTIFAMILY BLDG #2

Runoff = 2.43 cfs @ 12.09 hrs, Volume= 8,851 cf, Depth> 6.05"

Routed to Pond OCS3: OCS#3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	Area (sf)	CN	Description				
	7,873	98	Roofs, HSC	A S			
	9,688	98	Roofs, HSC	G C			
	17,561	98	98 Weighted Average				
	17,561		100.00% In	Area			
	c Length		•	Capacity	Description		
<u>(mir</u>	n) (feet)) (ft/1	ft) (ft/sec)	(cfs)			
6.	0				Direct Entry,		

Summary for Subcatchment B3: MULTIFAMILY BUILDING #3

Runoff = 2.76 cfs @ 12.09 hrs, Volume= 10,070 cf, Depth> 6.05" Routed to Pond P214 : STORMTECH INFILTRATION SYSTEM #4

 Area (sf)	CN	Description
 608	98	Roofs, HSG A
 19,373	98	Roofs, HSG C
19,981 19,981	98	Weighted Average 100.00% Impervious Area

Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

<u>Page 231</u>

	• .				Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry	' ,

Summary for Subcatchment C10: CB #10

Runoff = 0.96 cfs @ 12.09 hrs, Volume= 3,508 cf, Depth> 6.05"

Routed to Pond CB10: CB #10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description					
	339	98	Paved road	s w/curbs &	& sewers, HSG B			
	5,703	98	Paved road	s w/curbs &	& sewers, HSG C			
	919	98	Paved road	s w/curbs &	& sewers, HSG D			
	6,961	98	Weighted Average					
	6,961		100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment C11: CB #11

Runoff = 0.99 cfs @ 12.09 hrs, Volume= 3,615 cf, Depth> 6.05"

Routed to Pond CB11 : CB #11

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Α	rea (sf)	CN [Description						
	7,173	98 F	Paved roads w/curbs & sewers, HSG C						
	7,173	,	100.00% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry.				

Summary for Subcatchment C12: CB #12

Runoff = 0.72 cfs @ 12.09 hrs, Volume= 2,640 cf, Depth> 6.05"

Routed to Pond CB12 : CB #12

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 232

	Area (sf)	CN E	Description					
	5,238	98 F	Paved roads w/curbs & sewers, HSG C					
	5,238	1	100.00% Impervious Area					
To	Longth	Slope	Velocity	Capacity	Description			
(min)	Length (feet)	(ft/ft)	(ft/sec)	(cfs)	Description			
6.0			, ,		Direct Entry,			

Summary for Subcatchment C13: CB #13

Runoff = 1.49 cfs @ 12.09 hrs, Volume=

5,267 cf, Depth> 5.81"

Routed to Pond CB13: CB #13

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	rea (sf)	CN	Description					
	1,003	74	>75% Gras	s cover, Go	lood, HSG C			
	7,547	98	Paved park	ing, HSG C	C			
	2,323	98	Roofs, HSC	S C				
	10,873	96	Weighted Average					
	1,003		9.22% Pervious Area					
	9,870		90.78% Impervious Area					
_								
Тс	3	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C14: CB #14

Runoff = 1.57 cfs @ 12.09 hrs, Volume=

5,281 cf, Depth> 5.24"

Routed to Pond CB14: CB #14

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

 Α	rea (sf)	CN	Description							
	1,195	39	>75% Grass cover, Good, HSG A							
	7,649	98	Paved park	ing, HSG A	١					
	472	74	>75% Gras	s cover, Go	ood, HSG C					
	2,783	98	Paved parking, HSG C							
	12,099	91	Weighted Average							
	1,667		13.78% Per	vious Area	I					
	10,432		86.22% Imp	pervious Ar	ea					
т.	ما العرب ال	Clan	- \/alaaitu	Conneitu	December					
Tc	Length	Slop	,	Capacity	Description					
 (min)	(feet)	(ft/f	t) (ft/sec)	(cfs)						
~ ~					D: (= (

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 233

Summary for Subcatchment C15: CB #15

Runoff = 0.92 cfs @ 12.09 hrs, Volume=

3,360 cf, Depth> 6.05"

Routed to Pond CB15: CB #15

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description							
	5,000	98	Paved parking, HSG A							
	1,666	98	Paved park	Paved parking, HSG C						
	6,666	98	Weighted Average							
	6,666		100.00% Impervious Area							
Tc	Length	Slope	e Velocity	Capacity	Description					
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
6.0					Direct Entry,					

• .

Summary for Subcatchment C16: CB #16

Runoff = 0.88 cfs @ 12.09 hrs, Volume= 2,79

2,794 cf, Depth> 3.94"

Routed to Pond CB16: CB #16

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description							
	2,467	39	>75% Gras	s cover, Go	Good, HSG A					
	4,380	98	Paved park	ing, HSG A	A					
	524	74	>75% Gras	s cover, Go	Good, HSG C					
	1,145	98	Paved park	ing, HSG C	C					
	8,516	79	Weighted Average							
	2,991		35.12% Pe	vious Area	a					
	5,525		64.88% Imp	ervious Ar	ırea					
Tc	Length	Slope	e Velocity Capacity Description							
(min)	(feet)	(ft/ft	t) (ft/sec) (cfs)							
6.0			Direct Entry,							

Summary for Subcatchment C17: CB #17

Runoff = 1.56 cfs @ 12.09 hrs, Volume= 5,278 cf, Depth> 5.35"

Routed to Pond CB17: CB #17

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 234

Α	rea (sf)	CN	Description							
	3,093	74	>75% Grass cover, Good, HSG C							
	8,743	98	Paved park	ing, HSG C	C					
	11,836	92	02 Weighted Average							
	3,093		26.13% Pei	rvious Area	a					
	8,743	•	73.87% Imp	pervious Ar	ırea					
_				_						
Tc	Length	Slope	Slope Velocity Capacity Description							
(min)	(feet)	(ft/ft)	(t/ft) (ft/sec) (cfs)							
6.0		Direct Entry								

6.0 Direct Entry,

Summary for Subcatchment C18: CB #18

Runoff = 2.38 cfs @ 12.09 hrs, Volume= 7,940 cf, Depth> 5.13"

Routed to Pond CB18: CB #18

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Aı	rea (sf)	CN	Description							
	6,255	74	>75% Grass cover, Good, HSG C							
	12,336	98	Paved park	ing, HSG C	C					
	18,591	8,591 90 Weighted Average								
	6,255 33.65% Pervious Area									
	12,336		66.35% Imp	pervious Ar	rea					
т.	l 4l-	Class.	\	0	Description					
Tc	Length	Slope	,	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment C20: CB #20

Runoff = 1.62 cfs @ 12.09 hrs, Volume= 5,667 cf, Depth> 5.70"

Routed to Pond CB20 : CB #20

Area(sf) CN	Description						
3,3	19 98	Paved parking, HSG A						
1,3	19 74	>75% Grass cover, Good, HSG C						
7,3	01 98	Paved parking, HSG C						
11,9	39 95	Weighted Average						
1,3	19	11.05% Pervious Area						
10,6	20	88.95% Impervious Area						

Printed 6/19/2023

Page 235

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment C21: CB #21

1.30 cfs @ 12.09 hrs, Volume=

4,345 cf, Depth> 5.13"

Routed to Pond CB21 : CB #21

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	Area (sf)	CN	Description						
	1,319	39	>75% Gras	s cover, Go	Good, HSG A				
	7,301	98	Paved park	ing, HSG A	A				
	1,554	98	Paved park	ing, HSG C	C				
	10,174	90	90 Weighted Average						
	1,319		12.96% Per	vious Area	a				
	8,855		87.04% Imp	ervious Ar	ırea				
_				_					
Tc	J	Slop	•	Capacity	•				
<u>(min)</u>	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry.				

Summary for Subcatchment C22: CB #22

1.64 cfs @ 12.09 hrs, Volume= Runoff

5,813 cf, Depth> 5.81"

Routed to Pond CB22 : CB #22

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Area (sf) CN	Description						
2,946	98	Paved park	ing, HSG A	1				
177	7 74	>75% Gras	s cover, Go	ood, HSG C				
2,641	1 98	Paved park	ing, HSG C	,				
829	80	>75% Gras	s cover, Go	ood, HSG D				
5,408	3 98	Paved park	ing, HSG D)				
12,001	1 96	Weighted A	verage					
1,006	3	8.38% Perv	ious Area					
10,995	5	91.62% Imp	pervious Ar	ea				
Tc Lengt		,	Capacity	Description				
(min) (fee	t) (ft/	ft) (ft/sec)	(cfs)					
6.0				Direct Entry,				

Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 236

Summary for Subcatchment C23: CB #23

Runoff = 1.22 cfs @ 12.09 hrs, Volume= 4,050 cf, Depth> 5.01"

Routed to Pond CB23: CB #23

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description					
	242	39	>75% Gras	s cover, Go	ood, HSG A			
	3,016	98	Paved park	ing, HSG A	١			
	1,267	74	>75% Gras	s cover, Go	ood, HSG C			
	218	98	Paved park	ing, HSG C	;			
	2,272	80	>75% Gras	s cover, Go	ood, HSG D			
	2,679	98	Paved park	ing, HSG [)			
	9,694	89	Weighted A	verage				
	3,781		39.00% Per	vious Area				
	5,913		61.00% Imp	ervious Ar	ea			
Tc	Length	Slop						
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C24: CB #24

Runoff = 1.06 cfs @ 12.09 hrs, Volume= 3,612 cf, Depth> 5.47"

Routed to Pond CB24: CB #24

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	rea (sf)	CN	Description						
	5,722	98	Paved parking, HSG D						
	2,208	80	>75% Ġras	s cover, Go	lood, HSG D				
	7,930	93	Weighted Average						
	2,208		27.84% Pe	vious Area	a				
	5,722		72.16% lm <mark>։</mark>	ervious Ar	rea				
_									
Тс	3	Slope	,	Capacity	·				
<u>(min)</u>	(feet)	(ft/ft)	t) (ft/sec) (cfs)						
6.0					Direct Entry,				

Summary for Subcatchment C25: CB #25

Runoff = 1.14 cfs @ 12.09 hrs, Volume= 3,947 cf, Depth> 5.58"

Routed to Pond CB25 : CB #25

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 237

A	rea (sf)	CN	Description					
	211	39	>75% Gras	s cover, Go	od, HSG A			
	519	98	Paved park	ing, HSG A				
	15	74	>75% Gras	s cover, Go	od, HSG C			
	300	98	Paved park	ing, HSG C				
	1,393	80	>75% Gras	s cover, Go	od, HSG D			
	6,049	98	Paved park	ing, HSG D				
	8,487	94	Weighted A	verage				
	1,619		19.08% Per	vious Area				
	6,868		80.92% Imp	ervious Ar	a			
Tc	Length	Slope	pe Velocity Capacity Description					
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C26: CB #26

Runoff 1.15 cfs @ 12.09 hrs, Volume= 3,856 cf, Depth> 5.24"

Routed to Pond CB26: CB #26

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description						
	3,203	80	>75% Grass cover, Good, HSG D						
	5,632	98	Paved parking, HSG D						
	8,835	91	Weighted Average						
	3,203	;	36.25% Pei	rvious Area	a				
	5,632	(33.75% Imp	pervious Ar	rea				
Tc	Length	Slope	,	Capacity	·				
(min)	(feet)	(ft/ft)	(ft/sec) (cfs)						
6.0			Direct Entry,						

Summary for Subcatchment C27: CB #27

2,960 cf, Depth> 5.81" 0.84 cfs @ 12.09 hrs, Volume= Runoff

Routed to Pond CB27: CB #27

Area (sf)	CN	Description			
98	39	>75% Grass cover, Good, HSG A			
131	98	Paved parking, HSG A			
397	80	0 >75% Grass cover, Good, HSG D			
5,485	98	Paved parking, HSG D			
6,111	96	Weighted Average			
495		8.10% Pervious Area			
5,616		91.90% Impervious Area			

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 238

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment C28: CB #28

1.29 cfs @ 12.09 hrs, Volume= 4,237 cf, Depth> 4.90" Runoff =

Routed to Pond CB28: CB #28

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Aı	rea (sf)	CN	Description					
	2,751	74	>75% Grass	cover, Go	Good, HSG C			
	2,841	98	Paved parki	ng, HSG C	C			
	2,297	80	>75% Grass	cover, Go	Good, HSG D			
	2,483	98	Paved parki	ng, HSG D	D			
•	10,372	88	Weighted Average					
	5,048		48.67% Per	vious Area	a			
	5,324		51.33% Imp	ervious Ar	rea			
_								
Тс	Length	Slope	•	Capacity	•			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C29: CB #29

1.14 cfs @ 12.09 hrs, Volume=

3,950 cf, Depth> 5.58"

Routed to Pond CB29: CB #29

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	Δ	rea (sf)	CN [Description						
-										
		1,341	74 >	∙75% Gras	s cover, Go	Good, HSG C				
		5,330	98 F	Paved park	ing, HSG C	C				
		1,824		Roofs, HSC						
		8,495	94 \	Weighted Average						
		1,341	1	5.79% Per	vious Area	a				
		7,154	8	34.21% Imp	pervious Ar	ırea				
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)							
-		(ICCI)	(10/10)	(10,000)	(013)					
	6.0					Direct Entry				

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 239

Summary for Subcatchment C30: CB #30

Runoff = 1.20 cfs @ 12.09 hrs, Volume= 4,154 cf, Depth> 5.58"

Routed to Pond CB30: CB #30

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description						
	1,572	74	>75% Grass cover, Good, HSG C						
	6,310	98	Paved park	ing, HSG C	C				
	1,051	98	Roofs, HSC	G C					
	8,933	94	Weighted Average						
	1,572		17.60% Pei	rvious Area	a				
	7,361		82.40% Imp	pervious Ar	rea				
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	•				
6.0	• •	, ,	, ,	,	Direct Entry,				

Summary for Subcatchment C31: CB #31

Runoff = 2.10 cfs @ 12.09 hrs, Volume= 6,989 cf, Depth> 5.13"

Routed to Pond CB31: CB #31

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	Area (sf)	CN	Description						
	5,132	74	>75% Grass cover, Good, HSG C						
	9,132	98	Paved park	ing, HSG C	C				
	2,101	98	Roofs, HSC	S Č					
	16,365	90	Weighted Average						
	5,132		31.36% Per	rvious Area	a				
	11,233		68.64% lmp	pervious Ar	rea				
_									
Тс	Length	Slope	,	Capacity	·				
<u>(min)</u>	(feet)	(ft/ft	(ft/sec) (cfs)						
6.0					Direct Entry,				

Summary for Subcatchment C32: CB #32

Runoff = 1.65 cfs @ 12.09 hrs, Volume= 5,548 cf, Depth> 5.24"

Routed to Pond CB32 : CB #32

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 240

Area	a (sf)	CN	Description						
3	3,753	74	>75% Gras	s cover, Go	Good, HSG C				
7	7,068	98	Paved park	ing, HSG C	C				
1	1,889	98	Roofs, HSG	G C					
12	2,710	91	Weighted Average						
3	3,753		29.53% Per	vious Area	a				
8	3,957		70.47% Imp	ervious Ar	ırea				
Tc L	enath	Slone	. Volocity	Canacity	Description				
(min)	ength	Slope (ft/ft	,	Capacity (cfs)	•				
	(feet)	(11/11) (II/Sec)	(CIS)					
6.0					Direct Entry,				

Summary for Subcatchment C33: CB #33

Runoff = 0.73 cfs @ 12.09 hrs, Volume= 2,521 cf, Depth> 5.58"

Routed to Pond CB33: CB #33

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Aı	rea (sf)	CN	Description						
	873	74	>75% Gras	s cover, Go	Good, HSG C				
	3,693	98	Paved park	ing, HSG C	C				
	855	98	Roofs, HSC	S Č					
	5,421	94	Weighted A	Weighted Average					
	873		16.10% Per	rvious Area	a				
	4,548		83.90% Imp	pervious Ar	rea				
Тс	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft							
6.0	•				Direct Entry,				

Summary for Subcatchment C34: CB #34

Runoff = 1.15 cfs @ 12.09 hrs, Volume= 3,927 cf, Depth> 5.47"

Routed to Pond CB34 : CB #34

 Area (sf)	CN	Description			
 1,680	74	>75% Grass cover, Good, HSG C			
5,115	98	Paved parking, HSG C			
 1,827	98	Roofs, HSG C			
8,622	93	Weighted Average			
1,680		19.49% Pervious Area			
6,942		80.51% Impervious Area			

Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 241

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•
6.0					Direct Entry

Direct Entry,

Summary for Subcatchment C35: CB #35

0.57 cfs @ 12.09 hrs, Volume=

2,091 cf, Depth> 6.05"

Routed to Pond CB35: CB #35

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description					
	3,578	98	Paved park	ing, HSG C	C			
	79	80	>75% Gras	s cover, Go	Good, HSG D			
	492	98	Paved park	ing, HSG D	D			
	4,149	98	Weighted A	verage				
	79		1.90% Perv	rious Area				
	4,070		98.10% Imp	pervious Ar	rea			
Tc	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(ft/sec) (cfs)				
6.0					Direct Entry.			

Summary for Subcatchment C36: CB #36

0.92 cfs @ 12.09 hrs, Volume=

3,337 cf, Depth> 6.05"

Routed to Pond CB36: CB #36

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	Area (sf)	CN I	Description						
	6,622	98 I	98 Paved parking, HSG C						
	6,622		100.00% Impervious Area						
Т	c Length	Slope	Velocity	Capacity	Description				
(mir	ı) (feet)	(ft/ft)	(ft/sec)	(cfs)					
6.	0				Direct Entry,				

Summary for Subcatchment C38: CB #38

1.06 cfs @ 12.09 hrs, Volume= 3,849 cf, Depth> 6.05" Runoff

Routed to Pond CB38: CB #38

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 242

	Α	rea (sf)	CN	Description					
		6,647	98	Paved park	ing, HSG B	3			
		392	98	Paved park	ing, HSG C				
		598	98	Paved park	ing, HSG D)			
		7,637	98	Weighted Average					
		7,637		100.00% Impervious Area					
	Тс	Length	Slope	e Velocity	Capacity	Description			
(1	min)	(feet)	(ft/ft	,	(cfs)	Description			
	6.0	(1001)	(1010	, (1900)	(010)	Direct Entry			

Summary for Subcatchment C39: CB #39

Runoff 1.05 cfs @ 12.09 hrs, Volume= 3,836 cf, Depth> 6.05"

Routed to Pond CB39: CB #39

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description					
	6,505	98	Paved park	ing, HSG B	В			
	519	98	Paved park	ing, HSG C	${\tt C}$			
	588	98	Paved park	ing, HSG D	0			
	7,612	98	Weighted Average					
	7,612		100.00% Im	100.00% Impervious Area				
Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment C40: CB #40

Runoff 0.58 cfs @ 12.09 hrs, Volume= 2,122 cf, Depth> 6.05"

Routed to Pond CB40: CB #40

A	rea (sf)	CN Description								
	4,211	98 F	98 Paved parking, HSG B							
	4,211	1	100.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	·					
6.0					Direct Entry,					

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 243

Summary for Subcatchment C41: CB #41

Runoff = 0.77 cfs @ 12.09 hrs, Volume=

2,815 cf, Depth> 6.05"

Routed to Pond CB41: CB #41

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN E	Description						
	5,586	98 F	Paved parking, HSG B						
	5,586	1	100.00% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment C43: CB #43

Runoff = 0.39 cfs @ 12.09 hrs, Volume= 1,299 cf, Depth> 5.01"

Routed to Pond CB43: CB #43

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description						
	2,343	98	Paved parking, HSG B						
	766	61	>75% Grass cover, Good, HSG B						
	3,109	89	Weighted Average						
	766		24.64% Pei	rvious Area	a				
	2,343		75.36% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	·				
6.0					Direct Entry,				

Summary for Subcatchment C44: CB #44

Runoff = 0.26 cfs @ 12.09 hrs, Volume= 882 cf, Depth> 5.35"

Routed to Pond CB44: CB #44

Area (sf)	CN	Description
1,670	98	Paved parking, HSG B
308	61	>75% Grass cover, Good, HSG B
1,978	92	Weighted Average
308		15.57% Pervious Area
1,670		84.43% Impervious Area

Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 244

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment C45: CB #45

Runoff = 0.26 cfs @ 12.09 hrs, Volume= 830 cf, Depth> 4.04"

Routed to Pond CB45: CB #45

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description							
	1,240	98	Paved parking, HSG B							
	1,225	61	>75% Gras	s cover, Go	lood, HSG B					
	2,465	80	Weighted Average							
	1,225		49.70% Pei	rvious Area	a					
	1,240	:	50.30% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	·					
6.0	(ICCI)	(11/11)	(10/360)	(013)	Direct Entry,					
0.0					Direct Entry,					

Summary for Subcatchment C46: CB #46

Runoff = 0.47 cfs @ 12.09 hrs, Volume= 1,481 cf, Depth> 4.04"

Routed to Pond CB46: CB #46

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

rea (sf)	CN I	Description					
2,241	98 I	Paved parking, HSG B					
2,156	61 :	>75% Gras	s cover, Go	ood, HSG B			
4,397	80 '	Neighted A	verage				
2,156	4	19.03% Per	vious Area	a			
2,241		50.97% Impervious Area					
_		,		Description			
(feet)	(ft/ft)	(ft/sec)	(cfs)				
				Direct Entry,			
	2,241 2,156 4,397 2,156	2,241 98 F 2,156 61 2 4,397 80 V 2,156 2 2,241 8	2,241 98 Paved park 2,156 61 >75% Gras 4,397 80 Weighted A 2,156 49.03% Per 2,241 50.97% Imp	2,241 98 Paved parking, HSG I 2,156 61 >75% Grass cover, G 4,397 80 Weighted Average 2,156 49.03% Pervious Area 2,241 50.97% Impervious A Length Slope Velocity Capacity			

Summary for Subcatchment C47: CB #47

Runoff = 0.42 cfs @ 12.09 hrs, Volume= 1,518 cf, Depth> 6.05"

Routed to Pond CB47: CB#47

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 245

	Area (sf)	CN E	Description					
	3,012	98 F	8 Paved roads w/curbs & sewers, HSG B					
·	3,012	1	100.00% Impervious Area					
To (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment C48: CB #48

Runoff 4.02 cfs @ 12.17 hrs, Volume= 15,183 cf, Depth> 3.03"

Routed to Pond CB48: CB#48

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	Α	rea (sf)	CN D	escription		
		3,877				& sewers, HSG B
		56,251	68 1	acre lots,	20% imp, ł	HSG B
		60,128	70 V	Veighted A	verage	
		45,001	7	4.84% Per	vious Area	
		15,127	2	5.16% Imp	ervious Ar	ea
	Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.0	50	0.0800	0.12		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.27"
	4.8	350	0.0600	1.22		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	11.8	400	Total			

Summary for Subcatchment C49: CB #49

0.70 cfs @ 12.09 hrs, Volume= 2,436 cf, Depth> 5.58" Runoff

Routed to Pond CB49: CB #49

 Area (sf)	CN	Description				
4,431	98	Paved roads w/curbs & sewers, HSG C				
 807	74	75% Grass cover, Good, HSG C				
5 000		Weighted Average				
5,238	94	Weighted Average				
5,238 807	94	Weighted Average 15.41% Pervious Area				

Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 246

Тс	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry	,

Summary for Subcatchment C50: CB #50

Runoff = 2.00 cfs @ 12.09 hrs, Volume= 6,850 cf, Depth> 5.47"

Routed to Pond CB50 : CB #50

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Ar	rea (sf)	CN	Description						
	3,429	74	>75% Grass cover, Good, HSG C						
	11,611	98	Paved road	s w/curbs &	& sewers, HSG C				
	15,040	93	Weighted A	verage					
	3,429		22.80% Pei	vious Area	1				
	11,611	•	77.20% lmp	pervious Ar	rea				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)							
6.0	,	, ,	Direct Entry,						

Summary for Subcatchment C51: CB #51

Runoff = 0.94 cfs @ 12.09 hrs, Volume= 3,439 cf, Depth> 6.05"

Routed to Pond CB51: CB #51

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	Α	rea (sf)	CN	Description					
		3,147	98	Roofs, HSG C					
		3,676	98	Paved park	ing, HSG C				
		6,823	98	Weighted A	verage				
		6,823		100.00% Im	npervious A	Area			
	Tc	Length	Slop	e Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft	(ft/sec)	(cfs)				
	6.0					Direct Entry,			

Summary for Subcatchment C52: CB#52

Runoff = 1.23 cfs @ 12.09 hrs, Volume= 4,297 cf, Depth> 5.70"

Routed to Pond CB52 : CB #52

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 247

Α	rea (sf)	CN	Description						
	1,164	74	>75% Grass cover, Good, HSG C						
	7,888	98	Paved park	ing, HSG C	C				
	9,052	95	Weighted A	verage					
	1,164		12.86% Pei	rvious Area	a				
	7,888		87.14% lm <mark>բ</mark>	pervious Ar	ırea				
_									
Tc	Length	Slope	e Velocity Capacity Description						
(min)	(feet)	(ft/ft)	(cfs) (ft/sec) (cfs)						
6.0	•	_	Direct Entry						

.0 Direct Entry,

Summary for Subcatchment C53: CB #53

Runoff = 1.05 cfs @ 12.09 hrs, Volume= 3,581 cf, Depth> 5.47"

Routed to Pond CB53: CB #53

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description							
	287	39	>75% Gras	s cover, Go	Good, HSG A					
	3,287	98	Paved park	ing, HSG A	A					
	773	74	>75% Gras	s cover, Go	Good, HSG C					
	3,516	98	Paved park	ing, HSG C	C					
	7,863	93	Weighted A	verage						
	1,060		13.48% Pe	rvious Area	a					
	6,803		86.52% lmp	pervious Ar	ırea					
Tc	Length	Slope	,	Capacity	·					
(min)	(feet)	(ft/ft)	(ft/sec) (cfs)							
6.0					Direct Entry,					

Summary for Subcatchment C54: CB #54

Runoff = 0.63 cfs @ 12.09 hrs, Volume= 2,104 cf, Depth> 5.24"

Routed to Pond CB54 : CB #54

_	Area (sf)	CN	Description						
	550	39	>75% Grass cover, Good, HSG A						
	4,176	98	Paved parking, HSG A						
	84	74	>75% Grass cover, Good, HSG C						
_	11	98	Paved parking, HSG C						
	4,821	91	Weighted Average						
	634		13.15% Pervious Area						
	4,187		86.85% Impervious Area						

Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 248

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•	
6.0					Direct Entry,	

Summary for Subcatchment C7: CB #5

Runoff = 0.64 cfs @ 12.09 hrs, Volume= 2

2,344 cf, Depth> 6.05"

Routed to Pond CB7: CB#5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN [CN Description							
	4,650	98 F	98 Paved roads w/curbs & sewers, HSG B							
	4,650	1	100.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)								
6.0			Direct Entry,							

Summary for Subcatchment C8: CB #8

Runoff = 0.73 cfs @ 12.09 hrs, Volume= 2,534 cf, Depth> 5.58"

Routed to Pond CB8: CB#8

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	Α	rea (sf)	CN	Description						
		4,837	98	Paved road	s w/curbs &	& sewers, HSG B				
		613	61	>75% Gras	s cover, Go	Good, HSG B				
		5,450	94	Weighted Average						
		613		11.25% Pe	rvious Area	a				
		4,837		88.75% lm	pervious Ar	rea				
	_		01			D				
	Tc	Length	Slope	,	Capacity	·				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry.				

Summary for Subcatchment C9: CB #9

Runoff = 2.24 cfs @ 12.09 hrs, Volume= 8,058 cf, Depth> 5.93"

Routed to Pond CB9: CB #9

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 249

A	rea (sf)	CN	Description	Description					
	31	61	>75% Gras	s cover, Go	ood, HSG B				
	433	98	Paved road	s w/curbs &	& sewers, HSG B				
	904	74	>75% Gras	s cover, Go	ood, HSG C				
	12,073	98	Paved park	ing, HSG C					
	2,305	98	Roofs, HSC	S C					
	52	80	>75% Gras	s cover, Go	ood, HSG D				
	509	98	Paved park	ing, HSG D					
	16,307	97	Weighted A	verage					
	987		6.05% Perv	rious Area					
	15,320		93.95% Imp	pervious Ar	rea				
Tc	Length	Slop	e Velocity	Capacity	Description				
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment CH1: CLUBHOUSE

Runoff = 0.86 cfs @ 12.09 hrs, Volume= 3,033 cf, Depth> 5.81"

Routed to Pond DECH: DRIP #CH

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description		
	5,802	98	Roofs, HSC	G C	
	3	98	Roofs, HSG	G D	
	435	74	>75% Gras	s cover, Go	lood, HSG C
	22	80	>75% Gras	s cover, Go	lood, HSG D
	6,262	96	Weighted A	verage	
	457		7.30% Perv	ious Area	
	5,805		92.70% Imp	ervious Ar	rea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

•

Summary for Subcatchment MB1: MAIL KIOSK

Runoff = 0.13 cfs @ 12.09 hrs, Volume= 473 cf, Depth> 6.05" Routed to Link AP2 : ANALYSIS POINT 2

 Area (sf)	CN	Description
938	98	Roofs, HSG B
938		100.00% Impervious Area

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 250

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment S201: SUMMER STREET ACCESS APRON

Runoff = 1.35 cfs @ 12.09 hrs, Volume=

4,720 cf, Depth> 5.70"

Routed to Link AP1: ANALYSIS POINT 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description							
	717	61	>75% Grass cover, Good, HSG B							
	9,226	98	Paved parking, HSG B							
	9,943	95	Weighted Average							
	717		7.21% Pervious Area							
	9,226	!	92.79% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	·					
6.0					Direct Entry,					

Summary for Subcatchment S202: EXISTING WETLAND

Runoff = 26.72 cfs @ 12.32 hrs, Volume=

130,245 cf, Depth> 3.62"

Routed to Reach SC1: Stream Crossing #1

Area (sf)	CN	Description
136,496	61	>75% Grass cover, Good, HSG B
83,935	55	Woods, Good, HSG B
29	98	Paved parking, HSG B
13,946	98	Roofs, HSG B
9,038	48	Brush, Good, HSG B
2,573	74	>75% Grass cover, Good, HSG C
17,121	70	Woods, Good, HSG C
98	98	Paved parking, HSG C
1,097	65	Brush, Good, HSG C
126	80	>75% Grass cover, Good, HSG D
132	98	Paved parking, HSG D
167,678	98	Water Surface, HSG D
432,269	76	Weighted Average
250,386		57.92% Pervious Area
181,883		42.08% Impervious Area

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 251

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
Ī	5.6	50	0.0200	0.15		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.27"
	1.4	118	0.0400	1.40		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	16.2	688	0.0200	0.71		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	23.2	856	Total			

Summary for Subcatchment S203: POCKET WETLAND #1

Runoff = 2.18 cfs @ 12.09 hrs, Volume=

6,885 cf, Depth> 3.23"

Routed to Pond p210: POCKET WETLAND #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Area (sf)	CN	Description						
12,682	61	>75% Grass cover, Good, HSG B						
1,060	98	Water Surface, 0% imp, HSG B						
7,785	74	>75% Grass cover, Good, HSG C						
4,060	98	Water Surface, 0% imp, HSG C	Water Surface, 0% imp, HSG C					
25,587	72	Weighted Average						
25,587		100.00% Pervious Area						
Tc Length								
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)	_					
6.0		Direct Entry,						

Summary for Subcatchment S204: EXISTING WETLANDS

Runoff = 18.73 cfs @ 12.31 hrs, Volume= 9

90,283 cf, Depth> 3.52"

Routed to Link ap2: ANALYSIS POINT 2

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 252

A	rea (sf)	CN E	CN Description						
	53,739	61 >	61 >75% Grass cover, Good, HSG B						
	17,975	55 V	Voods, Go	od, HSG B					
	20,940	48 E	Brush, Goo	d, HSG B					
	41,421	74 >	75% Gras	s cover, Go	ood, HSG C				
	68,342		,	od, HSG C					
	116	98 F	aved park	ing, HSG C					
	1,904		Brush, Goo	,					
	1,528		Brush, Goo	,					
	2,508		,	od, HSG D					
	161			ing, HSG D)				
	4,073		Brush, Goo	•					
	95,496	98 V	Vater Surfa	ace, HSG D)				
3	808,203	75 V	Veighted A	verage					
2	212,430	_		vious Area					
	95,773	3	1.07% lmp	pervious Ar	ea				
Tc	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
3.2	50	0.2000	0.26		Sheet Flow,				
					Grass: Dense n= 0.240 P2= 3.27"				
19.4	582	0.0100	0.50		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
22.6	632	Total							

Summary for Subcatchment S205: ISOLATED WETLAND

noff = 4.43 cfs @ 12.09 hrs, Volume= Routed to Link AP3 : ANALYSIS POINT 3 Runoff

14,013 cf, Depth> 3.03"

Area (sf)	CN	Description					
10,910	30	Woods, Good, HSG A					
3,684	74	>75% Grass cover, Good, HSG C					
2,275	70	Woods, Good, HSG C					
171	98	Paved parking, HSG C					
1,706	65	Brush, Good, HSG C					
1,940	80	>75% Grass cover, Good, HSG D					
23,513	77	Woods, Good, HSG D					
393	98	Paved parking, HSG D					
2,208	73	Brush, Good, HSG D					
8,620	98	Water Surface, HSG D					
55,420	70	Weighted Average					
46,236		83.43% Pervious Area					
9,184		16.57% Impervious Area					

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 253

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0	•	•			Direct Entry,	

Summary for Subcatchment S206: OVERLAND FLOW

Runoff = 31.85 cfs @ 12.50 hrs, Volume=

188,907 cf, Depth> 2.54"

Routed to Link AP4: ANALYSIS POINT #4

A	rea (sf)	CN E	Description							
	16,514	39 >	9 >75% Grass cover, Good, HSG A							
	18,226		Woods, Good, HSG A							
	713		,	ing, HSG A						
	41,148		Brush, Goo							
	17,568			20% imp, I	HSG A					
	37,410				ood, HSG B					
	13,900	55 V	Voods, Go	od, HSG B						
	54,538	48 E	Brush, Goo	d, HSG B						
	91,202	68 1	acre lots,	20% imp, I	HSG B					
	77,444			,	ood, HSG C					
1	14,763		,	od, HSG C						
	3,493			ing, HSG C						
	57,740		Brush, Goo							
	5,763				ood, HSG D					
	26,141			od, HSG D						
	14,732			ace, 0% imp	p, HSG D					
	91,295		Veighted A							
	65,335	_		rvious Area						
	25,960	2	91% impe	ervious Are	a					
Tc	Length	Slope		Capacity	Description					
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)						
8.5	50	0.0500	0.10		Sheet Flow,					
					Woods: Light underbrush n= 0.400 P2= 3.27"					
5.0	334	0.0250	1.11		Shallow Concentrated Flow,					
					Short Grass Pasture Kv= 7.0 fps					
1.9	91	0.0250	0.79		Shallow Concentrated Flow,					
40.0	404	0.0400			Woodland Kv= 5.0 fps					
10.2	491	0.0400	0.80		Shallow Concentrated Flow, BRUSH					
8.9	501	0.0350	0.94		Kv= 4.0 fps Shallow Concentrated Flow,					
0.9	301	0.0330	0.54		Woodland Kv= 5.0 fps					
34.5	1 167	Total			vvoodiana itv- 0.0 ips					
34.3	1,467	Total								

Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 254

Summary for Subcatchment S207: INFILTRATION POND #2

Runoff = 2.59 cfs @ 12.09 hrs, Volume=

8,498 cf, Depth> 4.90"

Routed to Pond P207: INFILTRATION POND #2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Area	(sf) CN	Description	Description						
3	339 98	Water Surfa	ace, 0% im	o, HSG A					
8,8	302 74	>75% Gras	>75% Grass cover, Good, HSG C						
11,1	162 98	Water Surfa	Water Surface, 0% imp, HSG C						
20,8	303 88	88 Weighted Average							
20,8	303	100.00% Pervious Area							
	ngth Slo	,	Capacity	Description					
(min) (1	eet) (ft/	ft) (ft/sec)	(cfs)						
6.0				Direct Entry.					

Summary for Subcatchment S208: GRASS AREA

Runoff = 1.28 cfs @ 12.09 hrs, Volume=

4,045 cf, Depth> 3.53"

Routed to Pond OCS4: OCS#4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Area	(sf) CN	l De	Description							
4	176 39	>7	5% Grass	s cover, Go	ood, HSG A					
12,0	000 74	>7	5% Grass	s cover, Go	lood, HSG C					
•	168 98	B Pa	ved parki	ng, HSG A	A					
1,1	116 98	B Pa	ved parki	ng, HSG C	C					
13,7	760 75	. We	Weighted Average							
12,4	176	90	.67% Per	vious Area	a					
1,2	284	9.3	33% Impe	rvious Area	ea					
Tc Le	ngth Sl	ope	Velocity	Capacity	Description					
<u>(min)</u> (1	eet) (f	ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment S209: WETLAND C

Runoff = 5.50 cfs @ 12.41 hrs, Volume=

29,544 cf, Depth> 3.31"

Routed to Reach 11R: 4x4 Open Bottom Culvert

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 255

A	rea (sf)	CN	Description					
	10,826	39	>75% Gras	s cover, Go	ood, HSG A			
	16,826	30	Woods, Go	od, HSG A				
	8,863	74	>75% Gras	s cover, Go	ood, HSG C			
	26,084	70	Woods, Go	od, HSG C				
	44,067	98	Water Surfa	ace, 0% im	p, HSG D			
	304	98	Paved park	ing, HSG A				
	103	98	Paved park	ing, HSG C				
•	107,073	73	Weighted A	verage				
•	106,666		99.62% Pervious Area					
407 0.38% Impervious Area			0.38% Impe	ervious Are	a			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
13.7	50	0.0150	0.06		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.27"			
15.2	557	0.0150	0.61		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
28.9	607	Total						

Summary for Subcatchment S210: INFILTRATION POND #1

Runoff 5.67 cfs @ 12.23 hrs, Volume=

Routed to Pond P212: INFILTRATION POND #1

24,190 cf, Depth> 3.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	Α	rea (sf)	CN [CN Description								
		13,844	98 V	98 Water Surface, 0% imp, HSG C								
		59,814	74 >	75% Gras	s cover, Go	ood, HSG C						
		2,232	65 E	Brush, Goo	d, HSG C							
		75,890	78 V	Veighted A	verage							
		75,890	1	100.00% Pervious Area								
	Тс	Length	Slope	Velocity	Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	6.2	50	0.0150	0.13		Sheet Flow,						
						Grass: Short n= 0.150 P2= 3.27"						
	10.3	530	0.0150	0.86		Shallow Concentrated Flow,						
						Short Grass Pasture Kv= 7.0 fps						
_	16.5	580	Total									

Summary for Subcatchment S211: S211

1.60 cfs @ 12.09 hrs, Volume= Runoff

5,064 cf, Depth> 3.94"

Routed to Pond P205: INFILTRATION POND #3

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 256

A	rea (sf)	CN	Description					
	8,108	61	>75% Gras	s cover, Go	ood, HSG B			
	7,328	98	Water Surfa	ace, HSG B	В			
	15,436	79	Weighted Average					
	8,108		52.53% Pervious Area					
	7,328		47.47% Impervious Area					
_		0.1						
Тс	Length	Slope	,	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment S212: SWALE

Runoff = 2.25 cfs @ 12.34 hrs, Volume= 11,217 cf, Depth> 2.55"

Routed to Reach SC2 : Stream Crossing #2

	Α	rea (sf)	CN I	Description				
		4,100	61 :	61 >75% Grass cover, Good, HSG B				
		7,192	55 \	Noods, Go	od, HSG B			
		1,180	74	>75% Gras	s cover, Go	ood, HSG C		
		3,436	70 \	Woods, Go	od, HSG C			
		13,180	98 \	Nater Surfa	ace, 0% im _l	p, HSG D		
		72	98 F	Paved park	ing, HSG E	3		
		22,663	48 I	Brush, Goo	d, HSG B			
		545		Brush, Goo	•			
		107		•	ing, HSG C			
		135			ing, HSG D			
_		158	80 >	>75% Gras	s cover, Go	ood, HSG D		
	52,768 65 Weighted Average							
	52,454 99.40% Pervious Area							
		314	(0.60% Impe	ervious Are	a		
	_				_			
	Tc	Length	Slope	•		Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	16.1	50	0.0400	0.05		Sheet Flow,		
						Woods: Dense underbrush n= 0.800 P2= 3.27"		
	3.9	232	0.0600	0.98		Shallow Concentrated Flow, BRUSH		
						Kv= 4.0 fps		
	3.1	136	0.0220	0.74		Shallow Concentrated Flow,		
_						Woodland Kv= 5.0 fps		
	23.1	418	Total					

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 257

Summary for Subcatchment S213: COURTYARD

2.21 cfs @ 12.09 hrs, Volume= 7,022 cf, Depth> 3.94" Runoff

Routed to Pond 11P: YARD DRAIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Ar	rea (sf)	CN	Description						
	5,047	39	>75% Grass cover, Good, HSG A						
	1,678	98	Paved parking, HSG A						
	168	98	Roofs, HSG A						
	532	98	Water Surface, 0% imp, HSG A						
	4,518	74	>75% Grass cover, Good, HSG C						
	7,080	98	Paved parking, HSG C						
	878	98	Roofs, HSG C						
	718	98	Water Surface, 0% imp, HSG C						
	296	80	>75% Grass cover, Good, HSG D						
	492	98	Paved parking, HSG D						
	21,407	79	Weighted Average						
	11,111 51.90% Pervious Area								
	10,296 48.10% Impervious Area								
_		0.	VI. "						
Tc	Length	Slop							
(min)	(feet)	(ft/f							
6.0			Direct Entry,						

Summary for Subcatchment T1: Trench Drain 1

1.49 cfs @ 12.09 hrs, Volume= 5,089 cf, Depth> 5.47" Runoff

Routed to Pond 5R: TRENCH DRAIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description						
	1,443	74	>75% Grass cover, Good, HSG C						
	4,228	98	Paved park	ing, HSG C	C				
	1,339	80	>75% Gras	s cover, Go	Good, HSG D				
	4,163	98	Paved park	ing, HSG D	D				
	11,173	93	Weighted Average						
	2,782		24.90% Pervious Area						
	8,391		75.10% Impervious Area						
Tc	Length	Slope	,	Capacity	•				
<u>(min)</u>	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 258

Summary for Subcatchment T2: Drive Under B2

Runoff = 0.45 cfs @ 12.09 hrs, Volume= 1

1,420 cf, Depth> 3.83"

Routed to Reach 11R: 4x4 Open Bottom Culvert

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description						
	1,510	39	>75% Grass	cover, Go	Good, HSG A				
	2,313	98	Paved parking, HSG A						
	77	74	>75% Grass	cover, Go	Good, HSG C				
	545	98	Paved parki	ng, HSG C	C				
•	4,445	78	Weighted Average						
	1,587		35.70% Pervious Area						
	2,858		64.30% Impervious Area						
Тс	Length	Slop	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment TH1: TOWN HOUSE #1

Runoff = 0.58 cfs @ 12.09 hrs, Volume=

2,057 cf, Depth> 5.81"

Routed to Pond DE61: DRIP #61

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description						
	3,936	98	Roofs, HSG C						
	311	74	>75% Grass cover, Good, HSG C						
	4,247	96	Weighted Average						
	311		7.32% Pervious Area						
	3,936		92.68% Impervious Area						
Tc	Length	Slope	,	Capacity	·				
<u>(min)</u>	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment TH10: TOWN HOUSE #10

Runoff = 0.48 cfs @ 12.09 hrs, Volume= 1,684 cf, Depth> 5.81" Routed to Pond DE70 : DRIP #70

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 259

Α	rea (sf)	CN	Description							
	3,184	98	Roofs, HSG C							
	292	74	>75% Grass cover, Good, HSG C							
	3,476	96	Weighted Average							
	292		8.40% Pervious Area							
	3,184	!	91.60% Impervious Area							
_		٠.								
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry					

Direct Entry,

Summary for Subcatchment TH11: TOWN HOUSE #11

Runoff = 0.58 cfs @ 12.09 hrs, Volume= 2,039 cf, Depth> 5.81"

Routed to Pond DE71: DRIP #71

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

А	rea (sf)	CN	Description					
•	3,899	98	Roofs, HSG C					
	311	74	>75% Gras	s cover, Go	Good, HSG C			
	4,210 311 3,899		Weighted Average 7.39% Pervious Area 92.61% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	•			
6.0					Direct Entry,			

Summary for Subcatchment TH2: TOWN HOUSE #2

Runoff = 0.58 cfs @ 12.09 hrs, Volume= 2,057 cf, Depth> 5.81"

Routed to Pond DE62 : DRIP #62

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description						
	3,936	98	Roofs, HSG C						
	311	74	>75% Grass cover, Good, HSG C						
	4,247	96	Weighted Average						
	311		7.32% Pervious Area						
	3,936		92.68% Impervious Area						
Tc	Length	Slope	,	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 260

Summary for Subcatchment TH3: TOWN HOUSE #3

Runoff = 0.41 cfs @ 12.09 hrs, Volume= 1,430 cf, Depth> 5.70"

Routed to Pond DE63: DRIP #63

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description						
	2,672	98	Roofs, HSG C						
	341	74	>75% Grass cover, Good, HSG C						
	3,013	95	Weighted Average						
	341		11.32% Pervious Area						
	2,672		88.68% Impervious Area						
-		01	\	0 ''	5				
Tc	Length	Slope	,	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry.				

Summary for Subcatchment TH4: TOWN HOUSE #4

Runoff = 0.47 cfs @ 12.09 hrs, Volume= 1,681 cf, Depth> 5.81"

Routed to Pond DE64: DRIP #64

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description							
	3,178	98	Roofs, HSG C							
	292	74	>75% Grass cover, Good, HSG C							
	3,470	96	Weighted Average							
	292		8.41% Pervious Area							
	3,178		91.59% Impervious Area							
_										
Tc	Length	Slope	,	Capacity	·					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment TH5: TOWN HOUSE #5

Runoff = 0.41 cfs @ 12.09 hrs, Volume= 1,432 cf, Depth> 5.70"

Routed to Pond DE65: DRIP #65

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 261

A	rea (sf)	CN	Description							
	2,675	98	Roofs, HSG C							
	341	74	>75% Grass cover, Good, HSG C							
	3,016	95	Weighted Average							
	341		11.31% Pervious Area							
	2,675		88.69% Impervious Area							
_				_						
Tc	J	Slope	,	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry					

Summary for Subcatchment TH6: TOWN HOUSE #6

Runoff = 0.47 cfs @ 12.09 hrs, Volume=

1,650 cf, Depth> 5.81"

Routed to Pond DE66: DRIP #66

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Aı	rea (sf)	CN	Description						
•	3,116	98	Roofs, HSG C						
	291	74	>75% Gras	>75% Grass cover, Good, HSG C					
	3,407 291 3,116		Weighted Average 8.54% Pervious Area 91.46% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	•				
6.0					Direct Entry,				

Summary for Subcatchment TH7: TOWN HOUSE #7

Runoff = 0.48 cfs @ 12.09 hrs, Volume= 1,686 cf, Depth> 5.81"

Routed to Pond DE67 : DRIP #67

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

Are	ea (sf)	CN	Description					
	3,189	98	Roofs, HSG	G C				
	292	74	>75% Grass cover, Good, HSG C					
	3,481 292 3,189		Weighted A 8.39% Perv 91.61% Imp	ious Area	ea			
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description			

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Page 262

Summary for Subcatchment TH8: TOWN HOUSE #8

Runoff = 0.58 cfs @ 12.09 hrs, Volume= 2,040 cf, Depth> 5.81"

Routed to Pond DE68: DRIP #68

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

	Α	rea (sf)	CN	Description						
		3,901	98	Roofs, HSG C						
_		311	74	>75% Grass cover, Good, HSG C						
		4,212	96	Weighted Average						
		311		7.38% Pervious Area						
		3,901		92.62% Impervious Area						
	т.	ما المحمد ا	Clana	Valacity	Consoitu	Decemention				
	Tc	Length	Slope	,	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	6.0					Direct Entry,				

Summary for Subcatchment TH9: TOWN HOUSE #9

Runoff = 0.48 cfs @ 12.09 hrs, Volume= 1,686 cf, Depth> 5.81"

Routed to Pond DE69: DRIP #69

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

A	rea (sf)	CN	Description							
	3,188	98	Roofs, HSG C							
	292	74	>75% Grass cover, Good, HSG C							
	3,480	96	Weighted Average							
	292		8.39% Pervious Area							
	3,188	!	91.61% Impervious Area							
_		01		0 "						
Tc	Length	Slope	,	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Reach 8R: OVERLAND FLOW

Inflow Area = 11,975 sf, 92.37% Impervious, Inflow Depth > 4.86" for 25YR event

Inflow = 1.38 cfs @ 12.14 hrs, Volume= 4,854 cf

Outflow = 0.40 cfs @ 12.54 hrs, Volume= 4,526 cf, Atten= 71%, Lag= 23.9 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.10 fps, Min. Travel Time= 94.0 min

Avg. Velocity = 0.05 fps, Avg. Travel Time= 197.1 min

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 263

Peak Storage= 2,258 cf @ 12.54 hrs Average Depth at Peak Storage= 0.08', Surface Width= 50.80' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 28.09 cfs

50.00' x 1.00' deep channel, n= 0.400 Side Slope Z-value= 5.0 '/' Top Width= 60.00' Length= 563.0' Slope= 0.0213 '/' Inlet Invert= 208.00', Outlet Invert= 196.00'



Summary for Reach 9R: OVERLAND FLOW

Inflow Area = 32,665 sf, 94.81% Impervious, Inflow Depth = 0.00" for 25YR event

Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs

Average Depth at Peak Storage= 0.00'

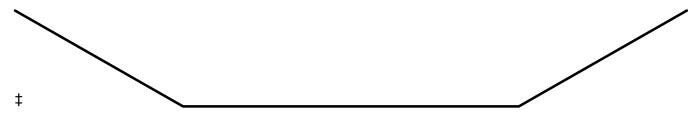
Bank-Full Depth= 1.00' Flow Area= 30.0 sf, Capacity= 23.45 cfs

20.00' x 1.00' deep channel, n= 0.400

Side Slope Z-value= 10.0 '/' Top Width= 40.00'

Length= 211.0' Slope= 0.0652 '/'

Inlet Invert= 201.75', Outlet Invert= 188.00'



Summary for Reach 10R: OVERLAND FLOW

Inflow Area = 129,716 sf, 63.13% Impervious, Inflow Depth = 1.18" for 25YR event

Inflow = 1.47 cfs @ 12.61 hrs, Volume= 12,790 cf

Outflow = 1.45 cfs @ 12.81 hrs, Volume= 12,790 cf, Atten= 2%, Lag= 12.4 min

Routed to Link AP4: ANALYSIS POINT #4

Type III 24-hr 25YR Rainfall=6.29"

19097 Post-Development

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 264

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.26 fps, Min. Travel Time= 10.5 min Avg. Velocity = 0.10 fps, Avg. Travel Time= 28.4 min

Peak Storage= 910 cf @ 12.81 hrs

Average Depth at Peak Storage= 0.25', Surface Width= 24.94'

Bank-Full Depth= 1.00' Flow Area= 30.0 sf, Capacity= 17.57 cfs

20.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 40.00'

Length= 164.0' Slope= 0.0366 '/'

Inlet Invert= 192.00', Outlet Invert= 186.00'



Summary for Reach 11R: 4x4 Open Bottom Culvert

[52] Hint: Inlet/Outlet conditions not evaluated

[62] Hint: Exceeded Reach 20R OUTLET depth by 0.11' @ 12.50 hrs

[61] Hint: Exceeded Reach R211 outlet invert by 0.31' @ 12.55 hrs

Inflow Area = 424,818 sf, 45.99% Impervious, Inflow Depth > 2.00" for 25YR event

Inflow = 9.42 cfs @ 12.53 hrs, Volume= 70,760 cf

Outflow = 9.42 cfs @ 12.53 hrs, Volume= 70,749 cf, Atten= 0%, Lag= 0.2 min

Routed to Reach 23R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 2.11 fps, Min. Travel Time= 0.2 min

Avg. Velocity = 0.82 fps, Avg. Travel Time= 0.6 min

Peak Storage= 134 cf @ 12.53 hrs

Average Depth at Peak Storage= 1.12', Surface Width= 4.00'

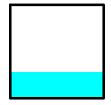
Bank-Full Depth= 4.00' Flow Area= 16.0 sf, Capacity= 42.20 cfs

48.0" W x 48.0" H Box Pipe

n= 0.069 Riprap, 6-inch

Length= 30.0' Slope= 0.0150 '/'

Inlet Invert= 194.00', Outlet Invert= 193.55'



Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 265

Summary for Reach 12R: OVERLAND FLOW

Inflow Area = 12,906 sf, 90.20% Impervious, Inflow Depth > 4.91" for 25YR event

Inflow = 1.58 cfs @ 12.13 hrs, Volume= 5,279 cf

Outflow = 0.85 cfs @ 12.30 hrs, Volume= 5,158 cf, Atten= 46%, Lag= 10.7 min

Routed to Link AP2: ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.14 fps, Min. Travel Time= 29.9 min

Avg. Velocity = 0.05 fps, Avg. Travel Time= 85.1 min

Peak Storage= 1,523 cf @ 12.30 hrs

Average Depth at Peak Storage= 0.12', Surface Width= 51.20' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 29.80 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'

Length= 250.0' Slope= 0.0240 '/'

Inlet Invert= 202.00', Outlet Invert= 196.00'



Summary for Reach 14R: OVERLAND FLOW

Inflow Area = 52,768 sf, 0.60% Impervious, Inflow Depth > 2.55" for 25YR event

Inflow = 2.25 cfs @ 12.34 hrs, Volume= 11,215 cf

Outflow = 0.65 cfs @ 12.97 hrs, Volume= 10,062 cf, Atten= 71%, Lag= 37.7 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.13 fps, Min. Travel Time= 113.6 min

Avg. Velocity = 0.07 fps, Avg. Travel Time= 199.3 min

Peak Storage= 4,397 cf @ 12.97 hrs

Average Depth at Peak Storage= 0.10', Surface Width= 52.02'

Bank-Full Depth= 1.00' Flow Area= 60.0 sf, Capacity= 31.55 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 70.00'

Length= 852.0' Slope= 0.0246 '/'

Inlet Invert= 207.00', Outlet Invert= 186.00'

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 266



Summary for Reach 15R: OVERLAND FLOW

Inflow Area = 62,582 sf, 52.00% Impervious, Inflow Depth > 1.86" for 25YR event

Inflow = 0.23 cfs @ 15.93 hrs, Volume= 9,701 cf

Outflow = 0.23 cfs @ 16.99 hrs, Volume= 8,871 cf, Atten= 0%, Lag= 63.2 min

Routed to Link AP2: ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.08 fps, Min. Travel Time= 63.7 min

Avg. Velocity = 0.07 fps, Avg. Travel Time= 70.7 min

Peak Storage= 872 cf @ 16.99 hrs

Average Depth at Peak Storage= 0.06', Surface Width= 50.58' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 27.21 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'

Length= 300.0' Slope= 0.0200 '/'

Inlet Invert= 202.00', Outlet Invert= 196.00'



Summary for Reach 18R: OVERLAND FLOW

Inflow Area = 88,676 sf, 39.42% Impervious, Inflow Depth = 1.01" for 25YR event

Inflow = 3.39 cfs @ 12.37 hrs, Volume= 7,468 cf

Outflow = 0.80 cfs @ 12.90 hrs, Volume= 7,269 cf, Atten= 76%, Lag= 31.8 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.14 fps, Min. Travel Time= 73.6 min

Avg. Velocity = 0.06 fps, Avg. Travel Time= 163.7 min

Peak Storage= 3,543 cf @ 12.90 hrs

Average Depth at Peak Storage= 0.11', Surface Width= 55.51'

Bank-Full Depth= 1.00' Flow Area= 75.0 sf, Capacity= 38.42 cfs

Type III 24-hr 25YR Rainfall=6.29"

19097 Post-Development

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 267

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 100.00'

Length= 609.0' Slope= 0.0279 '/'

Inlet Invert= 203.00', Outlet Invert= 186.00'



Summary for Reach 20R: OVERLAND FLOW

Inflow Area = 72,222 sf, 68.72% Impervious, Inflow Depth > 3.44" for 25YR event

Inflow = 4.31 cfs @ 12.27 hrs, Volume= 20,716 cf

Outflow = 1.57 cfs @ 13.10 hrs, Volume= 19,942 cf, Atten= 63%, Lag= 49.8 min

Routed to Reach 11R: 4x4 Open Bottom Culvert

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.13 fps, Min. Travel Time= 70.3 min

Avg. Velocity = 0.07 fps, Avg. Travel Time= 138.3 min

Peak Storage= 6,626 cf @ 13.10 hrs

Average Depth at Peak Storage= 0.23', Surface Width= 52.31' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 18.54 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'

Length= 560.0' Slope= 0.0093 '/'

Inlet Invert= 200.00', Outlet Invert= 194.80'



Summary for Reach 23R: OVERLAND FLOW

Inflow Area = 424,818 sf, 45.99% Impervious, Inflow Depth > 2.00" for 25YR event

Inflow = 9.42 cfs @ 12.53 hrs, Volume= 70,749 cf

Outflow = 8.56 cfs @ 12.72 hrs, Volume= 70,122 cf, Atten= 9%, Lag= 11.0 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.30 fps, Min. Travel Time= 13.2 min

Avg. Velocity = 0.11 fps, Avg. Travel Time= 35.1 min

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 268

Peak Storage= 6,778 cf @ 12.72 hrs Average Depth at Peak Storage= 0.48', Surface Width= 69.20' Bank-Full Depth= 1.00' Flow Area= 70.0 sf, Capacity= 31.93 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 20.0 '/' Top Width= 90.00'

Length= 237.0' Slope= 0.0211 '/'

Inlet Invert= 193.00', Outlet Invert= 188.00'



Summary for Reach R202: OVERLAND FLOW

[62] Hint: Exceeded Reach SC1 OUTLET depth by 0.28' @ 12.95 hrs

Inflow Area = 432,269 sf, 42.08% Impervious, Inflow Depth > 3.62" for 25YR event

Inflow = 26.73 cfs @ 12.32 hrs, Volume= 130,223 cf

Outflow = 13.59 cfs @ 12.69 hrs, Volume= 124,425 cf, Atten= 49%, Lag= 21.9 min

Routed to Link AP2: ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.23 fps, Min. Travel Time= 50.5 min Avg. Velocity = 0.10 fps, Avg. Travel Time= 122.8 min

Peak Storage= 41,142 cf @ 12.69 hrs

Average Depth at Peak Storage= 0.52', Surface Width= 126.01' Bank-Full Depth= 1.00' Flow Area= 125.0 sf, Capacity= 42.56 cfs

100.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 150.00'

Length= 700.0' Slope= 0.0107 '/'

Inlet Invert= 205.50', Outlet Invert= 198.00'



Summary for Reach R211: OVERLAND FLOW

Inflow Area = 241,078 sf, 59.10% Impervious, Inflow Depth = 1.00" for 25YR event

Inflow = 11.59 cfs @ 12.30 hrs, Volume= 20.184 cf

Outflow = 3.47 cfs @ 12.70 hrs, Volume= 19,854 cf, Atten= 70%, Lag= 24.3 min

Routed to Reach 11R: 4x4 Open Bottom Culvert

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 269

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.18 fps, Min. Travel Time= 54.3 min Avg. Velocity = 0.06 fps, Avg. Travel Time= 157.4 min

Peak Storage= 11,311 cf @ 12.70 hrs

Average Depth at Peak Storage= 0.45', Surface Width= 48.54'

Bank-Full Depth= 1.00' Flow Area= 50.0 sf, Capacity= 14.51 cfs

35.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 15.0 '/' Top Width= 65.00'

Length= 600.0' Slope= 0.0087 '/'

Inlet Invert= 200.00', Outlet Invert= 194.80'



Summary for Reach SC1: Stream Crossing #1

[52] Hint: Inlet/Outlet conditions not evaluated

[90] Warning: Qout>Qin may require smaller dt or Finer Routing

Inflow Area = 432,269 sf, 42.08% Impervious, Inflow Depth > 3.62" for 25YR event

Inflow = 26.72 cfs @ 12.32 hrs, Volume= 130,245 cf

Outflow = 26.73 cfs @ 12.32 hrs, Volume= 130,223 cf, Atten= 0%, Lag= 0.1 min

Routed to Reach R202: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 3.86 fps, Min. Travel Time= 0.2 min

Avg. Velocity = 1.28 fps, Avg. Travel Time= 0.6 min

Peak Storage= 298 cf @ 12.32 hrs

Average Depth at Peak Storage= 0.43', Surface Width= 16.00'

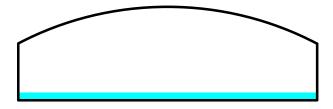
Bank-Full Depth= 5.00' Flow Area= 69.8 sf, Capacity= 722.91 cfs

192.0" W x 60.0" H, R=207.0" Arch Pipe

n= 0.030 Stream, clean & straight

Length= 43.1' Slope= 0.0200 '/'

Inlet Invert= 206.37', Outlet Invert= 205.51'



Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 270

Summary for Reach SC2: Stream Crossing #2

[52] Hint: Inlet/Outlet conditions not evaluated

Inflow Area = 52,768 sf, 0.60% Impervious, Inflow Depth > 2.55" for 25YR event

Inflow = 2.25 cfs @ 12.34 hrs, Volume= 11,217 cf

Outflow = 2.25 cfs @ 12.34 hrs, Volume= 11,215 cf, Atten= 0%, Lag= 0.3 min

Routed to Reach 14R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 1.57 fps, Min. Travel Time= 0.4 min Avg. Velocity = 1.06 fps, Avg. Travel Time= 0.6 min

Peak Storage= 52 cf @ 12.34 hrs

Average Depth at Peak Storage= 0.09', Surface Width= 16.00' Bank-Full Depth= 5.00' Flow Area= 68.1 sf, Capacity= 768.96 cfs

192.0" W x 60.0" H, R=180.0" Arch Pipe n= 0.030 Stream, clean & straight Length= 36.5' Slope= 0.0241 '/'

Inlet Invert= 208.52', Outlet Invert= 207.64'



Summary for Pond 1P: DMH #33

Inflow Area = 16,111 sf, 93.77% Impervious, Inflow Depth > 5.89" for 25YR event

Inflow = 2.21 cfs @ 12.09 hrs, Volume= 7,907 cf

Outflow = 2.21 cfs @ 12.09 hrs, Volume= 7,907 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.21 cfs @ 12.09 hrs, Volume= 7.907 cf

Routed to Pond OCS6: OCS #6

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.34' @ 12.09 hrs

Flood Elev= 209.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.50'	12.0" Round Culvert L= 46.7' Ke= 0.500
			Inlet / Outlet Invert= 205.50' / 204.33' S= 0.0251 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.15 cfs @ 12.09 hrs HW=206.33' TW=202.51' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.15 cfs @ 3.10 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 271

Summary for Pond 3P: OCS #8

Inflow Area = 12,684 sf, 86.64% Impervious, Inflow Depth > 5.38" for 25YR event

Inflow 1.67 cfs @ 12.09 hrs, Volume= 5.685 cf

1.67 cfs @ 12.09 hrs, Volume= Outflow 5,685 cf, Atten= 0%, Lag= 0.0 min

Primary 1.67 cfs @ 12.09 hrs, Volume= 5.685 cf Routed to Pond P214: STORMTECH INFILTRATION SYSTEM #4

Routing by Dvn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 202.09' @ 12.60 hrs

Flood Elev= 206.36'

Device Routing Invert Outlet Devices #1 200.62' 12.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads Primary

Primary OutFlow Max=1.67 cfs @ 12.09 hrs HW=201.39' TW=201.11' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 1.67 cfs @ 2.57 fps)

Summary for Pond 5R: TRENCH DRAIN

Inflow Area = 11,173 sf, 75.10% Impervious, Inflow Depth > 5.47" for 25YR event

Inflow 1.49 cfs @ 12.09 hrs, Volume= 5,089 cf

1.49 cfs @ 12.09 hrs, Volume= 5,089 cf, Atten= 0%, Lag= 0.0 min Outflow

Primary 1.49 cfs @ 12.09 hrs, Volume= 5.089 cf Routed to Pond P206: STORMTECH INFILTRATION SYSTEM #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 198.33' @ 12.09 hrs

Flood Elev= 199.50'

Device Routing Invert Outlet Devices #1 197.22' **8.0" Round Culvert** L= 36.0' Ke= 0.500 Primary Inlet / Outlet Invert= 197.22' / 196.50' S= 0.0200 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=1.45 cfs @ 12.09 hrs HW=198.30' TW=195.96' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.45 cfs @ 4.15 fps)

Summary for Pond 11P: YARD DRAIN

21,407 sf, 48.10% Impervious, Inflow Depth > 3.94" for 25YR event Inflow Area =

Inflow 2.21 cfs @ 12.09 hrs, Volume= 7,022 cf

Outflow 6,973 cf, Atten= 25%, Lag= 4.5 min =

1.67 cfs @ 12.17 hrs, Volume= 1.67 cfs @ 12.17 hrs, Volume= Primary 6,973 cf

Routed to Pond D13: DMH #13

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.41' @ 12.17 hrs Surf.Area= 6,009 sf Storage= 883 cf

Plug-Flow detention time= 16.5 min calculated for 6,973 cf (99% of inflow)

Center-of-Mass det. time= 12.2 min (826.4 - 814.2)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 272

Volume	Inve	rt Avail.Sto	rage Storage	Description	
#1	207.2	5' 5,47	75 cf Custom	n Stage Data (Pri	smatic)Listed below (Recalc)
Elevation (fee	et)	Surf.Area (sq-ft) 5,050	Inc.Store (cubic-feet) 0	Cum.Store (cubic-feet) 0	
208.0	00	9,550	5,475	5,475	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	203.25'	12.0" Round	d Culvert L= 61.0	0' Ke= 0.500
#2	Device 1	207.25'	n= 0.012 Cor 4.0" x 4.0" H X 4 rows C= 0	rrugated PP, smo oriz. Orifice/Gra	202.94' S= 0.0051 '/' Cc= 0.900 oth interior, Flow Area= 0.79 sf te X 4.00 columns 4.0" Grate (44% open area) ds

Primary OutFlow Max=1.64 cfs @ 12.17 hrs HW=207.41' TW=203.36' (Dynamic Tailwater) -1=Culvert (Passes 1.64 cfs of 6.63 cfs potential flow) **2=Orifice/Grate** (Weir Controls 1.64 cfs @ 1.30 fps)

Summary for Pond CB10: CB #10

Inflow Area = 6,961 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow = 0.96 cfs @ 12.09 hrs, Volume= 3,508 cf

0.96 cfs @ 12.09 hrs, Volume= 0.96 cfs @ 12.09 hrs, Volume= 3,508 cf, Atten= 0%, Lag= 0.0 min Outflow

Primary = 3,508 cf

Routed to Pond D5: DMH #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.46' @ 12.09 hrs

Flood Elev= 212.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.76'	12.0" Round Culvert L= 33.8' Ke= 0.500 Inlet / Outlet Invert= 209.76' / 209.59' S= 0.0050'/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior Flow Area= 0.79 sf

Primary OutFlow Max=0.93 cfs @ 12.09 hrs HW=210.44' TW=210.24' (Dynamic Tailwater) **1=Culvert** (Outlet Controls 0.93 cfs @ 2.30 fps)

Summary for Pond CB11: CB #11

7,173 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event Inflow Area =

Inflow = 0.99 cfs @ 12.09 hrs, Volume= 3,615 cf

Outflow 0.99 cfs @ 12.09 hrs, Volume= 3,615 cf, Atten= 0%, Lag= 0.0 min

 0.99 cfs @
 12.09 hrs, Volume=
 3,615 cf,

 0.99 cfs @
 12.09 hrs, Volume=
 3,615 cf

 Primary =

Routed to Pond D5: DMH #5

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 273

Peak Elev= 210.55' @ 12.09 hrs

Flood Elev= 213.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.94'	12.0" Round Culvert L= 26.3' Ke= 0.500 Inlet / Outlet Invert= 209.94' / 209.67' S= 0.0103 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.96 cfs @ 12.09 hrs HW=210.54' TW=210.24' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.96 cfs @ 2.83 fps)

Summary for Pond CB12: CB #12

Inflow Area = 5,238 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow 0.72 cfs @ 12.09 hrs, Volume= 2.640 cf

2,640 cf, Atten= 0%, Lag= 0.0 min 2,640 cf 0.72 cfs @ 12.09 hrs, Volume= Outflow

Primary = 0.72 cfs @ 12.09 hrs, Volume=

Routed to Pond 1P: DMH #33

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.11' @ 12.09 hrs

Flood Elev= 209.84'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.68'	12.0" Round Culvert L= 41.3' Ke= 0.500 Inlet / Outlet Invert= 206.68' / 205.65' S= 0.0249 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.70 cfs @ 12.09 hrs HW=207.10' TW=206.33' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.70 cfs @ 2.22 fps)

Summary for Pond CB13: CB #13

Inflow Area = 10,873 sf, 90.78% Impervious, Inflow Depth > 5.81" for 25YR event

1.49 cfs @ 12.09 hrs, Volume= 5,267 cf Inflow =

Outflow 1.49 cfs @ 12.09 hrs, Volume= 5,267 cf, Atten= 0%, Lag= 0.0 min

1.49 cfs @ 12.09 hrs, Volume= Primary = 5,267 cf

Routed to Pond 1P: DMH #33

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.35' @ 12.09 hrs

Flood Elev= 209.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.70'	12.0" Round Culvert L= 43.7' Ke= 0.500 Inlet / Outlet Invert= 206.70' / 205.61' S= 0.0249 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.45 cfs @ 12.09 hrs HW=207.34' TW=206.33' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.45 cfs @ 2.72 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 274

Summary for Pond CB14: CB #14

Inflow Area = 12,099 sf, 86.22% Impervious, Inflow Depth > 5.24" for 25YR event

Inflow = 1.57 cfs @ 12.09 hrs, Volume= 5,281 cf

Outflow = 1.57 cfs @ 12.09 hrs, Volume= 5,281 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.57 cfs @ 12.09 hrs, Volume= 5,281 cf

Routed to Pond D8: DMH #8

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 201.95' @ 12.09 hrs

Flood Elev= 203.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.79'	12.0" Round Culvert L= 23.2' Ke= 0.500 Inlet / Outlet Invert= 200.79' / 200.67' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.54 cfs @ 12.09 hrs HW=201.92' TW=201.76' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.54 cfs @ 1.96 fps)

Summary for Pond CB15: CB #15

Inflow Area = 6,666 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow = 0.92 cfs @ 12.09 hrs, Volume= 3,360 cf

Outflow = 0.92 cfs @ 12.09 hrs, Volume= 3,360 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.92 cfs @ 12.09 hrs, Volume= 3,360 cf

Routed to Pond D8: DMH #8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 201.84' @ 12.09 hrs

Flood Elev= 203.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.79'	12.0" Round Culvert L= 15.6' Ke= 0.500
			Inlet / Outlet Invert= 200.79' / 200.71' S= 0.0051 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.91 cfs @ 12.09 hrs HW=201.81' TW=201.75' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.91 cfs @ 1.16 fps)

Summary for Pond CB16: CB #16

Inflow Area = 8,516 sf, 64.88% Impervious, Inflow Depth > 3.94" for 25YR event

Inflow = 0.88 cfs @ 12.09 hrs, Volume= 2,794 cf

Outflow = 0.88 cfs @ 12.09 hrs, Volume= 2,794 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.88 cfs @ 12.09 hrs, Volume= 2,794 cf

Routed to Pond D10: DMH #10

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 275

Peak Elev= 204.11' @ 12.09 hrs Flood Elev= 206.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.47'	12.0" Round Culvert L= 20.9' Ke= 0.500 Inlet / Outlet Invert= 203.47' / 203.33' S= 0.0067 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.86 cfs @ 12.09 hrs HW=204.10' TW=203.91' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.86 cfs @ 2.37 fps)

Summary for Pond CB17: CB #17

Inflow Area = 11,836 sf, 73.87% Impervious, Inflow Depth > 5.35" for 25YR event

Inflow 1.56 cfs @ 12.09 hrs, Volume= 5,278 cf

1.56 cfs @ 12.09 hrs, Volume= 5,278 cf, Atten= 0%, Lag= 0.0 min 5,278 cf Outflow

Primary = 1.56 cfs @ 12.09 hrs, Volume=

Routed to Pond D11: DMH #11

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.78' @ 12.09 hrs

Flood Elev= 208.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.99'	12.0" Round Culvert L= 13.8' Ke= 0.500 Inlet / Outlet Invert= 204.99' / 204.86' S= 0.0094 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.20 cfs @ 12.09 hrs HW=205.76' TW=205.57' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.20 cfs @ 2.54 fps)

Summary for Pond CB18: CB #18

Inflow Area = 24,853 sf, 72.99% Impervious, Inflow Depth > 4.50" for 25YR event

2.77 cfs @ 12.09 hrs, Volume= 9.314 cf Inflow =

Outflow 2.77 cfs @ 12.09 hrs, Volume= 9,314 cf, Atten= 0%, Lag= 0.0 min

2.77 cfs @ 12.09 hrs, Volume= Primary = 9,314 cf

Routed to Pond D11: DMH #11

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.80' @ 12.10 hrs

Flood Elev= 208.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.72'	15.0" Round Culvert L= 25.1' Ke= 0.500 Inlet / Outlet Invert= 204.72' / 204.59' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.12 cfs @ 12.09 hrs HW=205.79' TW=205.59' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.12 cfs @ 2.55 fps)

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 276

Summary for Pond CB20: CB #20

Inflow Area = 11,939 sf, 88.95% Impervious, Inflow Depth > 5.70" for 25YR event

Inflow = 1.62 cfs @ 12.09 hrs, Volume= 5,667 cf

Outflow = 1.62 cfs @ 12.09 hrs, Volume= 5,667 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.62 cfs @ 12.09 hrs, Volume= 5,667 cf

Routed to Pond D12: DMH #12

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.88' @ 12.09 hrs

Flood Elev= 207.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.97'	12.0" Round Culvert L= 30.3' Ke= 0.500 Inlet / Outlet Invert= 203.97' / 203.81' S= 0.0053 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.59 cfs @ 12.09 hrs HW=204.85' TW=204.55' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.59 cfs @ 2.90 fps)

Summary for Pond CB21: CB #21

Inflow Area = 10,174 sf, 87.04% Impervious, Inflow Depth > 5.13" for 25YR event

Inflow = 1.30 cfs @ 12.09 hrs, Volume= 4,345 cf

Outflow = 1.30 cfs @ 12.09 hrs, Volume= 4,345 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.30 cfs @ 12.09 hrs, Volume= 4,345 cf

Routed to Pond D12: DMH #12

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.05' @ 12.09 hrs

Flood Elev= 208.02'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.32'	12.0" Round Culvert L= 26.0' Ke= 0.500 Inlet / Outlet Invert= 204.32' / 204.19' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.27 cfs @ 12.09 hrs HW=205.04' TW=204.55' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.27 cfs @ 2.96 fps)

Summary for Pond CB22: CB #22

Inflow Area = 12,001 sf, 91.62% Impervious, Inflow Depth > 5.81" for 25YR event

Inflow = 1.64 cfs @ 12.09 hrs, Volume= 5,813 cf

Outflow = 1.64 cfs @ 12.09 hrs, Volume= 5,813 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.64 cfs @ 12.09 hrs, Volume= 5,813 cf

Routed to Pond D14: DMH #14

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 277

Peak Elev= 206.17' @ 12.09 hrs Flood Elev= 208.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.33'	12.0" Round Culvert L= 16.1' Ke= 0.500 Inlet / Outlet Invert= 205.33' / 205.25' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.60 cfs @ 12.09 hrs HW=206.15' TW=205.32' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.60 cfs @ 3.14 fps)

Summary for Pond CB23: CB #23

Inflow Area = 9,694 sf, 61.00% Impervious, Inflow Depth > 5.01" for 25YR event

Inflow = 1.22 cfs @ 12.09 hrs, Volume= 4,050 cf

Outflow = 1.22 cfs @ 12.09 hrs, Volume= 4,050 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.22 cfs @ 12.09 hrs, Volume= 4.050 cf

Routed to Pond D14: DMH #14

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.10' @ 12.09 hrs

Flood Elev= 208.57'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.41'	12.0" Round Culvert L= 16.3' Ke= 0.500 Inlet / Outlet Invert= 205.41' / 205.32' S= 0.0055 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.19 cfs @ 12.09 hrs HW=206.09' TW=205.32' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.19 cfs @ 2.95 fps)

Summary for Pond CB24: CB #24

Inflow Area = 7,930 sf, 72.16% Impervious, Inflow Depth > 5.47" for 25YR event

Inflow = 1.06 cfs @ 12.09 hrs, Volume= 3.612 cf

Outflow = 1.06 cfs @ 12.09 hrs, Volume= 3,612 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.06 cfs @ 12.09 hrs, Volume= 3,612 cf

Routed to Pond D16: DMH #16

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.98' @ 12.09 hrs

Flood Elev= 209.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.21'	12.0" Round Culvert L= 12.1' Ke= 0.500
			Inlet / Outlet Invert= 205.21' / 205.15' S= 0.0050 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.03 cfs @ 12.09 hrs HW=205.96' TW=205.82' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.03 cfs @ 2.26 fps)

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 278

Summary for Pond CB25: CB #25

Inflow Area = 8,487 sf, 80.92% Impervious, Inflow Depth > 5.58" for 25YR event

Inflow = 1.14 cfs @ 12.09 hrs, Volume= 3,947 cf

Outflow = 1.14 cfs @ 12.09 hrs, Volume= 3,947 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.14 cfs @ 12.09 hrs, Volume= 3,947 cf

Routed to Pond D16: DMH #16

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 205.96' @ 12.09 hrs

Flood Elev= 208.38'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.22'	15.0" Round Culvert L= 11.4' Ke= 0.500 Inlet / Outlet Invert= 205.22' / 205.16' S= 0.0053 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.11 cfs @ 12.09 hrs HW=205.95' TW=205.82' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.11 cfs @ 2.16 fps)

Summary for Pond CB26: CB #26

Inflow Area = 8,835 sf, 63.75% Impervious, Inflow Depth > 5.24" for 25YR event

Inflow = 1.15 cfs @ 12.09 hrs, Volume= 3,856 cf

Outflow = 1.15 cfs @ 12.09 hrs, Volume= 3,856 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.15 cfs @ 12.09 hrs, Volume= 3,856 cf

Routed to Pond D17: DMH #17

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 202.43' @ 12.09 hrs

Flood Elev= 204.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	201.77'	12.0" Round Culvert L= 42.5' Ke= 0.500 Inlet / Outlet Invert= 201.77' / 201.55' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.12 cfs @ 12.09 hrs HW=202.42' TW=201.32' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.12 cfs @ 2.93 fps)

Summary for Pond CB27: CB #27

Inflow Area = 6,111 sf, 91.90% Impervious, Inflow Depth > 5.81" for 25YR event

Inflow = 0.84 cfs @ 12.09 hrs, Volume= 2,960 cf

Outflow = 0.84 cfs @ 12.09 hrs, Volume= 2,960 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.84 cfs @ 12.09 hrs, Volume= 2,960 cf

Routed to Pond D17: DMH #17

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 279

Peak Elev= 201.56' @ 12.09 hrs Flood Elev= 204.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	201.00'	12.0" Round Culvert L= 18.0' Ke= 0.500 Inlet / Outlet Invert= 201.00' / 200.90' S= 0.0056 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.82 cfs @ 12.09 hrs HW=201.55' TW=201.32' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.82 cfs @ 2.65 fps)

Summary for Pond CB28: CB #28

Inflow Area = 10,372 sf, 51.33% Impervious, Inflow Depth > 4.90" for 25YR event

Inflow 1.29 cfs @ 12.09 hrs, Volume= 4,237 cf

1.29 cfs @ 12.09 hrs, Volume= 4,237 cı, 4,237 cf Outflow 4,237 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.29 cfs @ 12.09 hrs, Volume=

Routed to Pond D18: DMH #18

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 198.60' @ 12.09 hrs

Flood Elev= 200.92'

Device	Routing	Invert	Outlet Devices
#1	Primary	197.75'	12.0" Round Culvert L= 13.7' Ke= 0.500 Inlet / Outlet Invert= 197.75' / 197.69' S= 0.0044 '/' Cc= 0.900
			n= 0.013 Corrugated PE_smooth interior_Flow Area= 0.79 sf

Primary OutFlow Max=1.26 cfs @ 12.09 hrs HW=198.58' TW=198.41' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.26 cfs @ 2.43 fps)

Summary for Pond CB29: CB #29

Inflow Area = 8,495 sf, 84.21% Impervious, Inflow Depth > 5.58" for 25YR event

1.14 cfs @ 12.09 hrs, Volume= 3,950 cf Inflow =

Outflow 1.14 cfs @ 12.09 hrs, Volume= 3,950 cf, Atten= 0%, Lag= 0.0 min

1.14 cfs @ 12.09 hrs, Volume= Primary = 3,950 cf

Routed to Pond D19: DMH #19

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.22' @ 12.09 hrs

Flood Elev= 208.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.38'	12.0" Round Culvert L= 13.5' Ke= 0.500 Inlet / Outlet Invert= 205.38' / 205.31' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.09 cfs @ 12.09 hrs HW=206.20' TW=206.07' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.09 cfs @ 2.13 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 280

Summary for Pond CB30: CB #30

Inflow Area = 8,933 sf, 82.40% Impervious, Inflow Depth > 5.58" for 25YR event

Inflow = 1.20 cfs @ 12.09 hrs, Volume= 4,154 cf

Outflow = 1.20 cfs @ 12.09 hrs, Volume= 4,154 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.20 cfs @ 12.09 hrs, Volume= 4,154 cf

Routed to Pond D19: DMH #19

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 206.24' @ 12.09 hrs

Flood Elev= 208.54'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.38'	12.0" Round Culvert L= 17.5' Ke= 0.500 Inlet / Outlet Invert= 205.38' / 205.29' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.15 cfs @ 12.09 hrs HW=206.22' TW=206.07' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.15 cfs @ 2.20 fps)

Summary for Pond CB31: CB #31

Inflow Area = 16,365 sf, 68.64% Impervious, Inflow Depth > 5.13" for 25YR event

Inflow = 2.10 cfs @ 12.09 hrs, Volume= 6,989 cf

Outflow = 2.10 cfs @ 12.09 hrs, Volume= 6,989 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.10 cfs @ 12.09 hrs, Volume= 6,989 cf

Routed to Pond D21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.18' @ 12.09 hrs

Flood Elev= 207.36'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.19'	12.0" Round Culvert L= 16.4' Ke= 0.500 Inlet / Outlet Invert= 204.19' / 204.11' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=2.04 cfs @ 12.09 hrs HW=205.17' TW=204.63' (Dynamic Tailwater) 1=Culvert (Barrel Controls 2.04 cfs @ 3.31 fps)

Summary for Pond CB32: CB #32

Inflow Area = 12,710 sf, 70.47% Impervious, Inflow Depth > 5.24" for 25YR event

Inflow = 1.65 cfs @ 12.09 hrs, Volume= 5,548 cf

Outflow = 1.65 cfs @ 12.09 hrs, Volume= 5,548 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.65 cfs @ 12.09 hrs, Volume= 5,548 cf

Routed to Pond D21: DMH #21

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 281

Peak Elev= 205.04' @ 12.09 hrs

Flood Elev= 207.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.19'	12.0" Round Culvert L= 16.3' Ke= 0.500 Inlet / Outlet Invert= 204.19' / 204.11' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.61 cfs @ 12.09 hrs HW=205.03' TW=204.63' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.61 cfs @ 3.10 fps)

Summary for Pond CB33: CB #33

Inflow Area = 5,421 sf, 83.90% Impervious, Inflow Depth > 5.58" for 25YR event

Inflow 0.73 cfs @ 12.09 hrs, Volume= 2.521 cf

0.73 cfs @ 12.09 hrs, Volume= 2,521 cf, Atten= 0%, Lag= 0.0 min 2,521 cf Outflow =

Primary = 0.73 cfs @ 12.09 hrs, Volume=

Routed to Pond D22: DMH #22

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.97' @ 12.09 hrs

Flood Elev= 208.45'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.28'	12.0" Round Culvert L= 11.7' Ke= 0.500 Inlet / Outlet Invert= 205.28' / 205.22' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.71 cfs @ 12.09 hrs HW=205.96' TW=205.87' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.71 cfs @ 1.77 fps)

Summary for Pond CB34: CB #34

Inflow Area = 8,622 sf, 80.51% Impervious, Inflow Depth > 5.47" for 25YR event

1.15 cfs @ 12.09 hrs, Volume= 3,927 cf Inflow =

Outflow 1.15 cfs @ 12.09 hrs, Volume= 3,927 cf, Atten= 0%, Lag= 0.0 min

1.15 cfs @ 12.09 hrs, Volume= Primary = 3,927 cf

Routed to Pond D22: DMH #22

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.04' @ 12.09 hrs

Flood Elev= 208.38'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.21'	12.0" Round Culvert L= 16.5' Ke= 0.500
			Inlet / Outlet Invert= 205.21' / 205.13' S= 0.0048 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.12 cfs @ 12.09 hrs HW=206.02' TW=205.87' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.12 cfs @ 2.24 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 282

Summary for Pond CB35: CB #35

Inflow Area = 4,149 sf, 98.10% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow = 0.57 cfs @ 12.09 hrs, Volume= 2,091 cf

Outflow = 0.57 cfs @ 12.09 hrs, Volume= 2,091 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.57 cfs @ 12.09 hrs, Volume= 2,091 cf

Routed to Pond D23: DMH #23

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.50' @ 12.09 hrs

Flood Elev= 210.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	207.04'	12.0" Round Culvert L= 15.2' Ke= 0.500 Inlet / Outlet Invert= 207.04' / 206.96' S= 0.0053 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.56 cfs @ 12.09 hrs HW=207.49' TW=207.30' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.56 cfs @ 2.38 fps)

Summary for Pond CB36: CB #36

Inflow Area = 6,622 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow = 0.92 cfs @ 12.09 hrs, Volume= 3,337 cf

Outflow = 0.92 cfs @ 12.09 hrs, Volume= 3,337 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.92 cfs @ 12.09 hrs, Volume= 3,337 cf

Routed to Pond D23: DMH #23

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.64' @ 12.09 hrs

Flood Elev= 210.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	207.04'	12.0" Round Culvert L= 16.1' Ke= 0.500 Inlet / Outlet Invert= 207.04' / 206.96' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.89 cfs @ 12.09 hrs HW=207.63' TW=207.30' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.89 cfs @ 2.66 fps)

Summary for Pond CB38: CB #38

Inflow Area = 7,637 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow = 1.06 cfs @ 12.09 hrs, Volume= 3,849 cf

Outflow = 1.06 cfs @ 12.09 hrs, Volume= 3,849 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.06 cfs @ 12.09 hrs, Volume= 3,849 cf

Routed to Pond D25: DMH #25

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 283

Peak Elev= 210.83' @ 12.09 hrs Flood Elev= 212.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.69'	12.0" Round Culvert L= 16.7' Ke= 0.500 Inlet / Outlet Invert= 209.69' / 209.61' S= 0.0048 '/' Cc= 0.900 n= 0.012 Corrugated PP. smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.03 cfs @ 12.09 hrs HW=210.80' TW=210.72' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.03 cfs @ 1.31 fps)

Summary for Pond CB39: CB #39

Inflow Area = 7,612 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow = 1.05 cfs @ 12.09 hrs, Volume= 3,836 cf

Outflow = 1.05 cfs @ 12.09 hrs, Volume= 3,836 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.05 cfs @ 12.09 hrs, Volume= 3,836 cf

Routed to Pond D25: DMH #25

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.83' @ 12.09 hrs

Flood Elev= 212.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.69'	12.0" Round Culvert L= 16.4' Ke= 0.500 Inlet / Outlet Invert= 209.69' / 209.61' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.02 cfs @ 12.09 hrs HW=210.80' TW=210.72' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.02 cfs @ 1.30 fps)

Summary for Pond CB40: CB #40

Inflow Area = 4,211 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow = 0.58 cfs @ 12.09 hrs, Volume= 2,122 cf

Outflow = 0.58 cfs @ 12.09 hrs, Volume= 2,122 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.58 cfs @ 12.09 hrs, Volume= 2,122 cf

Routed to Pond D27: DMH #27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 214.28' @ 12.09 hrs

Flood Elev= 217.04'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.68'	12.0" Round Culvert L= 17.8' Ke= 0.500 Inlet / Outlet Invert= 213.68' / 213.55' S= 0.0073 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.57 cfs @ 12.09 hrs HW=214.26' TW=214.17' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.57 cfs @ 1.72 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 284

Summary for Pond CB41: CB #41

Inflow Area = 5,586 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow = 0.77 cfs @ 12.09 hrs, Volume= 2,815 cf

Outflow = 0.77 cfs @ 12.09 hrs, Volume= 2,815 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.77 cfs @ 12.09 hrs, Volume= 2,815 cf

Routed to Pond D27: DMH #27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 214.43' @ 12.09 hrs

Flood Elev= 217.06'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.89'	12.0" Round Culvert L= 18.4' Ke= 0.500 Inlet / Outlet Invert= 213.89' / 213.80' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.75 cfs @ 12.09 hrs HW=214.42' TW=214.17' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.75 cfs @ 2.55 fps)

Summary for Pond CB43: CB #43

Inflow Area = 3,109 sf, 75.36% Impervious, Inflow Depth > 5.01" for 25YR event

Inflow = 0.39 cfs @ 12.09 hrs, Volume= 1,299 cf

Outflow = 0.39 cfs @ 12.09 hrs, Volume= 1,299 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.39 cfs @ 12.09 hrs, Volume= 1,299 cf

Routed to Pond D29: DMH #29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 220.52' @ 12.09 hrs

Flood Elev= 223.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	220.00'	12.0" Round Culvert L= 14.9' Ke= 0.500
			Inlet / Outlet Invert= 220.00' / 219.93' S= 0.0047 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.38 cfs @ 12.09 hrs HW=220.51' TW=220.44' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.38 cfs @ 1.40 fps)

Summary for Pond CB44: CB #44

Inflow Area = 1,978 sf, 84.43% Impervious, Inflow Depth > 5.35" for 25YR event

Inflow = 0.26 cfs @ 12.09 hrs, Volume= 882 cf

Outflow = 0.26 cfs @ 12.09 hrs, Volume= 882 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.26 cfs @ 12.09 hrs, Volume= 882 cf

Routed to Pond D29: DMH #29

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 285

Peak Elev= 220.49' @ 12.09 hrs

Flood Elev= 223.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	220.00'	12.0" Round Culvert L= 14.9' Ke= 0.500 Inlet / Outlet Invert= 220.00' / 219.93' S= 0.0047 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.25 cfs @ 12.09 hrs HW=220.48' TW=220.44' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.25 cfs @ 1.01 fps)

Summary for Pond CB45: CB #45

Inflow Area = 2,465 sf, 50.30% Impervious, Inflow Depth > 4.04" for 25YR event

Inflow = 0.26 cfs @ 12.09 hrs, Volume= 830 cf

Outflow = 0.26 cfs @ 12.09 hrs, Volume= 830 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.26 cfs @ 12.09 hrs, Volume= 830 cf

Routed to Pond D30: DMH #30

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 221.59' @ 12.09 hrs

Flood Elev= 224.46'

Device	Routing	Invert	Outlet Devices
#1	Primary	221.29'	12.0" Round Culvert L= 18.2' Ke= 0.500 Inlet / Outlet Invert= 221.29' / 221.20' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior Flow Area= 0.79 sf

Primary OutFlow Max=0.26 cfs @ 12.09 hrs HW=221.59' TW=221.42' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.26 cfs @ 1.94 fps)

Summary for Pond CB46: CB #46

Inflow Area = 4,397 sf, 50.97% Impervious, Inflow Depth > 4.04" for 25YR event

Inflow = 0.47 cfs @ 12.09 hrs, Volume= 1.481 cf

Outflow = 0.47 cfs @ 12.09 hrs, Volume= 1,481 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.47 cfs @ 12.09 hrs, Volume= 1,481 cf

Routed to Pond D30: DMH #30

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 221.94' @ 12.09 hrs

Flood Elev= 224.69'

Device	Routing	Invert	Outlet Devices
#1	Primary	221.53'	12.0" Round Culvert L= 15.3' Ke= 0.500 Inlet / Outlet Invert= 221.53' / 221.45' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.46 cfs @ 12.09 hrs HW=221.94' TW=221.42' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.46 cfs @ 2.26 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 286

Summary for Pond CB47: CB#47

Inflow Area = 3,012 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow = 0.42 cfs @ 12.09 hrs, Volume= 1,518 cf

Outflow = 0.42 cfs @ 12.09 hrs, Volume= 1,518 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.42 cfs @ 12.09 hrs, Volume= 1,518 cf

Routed to Pond D31: DMH#31

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 225.79' @ 12.16 hrs

Flood Elev= 230.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	225.05'	12.0" Round Culvert L= 20.9' Ke= 0.500 Inlet / Outlet Invert= 225.05' / 224.95' S= 0.0048 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.42 cfs @ 12.09 hrs HW=225.66' TW=225.62' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.42 cfs @ 1.18 fps)

Summary for Pond CB48: CB#48

Inflow Area = 60,128 sf, 25.16% Impervious, Inflow Depth > 3.03" for 25YR event

Inflow = 4.02 cfs @ 12.17 hrs, Volume= 15,183 cf

Outflow = 4.02 cfs @ 12.17 hrs, Volume= 15,183 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.02 cfs @ 12.17 hrs, Volume= 15,183 cf

Routed to Pond D31: DMH#31

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 226.24' @ 12.16 hrs

Flood Elev= 230.25'

Device	Routing	Invert	Outlet Devices
#1	Primary	224.82'	15.0" Round Culvert L= 16.9' Ke= 0.500
			Inlet / Outlet Invert= 224.82' / 224.74' S= 0.0047 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.94 cfs @ 12.17 hrs HW=226.21' TW=225.76' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.94 cfs @ 3.21 fps)

Summary for Pond CB49: CB #49

Inflow Area = 5,238 sf, 84.59% Impervious, Inflow Depth > 5.58" for 25YR event

Inflow = 0.70 cfs @ 12.09 hrs, Volume= 2,436 cf

Outflow = 0.70 cfs @ 12.09 hrs, Volume= 2,436 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.70 cfs @ 12.09 hrs, Volume= 2,436 cf

Routed to Pond DMH32: DMH #32

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 287

Peak Elev= 203.65' @ 12.09 hrs

Flood Elev= 205.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.76'	12.0" Round Culvert L= 15.5' Ke= 0.500 Inlet / Outlet Invert= 202.76' / 202.68' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.69 cfs @ 12.09 hrs HW=203.62' TW=203.57' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.69 cfs @ 1.28 fps)

Summary for Pond CB50: CB #50

Inflow Area = 15,040 sf, 77.20% Impervious, Inflow Depth > 5.47" for 25YR event

Inflow = 2.00 cfs @ 12.09 hrs, Volume= 6.850 cf

2.00 cfs @ 12.09 hrs, Volume= 6,850 cf, Atten= 0%, Lag= 0.0 min 6,850 cf Outflow =

Primary = 2.00 cfs @ 12.09 hrs, Volume=

Routed to Pond DMH32: DMH #32

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.87' @ 12.09 hrs

Flood Elev= 205.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.78'	12.0" Round Culvert L= 15.3' Ke= 0.500 Inlet / Outlet Invert= 202.78' / 202.70' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.96 cfs @ 12.09 hrs HW=203.84' TW=203.57' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.96 cfs @ 2.50 fps)

Summary for Pond CB51: CB #51

Inflow Area = 6,823 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event

0.94 cfs @ 12.09 hrs, Volume= 3,439 cf Inflow =

0.94 cfs @ 12.09 hrs, Volume= Outflow 3,439 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.94 cfs @ 12.09 hrs, Volume= 3,439 cf

Routed to Pond OCS7: OCS #7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.36' @ 12.95 hrs

Flood Elev= 212.77'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.35'	12.0" Round Culvert L= 31.4' Ke= 0.500 Inlet / Outlet Invert= 202.35' / 202.19' S= 0.0051 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.92 cfs @ 12.09 hrs HW=202.94' TW=202.62' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.92 cfs @ 2.76 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 288

Summary for Pond CB52: CB #52

Inflow Area = 9,052 sf, 87.14% Impervious, Inflow Depth > 5.70" for 25YR event

Inflow = 1.23 cfs @ 12.09 hrs, Volume= 4,297 cf

Outflow = 1.23 cfs @ 12.09 hrs, Volume= 4,297 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.23 cfs @ 12.09 hrs, Volume= 4,297 cf

Routed to Pond OCS7: OCS #7

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 203.38' @ 12.09 hrs

Flood Elev= 205.84'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.68'	12.0" Round Culvert L= 25.5' Ke= 0.500 Inlet / Outlet Invert= 202.68' / 202.55' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.20 cfs @ 12.09 hrs HW=203.37' TW=202.62' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.20 cfs @ 2.92 fps)

Summary for Pond CB53: CB #53

Inflow Area = 7,863 sf, 86.52% Impervious, Inflow Depth > 5.47" for 25YR event

Inflow = 1.05 cfs @ 12.09 hrs, Volume= 3,581 cf

Outflow = 1.05 cfs @ 12.09 hrs, Volume= 3,581 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.05 cfs @ 12.09 hrs, Volume= 3,581 cf

Routed to Pond 3P: OCS #8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.42' @ 12.09 hrs

Flood Elev= 205.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.78'	12.0" Round Culvert L= 32.0' Ke= 0.500 Inlet / Outlet Invert= 202.78' / 202.62' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.02 cfs @ 12.09 hrs HW=203.41' TW=201.39' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.02 cfs @ 2.82 fps)

Summary for Pond CB54: CB #54

Inflow Area = 4,821 sf, 86.85% Impervious, Inflow Depth > 5.24" for 25YR event

Inflow = 0.63 cfs @ 12.09 hrs, Volume= 2,104 cf

Outflow = 0.63 cfs @ 12.09 hrs, Volume= 2,104 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.63 cfs @ 12.09 hrs, Volume= 2,104 cf

Routed to Pond 3P: OCS #8

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 289

Peak Elev= 203.14' @ 12.09 hrs

Flood Elev= 205.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.66'	12.0" Round Culvert L= 36.7' Ke= 0.500 Inlet / Outlet Invert= 202.66' / 202.48' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.61 cfs @ 12.09 hrs HW=203.13' TW=201.39' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.61 cfs @ 2.48 fps)

Summary for Pond CB7: CB#5

Inflow Area = 4,650 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow 0.64 cfs @ 12.09 hrs, Volume= 2,344 cf

2,344 cf, Atten= 0%, Lag= 0.0 min 2,344 cf 0.64 cfs @ 12.09 hrs, Volume= Outflow =

Primary = 0.64 cfs @ 12.09 hrs, Volume=

Routed to Pond D4: DMH#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 213.04' @ 12.09 hrs

Flood Elev= 215.79'

Device	Routing	Invert	Outlet Devices
#1	Primary	212.60'	12.0" Round Culvert L= 15.1' Ke= 0.500 Inlet / Outlet Invert= 212.60' / 212.45' S= 0.0099 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=0.63 cfs @ 12.09 hrs HW=213.04' TW=211.65' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.63 cfs @ 2.81 fps)

Summary for Pond CB8: CB#8

Inflow Area = 5,450 sf, 88.75% Impervious, Inflow Depth > 5.58" for 25YR event

0.73 cfs @ 12.09 hrs, Volume= 2,534 cf Inflow =

0.73 cfs @ 12.09 hrs, Volume= Outflow 2,534 cf, Atten= 0%, Lag= 0.0 min

0.73 cfs @ 12.09 hrs, Volume= Primary = 2,534 cf

Routed to Pond D4: DMH#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 214.27' @ 12.09 hrs

Flood Elev= 215.79'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.79'	12.0" Round Culvert L= 15.1' Ke= 0.500 Inlet / Outlet Invert= 213.79' / 213.64' S= 0.0099 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.71 cfs @ 12.09 hrs HW=214.27' TW=211.65' (Dynamic Tailwater) **1=Culvert** (Barrel Controls 0.71 cfs @ 2.84 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 290

Summary for Pond CB9: CB #9

Inflow Area = 16,307 sf, 93.95% Impervious, Inflow Depth > 5.93" for 25YR event

Inflow = 2.24 cfs @ 12.09 hrs, Volume= 8,058 cf

Outflow = 2.24 cfs @ 12.09 hrs, Volume= 8,058 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.24 cfs @ 12.09 hrs, Volume= 8,058 cf

Routed to Pond D5: DMH #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.95' @ 12.09 hrs

Flood Elev= 213.27'

Device	Routing	Invert	Outlet Devices
#1	Primary	210.10'	12.0" Round Culvert L= 19.9' Ke= 0.500 Inlet / Outlet Invert= 210.10' / 209.71' S= 0.0196 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.18 cfs @ 12.09 hrs HW=210.94' TW=210.24' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.18 cfs @ 3.11 fps)

Summary for Pond D10: DMH #10

Inflow Area = 8,516 sf, 64.88% Impervious, Inflow Depth > 3.94" for 25YR event

Inflow = 0.88 cfs @ 12.09 hrs, Volume= 2,794 cf

Outflow = 0.88 cfs @ 12.09 hrs, Volume= 2,794 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.88 cfs @ 12.09 hrs, Volume= 2,794 cf

Routed to Pond P207: INFILTRATION POND #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.91' @ 12.09 hrs

Flood Elev= 206.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.33'	12.0" Round Culvert L= 15.6' Ke= 0.500
	-		Inlet / Outlet Invert= 203.33' / 203.25' S= 0.0051 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.86 cfs @ 12.09 hrs HW=203.91' TW=197.78' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.86 cfs @ 2.65 fps)

Summary for Pond D11: DMH #11

Inflow Area = 36,689 sf, 73.28% Impervious, Inflow Depth > 4.77" for 25YR event

Inflow = 4.32 cfs @ 12.09 hrs, Volume= 14,592 cf

Outflow = 4.32 cfs @ 12.09 hrs, Volume= 14,592 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.32 cfs @ 12.09 hrs, Volume= 14,592 cf

Routed to Pond OCS3: OCS#3

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 291

Peak Elev= 205.64' @ 12.12 hrs

Flood Elev= 208.33'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.25'	18.0" Round Culvert L= 44.6' Ke= 0.500 Inlet / Outlet Invert= 204.25' / 204.03' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=4.27 cfs @ 12.09 hrs HW=205.58' TW=205.19' (Dynamic Tailwater) 1=Culvert (Outlet Controls 4.27 cfs @ 3.42 fps)

Summary for Pond D12: DMH #12

Inflow Area = 22,113 sf, 88.07% Impervious, Inflow Depth > 5.43" for 25YR event

Inflow = 2.92 cfs @ 12.09 hrs, Volume= 10,012 cf

Outflow = 2.92 cfs @ 12.09 hrs, Volume= 10,012 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.92 cfs @ 12.09 hrs, Volume= 10.012 cf

Routed to Pond D13: DMH #13

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.60' @ 12.09 hrs

Flood Elev= 207.78'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.21'	12.0" Round Culvert L= 41.9' Ke= 0.500 Inlet / Outlet Invert= 203.21' / 203.00' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=2.79 cfs @ 12.09 hrs HW=204.55' TW=203.56' (Dynamic Tailwater) 1=Culvert (Barrel Controls 2.79 cfs @ 3.55 fps)

Summary for Pond D13: DMH #13

Inflow Area = 81,632 sf, 72.61% Impervious, Inflow Depth > 5.06" for 25YR event

Inflow = 9.39 cfs @ 12.09 hrs, Volume= 34.406 cf

Outflow = 9.39 cfs @ 12.09 hrs, Volume= 34,406 cf, Atten= 0%, Lag= 0.0 min

Primary = 9.39 cfs @ 12.09 hrs, Volume= 34,406 cf

Routed to Pond P207: INFILTRATION POND #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.59' @ 12.09 hrs

Flood Elev= 208.12'

Device	Routing	Invert	Outlet Devices
#1	Primary	201.95'	24.0" Round Culvert L= 60.1' Ke= 0.500 Inlet / Outlet Invert= 201.95' / 201.65' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=9.26 cfs @ 12.09 hrs HW=203.58' TW=197.80' (Dynamic Tailwater) 1=Culvert (Barrel Controls 9.26 cfs @ 4.61 fps)

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 292

Summary for Pond D14: DMH #14

Inflow Area = 38,112 sf, 77.40% Impervious, Inflow Depth > 5.49" for 25YR event

Inflow = 5.06 cfs @ 12.09 hrs, Volume= 17,421 cf

Outflow = 5.06 cfs @ 12.09 hrs, Volume= 17,421 cf, Atten= 0%, Lag= 0.0 min

Primary = 5.06 cfs @ 12.09 hrs, Volume= 17,421 cf

Routed to Pond d13: DMH #13

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 205.34' @ 12.09 hrs

Flood Elev= 208.78'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.13'	18.0" Round Culvert L= 256.3' Ke= 0.500 Inlet / Outlet Invert= 204.13' / 202.85' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=4.93 cfs @ 12.09 hrs HW=205.32' TW=203.56' (Dynamic Tailwater) 1=Culvert (Barrel Controls 4.93 cfs @ 4.51 fps)

Summary for Pond D16: DMH #16

Inflow Area = 16,417 sf, 76.69% Impervious, Inflow Depth > 5.52" for 25YR event

Inflow = 2.20 cfs @ 12.09 hrs, Volume= 7,558 cf

Outflow = 2.20 cfs @ 12.09 hrs, Volume= 7,558 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.20 cfs @ 12.09 hrs, Volume= 7,558 cf

Routed to Pond D14: DMH #14

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.84' @ 12.09 hrs

Flood Elev= 208.59'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.90'	15.0" Round Culvert L= 103.5' Ke= 0.500 Inlet / Outlet Invert= 204.90' / 204.38' S= 0.0050 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.14 cfs @ 12.09 hrs HW=205.82' TW=205.32' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.14 cfs @ 3.08 fps)

Summary for Pond D17: DMH #17

Inflow Area = 14,946 sf, 75.26% Impervious, Inflow Depth > 5.47" for 25YR event

Inflow = 1.98 cfs @ 12.09 hrs, Volume= 6,816 cf

Outflow = 1.98 cfs @ 12.09 hrs, Volume= 6,816 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.98 cfs @ 12.09 hrs, Volume= 6,816 cf

Routed to Pond D18: DMH #18

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 293

Peak Elev= 201.33' @ 12.09 hrs Flood Elev= 204.84'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.55'	12.0" Round Culvert L= 91.6' Ke= 0.500 Inlet / Outlet Invert= 200.55' / 197.69' S= 0.0312 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.93 cfs @ 12.09 hrs HW=201.32' TW=198.41' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.93 cfs @ 2.98 fps)

Summary for Pond D18: DMH #18

Inflow Area = 25,318 sf, 65.46% Impervious, Inflow Depth > 5.24" for 25YR event

Inflow = 3.27 cfs @ 12.09 hrs, Volume= 11,053 cf

Outflow = 3.27 cfs @ 12.09 hrs, Volume= 11,053 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.27 cfs @ 12.09 hrs, Volume= 11.053 cf

Routed to Pond OCS1: OCS#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 198.43' @ 12.09 hrs

Flood Elev= 201.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	197.44'	15.0" Round Culvert L= 46.3' Ke= 0.500 Inlet / Outlet Invert= 197.44' / 196.98' S= 0.0099 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.19 cfs @ 12.09 hrs HW=198.41' TW=196.32' (Dynamic Tailwater) 1=Culvert (Barrel Controls 3.19 cfs @ 4.29 fps)

Summary for Pond D19: DMH #19

Inflow Area = 17,428 sf, 83.29% Impervious, Inflow Depth > 5.58" for 25YR event

Inflow = 2.34 cfs @ 12.09 hrs, Volume= 8,105 cf

Outflow = 2.34 cfs @ 12.09 hrs, Volume= 8,105 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.34 cfs @ 12.09 hrs, Volume= 8,105 cf

Routed to Pond d20: DMH #20

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.09' @ 12.09 hrs

Flood Elev= 208.57'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.19'	12.0" Round Culvert L= 82.5' Ke= 0.500 Inlet / Outlet Invert= 205.19' / 204.43' S= 0.0092 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.28 cfs @ 12.09 hrs HW=206.07' TW=205.11' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.28 cfs @ 4.14 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 294

Summary for Pond D2: DMH#2

Inflow Area = 73,240 sf, 37.72% Impervious, Inflow Depth > 3.54" for 25YR event

Inflow = 5.38 cfs @ 12.14 hrs, Volume= 21,579 cf

Outflow = 5.38 cfs @ 12.14 hrs, Volume= 21,579 cf, Atten= 0%, Lag= 0.0 min

Primary = 5.38 cfs @ 12.14 hrs, Volume= 21,579 cf

Routed to Pond P205: INFILTRATION POND #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 208.37' @ 12.14 hrs

Flood Elev= 212.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.90'	15.0" Round Culvert L= 38.2' Ke= 0.500 Inlet / Outlet Invert= 206.90' / 206.52' S= 0.0099 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=5.33 cfs @ 12.14 hrs HW=208.36' TW=206.33' (Dynamic Tailwater) 1=Culvert (Barrel Controls 5.33 cfs @ 4.68 fps)

Summary for Pond D20: DMH #20

Inflow Area = 17,428 sf, 83.29% Impervious, Inflow Depth > 5.58" for 25YR event

Inflow = 2.34 cfs @ 12.09 hrs, Volume= 8,105 cf

Outflow = 2.34 cfs @ 12.09 hrs, Volume= 8,105 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.34 cfs @ 12.09 hrs, Volume= 8,105 cf

Routed to Pond D21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.13' @ 12.09 hrs

Flood Elev= 207.68'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.19'	15.0" Round Culvert L= 63.5' Ke= 0.500
			Inlet / Outlet Invert= 204.19' / 203.87' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.28 cfs @ 12.09 hrs HW=205.11' TW=204.63' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.28 cfs @ 3.28 fps)

Summary for Pond D21: DMH #21

Inflow Area = 71,317 sf, 79.77% Impervious, Inflow Depth > 5.47" for 25YR event

Inflow = 9.46 cfs @ 12.09 hrs, Volume= 32,518 cf

Outflow = 9.46 cfs @ 12.09 hrs, Volume= 32,518 cf, Atten= 0%, Lag= 0.0 min

Primary = 9.46 cfs @ 12.09 hrs, Volume= 32,518 cf

Routed to Pond p212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 295

Peak Elev= 204.66' @ 12.09 hrs Flood Elev= 207.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.02'	24.0" Round Culvert L= 72.4' Ke= 0.500 Inlet / Outlet Invert= 203.02' / 202.66' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE. smooth interior. Flow Area= 3.14 sf

Primary OutFlow Max=9.21 cfs @ 12.09 hrs HW=204.63' TW=202.18' (Dynamic Tailwater) 1=Culvert (Barrel Controls 9.21 cfs @ 4.64 fps)

Summary for Pond D22: DMH #22

Inflow Area = 24,814 sf, 89.39% Impervious, Inflow Depth > 5.74" for 25YR event

Inflow = 3.37 cfs @ 12.09 hrs, Volume= 11,876 cf

Outflow = 3.37 cfs @ 12.09 hrs, Volume= 11,876 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.37 cfs @ 12.09 hrs, Volume= 11,876 cf

Routed to Pond d21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.89' @ 12.09 hrs

Flood Elev= 208.46'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.87'	15.0" Round Culvert L= 134.2' Ke= 0.500 Inlet / Outlet Invert= 204.87' / 203.92' S= 0.0071 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 1.23 sf

Primary OutFlow Max=3.28 cfs @ 12.09 hrs HW=205.87' TW=204.63' (Dynamic Tailwater) 1=Culvert (Barrel Controls 3.28 cfs @ 4.28 fps)

Summary for Pond D23: DMH #23

Inflow Area = 10,771 sf, 99.27% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow = 1.49 cfs @ 12.09 hrs, Volume= 5,429 cf

Outflow = 1.49 cfs @ 12.09 hrs, Volume= 5,429 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.49 cfs @ 12.09 hrs, Volume= 5,429 cf

Routed to Pond D22: DMH #22

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.31' @ 12.09 hrs

Flood Elev= 210.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.70'	15.0" Round Culvert L= 173.3' Ke= 0.500 Inlet / Outlet Invert= 206.70' / 204.97' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.45 cfs @ 12.09 hrs HW=207.30' TW=205.87' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 1.45 cfs @ 3.63 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 296

Summary for Pond D25: DMH #25

Inflow Area = 36,995 sf, 87.96% Impervious, Inflow Depth > 5.55" for 25YR event

Inflow = 4.84 cfs @ 12.09 hrs, Volume= 17,114 cf

Outflow = 4.84 cfs @ 12.09 hrs, Volume= 17,114 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.84 cfs @ 12.09 hrs, Volume= 17,114 cf

Routed to Pond P210: POCKET WETLAND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.76' @ 12.09 hrs

Flood Elev= 213.11'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.36'	15.0" Round Culvert L= 237.6' Ke= 0.500 Inlet / Outlet Invert= 209.36' / 208.17' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=4.72 cfs @ 12.09 hrs HW=210.73' TW=203.34' (Dynamic Tailwater) 1=Culvert (Barrel Controls 4.72 cfs @ 4.38 fps)

Summary for Pond D27: DMH #27

Inflow Area = 21,746 sf, 79.51% Impervious, Inflow Depth > 5.20" for 25YR event

Inflow = 2.73 cfs @ 12.09 hrs, Volume= 9,429 cf

Outflow = 2.73 cfs @ 12.09 hrs, Volume= 9,429 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.73 cfs @ 12.09 hrs, Volume= 9,429 cf

Routed to Pond D35: DMH #35

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 214.18' @ 12.09 hrs

Flood Elev= 217.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.34'	15.0" Round Culvert L= 63.9' Ke= 0.500 Inlet / Outlet Invert= 213.34' / 212.38' S= 0.0150 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.66 cfs @ 12.09 hrs HW=214.17' TW=213.11' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.66 cfs @ 3.09 fps)

Summary for Pond D28: DMH #28

Inflow Area = 11,949 sf, 62.72% Impervious, Inflow Depth > 4.51" for 25YR event

Inflow = 1.38 cfs @ 12.09 hrs, Volume= 4,491 cf

Outflow = 1.38 cfs @ 12.09 hrs, Volume= 4,491 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.38 cfs @ 12.09 hrs, Volume= 4,491 cf

Routed to Pond D27: DMH #27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 297

Peak Elev= 218.08' @ 12.09 hrs

Flood Elev= 220.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	217.46'	12.0" Round Culvert L= 158.3' Ke= 0.500 Inlet / Outlet Invert= 217.46' / 214.29' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.35 cfs @ 12.09 hrs HW=218.07' TW=214.17' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.35 cfs @ 2.67 fps)

Summary for Pond D29: DMH #29

Inflow Area = 11,949 sf, 62.72% Impervious, Inflow Depth > 4.51" for 25YR event

Inflow = 1.38 cfs @ 12.09 hrs, Volume= 4,491 cf

Outflow = 1.38 cfs @ 12.09 hrs, Volume= 4,491 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.38 cfs @ 12.09 hrs, Volume= 4,491 cf

Routed to Pond D28: DMH #28

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 220.45' @ 12.09 hrs

Flood Elev= 223.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	219.83'	12.0" Round Culvert L= 150.9' Ke= 0.500 Inlet / Outlet Invert= 219.83' / 217.55' S= 0.0151 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.35 cfs @ 12.09 hrs HW=220.44' TW=218.07' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.35 cfs @ 2.67 fps)

Summary for Pond D30: DMH #30

Inflow Area = 6,862 sf, 50.73% Impervious, Inflow Depth > 4.04" for 25YR event

Inflow = 0.73 cfs @ 12.09 hrs, Volume= 2.311 cf

Outflow = 0.73 cfs @ 12.09 hrs, Volume= 2,311 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.73 cfs @ 12.09 hrs, Volume= 2,311 cf

Routed to Pond D29: DMH #29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 221.43' @ 12.09 hrs

Flood Elev= 224.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	220.92'	12.0" Round Culvert L= 184.2' Ke= 0.500 Inlet / Outlet Invert= 220.92' / 220.00' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.71 cfs @ 12.09 hrs HW=221.42' TW=220.44' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.71 cfs @ 2.64 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 298

Summary for Pond D31: DMH#31

Inflow Area = 63,140 sf, 28.73% Impervious, Inflow Depth > 3.17" for 25YR event

Inflow = 4.31 cfs @ 12.16 hrs, Volume= 16,701 cf

Outflow = 4.31 cfs @ 12.16 hrs, Volume= 16,701 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.31 cfs @ 12.16 hrs, Volume= 16,701 cf

Routed to Pond D4: DMH#4

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 225.78' @ 12.16 hrs

Flood Elev= 229.59'

Device	Routing	Invert	Outlet Devices
#1	Primary	224.63'	15.0" Round Culvert L= 288.5' Ke= 0.500 Inlet / Outlet Invert= 224.63' / 213.09' S= 0.0400 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=4.25 cfs @ 12.16 hrs HW=225.77' TW=211.76' (Dynamic Tailwater) 1=Culvert (Inlet Controls 4.25 cfs @ 3.63 fps)

Summary for Pond D34: DMH #34

Inflow Area = 23,255 sf,100.00% Impervious, Inflow Depth > 6.05" for 25YR event

Inflow = 3.21 cfs @ 12.09 hrs, Volume= 11,720 cf

Outflow = 3.21 cfs @ 12.09 hrs, Volume= 11,720 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.21 cfs @ 12.09 hrs, Volume= 11,720 cf

Routed to Pond OCS1: OCS#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 199.29' @ 12.09 hrs

Flood Elev= 202.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	198.07'	12.0" Round Culvert L= 52.0' Ke= 0.500 Inlet / Outlet Invert= 198.07' / 197.03' S= 0.0200 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.13 cfs @ 12.09 hrs HW=199.25' TW=196.32' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.13 cfs @ 3.98 fps)

Summary for Pond D35: DMH #35

Inflow Area = 21,746 sf, 79.51% Impervious, Inflow Depth > 5.20" for 25YR event

Inflow = 2.73 cfs @ 12.09 hrs, Volume= 9,429 cf

Outflow = 2.73 cfs @ 12.09 hrs, Volume= 9,429 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.73 cfs @ 12.09 hrs, Volume= 9,429 cf

Routed to Pond D25: DMH #25

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 299

Peak Elev= 213.12' @ 12.09 hrs

Flood Elev= 215.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	212.28'	15.0" Round Culvert L= 171.5' Ke= 0.500
			Inlet / Outlet Invert= 212.28' / 209.71' S= 0.0150 '/' Cc= 0.900
			n= 0.012. Flow Area= 1.23 sf

Primary OutFlow Max=2.66 cfs @ 12.09 hrs HW=213.11' TW=210.73' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.66 cfs @ 3.09 fps)

Summary for Pond D4: DMH#4

Inflow Area = 73,240 sf, 37.72% Impervious, Inflow Depth > 3.54" for 25YR event

Inflow = 5.38 cfs @ 12.14 hrs, Volume= 21,579 cf

Outflow = 5.38 cfs @ 12.14 hrs, Volume= 21,579 cf, Atten= 0%, Lag= 0.0 min

Primary = 5.38 cfs @ 12.14 hrs, Volume= 21,579 cf

Routed to Pond D2: DMH#2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 211.79' @ 12.14 hrs

Flood Elev= 217.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	210.34'	15.0" Round Culvert L= 222.3' Ke= 0.500 Inlet / Outlet Invert= 210.34' / 207.01' S= 0.0150 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=5.33 cfs @ 12.14 hrs HW=211.78' TW=208.36' (Dynamic Tailwater) 1=Culvert (Inlet Controls 5.33 cfs @ 4.34 fps)

Summary for Pond D5: DMH #5

Inflow Area = 30,441 sf, 96.76% Impervious, Inflow Depth > 5.98" for 25YR event

Inflow = 4.20 cfs @ 12.09 hrs, Volume= 15,182 cf

Outflow = 4.20 cfs @ 12.09 hrs, Volume= 15,182 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.20 cfs @ 12.09 hrs, Volume= 15,182 cf

Routed to Pond D6: DMH #6

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.26' @ 12.09 hrs

Flood Elev= 212.97'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.09'	18.0" Round Culvert L= 183.0' Ke= 0.500 Inlet / Outlet Invert= 209.09' / 208.17' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=4.08 cfs @ 12.09 hrs HW=210.24' TW=209.17' (Dynamic Tailwater) 1=Culvert (Outlet Controls 4.08 cfs @ 3.90 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 300

Summary for Pond D6: DMH #6

Inflow Area = 30,441 sf, 96.76% Impervious, Inflow Depth > 5.98" for 25YR event

Inflow = 4.20 cfs @ 12.09 hrs, Volume= 15,182 cf

Outflow = 4.20 cfs @ 12.09 hrs, Volume= 15,182 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.20 cfs @ 12.09 hrs, Volume= 15,182 cf

Routed to Pond D7: DMH #7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 209.19' @ 12.09 hrs

Flood Elev= 214.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	208.07'	18.0" Round Culvert L= 299.7' Ke= 0.500 Inlet / Outlet Invert= 208.07' / 206.57' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=4.08 cfs @ 12.09 hrs HW=209.17' TW=207.44' (Dynamic Tailwater) 1=Culvert (Outlet Controls 4.08 cfs @ 4.11 fps)

Summary for Pond D7: DMH #7

Inflow Area = 30,441 sf, 96.76% Impervious, Inflow Depth > 5.98" for 25YR event

Inflow = 4.20 cfs @ 12.09 hrs, Volume= 15,182 cf

Outflow = 4.20 cfs @ 12.09 hrs, Volume= 15,182 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.20 cfs @ 12.09 hrs, Volume= 15,182 cf

Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.46' @ 12.09 hrs

Flood Elev= 213.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.47'	18.0" Round Culvert L= 44.2' Ke= 0.500
			Inlet / Outlet Invert= 206.47' / 204.04' S= 0.0550 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=4.08 cfs @ 12.09 hrs HW=207.44' TW=202.18' (Dynamic Tailwater) 1=Culvert (Inlet Controls 4.08 cfs @ 3.36 fps)

Summary for Pond D8: DMH #8

Inflow Area = 18,765 sf, 91.12% Impervious, Inflow Depth > 5.53" for 25YR event

Inflow = 2.49 cfs @ 12.09 hrs, Volume= 8,641 cf

Outflow = 2.49 cfs @ 12.09 hrs, Volume= 8,641 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.49 cfs @ 12.09 hrs, Volume= 8,641 cf

Routed to Pond D9: DMH #9

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 301

Peak Elev= 201.79' @ 12.09 hrs Flood Elev= 204.72'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.57'	12.0" Round Culvert L= 87.7' Ke= 0.500 Inlet / Outlet Invert= 200.57' / 200.13' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.43 cfs @ 12.09 hrs HW=201.76' TW=201.14' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.43 cfs @ 3.30 fps)

Summary for Pond D9: DMH #9

Inflow Area = 18,765 sf, 91.12% Impervious, Inflow Depth > 5.53" for 25YR event
Inflow = 2.49 cfs @ 12.09 hrs, Volume= 8,641 cf
Outflow = 2.49 cfs @ 12.09 hrs, Volume= 8,641 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.49 cfs @ 12.09 hrs, Volume= 8,641 cf
Routed to Pond P207 : INFILTRATION POND #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 201.16' @ 12.09 hrs Flood Elev= 204.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.03'	12.0" Round Culvert L= 11.9' Ke= 0.500 Inlet / Outlet Invert= 200.03' / 199.97' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=2.43 cfs @ 12.09 hrs HW=201.14' TW=197.77' (Dynamic Tailwater) —1=Culvert (Barrel Controls 2.43 cfs @ 3.48 fps)

Summary for Pond DE61: DRIP #61

Inflow Area =	4,247 sf, 92.68% Impervious,	Inflow Depth > 5.81" for 25YR event		
Inflow =	0.58 cfs @ 12.09 hrs, Volume=	2,057 cf		
Outflow =	0.48 cfs @ 12.14 hrs, Volume=	1,911 cf, Atten= 17%, Lag= 3.4 min		
Discarded =	0.00 cfs @ 4.10 hrs, Volume=	203 cf		
Primary =	0.48 cfs @ 12.14 hrs, Volume=	1,708 cf		
Routed to Reach 8R : OVERLAND FLOW				

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 213.33' @ 12.14 hrs Surf.Area= 665 sf Storage= 303 cf

Plug-Flow detention time= 69.7 min calculated for 1,907 cf (93% of inflow) Center-of-Mass det. time= 31.8 min (788.6 - 756.8)

Volume	Invert	Avail.Storage	Storage Description
#1	212.19'	539 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 302

Elevation	Surf.Area	Voids	Inc.Store	Cum.Store
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)
212.19	665	0.0	0	0
212.20	665	40.0	3	3
214.19	665	40.0	529	532
214.20	665	100.0	7	539

Device	Routing	Invert	Outlet Devices
#1	Primary	214.10'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	212.70'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 212.70' / 212.65' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	212.19'	0.170 in/hr Exfiltration over Surface area Phase-ln= 0.01'

Discarded OutFlow Max=0.00 cfs @ 4.10 hrs HW=212.21' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.48 cfs @ 12.14 hrs HW=213.33' TW=208.06' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Barrel Controls 0.48 cfs @ 2.49 fps)

Summary for Pond DE62: DRIP #62

Inflow Area =	4,247 sf	, 92.68% Impervious,	Inflow Depth > 5.81"	for 25YR event	
Inflow =	0.58 cfs @	12.09 hrs, Volume=	2,057 cf		
Outflow =	0.48 cfs @	12.14 hrs, Volume=	1,911 cf, Atte	n= 17%, Lag= 3.4 min	
Discarded =	0.00 cfs @	4.10 hrs, Volume=	203 cf	_	
Primary =	0.48 cfs @	12.14 hrs, Volume=	1,708 cf		
Routed to Reach 8R : OVERLAND FLOW					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 213.33' @ 12.14 hrs Surf.Area= 665 sf Storage= 303 cf

Plug-Flow detention time= 69.7 min calculated for 1,907 cf (93% of inflow) Center-of-Mass det. time= 31.8 min (788.6 - 756.8)

Volume	Invert Ava	il.Storage	Storage Descrip	tion	
#1	212.19'	539 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
Elevation	Surf.Area	Voids	Inc.Store	Cum.Store	
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
212.19	665	0.0	0	0	
212.20	665	40.0	3	3	
214.19	665	40.0	529	532	
214.20	665	100.0	7	539	

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 303

Device	Routing	Invert	Outlet Devices
#1	Primary	214.10'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	212.70'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 212.70' / 212.65' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	212.19'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 4.10 hrs HW=212.21' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.48 cfs @ 12.14 hrs HW=213.33' TW=208.06' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.48 cfs @ 2.49 fps)

Summary for Pond DE63: DRIP #63

Inflow Area = 3,013 sf, 88.68% Impervious, Inflow Depth > 5.70" for 25YR event

Inflow = 0.41 cfs @ 12.09 hrs, Volume= 1,430 cf

Outflow = 0.37 cfs @ 12.12 hrs, Volume= 1,342 cf, Atten= 8%, Lag= 2.2 min

Discarded = 0.00 cfs @ 4.30 hrs, Volume= 121 cf Primary = 0.37 cfs @ 12.12 hrs, Volume= 1,221 cf

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 208.01' @ 12.12 hrs Surf.Area= 404 sf Storage= 164 cf

Plug-Flow detention time= 60.7 min calculated for 1,342 cf (94% of inflow)

Center-of-Mass det. time= 26.6 min (788.7 - 762.1)

Volume	Invert	Avail.S	torage	Storage Descri	ption	
#1	206.99'		327 cf	Custom Stage	Data (Prismatio	Listed below (Recalc)
Elevation	Surf.A		oids	Inc.Store	Cum.Store	
(feet)	(sc	-ft)	(%)	(cubic-feet)	(cubic-feet)	
206.99	4	104	0.0	0	0	
207.00	4	404 4	10.0	2	2	
208.99	4	404 4	10.0	322	323	
209.00	4	104 10	0.00	4	327	

Device	Routing	Invert	Outlet Devices
#1	Primary	208.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	207.50'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 207.50' / 207.45' S= 0.0050 '/' Cc= 0.900
#3	Discarded	206.99'	n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf 0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 304

Discarded OutFlow Max=0.00 cfs @ 4.30 hrs HW=207.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.36 cfs @ 12.12 hrs HW=208.00' TW=202.10' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.36 cfs @ 2.31 fps)

Summary for Pond DE64: DRIP #64

Inflow Area = 3,470 sf, 91.59% Impervious, Inflow Depth > 5.81" for 25YR event Inflow = 0.47 cfs @ 12.09 hrs, Volume= 1,681 cf

Outflow = 0.42 cfs @ 12.13 hrs, Volume= 1,578 cf, Atten= 11%, Lag= 2.5 min

Discarded = 0.00 cfs @ 3.80 hrs, Volume= 144 cf Primary = 0.42 cfs @ 12.13 hrs, Volume= 1,433 cf

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 206.06' @ 12.13 hrs Surf.Area= 470 sf Storage= 201 cf

Plug-Flow detention time= 62.4 min calculated for 1,578 cf (94% of inflow)

Center-of-Mass det. time= 28.2 min (785.0 - 756.8)

Volume	Inve	ert Ava	il.Storage	Storage Descrip	otion	
#1	204.9	99'	381 cf	Custom Stage	Data (Prismatio	c)Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
204.9	99	470	0.0	0	0	
205.0	00	470	40.0	2	2	
206.9	9	470	40.0	374	376	
207.0	00	470	100.0	5	381	
Device	Routing	In	vert Out	let Devices		
#1	Primary	206	6.90' 180	.0' long x 0.5' bi	readth Broad-C	rested Rectangular Weir

	<u> </u>		-
#1	Primary	206.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	205.50'	6.0" Round Culvert L= 10.0' Ke= 0.500
	•		Inlet / Outlet Invert= 205.50' / 205.45' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	204.99'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 3.80 hrs HW=205.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.41 cfs @ 12.13 hrs HW=206.05' TW=202.10' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.41 cfs @ 2.39 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 305

Summary for Pond DE65: DRIP #65

Inflow Area = 3,016 sf, 88.69% Impervious, Inflow Depth > 5.70" for 25YR event

Inflow = 0.41 cfs @ 12.09 hrs, Volume= 1,432 cf

Outflow = 0.37 cfs @ 12.12 hrs, Volume= 1,343 cf, Atten= 8%, Lag= 2.2 min

Discarded = 0.00 cfs @ 4.30 hrs, Volume= 121 cf Primary = 0.37 cfs @ 12.12 hrs, Volume= 1,222 cf

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.01' @ 12.12 hrs Surf.Area= 404 sf Storage= 165 cf

Plug-Flow detention time= 60.7 min calculated for 1,343 cf (94% of inflow)

Center-of-Mass det. time= 26.6 min (788.7 - 762.1)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion		
#1	205.9	99'	327 cf	Custom Stage	Data (Prismatic)Li	sted below (Recalc)	
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)		
205.9	99	404	0.0	0	0		
206.0	00	404	40.0	2	2		
207.9	99	404	40.0	322	323		
208.0	00	404	100.0	4	327		
Device	Routing	In	vert Out	let Devices			_
#1	Primary	207	7.90' 180	.0' long x 0.5' br	eadth Broad-Cres	ted Rectangular Weir	
	•		Hea	ad (feet) 0.20 0.4	0 0.60 0.80 1.00	•	
			Coe	ef. (English) 2.80	2.92 3.08 3.30 3	.32	
#2	Primary	206			L= 10.0' Ke= 0.5		
						= 0.0050 '/' Cc= 0.900	
					•	r, Flow Area= 0.20 sf	
#3	Discarde	ed 205	5.99' 0.1 7	70 in/hr Exfiltration	on over Surface a	rea Phase-In= 0.01'	

Discarded OutFlow Max=0.00 cfs @ 4.30 hrs HW=206.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.36 cfs @ 12.12 hrs HW=207.00' TW=202.10' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.36 cfs @ 2.31 fps)

Summary for Pond DE66: DRIP #66

Inflow Area = 3,407 sf, 91.46% Impervious, Inflow Depth > 5.81" for 25YR event

Inflow = 0.47 cfs @ 12.09 hrs, Volume= 1,650 cf

Outflow = 0.42 cfs @ 12.13 hrs, Volume= 1,547 cf, Atten= 11%, Lag= 2.5 min

Discarded = 0.00 cfs @ 3.15 hrs, Volume= 144 cf Primary = 0.41 cfs @ 12.13 hrs, Volume= 1,403 cf

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 306

Peak Elev= 208.85' @ 12.13 hrs Surf.Area= 470 sf Storage= 199 cf

Plug-Flow detention time= 62.6 min calculated for 1,544 cf (94% of inflow)

Invest Aveil Otensons Otensons Description

Center-of-Mass det. time= 28.5 min (785.2 - 756.8)

Volume	Inve	ert Avai	II.Storage	Storage Descrip	tion		_
#1	207.7	79'	381 cf	Custom Stage	Data (Prismatic) Lis	ted below (Recalc)	_
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)		
207.7	79	470	0.0	0	0		
207.8	30	470	40.0	2	2		
209.7	79	470	40.0	374	376		
209.8	30	470	100.0	5	381		
Device	Routing	In	vert Out	let Devices			
#1	Primary	209	.70' 180	.0' long x 0.5' br	eadth Broad-Crest	ed Rectangular Weir	_
	•		Hea	nd (feet) 0.20 0.4	0 0.60 0.80 1.00	•	
			Coe	f. (English) 2.80	2.92 3.08 3.30 3.3	32	
#2	Primary	208	3.30' 6.0'	' Round Culvert	L= 10.0' Ke= 0.50	00	
			Inle	t / Outlet Invert= 2	.08.30' / 208.25' S=	= 0.0050 '/' Cc= 0.900	
				•		r, Flow Area= 0.20 sf	
#3	Discarde	ed 207	'.79' 0.17	70 in/hr Exfiltratio	on over Surface ar	ea Phase-In= 0.01'	

Discarded OutFlow Max=0.00 cfs @ 3.15 hrs HW=207.80' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.41 cfs @ 12.13 hrs HW=208.84' TW=202.10' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.41 cfs @ 2.38 fps)

Summary for Pond DE67: DRIP #67

Inflow Area = 3,481 sf, 91.61% Impervious, Inflow Depth > 5.81" for 25YR event Inflow = 0.48 cfs @ 12.09 hrs, Volume= 1,686 cf

Outflow = 0.42 cfs @ 12.13 hrs, Volume= 1,583 cf, Atten= 11%, Lag= 2.5 min

Discarded = 0.00 cfs @ 3.80 hrs, Volume= 144 cf Primary = 0.42 cfs @ 12.13 hrs, Volume= 1,439 cf

Routed to Reach 8R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 209.06' @ 12.13 hrs Surf.Area= 470 sf Storage= 201 cf

Plug-Flow detention time= 61.7 min calculated for 1,580 cf (94% of inflow) Center-of-Mass det. time= 28.2 min (784.9 - 756.8)

Volume	Invert	Avail.Storage	Storage Description
#1	207.99'	381 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 307

Elevation	Surf.Area	Voids	Inc.Store	Cum.Store
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)
207.99	470	0.0	0	0
208.00	470	40.0	2	2
209.99	470	40.0	374	376
210.00	470	100.0	5	381

Device	Routing	Invert	Outlet Devices
#1	Primary	209.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	208.50'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 208.50' / 208.45' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	207.99'	0.170 in/hr Exfiltration over Surface area Phase-ln= 0.01'

Discarded OutFlow Max=0.00 cfs @ 3.80 hrs HW=208.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.41 cfs @ 12.13 hrs HW=209.05' TW=208.05' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Barrel Controls 0.41 cfs @ 2.39 fps)

Summary for Pond DE68: DRIP #68

Inflow Area =	4,212 sf, 9	2.62% Impervious,	Inflow Depth > 5.81"	for 25YR event			
Inflow =	0.58 cfs @ 12	2.09 hrs, Volume=	2,040 cf				
Outflow =	0.49 cfs @ 12	2.14 hrs, Volume=	1,897 cf, Atte	n= 14%, Lag= 3.1 min			
Discarded =	0.00 cfs @ 4	1.15 hrs, Volume=	203 cf				
Primary =	0.49 cfs @ 12	2.14 hrs, Volume=	1,694 cf				
Routed to Pond OCS4 : OCS#4							

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 208.02' @ 12.14 hrs Surf.Area= 665 sf Storage= 274 cf

Plug-Flow detention time= 68.3 min calculated for 1,897 cf (93% of inflow) Center-of-Mass det. time= 30.2 min (787.0 - 756.8)

Volume	Invert Av	ail.Storage	Storage Descrip	otion	
#1	206.99'	539 cf	Custom Stage	Data (Prismatic	Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
206.99	665		Ó	0	
207.00	665	40.0	3	3	
208.99	665	40.0	529	532	
209.00	665	100.0	7	539	

#3

Discarded

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 308

Device	Routing	Invert	Outlet Devices
#1	Primary	208.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	207.50'	6.0" Round Culvert L= 20.0' Ke= 0.500
			Inlet / Outlet Invert= 207.50' / 206.00' S= 0.0750 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	206.99'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 4.15 hrs HW=207.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.49 cfs @ 12.14 hrs HW=208.01' TW=204.88' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Inlet Controls 0.49 cfs @ 2.47 fps)

Summary for Pond DE69: DRIP #69

Inflow Area = 3,480 sf, 91.61% Impervious, Inflow Depth > 5.81" for 25YR event Inflow 0.48 cfs @ 12.09 hrs, Volume= 1.686 cf 0.42 cfs @ 12.13 hrs, Volume= Outflow 1,582 cf, Atten= 11%, Lag= 2.5 min 3.80 hrs, Volume= Discarded = 0.00 cfs @ 144 cf 0.42 cfs @ 12.13 hrs, Volume= Primary 1.438 cf Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 206.56' @ 12.13 hrs Surf.Area= 470 sf Storage= 201 cf

Plug-Flow detention time= 62.2 min calculated for 1,582 cf (94% of inflow) Center-of-Mass det. time= 28.2 min (784.9 - 756.8)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	205.4	49'	381 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
205.4	19	470	0.0	0	0	
205.5	50	470	40.0	2	2	
207.4	19	470	40.0	374	376	
207.5	50	470	100.0	5	381	
Device	Routing	In	vert Out	tlet Devices		
#1	Primary	207				ested Rectangular Weir
				ad (feet) 0.20 0.4		
				ef. (English) 2.80		
#2	Primary	206		" Round Culvert		
			Inle	et / Outlet Invert= 2	206.00' / 205.95'	S= 0.0050 '/' Cc= 0.900

n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf **0.170 in/hr Exfiltration over Surface area** Phase-In= 0.01'

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 309

Discarded OutFlow Max=0.00 cfs @ 3.80 hrs HW=205.51' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.41 cfs @ 12.13 hrs HW=206.55' TW=202.43' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.41 cfs @ 2.39 fps)

Summary for Pond DE70: DRIP #70

3,476 sf, 91.60% Impervious, Inflow Depth > 5.81" for 25YR event Inflow Area = Inflow 0.48 cfs @ 12.09 hrs, Volume= 1,684 cf Outflow 0.42 cfs @ 12.13 hrs, Volume= 1,581 cf. Atten= 11%, Lag= 2.5 min Discarded = 3.15 hrs, Volume= 0.00 cfs @ 144 cf 0.42 cfs @ 12.13 hrs, Volume= 1,436 cf Primary Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 206.96' @ 12.13 hrs Surf.Area= 470 sf Storage= 201 cf

Plug-Flow detention time= 61.8 min calculated for 1,577 cf (94% of inflow) Center-of-Mass det. time= 28.2 min (785.0 - 756.8)

Volume	Invert A	vail.Storage	Storage Descri	ption	
#1	205.89'	381 cf	Custom Stage	Data (Prismatio	Listed below (Recalc)
Elevation (feet)	Surf.Are (sq-		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
205.89	47	70 0.0	0	0	
205.90	47	70 40.0	2	2	
207.89	47	70 40.0	374	376	
207.90	47	70 100.0	5	381	
Device Ro	outing	Invert Ou	tlet Devices		

201100	1 10 4 111 19		Callet Bettleec
#1	Primary	207.80'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	206.40'	6.0" Round Culvert L= 10.0' Ke= 0.500
	•		Inlet / Outlet Invert= 206.40' / 206.35' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	205.89'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 3.15 hrs HW=205.90' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.41 cfs @ 12.13 hrs HW=206.95' TW=202.43' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.41 cfs @ 2.39 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Page 310

Summary for Pond DE71: DRIP #71

Inflow Area = 4,210 sf, 92.61% Impervious, Inflow Depth > 5.81" for 25YR event

Inflow = 0.58 cfs @ 12.09 hrs, Volume= 2,039 cf

Outflow = 0.48 cfs @ 12.14 hrs, Volume= 1,893 cf, Atten= 17%, Lag= 3.4 min

Discarded = 0.00 cfs @ 4.50 hrs, Volume= 203 cf Primary = 0.48 cfs @ 12.14 hrs, Volume= 1,690 cf

Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.62' @ 12.14 hrs Surf.Area= 665 sf Storage= 302 cf

Plug-Flow detention time= 70.6 min calculated for 1,893 cf (93% of inflow)

Center-of-Mass det. time= 32.0 min (788.7 - 756.8)

Volume	Inv	ert Ava	il.Storage	Storage Descri	ption	
#1	206.4	49'	805 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
206.4	49	665	0.0	0	0	
206.5	50	665	40.0	3	3	
209.4	49	665	40.0	795	798	
209.5	50	665	100.0	7	805	
Device	Routing	In	vert Ou	tlet Devices		
#1	Primary	209	9.40' 18	0.0' long x 0.5' b	readth Broad-Cr	ested Rectangular Weir
	,			ad (feet) 0.20 0.4		
				ef. (English) 2.80		
#2	Primary	207		" Round Culver		
						S= 0.0050 '/' Cc= 0.900
						rior, Flow Area= 0.20 sf
#3	Discarde	ad 206				area Phase-In= 0.01'
#5	Discarde	-u 200). T 0 U. I	70 III/III EXIIIII au	ion over ourrace	area 1 11030-111- 0.01

Discarded OutFlow Max=0.00 cfs @ 4.50 hrs HW=206.52' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.47 cfs @ 12.14 hrs HW=207.62' TW=202.51' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.47 cfs @ 2.48 fps)

Summary for Pond DECH: DRIP #CH

Inflow Area = 6,262 sf, 92.70% Impervious, Inflow Depth > 5.81" for 25YR event

Inflow = 0.86 cfs @ 12.09 hrs, Volume= 3.033 cf

Outflow = 0.45 cfs @ 12.27 hrs, Volume= 3,033 cf, Atten= 47%, Lag= 11.3 min

Discarded = 0.04 cfs @ 10.10 hrs, Volume= 1,659 cf Primary = 0.42 cfs @ 12.27 hrs, Volume= 1,373 cf

Routed to Pond CB18: CB #18

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

<u>Page 311</u>

Peak Elev= 209.99' @ 12.22 hrs Surf.Area= 636 sf Storage= 508 cf

Plug-Flow detention time= 21.3 min calculated for 3,026 cf (100% of inflow)

Center-of-Mass det. time= 21.1 min (777.9 - 756.8)

Volume	Inve	ert Ava	il.Storage	Storage Descrip	otion	
#1	207.9	99'	770 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
207.9	99	636	0.0	0	0	
208.0	00	636	40.0	3	3	
210.9	99	636	40.0	761	763	
211.0	00	636	100.0	6	770	
Device	Routing	In	vert Out	let Devices		
#1	Primary	210).90' 160	.0' long x 0.5' bi	readth Broad-Cro	ested Rectangular Weir
	,				10 0.60 0.80 1.0	
					2.92 3.08 3.30	
#2	Primary	208			t L= 80.0' Ke= 0	
<i>,,,</i>	1 minary	200				S= 0.0425 '/' Cc= 0.900
				· -		rior, Flow Area= 0.09 sf
4 2	Diagonda	-d 20-7				
#3	Discarde	eu 207	7.99' 2.4 '	iv in/nr Extiltrati	on over Surrace	area Phase-In= 0.01'

Discarded OutFlow Max=0.04 cfs @ 10.10 hrs HW=208.02' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=0.42 cfs @ 12.27 hrs HW=209.95' TW=205.55' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

—2=Culvert (Outlet Controls 0.42 cfs @ 4.78 fps)

Summary for Pond DMH32: DMH #32

Inflow Area = 20,278 sf, 79.11% Impervious, Inflow Depth > 5.50" for 25YR event

Inflow = 2.71 cfs @ 12.09 hrs, Volume= 9,286 cf

Outflow = 2.71 cfs @ 12.09 hrs, Volume= 9,286 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.71 cfs @ 12.09 hrs, Volume= 9,286 cf

Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.60' @ 12.09 hrs

Flood Elev= 206.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.59'	12.0" Round Culvert L= 19.2' Ke= 0.500 Inlet / Outlet Invert= 202.59' / 201.57' S= 0.0531 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.64 cfs @ 12.09 hrs HW=203.57' TW=202.18' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.64 cfs @ 3.38 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 312

Summary for Pond OCS1: OCS#1

Inflow Area = 48,573 sf, 81.99% Impervious, Inflow Depth > 5.63" for 25YR event

Inflow = 6.49 cfs @ 12.09 hrs, Volume= 22,774 cf

Outflow = 6.49 cfs @ 12.09 hrs, Volume= 22,774 cf, Atten= 0%, Lag= 0.0 min

Primary = 6.49 cfs @ 12.09 hrs, Volume= 22,774 cf Routed to Pond P206 : STORMTECH INFILTRATION SYSTEM #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 196.41' @ 12.16 hrs

Flood Elev= 201.48'

Device Routing Invert Outlet Devices

#1 Primary 195.00' **24.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=6.35 cfs @ 12.09 hrs HW=196.32' TW=195.96' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 6.35 cfs @ 2.89 fps)

Summary for Pond OCS3: OCS#3

Inflow Area = 54,250 sf, 81.93% Impervious, Inflow Depth > 5.19" for 25YR event

Inflow = 6.75 cfs @ 12.09 hrs, Volume= 23,442 cf

Outflow = 6.75 cfs @ 12.09 hrs, Volume= 23,442 cf, Atten= 0%, Lag= 0.0 min

Primary = 6.75 cfs @ 12.09 hrs, Volume= 23,442 cf Routed to Pond p204 : STORMTECH INFILTRATION SYSTEM #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.31' @ 12.15 hrs

Flood Elev= 209.00'

Device Routing Invert Outlet Devices

#1 Primary 203.10' **18.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=6.59 cfs @ 12.09 hrs HW=205.18' TW=204.58' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 6.59 cfs @ 3.73 fps)

Summary for Pond OCS4: OCS#4

Inflow Area = 17,972 sf, 28.85% Impervious, Inflow Depth > 3.83" for 25YR event

Inflow = 1.75 cfs @ 12.10 hrs, Volume= 5,738 cf

Outflow = 1.75 cfs @ 12.10 hrs, Volume= 5,738 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.75 cfs @ 12.10 hrs, Volume= 5,738 cf Routed to Pond P204 : STORMTECH INFILTRATION SYSTEM #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.10' @ 12.26 hrs

Flood Elev= 208.00'

Device	Routing	Invert	Outlet Devices		
#1	Primary	203.10'	18.0" Vert. Orifice/Grate	C= 0.600	Limited to weir flow at low heads

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 313

Primary OutFlow Max=1.75 cfs @ 12.10 hrs HW=204.69' TW=204.64' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 1.75 cfs @ 0.99 fps)

Summary for Pond OCS6: OCS #6

Inflow Area = 16,111 sf, 93.77% Impervious, Inflow Depth > 5.89" for 25YR event

Inflow = 2.21 cfs @ 12.09 hrs, Volume= 7,907 cf

Outflow = 2.21 cfs @ 12.09 hrs, Volume= 7,907 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.21 cfs @ 12.09 hrs. Volume = 7,907 cf

Routed to Pond P213: Stormtech Infiltration System #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.36' @ 12.95 hrs

Flood Elev= 206.96'

Device Routing Invert Outlet Devices

#1 Primary 201.20' **12.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.15 cfs @ 12.09 hrs HW=202.51' TW=202.19' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 2.15 cfs @ 2.74 fps)

Summary for Pond OCS7: OCS #7

Inflow Area = 15,875 sf, 92.67% Impervious, Inflow Depth > 5.85" for 25YR event

Inflow = 2.17 cfs @ 12.09 hrs, Volume= 7,735 cf

Outflow = 2.17 cfs @ 12.09 hrs, Volume= 7,735 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.17 cfs @ 12.09 hrs, Volume= 7,735 cf

Routed to Pond P213: Stormtech Infiltration System #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.36' @ 12.95 hrs

Flood Elev= 206.47'

Device Routing Invert Outlet Devices

#1 Primary 201.78' **12.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.21 cfs @ 12.09 hrs HW=202.62' TW=202.19' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 2.21 cfs @ 3.13 fps)

Summary for Pond P204: STORMTECH INFILTRATION SYSTEM #1

Inflow Area = 72,222 sf, 68.72% Impervious, Inflow Depth > 4.85" for 25YR event

Inflow = 8.49 cfs @ 12.09 hrs, Volume= 29,181 cf

Outflow = 4.39 cfs @ 12.27 hrs, Volume= 26,780 cf, Atten= 48%, Lag= 10.5 min

Discarded = 0.09 cfs @ 8.30 hrs, Volume= 6,064 cf Primary = 4.31 cfs @ 12.27 hrs, Volume= 20,716 cf

Routed to Reach 20r: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 314

Peak Elev= 205.09' @ 12.27 hrs Surf.Area= 5,670 sf Storage= 9,827 cf Flood Elev= 208.75' Surf.Area= 5,670 sf Storage= 13,379 cf

Plug-Flow detention time= 96.7 min calculated for 26,780 cf (92% of inflow)

Center-of-Mass det. time= 55.0 min (828.7 - 773.8)

Volume Invert Avail.Storage Storage Description	
#1A 202.50' 5,923 cf 77.50'W x 67.70'L x 4.08'H STORMTECH SC-740	
21,423 cf Overall - 6,615 cf Embedded = 14,808 cf x 4	10.0% Voids
#2A 203.08' 6,615 cf ADS_StormTech SC-740 +Cap x 144 Inside #1	
Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L =	45.9 cf
Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Ove	rlap
144 Chambers in 16 Rows	
#3B 202.50' 427 cf 6.25'W x 67.70'L x 3.50'H ISOLATOR ROW	
1,481 cf Overall - 413 cf Embedded = 1,067 cf x 40.09	% Voids
#4B 203.00' 413 cf ADS_StormTech SC-740 +Cap x 9 Inside #3	
Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L =	
Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Ove	rlap

13,379 cf Total Available Storage

Storage Group A created with Chamber Wizard Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	202.75'	15.0" Round Culvert L= 35.0' Ke= 0.500
	•		Inlet / Outlet Invert= 202.75' / 201.00' S= 0.0500 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf
#2	Device 1	204.75'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Device 1	203.25'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	202.50'	0.660 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.09 cfs @ 8.30 hrs HW=202.56' (Free Discharge) **4=Exfiltration** (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=4.25 cfs @ 12.27 hrs HW=205.08' TW=200.12' (Dynamic Tailwater)

-1=Culvert (Passes 4.25 cfs of 7.72 cfs potential flow)

-2=Broad-Crested Rectangular Weir (Weir Controls 2.20 cfs @ 1.66 fps)

-3=Orifice/Grate (Orifice Controls 2.06 cfs @ 5.89 fps)

Summary for Pond P205: INFILTRATION POND #3

Inflow Area = 88,676 sf, 39.42% Impervious, Inflow Depth > 3.61" for 25YR event 6.85 cfs @ 12.12 hrs, Volume= Inflow 26.643 cf Outflow 3.69 cfs @ 12.37 hrs, Volume= 22,614 cf, Atten= 46%, Lag= 14.9 min Discarded = 0.31 cfs @ 12.37 hrs, Volume= 15.146 cf 3.39 cfs @ 12.37 hrs, Volume= Primary 7,468 cf

Routed to Reach 18R: OVERLAND FLOW

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 315

Peak Elev= 206.84' @ 12.37 hrs Surf.Area= 5,490 sf Storage= 8,662 cf

Plug-Flow detention time= 169.0 min calculated for 22,567 cf (85% of inflow)

Invest Aveil Changes Changes December

Center-of-Mass det. time= 104.0 min (918.2 - 814.2)

Volume	Inve	rt Avail.Sto	rage Storage	e Description	
#1	205.0	0' 15,6	30 cf Custom	n Stage Data (Prismatic)Listed below (Recalc)	
Elevation		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
205.0	00	3,939	0	0	
206.0	00	4,756	4,348	4,348	
207.0	00	5,627	5,192	9,539	
208.0	00	6,555	6,091	15,630	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	206.65'	15.0' long x	15.0' breadth Broad-Crested Rectangular Weir	
			Head (feet) 0	0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60	
			Coef. (English	sh) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63	
#2	Discarded	d 205.00'	2.410 in/hr E	Exfiltration over Surface area Phase-In= 0.01'	

Discarded OutFlow Max=0.31 cfs @ 12.37 hrs HW=206.84' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.31 cfs)

Primary OutFlow Max=3.31 cfs @ 12.37 hrs HW=206.84' TW=203.04' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 3.31 cfs @ 1.17 fps)

Summary for Pond P206: STORMTECH INFILTRATION SYSTEM #2

Inflow Area = 59,746 sf, 80.70% Impervious, Inflow Depth > 5.60" for 25YR event Inflow = 7.97 cfs @ 12.09 hrs, Volume= 27,862 cf

Outflow = 4.16 cfs @ 12.23 hrs, Volume= 27,859 cf, Atten= 48%, Lag= 8.5 min Discarded = 0.49 cfs @ 11.15 hrs, Volume= 21,285 cf

Primary = 3.67 cfs @ 12.23 hrs, Volume= 6,574 cf

Routed to Link AP4 : ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 196.29' @ 12.23 hrs Surf.Area= 6,072 sf Storage= 6,969 cf

Plug-Flow detention time= 60.4 min calculated for 27,801 cf (100% of inflow) Center-of-Mass det. time= 60.2 min (822.3 - 762.1)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 316

Volume	Invert	Avail.Storage	Storage Description
#1A	194.67'	1,786 cf	39.50'W x 53.46'L x 3.33'H FIELD A
			7,038 cf Overall - 2,573 cf Embedded = 4,466 cf x 40.0% Voids
#2A	195.00'	2,573 cf	ADS_StormTech SC-740 +Cap x 56 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			56 Chambers in 8 Rows
#3B	194.67'	3,296 cf	58.50'W x 67.70'L x 3.33'H FIELD B
			13,201 cf Overall - 4,962 cf Embedded = 8,239 cf x 40.0% Voids
#4B	195.00'	4,962 cf	ADS_StormTech SC-740 +Cap x 108 Inside #3
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			108 Chambers in 12 Rows
		10 C1C of	Total Available Ctarers

12,616 cf Total Available Storage

Storage Group A created with Chamber Wizard Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	194.65'	18.0" Round Culvert L= 30.0' Ke= 0.200
	•		Inlet / Outlet Invert= 194.65' / 194.50' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf
#2	Device 1	195.85'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Discarded	194.67'	3.500 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.49 cfs @ 11.15 hrs HW=194.71' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.49 cfs)

Primary OutFlow Max=3.64 cfs @ 12.23 hrs HW=196.28' TW=0.00' (Dynamic Tailwater)

1=Culvert (Passes 3.64 cfs of 7.27 cfs potential flow)

2=Sharp-Crested Rectangular Weir (Weir Controls 3.64 cfs @ 2.15 fps)

Summary for Pond P207: INFILTRATION POND #2

Inflow Area =	129,716 sf, 63.13% Impervious,	Inflow Depth > 5.03" for 25YR event
Inflow =	15.34 cfs @ 12.09 hrs, Volume=	54,338 cf
Outflow =	2.56 cfs @ 12.61 hrs, Volume=	54,320 cf, Atten= 83%, Lag= 30.9 min
Discarded =	1.09 cfs @ 12.61 hrs, Volume=	41,530 cf
Primary =	1.47 cfs @ 12.61 hrs, Volume=	12,790 cf
Routed to Rea	ich 10R : OVERLAND FLOW	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 198.50' @ 12.61 hrs Surf.Area= 12,751 sf Storage= 19,452 cf

Plug-Flow detention time= 83.2 min calculated for 54,320 cf (100% of inflow) Center-of-Mass det. time= 83.0 min (864.6 - 781.7)

Volume	Invert	Avail.Storage	Storage Description
#1	196.80'	40,260 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Elevation

#3

Device 2

Prepared by Howard Stein Hudson Associates

Surf.Area

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Inc.Store

Page 317

Licvatic	/I I	Curr./ (i Cu	1110.01010	Ourn.Olore	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
196.8	30	10,100	0	0	
198.0	00	12,000	13,260	13,260	
200.0	00	15,000	27,000	40,260	
Device	Routing	Invert	Outlet Devices		
#1	Primary	198.80'	20.0' long x 21	.0' breadth B	Broad-Crested Rectangular Weir
			Head (feet) 0.20	0.40 0.60	0.80 1.00 1.20 1.40 1.60
			Coef. (English)	2.68 2.70 2.	70 2.64 2.63 2.64 2.64 2.63
#2	Primary	194.75'	15.0" Round C	ulvert L= 40	.0' Ke= 0.500
			Inlet / Outlet Inv	ert= 194.75' /	194.55' S= 0.0050 '/' Cc= 0.900
			n= 0.012 Corru	gated PP, sm	ooth interior, Flow Area= 1.23 sf

Cum.Store

6.0" x 6.0" Horiz. Orifice/Grate X 6.00 columns X 6 rows C= 0.600 in 48.0" x 48.0" Grate (56% open area)

Limited to weir flow at low heads

#4 Device 2 197.40' **8.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads 196.80' **3.690 in/hr Exfiltration over Surface area** Phase-In= 0.01'

Discarded OutFlow Max=1.09 cfs @ 12.61 hrs HW=198.50' (Free Discharge)

Primary OutFlow Max=1.47 cfs @ 12.61 hrs HW=198.50' TW=192.24' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

198.80'

-2=Culvert (Passes 1.47 cfs of 10.45 cfs potential flow)

-3=Orifice/Grate (Controls 0.00 cfs)

5=Exfiltration (Exfiltration Controls 1.09 cfs)

-4=Orifice/Grate (Orifice Controls 1.47 cfs @ 4.22 fps)

Summary for Pond P210: POCKET WETLAND #1

Inflow Area = 62,582 sf, 52.00% Impervious, Inflow Depth > 4.60" for 25YR event

Inflow = 7.02 cfs @ 12.09 hrs, Volume= 23,999 cf

Outflow = 0.23 cfs @ 15.93 hrs, Volume= 9,701 cf, Atten= 97%, Lag= 230.7 min

Primary = 0.23 cfs @ 15.93 hrs, Volume= 9,701 cf

Routed to Reach 15R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Starting Elev= 201.00' Surf.Area= 376 sf Storage= 591 cf

Peak Elev= 204.35' @ 15.93 hrs Surf.Area= 8,975 sf Storage= 17,796 cf (17,205 cf above start)

Plug-Flow detention time= 463.9 min calculated for 9,091 cf (38% of inflow)

Center-of-Mass det. time= 293.9 min (1,072.7 - 778.8)

Volume	Invert	Avail.Storage	Storage Description
#1	199.00'	43,190 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 318

Elevation	Surf.Area	Inc.Store	Cum.Store
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)
199.00	218	0	0
200.00	294	256	256
201.00	376	335	591
202.00	3,991	2,184	2,775
204.00	8,073	12,064	14,839
206.00	13,272	21,345	36,184
206.50	14,753	7,006	43,190

Device	Routing	Invert	Outlet Devices
#1	Primary	205.10'	20.0' long x 15.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Primary	202.25'	12.0" Round Culvert L= 44.0' Ke= 0.500
			Inlet / Outlet Invert= 202.25' / 202.03' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	202.30'	2.5" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	204.50'	6.0" x 6.0" Horiz. Orifice/Grate X 6.00 columns
			X 6 rows C= 0.600 in 48.0" x 48.0" Grate (56% open area)
			Limited to weir flow at low heads

Primary OutFlow Max=0.23 cfs @ 15.93 hrs HW=204.35' TW=202.06' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Passes 0.23 cfs of 4.26 cfs potential flow)

-3=Orifice/Grate (Orifice Controls 0.23 cfs @ 6.71 fps)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond P212: INFILTRATION POND #1

Inflow Area = 241,078 sf, 59.10% Impervious, Inflow Depth > 4.27" for 25YR event

Inflow = 21.45 cfs @ 12.10 hrs, Volume= 85,741 cf

Outflow = 13.03 cfs @ 12.30 hrs, Volume= 84,823 cf, Atten= 39%, Lag= 11.9 min

Discarded = 1.44 cfs @ 12.30 hrs, Volume= 64,639 cf Primary = 11.59 cfs @ 12.30 hrs, Volume= 20,184 cf

Routed to Reach R211: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 202.81' @ 12.30 hrs Surf.Area= 12,157 sf Storage= 26,231 cf

Plug-Flow detention time= 127.2 min calculated for 84,823 cf (99% of inflow)

Center-of-Mass det. time= 120.4 min (902.4 - 782.1)

Volume	Invert	Avail.Storage	Storage Description
#1	200.00'	41,774 cf	Custom Stage Data (Conic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 319

Elevation	Surf.Area	Inc.Store	Cum.Store	Wet.Area
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	(sq-ft)
200.00	4,354	0	0	4,354
201.00	9,360	6,699	6,699	9,368
202.00	10,993	10,166	16,865	11,040
204.00	13,976	24,909	41,774	14,126

Device	Routing	Invert	Outlet Devices
#1	Primary	202.50'	25.0' long x 20.0' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.68 2.70 2.70 2.64 2.63 2.64 2.64 2.63
#2	Discarded	200.00'	5.130 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=1.44 cfs @ 12.30 hrs HW=202.81' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 1.44 cfs)

Primary OutFlow Max=11.56 cfs @ 12.30 hrs HW=202.81' TW=200.19' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 11.56 cfs @ 1.50 fps)

Summary for Pond P213: Stormtech Infiltration System #3

Inflow Area =	31,986 sf, 93.23% Impervious,	Inflow Depth > 5.87" for 25YR event			
Inflow =	4.38 cfs @ 12.09 hrs, Volume=	15,642 cf			
Outflow =	0.40 cfs @ 12.96 hrs, Volume=	15,641 cf, Atten= 91%, Lag= 52.3 min			
Discarded =	0.39 cfs @ 11.50 hrs, Volume=	15,640 cf			
Primary =	0.00 cfs @ 12.96 hrs, Volume=	1 cf			
Routed to Pond P212 : INFILTRATION POND #1					

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 203.36' @ 12.96 hrs Surf.Area= 3,317 sf Storage= 5,458 cf

Plug-Flow detention time= 95.9 min calculated for 15,641 cf (100% of inflow) Center-of-Mass det. time= 95.8 min (849.1 - 753.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	200.95'	2,990 cf	49.00'W x 67.70'L x 3.50'H Field A
			11,610 cf Overall - 4,135 cf Embedded = 7,475 cf x 40.0% Voids
#2A	201.45'	4,135 cf	ADS_StormTech SC-740 +Cap x 90 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			90 Chambers in 10 Rows
		7.405 (T () A () 1 0 (

7,125 cf Total Available Storage

Storage Group A created with Chamber Wizard

Routing	Invert	Outlet Devices
Primary	202.30'	12.0" Round Culvert L= 60.0' Ke= 0.500
-		Inlet / Outlet Invert= 202.30' / 202.00' S= 0.0050 '/' Cc= 0.900
		n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
Discarded	200.95'	5.130 in/hr Exfiltration over Surface area Phase-In= 0.01'
Device 1	204.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
	Primary Discarded	Primary 202.30' Discarded 200.95'

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 320

#4 Device 1 203.35' **6.0" W x 4.0" H Vert. Orifice/Grate** C= 0.600

Limited to weir flow at low heads

Discarded OutFlow Max=0.39 cfs @ 11.50 hrs HW=200.99' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.39 cfs)

Primary OutFlow Max=0.00 cfs @ 12.96 hrs HW=203.36' TW=202.58' (Dynamic Tailwater)

1=Culvert (Passes 0.00 cfs of 2.35 cfs potential flow)

3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 0.00 cfs @ 0.30 fps)

Summary for Pond P214: STORMTECH INFILTRATION SYSTEM #4

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=3)

Inflow Area = 32,665 sf, 94.81% Impervious, Inflow Depth > 5.79" for 25YR event

Inflow = 4.44 cfs @ 12.09 hrs, Volume= 15,756 cf

Outflow = 0.61 cfs @ 11.70 hrs, Volume= 15,774 cf, Atten= 86%, Lag= 0.0 min

Discarded = $0.61 \text{ cfs } \boxed{0}$ 11.70 hrs, Volume= 15,774 cf Primary = $0.00 \text{ cfs } \boxed{0}$ 0.00 hrs, Volume= 0 cf

Routed to Reach 9R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 202.09' @ 12.61 hrs Surf.Area= 3,201 sf Storage= 4,534 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 43.4 min (798.3 - 754.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	200.00'	1,851 cf	30.00'W x 67.70'L x 3.50'H Field A
			7,108 cf Overall - 2,481 cf Embedded = 4,627 cf x 40.0% Voids
#2A	200.50'	2,481 cf	ADS_StormTech SC-740 +Cap x 54 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			54 Chambers in 6 Rows
#3B	200.00'	1,087 cf	25.25'W x 46.34'L x 3.50'H Field B
			4,095 cf Overall - 1,378 cf Embedded = 2,717 cf x 40.0% Voids
#4B	200.50'	1,378 cf	ADS_StormTech SC-740 +Cap x 30 Inside #3
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			30 Chambers in 5 Rows
		0.707 (T () A ())) O(

6,797 cf Total Available Storage

Storage Group A created with Chamber Wizard Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	201.00'	12.0" Round Culvert L= 25.0' Ke= 0.500
	•		Inlet / Outlet Invert= 201.00' / 200.88' S= 0.0048 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	200.00'	8.280 in/hr Exfiltration over Surface area Phase-In= 0.01'

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

<u>Page 321</u>

#3 Device 1 203.40' 4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) 44 Device 1 202.40' 8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.61 cfs @ 11.70 hrs HW=200.06' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.61 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=200.00' TW=201.75' (Dynamic Tailwater)

-1=Culvert (Controls 0.00 cfs)

3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Orifice/Grate (Controls 0.00 cfs)

Summary for Link AP1: ANALYSIS POINT 1

Inflow Area = 9,943 sf, 92.79% Impervious, Inflow Depth > 5.70" for 25YR event

Inflow = 1.35 cfs @ 12.09 hrs, Volume= 4,720 cf

Primary = 1.35 cfs @ 12.09 hrs, Volume= 4,720 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP2: ANALYSIS POINT 2

Inflow Area = 816,898 sf, 39.51% Impervious, Inflow Depth > 3.37" for 25YR event

Inflow = 28.46 cfs @ 12.40 hrs, Volume= 229,211 cf

Primary = 28.46 cfs @ 12.40 hrs, Volume= 229,211 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP3: ANALYSIS POINT 3

Inflow Area = 55,420 sf, 16.57% Impervious, Inflow Depth > 3.03" for 25YR event

Inflow = 4.43 cfs @ 12.09 hrs, Volume= 14.013 cf

Primary = 4.43 cfs @ 12.09 hrs, Volume= 14,013 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP4: ANALYSIS POINT #4

Inflow Area = 1,691,659 sf, 25.34% Impervious, Inflow Depth > 2.13" for 25YR event

Inflow = 43.69 cfs @ 12.54 hrs, Volume= 300,250 cf

Primary = 43.69 cfs @ 12.54 hrs, Volume= 300,250 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Tc=6.0 min CN=89 Runoff=1.84 cfs 6,239 cf

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 322

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

· · · · · · · · · · · · · · · · · · ·	
SubcatchmentB1: MULTIFAMILYBLDG	Runoff Area=23,255 sf 100.00% Impervious Runoff Depth>8.81" Tc=6.0 min CN=98 Runoff=4.64 cfs 17,082 cf
SubcatchmentB2: MULTIFAMILYBLDG	Runoff Area=17,561 sf 100.00% Impervious Runoff Depth>8.81" Tc=6.0 min CN=98 Runoff=3.50 cfs 12,899 cf
SubcatchmentB3: MULTIFAMILY	Runoff Area=19,981 sf 100.00% Impervious Runoff Depth>8.81" Tc=6.0 min CN=98 Runoff=3.99 cfs 14,677 cf
SubcatchmentC10: CB #10	Runoff Area=6,961 sf 100.00% Impervious Runoff Depth>8.81" Tc=6.0 min CN=98 Runoff=1.39 cfs 5,113 cf
SubcatchmentC11: CB #11	Runoff Area=7,173 sf 100.00% Impervious Runoff Depth>8.81" Tc=6.0 min CN=98 Runoff=1.43 cfs 5,269 cf
SubcatchmentC12: CB #12	Runoff Area=5,238 sf 100.00% Impervious Runoff Depth>8.81" Tc=6.0 min CN=98 Runoff=1.04 cfs 3,847 cf
SubcatchmentC13: CB #13	Runoff Area=10,873 sf 90.78% Impervious Runoff Depth>8.57" Tc=6.0 min CN=96 Runoff=2.16 cfs 7,768 cf
SubcatchmentC14: CB #14	Runoff Area=12,099 sf 86.22% Impervious Runoff Depth>7.97" Tc=6.0 min CN=91 Runoff=2.33 cfs 8,033 cf
SubcatchmentC15: CB #15	Runoff Area=6,666 sf 100.00% Impervious Runoff Depth>8.81" Tc=6.0 min CN=98 Runoff=1.33 cfs 4,896 cf
SubcatchmentC16: CB #16	Runoff Area=8,516 sf 64.88% Impervious Runoff Depth>6.50" Tc=6.0 min CN=79 Runoff=1.43 cfs 4,611 cf
SubcatchmentC17: CB #17	Runoff Area=11,836 sf 73.87% Impervious Runoff Depth>8.09" Tc=6.0 min CN=92 Runoff=2.30 cfs 7,978 cf
SubcatchmentC18: CB #18	Runoff Area=18,591 sf 66.35% Impervious Runoff Depth>7.85" Tc=6.0 min CN=90 Runoff=3.56 cfs 12,155 cf
SubcatchmentC20: CB #20	Runoff Area=11,939 sf 88.95% Impervious Runoff Depth>8.45" Tc=6.0 min CN=95 Runoff=2.36 cfs 8,409 cf
SubcatchmentC21: CB #21	Runoff Area=10,174 sf 87.04% Impervious Runoff Depth>7.85" Tc=6.0 min CN=90 Runoff=1.95 cfs 6,652 cf
SubcatchmentC22: CB #22	Runoff Area=12,001 sf 91.62% Impervious Runoff Depth>8.57" Tc=6.0 min CN=96 Runoff=2.38 cfs 8,574 cf
SubcatchmentC23: CB #23	Runoff Area=9,694 sf 61.00% Impervious Runoff Depth>7.72"

SubcatchmentC39: CB #39

SubcatchmentC40: CB #40

SubcatchmentC41: CB #41

Page 323

SubcatchmentC24: CB #24	Runoff Area=7,930 sf 72.16% Impervious Runoff Depth>8.21" Tc=6.0 min CN=93 Runoff=1.55 cfs 5,426 cf
SubcatchmentC25: CB #25	Runoff Area=8,487 sf 80.92% Impervious Runoff Depth>8.33" Tc=6.0 min CN=94 Runoff=1.67 cfs 5,892 cf
SubcatchmentC26: CB #26	Runoff Area=8,835 sf 63.75% Impervious Runoff Depth>7.97" Tc=6.0 min CN=91 Runoff=1.70 cfs 5,866 cf
SubcatchmentC27: CB #27	Runoff Area=6,111 sf 91.90% Impervious Runoff Depth>8.57" Tc=6.0 min CN=96 Runoff=1.21 cfs 4,366 cf
SubcatchmentC28: CB #28	Runoff Area=10,372 sf 51.33% Impervious Runoff Depth>7.60" Tc=6.0 min CN=88 Runoff=1.95 cfs 6,570 cf
SubcatchmentC29: CB #29	Runoff Area=8,495 sf 84.21% Impervious Runoff Depth>8.33" Tc=6.0 min CN=94 Runoff=1.67 cfs 5,898 cf
SubcatchmentC30: CB #30	Runoff Area=8,933 sf 82.40% Impervious Runoff Depth>8.33" Tc=6.0 min CN=94 Runoff=1.76 cfs 6,202 cf
SubcatchmentC31: CB #31	Runoff Area=16,365 sf 68.64% Impervious Runoff Depth>7.85" Tc=6.0 min CN=90 Runoff=3.13 cfs 10,699 cf
SubcatchmentC32: CB #32	Runoff Area=12,710 sf 70.47% Impervious Runoff Depth>7.97" Tc=6.0 min CN=91 Runoff=2.45 cfs 8,439 cf
SubcatchmentC33: CB #33	Runoff Area=5,421 sf 83.90% Impervious Runoff Depth>8.33" Tc=6.0 min CN=94 Runoff=1.07 cfs 3,764 cf
SubcatchmentC34: CB #34	Runoff Area=8,622 sf 80.51% Impervious Runoff Depth>8.21" Tc=6.0 min CN=93 Runoff=1.69 cfs 5,899 cf
SubcatchmentC35: CB #35	Runoff Area=4,149 sf 98.10% Impervious Runoff Depth>8.81" Tc=6.0 min CN=98 Runoff=0.83 cfs 3,048 cf
SubcatchmentC36: CB #36	Runoff Area=6,622 sf 100.00% Impervious Runoff Depth>8.81" Tc=6.0 min CN=98 Runoff=1.32 cfs 4,864 cf
Subcatchment C38: CB #38	Runoff Area=7,637 sf 100.00% Impervious Runoff Depth>8.81" Tc=6.0 min CN=98 Runoff=1.52 cfs 5,610 cf

Runoff Area=7,612 sf 100.00% Impervious Runoff Depth>8.81"

Runoff Area=4,211 sf 100.00% Impervious Runoff Depth>8.81"

Runoff Area=5,586 sf 100.00% Impervious Runoff Depth>8.81"

Tc=6.0 min CN=98 Runoff=1.52 cfs 5,591 cf

Tc=6.0 min CN=98 Runoff=0.84 cfs 3,093 cf

Tc=6.0 min CN=98 Runoff=1.11 cfs 4,103 cf

Page 324

SubcatchmentC43: CB #43	Runoff Area=3,109 sf	75.36	% Imperv	/ious Runoff D	Depth>7.72"
	Tc=6	3.0 min	CN=89	Runoff=0.59	cfs 2,001 cf

SubcatchmentC44: CB #44	Runoff Area=1,978 sf	84.43% Imper	vious Runoff Depth>8.09"
	Tc=6	.0 min CN=92	Runoff=0.38 cfs 1,333 cf

SubcatchmentC46: CB #46	Runoff Area=4,397 sf 50.97% Impervious Runoff Depth>6.62"
	Tc=6.0 min CN=80 Runoff=0.75 cfs 2.426 cf

SubcatchmentC47: CB #47	Runoff Area=3,012 sf	100.00% Ir	mpervious	Runoff Dep	th>8.81"
	Tc=	6.0 min CN	N=98 Run	off=0.60 cfs	2.212 cf

SubcatchmentC48: CB #48	Runoff Area=60,128 sf	25.16% Impervious	Runoff Depth>5.38"
	Flow Length=400' Tc=11.8	3 min CN=70 Runo	ff=7.16 cfs 26.943 cf

SubcatchmentC49: CB #49	Runoff Area=5,238 sf 84.59% Impervious Runoff Depth>8.33"	
	Tc=6.0 min CN=94 Runoff=1.03 cfs 3.637 cf	

SubcatchmentC50: CB #50	Runoff Area=15,040 sf	77.20% Impe	rvious Runoff Dep	th>8.21"
	Tc=6.0) min CN=93	Runoff=2.94 cfs 1	10.290 cf

SubcatchmentC51: CB #51	Runoff Area=6,823 sf	100.00% Imperv	vious Runoff Depth>8.81"
	Tc=6	6.0 min CN=98	Runoff=1.36 cfs 5,012 cf

SubcatchmentC52: CB#52	Runoff Area=9,052 sf 87.14% Impervious Runoff Depth>8.45"
	Tc=6.0 min CN=95 Runoff=1.79 cfs 6.376 cf

SubcatchmentC53: CB #53	Runoff Area=7,863 sf	86.52% Imperv	ious Runoff Depth>8.21"
	Tc=6.	0 min CN=93	Runoff=1.54 cfs 5,380 cf

SubcatchmentC54: CB #54	Runoff Area=4,821 sf	86.85% I	mpervious	Runoff Depth>7.97"
	Tc=6.	.0 min CI	N=91 Run	off=0.93 cfs 3,201 cf

SubcatchmentC7: CB #5	Runoff Area=4,650 sf	100.009	% Imperv	rious Runoff Dep	th>8.81"
	Tc=	6.0 min	CN=98	Runoff=0.93 cfs	3,416 cf

SubcatchmentC9: CB #9	Runoff Area=16,307 sf 9	93.95% Imper	vious Runoff Depth>8.69"
	Tc=6.0 n	min CN=97	Runoff=3.25 cfs 11,814 cf

SubcatchmentCH1: CLUBHOUSE	Runoff Area=6,262 sf	92.709	% Imperv	ious	Runoff De	pth>8.57"
	Tc=6	.0 min	CN=96	Rund	off=1.24 cfs	4.474 cf

SubcatchmentMB1: MAIL KIOSK	Runoff Area=938 sf	100.00% Imperviou	s Runoff Depth>8.81"
	To	=6.0 min CN=08	Runoff=0.10 cfc 680 cf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 325

SubcatchmentS201: SUMMER STREETRunoff Area=9,943 sf 92.79% Impervious Runoff Depth>8.45"
Tc=6.0 min CN=95 Runoff=1.96 cfs 7,004 cf

SubcatchmentS202: EXISTING WETLANDRunoff Area=432,269 sf 42.08% Impervious Runoff Depth>6.11" Flow Length=856' Tc=23.2 min CN=76 Runoff=44.78 cfs 219,981 cf

SubcatchmentS203: POCKET WETLAND#1 Runoff Area=25,587 sf 0.00% Impervious Runoff Depth>5.63" Tc=6.0 min CN=72 Runoff=3.79 cfs 12,007 cf

SubcatchmentS204: EXISTING

Runoff Area=308,203 sf 31.07% Impervious Runoff Depth>5.98"
Flow Length=632' Tc=22.6 min CN=75 Runoff=31.68 cfs 153,689 cf

SubcatchmentS205: ISOLATEDWETLAND Runoff Area=55,420 sf 16.57% Impervious Runoff Depth>5.38"

Tc=6.0 min CN=70 Runoff=7.86 cfs 24.862 cf

SubcatchmentS206: OVERLANDFLOW Runoff Area=891,295 sf 2.91% Impervious Runoff Depth>4.73" Flow Length=1,467' Tc=34.5 min CN=65 Runoff=60.37 cfs 351,563 cf

SubcatchmentS207: INFILTRATIONPOND Runoff Area=20,803 sf 0.00% Impervious Runoff Depth>7.60" Tc=6.0 min CN=88 Runoff=3.91 cfs 13,178 cf

SubcatchmentS208: GRASS AREA Runoff Area=13,760 sf 9.33% Impervious Runoff Depth>6.00"

Tc=6.0 min CN=75 Runoff=2.16 cfs 6,883 cf

SubcatchmentS209: WETLANDC Runoff Area=107,073 sf 0.38% Impervious Runoff Depth>5.73" Flow Length=607' Slope=0.0150 '/' Tc=28.9 min CN=73 Runoff=9.50 cfs 51,122 cf

SubcatchmentS210: INFILTRATIONPOND Runoff Area=75,890 sf 0.00% Impervious Runoff Depth>6.36" Flow Length=580' Slope=0.0150 '/' Tc=16.5 min CN=78 Runoff=9.35 cfs 40,231 cf

SubcatchmentS211: S211Runoff Area=15,436 sf 47.47% Impervious Runoff Depth>6.50"

Tc=6.0 min CN=79 Runoff=2.59 cfs 8,357 cf

SubcatchmentS212: SWALE

Runoff Area=52,768 sf 0.60% Impervious Runoff Depth>4.75"

Flow Length=418' Tc=23.1 min CN=65 Runoff=4.28 cfs 20,867 cf

SubcatchmentS213: COURTYARD

Runoff Area=21,407 sf 48.10% Impervious Runoff Depth>6.50"

Tc=6.0 min CN=79 Runoff=3.59 cfs 11,590 cf

SubcatchmentT1: Trench Drain 1Runoff Area=11,173 sf 75.10% Impervious Runoff Depth>8.21"
Tc=6.0 min CN=93 Runoff=2.18 cfs 7,644 cf

SubcatchmentT2: Drive Under B2 Runoff Area=4,445 sf 64.30% Impervious Runoff Depth>6.37"
Tc=6.0 min CN=78 Runoff=0.73 cfs 2,361 cf

SubcatchmentTH1: TOWN HOUSE #1 Runoff Area=4,247 sf 92.68% Impervious Runoff Depth>8.57"
Tc=6.0 min CN=96 Runoff=0.84 cfs 3.034 cf

SubcatchmentTH10: TOWN HOUSE #10 Runoff Area=3,476 sf 91.60% Impervious Runoff Depth>8.57"

Tc=6.0 min CN=96 Runoff=0.69 cfs 2,483 cf

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 326

SubcatchmentTH11: TOWN HOUSE #11 Runoff Area=4,210 sf 92.61% Impervious Runoff Depth>8.57" Tc=6.0 min CN=96 Runoff=0.84 cfs 3.008 cf

SubcatchmentTH2: TOWN HOUSE #2 Runoff Area=4,247 sf 92.68% Impervious Runoff Depth>8.57"
Tc=6.0 min CN=96 Runoff=0.84 cfs 3.034 cf

SubcatchmentTH3: TOWN HOUSE #3 Runoff Area=3,013 sf 88.68% Impervious Runoff Depth>8.45"
Tc=6.0 min CN=95 Runoff=0.60 cfs 2.122 cf

SubcatchmentTH4: TOWN HOUSE #4 Runoff Area=3,470 sf 91.59% Impervious Runoff Depth>8.57"

Tc=6.0 min CN=96 Runoff=0.69 cfs 2,479 cf

SubcatchmentTH5: TOWN HOUSE #5 Runoff Area=3,016 sf 88.69% Impervious Runoff Depth>8.45"

Tc=6.0 min CN=95 Runoff=0.60 cfs 2.124 cf

SubcatchmentTH6: TOWN HOUSE #6Runoff Area=3,407 sf 91.46% Impervious Runoff Depth>8.57"
Tc=6.0 min CN=96 Runoff=0.68 cfs 2,434 cf

SubcatchmentTH7: TOWN HOUSE #7 Runoff Area=3,481 sf 91.61% Impervious Runoff Depth>8.57"

Tc=6.0 min CN=96 Runoff=0.69 cfs 2,487 cf

SubcatchmentTH8: TOWN HOUSE #8 Runoff Area=4,212 sf 92.62% Impervious Runoff Depth>8.57"

Tc=6.0 min CN=96 Runoff=0.84 cfs 3,009 cf

SubcatchmentTH9: TOWN HOUSE #9 Runoff Area=3,480 sf 91.61% Impervious Runoff Depth>8.57"

Tc=6.0 min CN=96 Runoff=0.69 cfs 2,486 cf

Reach 8R: OVERLAND FLOWAvg. Flow Depth=0.11' Max Vel=0.12 fps Inflow=1.97 cfs 7,579 cf n=0.400 L=563.0' S=0.0213 '/' Capacity=28.09 cfs Outflow=0.70 cfs 7,164 cf

Reach 9R: OVERLAND FLOWAvg. Flow Depth=0.16' Max Vel=0.27 fps Inflow=1.22 cfs 2,322 cf n=0.400 L=211.0' S=0.0652 '/' Capacity=23.45 cfs Outflow=0.97 cfs 2,322 cf

Reach 10R: OVERLAND FLOWAvg. Flow Depth=0.69' Max Vel=0.47 fps Inflow=10.24 cfs 31,579 cf n=0.400 L=164.0' S=0.0366 '/' Capacity=17.57 cfs Outflow=8.88 cfs 31,578 cf

Reach 11R: 4x4 Open Bottom CulvertAvg. Flow Depth=2.22' Max Vel=2.73 fps Inflow=24.22 cfs 144,775 cf 48.0" x 48.0" Box Pipe n=0.069 L=30.0' S=0.0150 '/' Capacity=42.20 cfs Outflow=24.22 cfs 144,761 cf

Reach 12R: OVERLAND FLOWAvg. Flow Depth=0.16' Max Vel=0.17 fps Inflow=2.22 cfs 8,218 cf n=0.400 L=250.0' S=0.0240 '/' Capacity=29.80 cfs Outflow=1.36 cfs 8,061 cf

Reach 14R: OVERLAND FLOWAvg. Flow Depth=0.18' Max Vel=0.18 fps Inflow=4.27 cfs 20,864 cf n=0.400 L=852.0' S=0.0246 '/' Capacity=31.55 cfs Outflow=1.64 cfs 19,373 cf

Reach 15R: OVERLAND FLOWAvg. Flow Depth=0.21' Max Vel=0.18 fps Inflow=3.51 cfs 20,698 cf n=0.400 L=300.0' S=0.0200 '/' Capacity=27.21 cfs Outflow=1.89 cfs 19,826 cf

Reach 18R: OVERLAND FLOWAvg. Flow Depth=0.26' Max Vel=0.24 fps Inflow=10.22 cfs 22,242 cf n=0.400 L=609.0' S=0.0279 '/' Capacity=38.42 cfs Outflow=3.56 cfs 21,983 cf

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 327

Reach 20R: OVERLAND FLOWAvg. Flow Depth=0.39' Max Vel=0.19 fps Inflow=8.98 cfs 35,788 cf n=0.400 L=560.0' S=0.0093 '/' Capacity=18.54 cfs Outflow=3.75 cfs 34,661 cf

Reach 23R: OVERLAND FLOWAvg. Flow Depth=0.83' Max Vel=0.41 fps Inflow=24.22 cfs 144,761 cf n=0.400 L=237.0' S=0.0211 '/' Capacity=31.93 cfs Outflow=22.58 cfs 143,931 cf

Reach R202: OVERLAND FLOWAvg. Flow Depth=0.75' Max Vel=0.29 fps Inflow=44.78 cfs 219,947 cf

n=0.400 L=700.0' S=0.0107 '/' Capacity=42.56 cfs Outflow=25.82 cfs 212,555 cf

Reach R211: OVERLAND FLOWAvg. Flow Depth=0.88' Max Vel=0.27 fps Inflow=27.69 cfs 57,014 cf n=0.400 L=600.0' S=0.0087 '/' Capacity=14.51 cfs Outflow=11.38 cfs 56,631 cf

Reach SC1: Stream Crossing #1 Avg. Flow Depth=0.59' Max Vel=4.71 fps Inflow=44.78 cfs 219,981 cf 192.0" x 60.0", R=207.0" Arch Pipe n=0.030 L=43.1' S=0.0200 '/' Capacity=722.91 cfs Outflow=44.78 cfs 219,947 cf

Reach SC2: Stream Crossing #2 Avg. Flow Depth=0.13' Max Vel=2.01 fps Inflow=4.28 cfs 20,867 cf 192.0" x 60.0", R=180.0" Arch Pipe n=0.030 L=36.5' S=0.0241 '/' Capacity=768.96 cfs Outflow=4.27 cfs 20,864 cf

Pond 1P: DMH #33 Peak Elev=206.71' Inflow=3.20 cfs 11,616 cf

12.0" Round Culvert n=0.013 L=46.7' S=0.0251 '/' Outflow=3.20 cfs 11,616 cf

Pond 3P: OCS #8 Peak Elev=203.30' Inflow=2.47 cfs 8,581 cf

Outflow=2.47 cfs 8,581 cf

Pond 5R: TRENCH DRAIN Peak Elev=199.24' Inflow=2.18 cfs 7,644 cf

8.0" Round Culvert n=0.012 L=36.0' S=0.0200 '/' Outflow=2.18 cfs 7,644 cf

Pond 11P: YARD DRAIN Peak Elev=207.48' Storage=1,296 cf Inflow=3.59 cfs 11,590 cf

Outflow=2.82 cfs 11,525 cf

Pond CB10: CB #10 Peak Elev=210.78' Inflow=1.39 cfs 5,113 cf

12.0" Round Culvert n=0.013 L=33.8' S=0.0050 '/' Outflow=1.39 cfs 5,113 cf

Pond CB11: CB #11 Peak Elev=210.83' Inflow=1.43 cfs 5,269 cf

12.0" Round Culvert n=0.013 L=26.3' S=0.0103'/' Outflow=1.43 cfs 5,269 cf

Pond CB12: CB #12 Peak Elev=207.24' Inflow=1.04 cfs 3,847 cf 12.0" Round Culvert n=0.013 L=41.3' S=0.0249 '/' Outflow=1.04 cfs 3.847 cf

Pond CB13: CB #13 Peak Elev=207.53' Inflow=2.16 cfs 7,768 cf

12.0" Round Culvert n=0.013 L=43.7' S=0.0249 '/' Outflow=2.16 cfs 7,768 cf

Pond CB14: CB #14 Peak Elev=203.42' Inflow=2.33 cfs 8,033 cf

Pond CB15: CB #15 Peak Elev=203.16' Inflow=1.33 cfs 4,896 cf 12.0" Round Culvert n=0.013 L=15.6' S=0.0051 '/' Outflow=1.33 cfs 4.896 cf

Pond CB16: CB #16 Peak Elev=204.33' Inflow=1.43 cfs 4.611 cf

12.0" Round Culvert n=0.013 L=20.9' S=0.0067'/' Outflow=1.43 cfs 4,611 cf

12.0" Round Culvert n=0.013 L=23.2' S=0.0052 '/' Outflow=2.33 cfs 8,033 cf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 328

Pond CB17: CB #17 Peak Elev=207.31' Inflow=2.30 cfs 7,978 cf 12.0" Round Culvert n=0.013 L=13.8' S=0.0094 '/' Outflow=2.30 cfs 7,978 cf

Pond CB18: CB #18 Peak Elev=207.38' Inflow=3.90 cfs 14,596 cf 15.0" Round Culvert n=0.013 L=25.1' S=0.0052 '/' Outflow=3.90 cfs 14.596 cf

Pond CB20: CB #20 Peak Elev=205.83' Inflow=2.36 cfs 8,409 cf

12.0" Round Culvert n=0.013 L=30.3' S=0.0053 '/' Outflow=2.36 cfs 8,409 cf

Pond CB21: CB #21 Peak Elev=205.70' Inflow=1.95 cfs 6,652 cf 12.0" Round Culvert n=0.013 L=26.0' S=0.0050 '/' Outflow=1.95 cfs 6,652 cf

Pond CB22: CB #22 Peak Elev=206.41' Inflow=2.38 cfs 8,574 cf

12.0" Round Culvert n=0.012 L=16.1' S=0.0050 '/' Outflow=2.38 cfs 8,574 cf

Pond CB23: CB #23 Peak Elev=206.30' Inflow=1.84 cfs 6,239 cf

12.0" Round Culvert n=0.012 L=16.3' S=0.0055 '/' Outflow=1.84 cfs 6,239 cf

Pond CB24: CB #24 Peak Elev=206.37' Inflow=1.55 cfs 5,426 cf

12.0" Round Culvert n=0.012 L=12.1' S=0.0050 '/' Outflow=1.55 cfs 5,426 cf

Pond CB25: CB #25 Peak Elev=206.30' Inflow=1.67 cfs 5,892 cf

15.0" Round Culvert n=0.012 L=11.4' S=0.0053 '/' Outflow=1.67 cfs 5,892 cf

Pond CB26: CB #26 Peak Elev=202.61' Inflow=1.70 cfs 5,866 cf 12.0" Round Culvert n=0.013 L=42.5' S=0.0052'/' Outflow=1.70 cfs 5,866 cf

Pond CB27: CB #27 Peak Elev=201.82' Inflow=1.21 cfs 4,366 cf 12.0" Round Culvert n=0.013 L=18.0' S=0.0056 '/' Outflow=1.21 cfs 4,366 cf

Pond CB28: CB #28 Peak Elev=199.02' Inflow=1.95 cfs 6,570 cf

12.0" Round Culvert n=0.013 L=13.7' S=0.0044 '/' Outflow=1.95 cfs 6,570 cf

Pond CB29: CB #29 Peak Elev=206.87' Inflow=1.67 cfs 5,898 cf

12.0" Round Culvert n=0.013 L=13.5' S=0.0052'/' Outflow=1.67 cfs 5,898 cf

Pond CB30: CB #30 Peak Elev=206.89' Inflow=1.76 cfs 6,202 cf 12.0" Round Culvert n=0.013 L=17.5' S=0.0051 '/' Outflow=1.76 cfs 6,202 cf

Pond CB31; CB #31 Peak Elev=205.84' Inflow=3.13 cfs 10,699 cf

12.0" Round Culvert n=0.013 L=16.4' S=0.0049 '/' Outflow=3.13 cfs 10,699 cf

Pond CB32: CB #32 Peak Elev=205.58' Inflow=2.45 cfs 8,439 cf

Pond CB33: CB #33 Peak Elev=206.39' Inflow=1.07 cfs 3,764 cf 12.0" Round Culvert n=0.013 L=11.7' S=0.0051 '/' Outflow=1.07 cfs 3.764 cf

Pond CB34; CB #34 Peak Elev=206.50' Inflow=1.69 cfs 5,899 cf

12.0" Round Culvert n=0.013 L=16.5' S=0.0048'/' Outflow=1.69 cfs 5,899 cf

12.0" Round Culvert n=0.013 L=16.3' S=0.0049 '/' Outflow=2.45 cfs 8,439 cf

Pond CB53: CB #53

Peak Elev=203.58' Inflow=1.54 cfs 5,380 cf

12.0" Round Culvert n=0.013 L=32.0' S=0.0050'/' Outflow=1.54 cfs 5,380 cf

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 329

11yu100AD® 10.20-30 3/1	102930 © 2023 Hydrochd Soliware Solutions LEC Fage 329
Pond CB35: CB #35	Peak Elev=207.66' Inflow=0.83 cfs 3,048 cf 12.0" Round Culvert n=0.013 L=15.2' S=0.0053 '/' Outflow=0.83 cfs 3,048 cf
Pond CB36: CB #36	Peak Elev=207.78' Inflow=1.32 cfs 4,864 cf 12.0" Round Culvert n=0.013 L=16.1' S=0.0050'/ Outflow=1.32 cfs 4,864 cf
Pond CB38: CB #38	Peak Elev=212.84' Inflow=1.52 cfs 5,610 cf 12.0" Round Culvert n=0.012 L=16.7' S=0.0048'/ Outflow=1.52 cfs 5,610 cf
Pond CB39: CB #39	Peak Elev=212.84' Inflow=1.52 cfs 5,591 cf 12.0" Round Culvert n=0.013 L=16.4' S=0.0049 '/' Outflow=1.52 cfs 5,591 cf
Pond CB40: CB #40	Peak Elev=214.52' Inflow=0.84 cfs 3,093 cf 12.0" Round Culvert n=0.013 L=17.8' S=0.0073'/ Outflow=0.84 cfs 3,093 cf
Pond CB41: CB #41	Peak Elev=214.63' Inflow=1.11 cfs 4,103 cf 12.0" Round Culvert n=0.013 L=18.4' S=0.0049'/ Outflow=1.11 cfs 4,103 cf
Pond CB43: CB #43	Peak Elev=220.71' Inflow=0.59 cfs 2,001 cf 12.0" Round Culvert n=0.013 L=14.9' S=0.0047 '/' Outflow=0.59 cfs 2,001 cf
Pond CB44: CB #44	Peak Elev=220.68' Inflow=0.38 cfs 1,333 cf 12.0" Round Culvert n=0.013 L=14.9' S=0.0047 '/' Outflow=0.38 cfs 1,333 cf
Pond CB45: CB #45	Peak Elev=221.73' Inflow=0.42 cfs 1,360 cf 12.0" Round Culvert n=0.013 L=18.2' S=0.0049 '/' Outflow=0.42 cfs 1,360 cf
Pond CB46: CB #46	Peak Elev=222.06' Inflow=0.75 cfs 2,426 cf 12.0" Round Culvert n=0.013 L=15.3' S=0.0052'/' Outflow=0.75 cfs 2,426 cf
Pond CB47: CB#47	Peak Elev=226.92' Inflow=0.60 cfs 2,212 cf 12.0" Round Culvert n=0.012 L=20.9' S=0.0048'/ Outflow=0.60 cfs 2,212 cf
Pond CB48: CB#48	Peak Elev=228.37' Inflow=7.16 cfs 26,943 cf 15.0" Round Culvert n=0.012 L=16.9' S=0.0047 '/' Outflow=7.16 cfs 26,943 cf
Pond CB49: CB #49	Peak Elev=204.26' Inflow=1.03 cfs 3,637 cf 12.0" Round Culvert n=0.013 L=15.5' S=0.0052 '/' Outflow=1.03 cfs 3,637 cf
Pond CB50: CB #50	Peak Elev=204.79' Inflow=2.94 cfs 10,290 cf 12.0" Round Culvert n=0.013 L=15.3' S=0.0052 '/' Outflow=2.94 cfs 10,290 cf
Pond CB51: CB #51	Peak Elev=204.55' Inflow=1.36 cfs 5,012 cf 12.0" Round Culvert n=0.013 L=31.4' S=0.0051 '/' Outflow=1.36 cfs 5,012 cf
Pond CB52: CB #52	Peak Elev=204.56' Inflow=1.79 cfs 6,376 cf 12.0" Round Culvert n=0.013 L=25.5' S=0.0051 '/' Outflow=1.79 cfs 6,376 cf

Pond CB54: CB #54

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 330

Peak Elev=203.32' Inflow=0.93 cfs 3,201 cf

12.0" Round Culvert n=0.013 L=36.7' S=0.0049 '/' Outflow=0.93 cfs 3,201 cf

Pond CB7: CB#5

Peak Elev=215.06' Inflow=0.93 cfs 3,416 cf

12.0" Round Culvert n=0.012 L=15.1' S=0.0099 '/' Outflow=0.93 cfs 3,416 cf

Pond CB8: CB#8 Peak Elev=215.06' Inflow=1.07 cfs 3,784 cf

12.0" Round Culvert n=0.013 L=15.1' S=0.0099 '/' Outflow=1.07 cfs 3,784 cf

Pond CB9: CB #9 Peak Elev=211.34' Inflow=3.25 cfs 11,814 cf

12.0" Round Culvert n=0.013 L=19.9' S=0.0196'/' Outflow=3.25 cfs 11,814 cf

Pond D10: DMH #10 Peak Elev=204.11' Inflow=1.43 cfs 4,611 cf 12.0" Round Culvert n=0.013 L=15.6' S=0.0051'/' Outflow=1.43 cfs 4.611 cf

Pond D11: DMH #11 Peak Elev=207.27' Inflow=6.20 cfs 22,574 cf 18.0" Round Culvert n=0.013 L=44.6' S=0.0049 '/' Outflow=6.20 cfs 22,574 cf

Pond D12: DMH #12 Peak Elev=205.44' Inflow=4.31 cfs 15,061 cf

12.0" Round Culvert n=0.013 L=41.9' S=0.0050 '/' Outflow=4.31 cfs 15,061 cf

Pond D13: DMH #13 Peak Elev=204.14' Inflow=14.18 cfs 52,717 cf

24.0" Round Culvert n=0.013 L=60.1' S=0.0050 '/' Outflow=14.18 cfs 52,717 cf

Pond D14: DMH #14 Peak Elev=205.77' Inflow=7.44 cfs 26,131 cf

18.0" Round Culvert n=0.012 L=256.3' S=0.0050 '/' Outflow=7.44 cfs 26,131 cf

Pond D16: DMH #16 Peak Elev=206.22' Inflow=3.22 cfs 11,318 cf

15.0" Round Culvert n=0.012 L=103.5' S=0.0050 '/' Outflow=3.22 cfs 11,318 cf

Pond D17: DMH #17 Peak Elev=201.64' Inflow=2.92 cfs 10,232 cf 12.0" Round Culvert n=0.013 L=91.6' S=0.0312 '/' Outflow=2.92 cfs 10,232 cf

Pond D18: DMH #18 Peak Elev=198.76' Inflow=4.87 cfs 16,802 cf

15.0" Round Culvert n=0.013 L=46.3' S=0.0099 '/' Outflow=4.87 cfs 16,802 cf

Pond D19: DMH #19 Peak Elev=206.75' Inflow=3.43 cfs 12,100 cf 12.0" Round Culvert n=0.013 L=82.5' S=0.0092 '/' Outflow=3.43 cfs 12,100 cf

Pond D2: DMH#2 Peak Elev=209.92' Inflow=9.15 cfs 36,354 cf

15.0" Round Culvert n=0.013 L=38.2' S=0.0099 '/' Outflow=9.15 cfs 36,354 cf

Pond D20: DMH #20 Peak Elev=205.57' Inflow=3.43 cfs 12,100 cf

15.0" Round Culvert n=0.013 L=63.5' S=0.0050 '/' Outflow=3.43 cfs 12,100 cf

Pond D21: DMH #21 Peak Elev=205.17' Inflow=13.91 cfs 48,812 cf 24.0" Round Culvert n=0.013 L=72.4' S=0.0050 '/' Outflow=13.91 cfs 48.812 cf

Pond D22: DMH #22 Peak Elev=206.31' Inflow=4.90 cfs 17,574 cf 15.0" Round Culvert n=0.013 L=134.2' S=0.0071 '/' Outflow=4.90 cfs 17,574 cf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 331

Pond D23: DMH #23	Pea	k Elev=207.50'	Inflow=2.15 cfs 7,9	12 cf
	15.0" Round Culvert n=0.013 L=173.3'	S=0.0100 '/'	Outflow=2.15 cfs 7,9	12 cf

Pond D25: DMH #25 Peak Elev=212.68' Inflow=7.14 cfs 25,517 cf

15.0" Round Culvert n=0.012 L=237.6' S=0.0050 '/' Outflow=7.14 cfs 25,517 cf

Pond D27: DMH #27 Peak Elev=214.46' Inflow=4.10 cfs 14,316 cf 15.0" Round Culvert n=0.012 L=63.9' S=0.0150 '/' Outflow=4.10 cfs 14,316 cf

Pond D28: DMH #28 Peak Elev=218.28' Inflow=2.14 cfs 7,120 cf

12.0" Round Culvert n=0.013 L=158.3' S=0.0200 '/' Outflow=2.14 cfs 7,120 cf

Pond D29: DMH #29 Peak Elev=220.65' Inflow=2.14 cfs 7,120 cf 12.0" Round Culvert n=0.013 L=150.9' S=0.0151 '/' Outflow=2.14 cfs 7.120 cf

Pond D30: DMH #30 Peak Elev=221.60' Inflow=1.17 cfs 3,786 cf 12.0" Round Culvert n=0.013 L=184.2' S=0.0050 '/' Outflow=1.17 cfs 3,786 cf

Pond D31: DMH#31 Peak Elev=226.91' Inflow=7.60 cfs 29,155 cf

15.0" Round Culvert n=0.012 L=288.5' S=0.0400 '/' Outflow=7.60 cfs 29,155 cf

Pond D34: DMH #34 Peak Elev=200.07' Inflow=4.64 cfs 17,082 cf 12.0" Round Culvert n=0.012 L=52.0' S=0.0200 '/' Outflow=4.64 cfs 17,082 cf

Pond D35: DMH #35 Peak Elev=213.61' Inflow=4.10 cfs 14,316 cf

15.0" Round Culvert n=0.012 L=171.5' S=0.0150 '/' Outflow=4.10 cfs 14,316 cf

Pond D4: DMH#4 Peak Elev=215.03' Inflow=9.15 cfs 36,354 cf 15.0" Round Culvert n=0.012 L=222.3' S=0.0150 '/' Outflow=9.15 cfs 36,354 cf

Pond D5: DMH #5

Peak Elev=210.62' Inflow=6.07 cfs 22,196 cf
18.0" Round Culvert n=0.013 L=183.0' S=0.0050 '/' Outflow=6.07 cfs 22,196 cf

Pond D6: DMH #6 Peak Elev=209.51' Inflow=6.07 cfs 22,196 cf 18.0" Round Culvert n=0.013 L=299.7' S=0.0050 '/' Outflow=6.07 cfs 22,196 cf

Pond D7: DMH #7

Peak Elev=207.73' Inflow=6.07 cfs 22,196 cf
18.0" Round Culvert n=0.013 L=44.2' S=0.0550 '/' Outflow=6.07 cfs 22,196 cf

Pond D8: DMH #8 Peak Elev=203.04' Inflow=3.66 cfs 12,929 cf

12.0" Round Culvert n=0.013 L=87.7' S=0.0050 '/' Outflow=3.66 cfs 12,929 cf

Pond D9: DMH #9 Peak Elev=201.60' Inflow=3.66 cfs 12,929 cf 12.0" Round Culvert n=0.013 L=11.9' S=0.0050 '/' Outflow=3.66 cfs 12,929 cf

Pond DE61: DRIP #61 Peak Elev=213.59' Storage=372 cf Inflow=0.84 cfs 3,034 cf

Discarded=0.00 cfs 211 cf Primary=0.69 cfs 2,673 cf Outflow=0.69 cfs 2,884 cf

Pond DE62: DRIP #62 Peak Elev=213.59' Storage=372 cf Inflow=0.84 cfs 3,034 cf Discarded=0.00 cfs 211 cf Primary=0.69 cfs 2,673 cf Outflow=0.69 cfs 2,884 cf

Pond OCS7: OCS #7

Page 332

Outflow=3.20 cfs 11,616 cf

Outflow=3.15 cfs 11.388 cf

Peak Elev=204.53' Inflow=3.15 cfs 11,388 cf

Pond DE63: DRIP #63 Peak Elev=208.20' Storage=196 cf Inflow=0.60 cfs 2,122 cf Discarded=0.00 cfs 127 cf Primary=0.52 cfs 1,906 cf Outflow=0.52 cfs 2,032 cf Peak Elev=206.28' Storage=242 cf Inflow=0.69 cfs 2,479 cf Pond DE64: DRIP #64 Discarded=0.00 cfs 149 cf Primary=0.60 cfs 2,224 cf Outflow=0.60 cfs 2,374 cf Pond DE65: DRIP #65 Peak Elev=207.21' Storage=196 cf Inflow=0.60 cfs 2,124 cf Discarded=0.00 cfs 127 cf Primary=0.52 cfs 1,908 cf Outflow=0.52 cfs 2,034 cf Peak Elev=209.07' Storage=240 cf Inflow=0.68 cfs 2,434 cf Pond DE66: DRIP #66 Discarded=0.00 cfs 149 cf Primary=0.58 cfs 2,180 cf Outflow=0.59 cfs 2,329 cf Pond DE67: DRIP #67 Peak Elev=209.28' Storage=242 cf Inflow=0.69 cfs 2,487 cf Discarded=0.00 cfs 149 cf Primary=0.60 cfs 2,232 cf Outflow=0.60 cfs 2,382 cf Pond DE68: DRIP #68 Peak Elev=208.26' Storage=338 cf Inflow=0.84 cfs 3,009 cf Discarded=0.00 cfs 211 cf Primary=0.68 cfs 2,652 cf Outflow=0.68 cfs 2,863 cf Peak Elev=206.78' Storage=242 cf Inflow=0.69 cfs 2,486 cf Pond DE69: DRIP #69 Discarded=0.00 cfs 149 cf Primary=0.60 cfs 2,232 cf Outflow=0.60 cfs 2,381 cf Pond DE70: DRIP #70 Peak Elev=207.18' Storage=242 cf Inflow=0.69 cfs 2,483 cf Discarded=0.00 cfs 149 cf Primary=0.60 cfs 2,229 cf Outflow=0.60 cfs 2,378 cf Pond DE71: DRIP #71 Peak Elev=207.88' Storage=370 cf Inflow=0.84 cfs 3,008 cf Discarded=0.00 cfs 211 cf Primary=0.68 cfs 2,647 cf Outflow=0.69 cfs 2,858 cf Pond DECH: DRIP #CH Peak Elev=210.92' Storage=746 cf Inflow=1.24 cfs 4,474 cf Discarded=0.04 cfs 2,032 cf Primary=1.05 cfs 2,441 cf Outflow=1.08 cfs 4,473 cf Peak Elev=204.19' Inflow=3.97 cfs 13,927 cf Pond DMH32: DMH #32 12.0" Round Culvert n=0.013 L=19.2' S=0.0531 '/' Outflow=3.97 cfs 13,927 cf Peak Elev=196.96' Inflow=9.51 cfs 33,884 cf Pond OCS1: OCS#1 Outflow=9.51 cfs 33.884 cf Pond OCS3: OCS#3 Peak Elev=206.75' Inflow=9.71 cfs 35,473 cf Outflow=9.71 cfs 35.473 cf Pond OCS4: OCS#4 Peak Elev=205.75' Inflow=2.79 cfs 9,535 cf Outflow=2.79 cfs 9,535 cf Pond OCS6: OCS #6 Peak Elev=204.53' Inflow=3.20 cfs 11,616 cf

Pond P204: STORMTECHINFILTRATION Peak Elev=205.68' Storage=11,436 cf Inflow=12.49 cfs 45,008 cf Discarded=0.09 cfs 6,554 cf Primary=8.98 cfs 35,788 cf Outflow=9.06 cfs 42,342 cf

19097	Post-Devel	lopment
-------	------------	---------

Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 333

Printed 6/19/2023

Pond P205: INFILTRATIONPOND #3 Peak Elev=207.05' Storage=9,818 cf Inflow=11.50 cfs 44,711 cf Discarded=0.32 cfs 16,679 cf Primary=10.22 cfs 22,242 cf Outflow=10.54 cfs 38,922 cf

Pond P206: STORMTECHINFILTRATION Peak Elev=196.64' Storage=8,490 cf Inflow=11.69 cfs 41,528 cf Discarded=0.49 cfs 26,156 cf Primary=8.72 cfs 15,367 cf Outflow=9.22 cfs 41,523 cf

Pond P207: INFILTRATIONPOND #2 Peak Elev=198.98' Storage=25,795 cf Inflow=23.17 cfs 83,435 cf Discarded=1.15 cfs 51,829 cf Primary=10.24 cfs 31,579 cf Outflow=11.39 cfs 83,408 cf

Pond P210: POCKET WETLAND#1 Peak Elev=204.66' Storage=20,706 cf Inflow=10.93 cfs 37,524 cf Outflow=3.51 cfs 20,698 cf

Pond P212: INFILTRATIONPOND #1 Peak Elev=203.05' Storage=29,225 cf Inflow=32.18 cfs 135,950 cf Discarded=1.49 cfs 74,502 cf Primary=27.69 cfs 57,014 cf Outflow=29.18 cfs 131,516 cf

Pond P213: Stormtech Infiltration System Peak Elev=204.44' Storage=7,117 cf Inflow=6.35 cfs 23,003 cf Discarded=0.39 cfs 19,325 cf Primary=1.88 cfs 3,677 cf Outflow=2.27 cfs 23,001 cf

Pond P214: STORMTECHINFILTRATION Peak Elev=203.26' Storage=6,493 cf Inflow=6.45 cfs 23,257 cf Discarded=0.61 cfs 20,934 cf Primary=1.22 cfs 2,322 cf Outflow=1.84 cfs 23,256 cf

Link AP1: ANALYSISPOINT 1 Inflow=1.96 cfs 7,004 cf Primary=1.96 cfs 7,004 cf

Link AP2: ANALYSISPOINT 2Inflow=52.47 cfs 394,820 cf
Primary=52.47 cfs 394,820 cf

Link AP3: ANALYSISPOINT 3 Inflow=7.86 cfs 24,862 cf Primary=7.86 cfs 24,862 cf

Link AP4: ANALYSISPOINT #4Inflow=98.54 cfs 593,282 cf
Primary=98.54 cfs 593,282 cf

Total Runoff Area = 2,573,920 sf Runoff Volume = 1,283,918 cf Average Runoff Depth = 5.99" 70.09% Pervious = 1,803,997 sf 29.91% Impervious = 769,923 sf

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 334

Summary for Subcatchment B1: MULTIFAMILY BLDG #1

Runoff = 4.64 cfs @ 12.09 hrs, Volume= 17,082 cf, Depth> 8.81"

Routed to Pond D34: DMH #34

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description					
	20,156	98	Roofs, HSG C					
	3,099	98	Roofs, HSG	D D				
	23,255	98	Weighted Average					
	23,255		100.00% Im	pervious A	Area			
Tc	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment B2: MULTIFAMILY BLDG #2

Runoff = 3.50 cfs @ 12.09 hrs, Volume= 12,899 cf, Depth> 8.81"

Routed to Pond OCS3: OCS#3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

	Area (sf)	CN	Description					
	7,873	98	Roofs, HSC	A S				
	9,688	98	Roofs, HSC	G C				
	17,561	98	98 Weighted Average					
	17,561 10		100.00% Impervious Area					
	c Length		•	Capacity	Description			
<u>(mir</u>	n) (feet)) (ft/1	ft) (ft/sec)	(cfs)				
6.	0				Direct Entry,			

Summary for Subcatchment B3: MULTIFAMILY BUILDING #3

Runoff = 3.99 cfs @ 12.09 hrs, Volume= 14,677 cf, Depth> 8.81" Routed to Pond P214 : STORMTECH INFILTRATION SYSTEM #4

 Area (sf)	CN	Description
608	98	Roofs, HSG A
 19,373	98	Roofs, HSG C
 19,981	98	Weighted Average
19,981		100.00% Impervious Area

Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 335

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment C10: CB #10

Runoff = 1.39 cfs @ 12.09 hrs, Volume= 5,113 cf, Depth> 8.81"

Routed to Pond CB10 : CB #10

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description								
	339	98	Paved road	Paved roads w/curbs & sewers, HSG B							
	5,703	98	Paved road	s w/curbs &	& sewers, HSG C						
	919	98	Paved road	s w/curbs &	& sewers, HSG D						
	6,961	98	Weighted Average								
	6,961		100.00% Impervious Area								
Tc (min)	Length (feet)	Slop (ft/ft	,	Capacity (cfs)	Description						
6.0					Direct Entry,						

Summary for Subcatchment C11: CB #11

Runoff = 1.43 cfs @ 12.09 hrs, Volume= 5,269 cf, Depth> 8.81"

Routed to Pond CB11 : CB #11

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Α	rea (sf)	CN [Description							
	7,173	98 F	Paved roads w/curbs & sewers, HSG C							
	7,173	,	100.00% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
6.0					Direct Entry.					

Summary for Subcatchment C12: CB #12

Runoff = 1.04 cfs @ 12.09 hrs, Volume= 3,847 cf, Depth> 8.81"

Routed to Pond CB12 : CB #12

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 336

A	rea (sf)	CN E	Description						
	5,238	98 F	Paved roads w/curbs & sewers, HSG C						
	5,238	1	100.00% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
6.0					Direct Entry,				

Summary for Subcatchment C13: CB #13

Runoff 2.16 cfs @ 12.09 hrs, Volume= 7,768 cf, Depth> 8.57"

Routed to Pond CB13: CB #13

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

	rea (sf)	CN	Description								
	1,003	74	>75% Grass cover, Good, HSG C								
	7,547	98	Paved park	ing, HSG C	${\tt C}$						
	2,323	98	Roofs, HSC	S C							
	10,873	96	Weighted Average								
	1,003		9.22% Perv	vious Area							
	9,870		90.78% lm <mark></mark>	pervious Ar	rea						
Tc	Length	Slope	,	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
6.0					Direct Entry,						

Summary for Subcatchment C14: CB #14

2.33 cfs @ 12.09 hrs, Volume= Runoff

8,033 cf, Depth> 7.97"

Routed to Pond CB14: CB #14

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Are	ea (sf)	CN	Description						
	1,195	39	>75% Gras	s cover, Go	ood, HSG A				
	7,649	98	Paved park	ing, HSG A	١				
	472	74	>75% Ġras	s cover, Go	ood, HSG C				
	2,783	98	Paved park	ing, HSG C)				
1	12,099	91	Weighted Average						
	1,667		13.78% Per	vious Area	l				
1	10,432		86.22% Imp	ervious Ar	ea				
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 337

Summary for Subcatchment C15: CB #15

Runoff = 1.33 cfs @ 12.09 hrs, Volume=

4,896 cf, Depth> 8.81"

Routed to Pond CB15: CB #15

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description		
	5,000	98	Paved park	ing, HSG A	4
	1,666	98	Paved park	ing, HSG C	
	6,666	98	Weighted A	verage	
	6,666		100.00% Im	npervious A	Area
Tc	Length	Slope	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment C16: CB #16

Runoff = 1.43 cfs @ 12.09 hrs, Volume=

4,611 cf, Depth> 6.50"

Routed to Pond CB16: CB #16

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description						
	2,467	39	>75% Gras	s cover, Go	Good, HSG A				
	4,380	98	Paved park	ing, HSG A	A				
	524	74	>75% Gras	s cover, Go	Good, HSG C				
	1,145	98	Paved park	ing, HSG C	C				
	8,516	79	Weighted A	Weighted Average					
	2,991		35.12% Per	35.12% Pervious Area					
	5,525		64.88% Imp	ervious Ar	ırea				
_									
Tc	Length	Slop	•	Capacity	•				
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment C17: CB #17

Runoff = 2.30 cfs @ 12.09 hrs, Volume= 7,978 cf, Depth> 8.09"

Routed to Pond CB17: CB #17

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 338

	Aı	rea (sf)	CN	Description						
		3,093	74	>75% Gras	s cover, Go	Good, HSG C				
		8,743	98	Paved park	ing, HSG C	C				
		11,836	92	Weighted A	verage					
		3,093		26.13% Pervious Area						
		8,743		73.87% Imp	pervious Ar	rea				
	_		0.1			B				
	Tc	Length	Slope	,	Capacity	•				
_	(min)	(feet)	(ft/ft	t) (ft/sec) (cfs)						
	6.0					Direct Entry				

Summary for Subcatchment C18: CB #18

Runoff = 3.56 cfs @ 12.09 hrs, Volume= 12,155 cf, Depth> 7.85"

Routed to Pond CB18: CB #18

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Aı	rea (sf)	CN	Description					
	6,255	74	>75% Gras	s cover, Go	ood, HSG C			
	12,336	98	Paved park	ing, HSG C	C			
	18,591	90	Weighted Average					
	6,255		33.65% Pervious Area					
	12,336		66.35% Imp	pervious Ar	rea			
т.	l 4l-	Class.	\	0	Description			
Tc	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C20: CB #20

Runoff = 2.36 cfs @ 12.09 hrs, Volume= 8,409 cf, Depth> 8.45"

Routed to Pond CB20: CB #20

Area(sf) CN	Description						
3,3	19 98	Paved parking, HSG A						
1,3	19 74	>75% Grass cover, Good, HSG C						
7,3	01 98	Paved parking, HSG C						
11,9	39 95	Weighted Average						
1,3	19	11.05% Pervious Area						
10,6	20	88.95% Impervious Area						

Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 339

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry

Direct Entry,

Summary for Subcatchment C21: CB #21

Runoff = 1.95 cfs @ 12.09 hrs, Volume=

6,652 cf, Depth> 7.85"

Routed to Pond CB21: CB #21

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

	Area (sf)	CN	Description					
	1,319	39	>75% Gras	s cover, Go	Good, HSG A			
	7,301	98	Paved park	ing, HSG A	A			
	1,554	98	Paved park	ing, HSG C	C			
	10,174	90	Weighted A	verage				
	1,319		12.96% Per	rvious Area	a			
	8,855		87.04% Imp	pervious Ar	ırea			
-		01		0 "	D			
Tc	9	Slope	,	Capacity	•			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry.			

Summary for Subcatchment C22: CB #22

Runoff = 2.38 cfs @ 12.09 hrs, Volume=

8,574 cf, Depth> 8.57"

Routed to Pond CB22 : CB #22

Area	(sf)	CN	Description					
2,	946	98	Paved park	ing, HSG A	١			
	177	74	>75% Ġras	s cover, Go	ood, HSG C			
2,	641	98	Paved park	ing, HSG C	;			
	829	80	>75% Ġras	s cover, Go	ood, HSG D			
5,	408	98	Paved park	ing, HSG D)			
12,	001	96	Weighted Average					
1,0	006		8.38% Perv	ious Area				
10,	995	!	91.62% Imp	pervious Ar	ea			
	ength	Slope	,	Capacity	Description			
(min)(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 340

Summary for Subcatchment C23: CB #23

Runoff = 1.84 cfs @ 12.09 hrs, Volume= 6,239 cf, Depth> 7.72"

Routed to Pond CB23: CB #23

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description					
	242	39	>75% Gras	s cover, Go	ood, HSG A			
	3,016	98	Paved park	ing, HSG A	١			
	1,267	74	>75% Gras	s cover, Go	ood, HSG C			
	218	98	Paved park	ing, HSG C				
	2,272	80	>75% Gras	s cover, Go	ood, HSG D			
	2,679	98	Paved park	ing, HSG [)			
	9,694	89	Weighted A	verage				
	3,781		39.00% Per	vious Area	1			
	5,913		61.00% Imp	ervious Ar	ea			
Tc	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C24: CB #24

Runoff = 1.55 cfs @ 12.09 hrs, Volume= 5,426 cf, Depth> 8.21"

Routed to Pond CB24: CB #24

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description					
	5,722	98	Paved park	ing, HSG D				
	2,208	80	>75% Gras	s cover, Go	od, HSG D			
	7,930	93	Weighted Average					
	2,208		27.84% Pervious Area					
	5,722	,	72.16% lm <mark></mark>	pervious Ar	ea			
_				_				
Tc	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C25: CB #25

Runoff = 1.67 cfs @ 12.09 hrs, Volume= 5,892 cf, Depth> 8.33"

Routed to Pond CB25 : CB #25

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 341

A	rea (sf)	CN	Description					
	211	39	>75% Gras	s cover, Go	ood, HSG A			
	519	98	Paved park	ing, HSG A				
	15	74	>75% Gras	s cover, Go	ood, HSG C			
	300	98	Paved park	ing, HSG C				
	1,393	80	>75% Gras	s cover, Go	ood, HSG D			
	6,049	98	Paved park	ing, HSG D				
	8,487	94	Weighted A	verage				
	1,619		19.08% Per	vious Area				
	6,868		80.92% Imp	ervious Ar	ea			
Tc	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C26: CB #26

Runoff = 1.70 cfs @ 12.09 hrs, Volume=

5,866 cf, Depth> 7.97"

Routed to Pond CB26: CB #26

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description					
	3,203	80	>75% Gras	s cover, Go	ood, HSG D			
	5,632	98	Paved park	ing, HSG D)			
	8,835	91	Veighted Average					
	3,203	;	36.25% Pervious Area					
	5,632	(33.75% Imp	ervious Ar	rea			
т.	1 41-	Class a	\/-l:t	0	Decements			
Tc	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C27: CB #27

Runoff = 1.21 cfs @ 12.09 hrs, Volume= 4,366 cf, Depth> 8.57"

Routed to Pond CB27: CB #27

Area (sf)	CN	Description			
98	39	>75% Grass cover, Good, HSG A			
131	98	Paved parking, HSG A			
397	80	>75% Grass cover, Good, HSG D			
5,485	98	Paved parking, HSG D			
6,111	96	Weighted Average			
495		8.10% Pervious Area			
5,616		91.90% Impervious Area			

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 342

(r	nin)	_	•	(ft/sec)	(cfs)	Description	
	6.0	,	, ,	,	, ,	Direct Entry,	

Summary for Subcatchment C28: CB #28

Runoff = 1.95 cfs @ 12.09 hrs, Volume=

6,570 cf, Depth> 7.60"

Routed to Pond CB28: CB #28

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description						
	2,751	74	>75% Grass cover, Good, HSG C						
	2,841	98	Paved parking, HSG C						
	2,297	80	>75% Grass cover, Good, HSG D						
	2,483	98	Paved parking, HSG D						
	10,372	88	Weighted Average						
	5,048		48.67% Pervious Area						
	5,324		51.33% Impervious Area						
_									
Tc	Length	Slop							
(min)	(feet)	(ft/f	ft) (ft/sec) (cfs)						
6.0			Direct Entry,						

Summary for Subcatchment C29: CB #29

Runoff = 1.67 cfs @ 12.09 hrs, Volume= 5,898 cf, Depth> 8.33"

Routed to Pond CB29: CB #29

A	rea (sf)	CN	Description						
	1,341	74	>75% Gras	s cover, Go	Good, HSG C				
	5,330	98	Paved park	ing, HSG C	C				
	1,824	98	Roofs, HSC	S Č					
	8,495	94	Weighted Average						
	1,341		15.79% Pe	rvious Area	a				
	7,154		34.21% lmp	pervious Ar	rea				
Tc	Length	Slope	,	Capacity	Description				
(min)_	(feet)	(ft/ft)	(ft/sec) (cfs)						
6.0					Direct Entry,				

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 343

Summary for Subcatchment C30: CB #30

Runoff = 1.76 cfs @ 12.09 hrs, Volume=

6,202 cf, Depth> 8.33"

Routed to Pond CB30: CB #30

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description						
	1,572	74	>75% Gras	s cover, Go	ood, HSG C				
	6,310	98	Paved park	ing, HSG C	C				
	1,051	98	Roofs, HSC	S Č					
	8,933	94	Weighted A	verage					
	1,572		17.60% Pei	rvious Area	a				
	7,361		82.40% lmp	pervious Ar	rea				
Tc	Length	Slope	,	Capacity	Description				
(min)	(feet)	(ft/ft)	ft) (ft/sec) (cfs)						
6.0					Direct Entry,				

Summary for Subcatchment C31: CB #31

Runoff = 3.13 cfs @ 12.09 hrs, Volume= 10,699 cf, Depth> 7.85"

Routed to Pond CB31: CB #31

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

	rea (sf)	CN	Description						
	5,132	74	>75% Gras	s cover, Go	ood, HSG C				
	9,132	98	Paved park	ing, HSG C	C				
	2,101	98	Roofs, HSC	S Č					
	16,365	90	Weighted A	verage					
	5,132		31.36% Pei	rvious Area	a				
	11,233		68.64% Imp	pervious Ar	rea				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment C32: CB #32

Runoff = 2.45 cfs @ 12.09 hrs, Volume= 8,439 cf, Depth> 7.97"

Routed to Pond CB32 : CB #32

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 344

Area (sf)	CN	Description	Description					
3,753	74	>75% Gras	s cover, Go	ood, HSG C				
7,068	98	Paved park	ing, HSG C	\circ				
1,889	98	Roofs, HSC	S Č					
12,710	91	Weighted A	verage					
3,753		29.53% Per	vious Area	a				
8,957		70.47% lmp	ervious Ar	rea				
Tc Length	Slop	oe Velocity	Capacity	Description				
(min) (feet)	(ft/	,	(cfs)	2000/ipileti				
6.0	•			Direct Entry,				

Summary for Subcatchment C33: CB #33

Runoff = 1.07 cfs @ 12.09 hrs, Volume= 3,764

3,764 cf, Depth> 8.33"

Routed to Pond CB33: CB #33

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description					
	873	74	>75% Gras	s cover, Go	lood, HSG C			
	3,693	98	Paved park	ing, HSG C	C			
	855	98	Roofs, HSC	G C				
	5,421 873 4,548		Weighted Average 16.10% Pervious Area 83.90% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	•			
6.0					Direct Entry,			

Summary for Subcatchment C34: CB #34

Runoff = 1.69 cfs @ 12.09 hrs, Volume= 5,899 cf, Depth> 8.21"

Routed to Pond CB34 : CB #34

Area (sf)	CN	Description			
1,680	74	>75% Grass cover, Good, HSG C			
5,115	98	Paved parking, HSG C			
1,827	98	Roofs, HSG C			
8,622	93	Weighted Average			
1,680		19.49% Pervious Area			
6,942		80.51% Impervious Area			

Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 345

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•	

6.0 Direct Entry,

Summary for Subcatchment C35: CB #35

Runoff = 0.83 cfs @ 12.09 hrs, Volume= 3,048 cf, Depth> 8.81"

Routed to Pond CB35 : CB #35

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

	Area (sf)	CN	Description					
	3,578	98	Paved park	ing, HSG C	С			
	79	80	>75% Gras	s cover, Go	Good, HSG D			
	492	98	Paved park	ing, HSG D	D			
	4,149	98	Weighted A	verage				
	79		1.90% Perv	rious Area				
	4,070		98.10% Imp	ervious Ar	ırea			
To	Length	Slope	e Velocity	Capacity	/ Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0)				Direct Entry.			

Summary for Subcatchment C36: CB #36

Runoff = 1.32 cfs @ 12.09 hrs, Volume= 4,864 cf, Depth> 8.81"

Routed to Pond CB36 : CB #36

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

	Α	rea (sf)	CN [CN Description					
		6,622	98 F	98 Paved parking, HSG C					
		6,622	1	100.00% Impervious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
_	6.0		•			Direct Entry,			

Summary for Subcatchment C38: CB #38

Runoff = 1.52 cfs @ 12.09 hrs, Volume= 5,610 cf, Depth> 8.81"

Routed to Pond CB38: CB #38

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 346

A	rea (sf)	CN	Description		
	6,647	98	Paved park	ing, HSG B	3
	392	98	Paved park	ing, HSG C	
	598	98	Paved park	ing, HSG D)
	7,637	98	Weighted A	verage	
	7,637		100.00% Im	pervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment C39: CB #39

Runoff 1.52 cfs @ 12.09 hrs, Volume= 5,591 cf, Depth> 8.81"

Routed to Pond CB39: CB #39

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Ar	ea (sf)	CN	Description		
	6,505	98	Paved park	ing, HSG B	В
	519	98	Paved park	ing, HSG C	C
	588	98	Paved park	ing, HSG D	D
	7,612	98	Weighted A	verage	
	7,612		100.00% Im	npervious A	Area
Tc (min)	Length (feet)	Slop (ft/f	,	Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment C40: CB #40

Runoff 0.84 cfs @ 12.09 hrs, Volume= 3,093 cf, Depth> 8.81"

Routed to Pond CB40: CB #40

A	rea (sf)	CN [Description		
	4,211	98 F	Paved park	ing, HSG B	}
	4,211	1	00.00% In	npervious A	ırea
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 347

Summary for Subcatchment C41: CB #41

Runoff = 1.11 cfs @ 12.09 hrs, Volume= 4

4,103 cf, Depth> 8.81"

Routed to Pond CB41: CB #41

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN [Description		
	5,586	98 F	Paved park	ing, HSG B	3
	5,586	1	00.00% In	pervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment C43: CB #43

Runoff = 0.59 cfs @ 12.09 hrs, Volume= 2,001 cf, Depth> 7.72"

Routed to Pond CB43: CB #43

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description		
	2,343	98	Paved park	ing, HSG E	В
	766	61	>75% Ġras	s cover, Go	ood, HSG B
	3,109	89	Weighted A	verage	
	766		24.64% Pei	rvious Area	a
	2,343		75.36% lmp	pervious Ar	rea
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment C44: CB #44

Runoff = 0.38 cfs @ 12.09 hrs, Volume= 1,333 cf, Depth> 8.09"

Routed to Pond CB44: CB #44

Area (sf)	CN	Description
1,670	98	Paved parking, HSG B
308	61	>75% Grass cover, Good, HSG B
1,978	92	Weighted Average
308		15.57% Pervious Area
1,670		84.43% Impervious Area

Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 348

•	6.0	· /		, ,		Direct Entry,	_
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	Tc	Length	Slope	Velocity	Capacity	Description	

Summary for Subcatchment C45: CB #45

0.42 cfs @ 12.09 hrs, Volume=

1,360 cf, Depth> 6.62"

Routed to Pond CB45: CB #45

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description		
	1,240	98	Paved park	ing, HSG E	В
	1,225	61	>75% Gras	s cover, Go	lood, HSG B
	2,465	80	Weighted A	verage	
	1,225		49.70% Pei	rvious Area	a
	1,240	:	50.30% lmp	pervious Ar	rea
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	·
6.0	(ICCI)	(11/11)	(10/360)	(013)	Direct Entry,
0.0					Direct Entry,

Summary for Subcatchment C46: CB #46

0.75 cfs @ 12.09 hrs, Volume= 2,426 cf, Depth> 6.62" Runoff

Routed to Pond CB46: CB #46

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description		
	2,241	98	Paved park	ing, HSG E	В
	2,156	61	>75% Gras	s cover, Go	Good, HSG B
	4,397	80	Weighted A	verage	
	2,156		49.03% Pe	rvious Area	a
	2,241		50.97% lmp	pervious Ar	rea
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	• • • • • • • • • • • • • • • • • • •
6.0					Direct Entry,

Summary for Subcatchment C47: CB #47

Runoff 0.60 cfs @ 12.09 hrs, Volume= 2,212 cf, Depth> 8.81"

Routed to Pond CB47: CB#47

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 349

_	A	rea (sf)	CN E	Description		
		3,012	98 F	aved road	s w/curbs &	& sewers, HSG B
		3,012	1	00.00% In	pervious A	rea
	т.	1 41-	Ol	\	0	Description
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	6.0	(1301)	(1011)	(14,000)	(010)	Direct Entry,

Summary for Subcatchment C48: CB #48

Runoff = 7.16 cfs @ 12.16 hrs, Volume=

26,943 cf, Depth> 5.38"

Routed to Pond CB48: CB#48

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

	Α	rea (sf)	CN E	escription		
		3,877	98 F	aved road	s w/curbs &	& sewers, HSG B
		56,251	68 1	acre lots,	20% imp, I	HSG B
		60,128	70 V	Veighted A	verage	
		45,001	7	4.84% Per	vious Area	
		15,127	2	5.16% Imp	ervious Ar	ea
	Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.0	50	0.0800	0.12		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.27"
	4.8	350	0.0600	1.22		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
·	11.8	400	Total	•		

Summary for Subcatchment C49: CB #49

Runoff = 1.03 cfs @ 12.09 hrs, Volume= 3,637 cf, Depth> 8.33"

Routed to Pond CB49: CB #49

 Area (sf)	CN	Description
4,431	98	Paved roads w/curbs & sewers, HSG C
 807	74	>75% Grass cover, Good, HSG C
5 000		
5,238	94	Weighted Average
5,238 807	94	Weighted Average 15.41% Pervious Area

Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 350

	_	•	•	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.0					Direct Entry,

Summary for Subcatchment C50: CB #50

Runoff = 2.94 cfs @ 12.09 hrs, Volume= 10,290 cf, Depth> 8.21"

Routed to Pond CB50 : CB #50

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Ar	ea (sf)	CN	Description					
	3,429	74	>75% Gras	s cover, Go	ood, HSG C			
1	11,611	98	98 Paved roads w/curbs & sewers, HSG C					
	15,040	93	Weighted A	verage				
	3,429 22.80% Pervious Area			vious Area	l .			
1	11,611		77.20% Imp	ervious Ar	rea			
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment C51: CB #51

Runoff = 1.36 cfs @ 12.09 hrs, Volume= 5,012 cf, Depth> 8.81"

Routed to Pond CB51: CB #51

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Aı	rea (sf)	CN	Description					
	3,147	98	Roofs, HSG C					
	3,676	98	Paved parking, HSG C					
	6,823	98	98 Weighted Average					
	6,823		100.00% Impervious Area					
Тс	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
6.0					Direct Entry,			
	Tc (min)	3,147 3,676 6,823 6,823 Tc Length (min) (feet)	3,147 98 3,676 98 6,823 98 6,823 Tc Length Slope (min) (feet) (ft/ft	3,147 98 Roofs, HSG 3,676 98 Paved park 6,823 98 Weighted A 6,823 100.00% In Tc Length Slope Velocity (min) (feet) (ft/ft) (ft/sec)	3,147 98 Roofs, HSG C 3,676 98 Paved parking, HSG C 6,823 98 Weighted Average 6,823 100.00% Impervious A Tc Length Slope Velocity Capacity (min) (feet) (ft/ft) (ft/sec) (cfs)			

Summary for Subcatchment C52: CB#52

Runoff = 1.79 cfs @ 12.09 hrs, Volume= 6,376 cf, Depth> 8.45"

Routed to Pond CB52 : CB #52

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 351

	rea (sf)	CN	Description					
	1,164	74	>75% Grass cover, Good, HSG C					
	7,888	98	Paved parking, HSG C					
	9,052	2 95 Weighted Average						
	1,164		12.86% Pervious Area					
	7,888		87.14% lmp	pervious Ar	ea			
Tc	J	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry			

Direct Entry,

Summary for Subcatchment C53: CB #53

Runoff = 1.54 cfs @ 12.09 hrs, Volume= 5,380 cf, Depth> 8.21"

Routed to Pond CB53: CB #53

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description					
	287	39	>75% Gras	s cover, Go	Good, HSG A			
	3,287	98	Paved park	ing, HSG A	A			
	773	74	>75% Gras	>75% Grass cover, Good, HSG C				
	3,516	98	Paved park	ing, HSG C	C			
	7,863	93	93 Weighted Average					
	1,060		13.48% Pervious Area					
	6,803		86.52% Imp	pervious Ar	ırea			
Tc	Length	Slope	,	Capacity	·			
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Summary for Subcatchment C54: CB #54

Runoff = 0.93 cfs @ 12.09 hrs, Volume= 3,201 cf, Depth> 7.97"

Routed to Pond CB54 : CB #54

_	Area (sf)	CN	Description		
	550	39	>75% Grass cover, Good, HSG A		
	4,176 98 Paved parking, HSG A				
	84	74	>75% Grass cover, Good, HSG C		
_	11	98	Paved parking, HSG C		
	4,821	91	Weighted Average		
	634		13.15% Pervious Area		
	4,187		86.85% Impervious Area		

Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 352

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment C7: CB #5

Runoff = 0.93 cfs @ 12.09 hrs, Volume= 3,416

3,416 cf, Depth> 8.81"

Routed to Pond CB7: CB#5

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN E	Description					
	4,650	98 F	98 Paved roads w/curbs & sewers, HSG B					
	4,650	1	100.00% Impervious Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
6.0					Direct Entry,			

Summary for Subcatchment C8: CB #8

Runoff = 1.07 cfs @ 12.09 hrs, Volume= 3,784 cf, Depth> 8.33"

Routed to Pond CB8: CB#8

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

_	Aı	rea (sf)	CN [Description					
		4,837	98 F	Paved roads w/curbs & sewers, HSG B					
		613	61 >	>75% Grass cover, Good, HSG B					
		5,450	94 \	Veighted A	verage				
		613	•	11.25% Pervious Area					
		4,837	3	38.75% Imp	pervious Ar	rea			
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•			
	6.0					Direct Entry.			

Summary for Subcatchment C9: CB #9

Runoff = 3.25 cfs @ 12.09 hrs, Volume= 11,814 cf, Depth> 8.69"

Routed to Pond CB9: CB #9

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 353

Are	a (sf)	CN Description						
	31	61	>75% Grass	s cover, Go	ood, HSG B			
	433	98	Paved road	s w/curbs &	& sewers, HSG B			
	904	74	>75% Grass	s cover, Go	ood, HSG C			
12	2,073	98	Paved park	ing, HSG C	;			
	2,305	98 Roofs, HSG C						
	52	80 >75% Grass cover, Good, HSG D						
	509	98	Paved park	ing, HSG D)			
16	6,307	97	Weighted A	verage				
	987		6.05% Perv	ious Area				
1:	5,320		93.95% Imp	ervious Ar	ea			
Tc l	₋ength	Slop	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)				
6.0					Direct Entry.			

Summary for Subcatchment CH1: CLUBHOUSE

Runoff = 1.24 cfs @ 12.09 hrs, Volume= 4,474 cf, Depth> 8.57"

Routed to Pond DECH: DRIP #CH

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description				
•	5,802	98	Roofs, HSG	G C			
	3	98	Roofs, HSC	B D			
	435	74	74 >75% Grass cover, Good, HSG C				
	22	80	80 >75% Grass cover, Good, HSG D				
	6,262	2 96 Weighted Average					
	457		7.30% Perv	ious Area			
	5,805		92.70% lmp	pervious Ar	ea		
_				_			
Tc	Length	Slope	•	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
6.0					Direct Entry,		

Summary for Subcatchment MB1: MAIL KIOSK

Runoff = 0.19 cfs @ 12.09 hrs, Volume= 689 cf, Depth> 8.81"

Routed to Link AP2: ANALYSIS POINT 2

 Area (sf)	CN	Description
938	98	Roofs, HSG B
938		100.00% Impervious Area

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 354

				•		Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
•	6.0					Direct Entry,	

Summary for Subcatchment S201: SUMMER STREET ACCESS APRON

1.96 cfs @ 12.09 hrs, Volume= Runoff

7,004 cf, Depth> 8.45"

Routed to Link AP1: ANALYSIS POINT 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description						
	717	61	>75% Grass cover, Good, HSG B						
	9,226	98	Paved park	Paved parking, HSG B					
	9,943	95	Weighted A	Weighted Average					
	717		7.21% Perv	rious Area					
	9,226		92.79% Impervious Area						
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description				
6.0					Direct Entry,				

Direct Entry,

Summary for Subcatchment S202: EXISTING WETLAND

Runoff 44.78 cfs @ 12.31 hrs, Volume=

219,981 cf, Depth> 6.11"

Routed to Reach SC1: Stream Crossing #1

Area (sf)	CN	Description
136,496	61	>75% Grass cover, Good, HSG B
83,935	55	Woods, Good, HSG B
29	98	Paved parking, HSG B
13,946	98	Roofs, HSG B
9,038	48	Brush, Good, HSG B
2,573	74	>75% Grass cover, Good, HSG C
17,121	70	Woods, Good, HSG C
98	98	Paved parking, HSG C
1,097	65	Brush, Good, HSG C
126	80	>75% Grass cover, Good, HSG D
132	98	Paved parking, HSG D
167,678	98	Water Surface, HSG D
432,269	76	Weighted Average
250,386		57.92% Pervious Area
181,883		42.08% Impervious Area

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 355

	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	5.6	50	0.0200	0.15		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.27"
	1.4	118	0.0400	1.40		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	16.2	688	0.0200	0.71		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
_	23.2	856	Total			

Summary for Subcatchment S203: POCKET WETLAND #1

Runoff = 3.79 cfs @ 12.09 hrs, Volume=

12,007 cf, Depth> 5.63"

Routed to Pond p210: POCKET WETLAND #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Area (sf)	CN	Description				
12,682	61	>75% Grass cover, Good, HSG B				
1,060	98	Water Surface, 0% imp, HSG B				
7,785	74	>75% Grass cover, Good, HSG C				
4,060	98	Water Surface, 0% imp, HSG C				
25,587	72	Weighted Average				
25,587		100.00% Pervious Area				
Tc Length	Slop	· · · · · · · · · · · · · · · · · · ·				
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)				
6.0		Direct Entry,				

Summary for Subcatchment S204: EXISTING WETLANDS

Runoff = 31.68 cfs @ 12.31 hrs, Volume= 153,6 Routed to Link ap2 : ANALYSIS POINT 2

153,689 cf, Depth> 5.98"

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 356

A	rea (sf)	CN E	CN Description						
	53,739	61 >	61 >75% Grass cover, Good, HSG B						
	17,975								
	20,940	48 E	Brush, Goo	d, HSG B					
	41,421	74 >	·75% Gras	s cover, Go	ood, HSG C				
	68,342	70 V	Voods, Go	od, HSG C					
	116			ing, HSG C					
	1,904		Brush, Goo						
	1,528		Brush, Goo	•					
	2,508		•	od, HSG D					
	161			ing, HSG D)				
	4,073		Brush, Goo						
	95,496			ace, HSG D)				
	808,203		Veighted A						
	12,430	_	68.93% Pervious Area						
	95,773	3	1.07% lmp	pervious Ar	ea				
_		01							
Tc	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
3.2	50	0.2000	0.26		Sheet Flow,				
					Grass: Dense n= 0.240 P2= 3.27"				
19.4	582	0.0100	0.50		Shallow Concentrated Flow,				
					Woodland Kv= 5.0 fps				
22.6	632	Total							

Summary for Subcatchment S205: ISOLATED WETLAND

Runoff = 7.86 cfs @ 12.09 hrs, Volume= 24,862 cf, Depth> 5.38"

Routed to Link AP3: ANALYSIS POINT 3

Area (sf)	CN	Description				
10,910	30	Woods, Good, HSG A				
3,684	74	>75% Grass cover, Good, HSG C				
2,275	70	Woods, Good, HSG C				
171	98	Paved parking, HSG C				
1,706	65	Brush, Good, HSG C				
1,940	80	>75% Grass cover, Good, HSG D				
23,513	77	Woods, Good, HSG D				
393	98	Paved parking, HSG D				
2,208	73	Brush, Good, HSG D				
8,620	98	Water Surface, HSG D				
55,420 46,236	70	Weighted Average 83.43% Pervious Area				
9,184		16.57% Impervious Area				

6.0

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 357

Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	-

Direct Entry,

Summary for Subcatchment S206: OVERLAND FLOW

Runoff = 60.37 cfs @ 12.48 hrs, Volume=

351,563 cf, Depth> 4.73"

Routed to Link AP4: ANALYSIS POINT #4

Aı	rea (sf)	CN Description						
	16,514	39 >	75% Gras	s cover, Go	ood, HSG A			
	18,226			od, HSG A				
	713			ing, HSG A				
	41,148		Brush, Goo					
	17,568		•	20% imp, I	HSG A			
	37,410				ood, HSG B			
	13,900			od, HSG B	,			
	54,538		Brush, Goo					
	91,202		•	20% imp, I	HSG B			
	77,444				ood, HSG C			
1	14,763			od, HSG C				
	3,493	98 F	aved park	ing, HSG C				
	57,740	65 E	Brush, Goo	d, HSG C				
	5,763	80 >	75% Gras	s cover, Go	ood, HSG D			
1	26,141	77 V	Voods, Go	od, HSG D				
1	14,732	98 V	Vater Surfa	ace, 0% imp	p, HSG D			
8	91,295	65 V	Veighted A	verage				
8	65,335	9	7.09% Per	vious Area				
	25,960	2	91% Impe	ervious Are	а			
_		٥.			—			
Tc	Length	Slope	•	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
8.5	50	0.0500	0.10		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.27"			
5.0	334	0.0250	1.11		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
1.9	91	0.0250	0.79		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
10.2	491	0.0400	0.80		Shallow Concentrated Flow, BRUSH			
0.0	50 4	0.0050	0.04		Kv= 4.0 fps			
8.9	501	0.0350	0.94		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
34.5	1,467	Total						

Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 358

Summary for Subcatchment S207: INFILTRATION POND #2

Runoff = 3.91 cfs @ 12.09 hrs, Volume=

13,178 cf, Depth> 7.60"

Routed to Pond P207: INFILTRATION POND #2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Area	(sf) CN	Description	Description					
3	339 98	Water Surfa	ace, 0% im	o, HSG A				
8,8	302 74	>75% Gras	s cover, G	ood, HSG C				
11,1	162 98	Water Surfa	ace, 0% im	o, HSG C				
20,8	303 88	88 Weighted Average						
20,8	303	100.00% P	ervious Are	а				
	ngth Slo	,	Capacity	Description				
(min) (1	eet) (ft/	ft) (ft/sec)	(cfs)					
6.0				Direct Entry.				

Summary for Subcatchment S208: GRASS AREA

Runoff = 2.16 cfs @ 12.09 hrs, Volume=

6,883 cf, Depth> 6.00"

Routed to Pond OCS4: OCS#4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Are	ea (sf)	CN I	Description						
	476	39 >	>75% Gras	s cover, Go	lood, HSG A				
1	2,000	74 >	>75% Gras	s cover, Go	lood, HSG C				
	168	98 F	Paved park	ing, HSG A	A				
	1,116	98 I	Paved park	ing, HSG C	C				
1	3,760	75 \	Weighted Average						
1	2,476	(90.67% Per	vious Area	a				
	1,284	(9.33% Impe	ervious Area	ea				
	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
6.0					Direct Entry,				

Summary for Subcatchment S209: WETLAND C

Runoff = 9.50 cfs @ 12.40 hrs, Volume= 51,122 Routed to Reach 11R: 4x4 Open Bottom Culvert

51,122 cf, Depth> 5.73"

Routed to Reach Till : 4x4 Open Bottom Odivert

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 359

A	rea (sf)	CN	Description					
	10,826	39	>75% Gras	s cover, Go	ood, HSG A			
	16,826	30	Woods, Go	od, HSG A				
	8,863	74	>75% Gras	s cover, Go	ood, HSG C			
	26,084	70	Woods, Go	od, HSG C				
	44,067	98	Water Surfa	ace, 0% imp	p, HSG D			
	304	98	Paved park	ing, HSG A				
	103	98	Paved park	ing, HSG C				
1	107,073	73	Weighted A	verage				
1	106,666	!	99.62% Pe	rvious Area				
	407		0.38% Impe	ervious Are	a			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
13.7	50	0.0150	0.06		Sheet Flow,			
					Woods: Light underbrush n= 0.400 P2= 3.27"			
15.2	557	0.0150	0.61		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
28.9	607	Total						

Summary for Subcatchment S210: INFILTRATION POND #1

Runoff = 9.35 cfs @ 12.22 hrs, Volume= 40,23

40,231 cf, Depth> 6.36"

Routed to Pond P212: INFILTRATION POND #1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

	Α	rea (sf)	CN [Description								
		13,844	98 V	98 Water Surface, 0% imp, HSG C								
		59,814	74 >	75% Gras	s cover, Go	ood, HSG C						
		2,232	65 E	Brush, Goo	d, HSG C							
		75,890	78 V	Veighted A	verage							
		75,890	1	00.00% Pe	ervious Are	a						
	Тс	Length	Slope	Velocity	Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	6.2	50	0.0150	0.13		Sheet Flow,						
						Grass: Short n= 0.150 P2= 3.27"						
	10.3	530	0.0150	0.86		Shallow Concentrated Flow,						
						Short Grass Pasture Kv= 7.0 fps						
	16.5	580	Total									

Summary for Subcatchment S211: S211

Runoff = 2.59 cfs @ 12.09 hrs, Volume= Routed to Pond P205 : INFILTRATION POND #3 8,357 cf, Depth> 6.50"

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 360

A	rea (sf)	CN	Description							
	8,108	61	>75% Grass cover, Good, HSG B							
	7,328	98	Water Surface, HSG B							
	15,436	79	Weighted Average							
	8,108		52.53% Pervious Area							
	7,328		47.47% lmp	pervious Ar	rea					
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	•					
6.0	, ,	/		7	Direct Entry,					

Summary for Subcatchment S212: SWALE

Runoff = 4.28 cfs @ 12.32 hrs, Volume= 20,86

20,867 cf, Depth> 4.75"

Routed to Reach SC2: Stream Crossing #2

A	rea (sf)	CN [Description									
	4,100	61 >	61 >75% Grass cover, Good, HSG B									
	7,192	55 V										
	1,180	74 >	75% Gras	s cover, Go	ood, HSG C							
	3,436	70 V	Voods, Go	od, HSG C								
	13,180			ace, 0% im _l								
	72		•	ing, HSG E	3							
	22,663		Brush, Goo	•								
	545		Brush, Goo	•								
	107			ing, HSG C								
	135			ing, HSG [
	158				ood, HSG D							
	52,768		Veighted A	•								
	52,454			rvious Area								
	314	C).60% Impe	ervious Are	a							
Тс	Length	Slope	Velocity	Capacity	Description							
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description							
16.1	50	0.0400	0.05	(010)	Sheet Flow,							
10.1	50	0.0400	0.00		Woods: Dense underbrush n= 0.800 P2= 3.27"							
3.9	232	0.0600	0.98		Shallow Concentrated Flow, BRUSH							
0.0	202	3.0000	0.00		Kv= 4.0 fps							
3.1	136	0.0220	0.74		Shallow Concentrated Flow,							
					Woodland Kv= 5.0 fps							
23.1	418	Total										

Printed 6/19/2023

Page 361

Summary for Subcatchment S213: COURTYARD

Runoff = 3.59 cfs @ 12.09 hrs, Volume= 11,590 cf, Depth> 6.50"

Routed to Pond 11P: YARD DRAIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description							
	5,047	39	>75% Grass cover, Good, HSG A							
	1,678	98	Paved parking, HSG A							
	168	98	Roofs, HSG A							
	532	98	Water Surface, 0% imp, HSG A							
	4,518	74	>75% Grass cover, Good, HSG C							
	7,080	98	Paved parking, HSG C							
	878	98	Roofs, HSG C							
	718	98	Water Surface, 0% imp, HSG C							
	296	80	>75% Grass cover, Good, HSG D							
	492	98	Paved parking, HSG D							
	21,407	79	Weighted Average							
	11,111		51.90% Pervious Area							
	10,296		48.10% Impervious Area							
Tc	Length	Slop	• • • • • • • • • • • • • • • • • • • •							
(min)	(feet)	(ft/f	ft) (ft/sec) (cfs)							
6.0			Direct Entry,							

Summary for Subcatchment T1: Trench Drain 1

Runoff = 2.18 cfs @ 12.09 hrs, Volume= 7,644 cf, Depth> 8.21"

Routed to Pond 5R: TRENCH DRAIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

	Area (s	f) CN	Description	Description							
	1,44	3 74	>75% Gras	s cover, Go	Good, HSG C						
	4,22	8 98	Paved park	ing, HSG C	C						
	1,33	9 80	>75% Gras	s cover, Go	Good, HSG D						
_	4,16	3 98	Paved park	ing, HSG D	D						
	11,17	3 93	Weighted A	Weighted Average							
	2,78	2	24.90% Per	rvious Area	a						
	8,39	1	75.10% lmp	pervious Ar	rea						
	Tc Leng	gth Slo	pe Velocity	Capacity	Description						
	(min) (fee	et) (ft.	/ft) (ft/sec)) (ft/sec) (cfs)							
	6.0				Direct Entry						

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 362

Summary for Subcatchment T2: Drive Under B2

Runoff = 0.73 cfs @ 12.09 hrs, Volume=

2,361 cf, Depth> 6.37"

Routed to Reach 11R: 4x4 Open Bottom Culvert

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description							
	1,510	39	>75% Grass	cover, Go	Good, HSG A					
	2,313	98	Paved parki	ng, HSG A	A					
	77	74	>75% Grass	cover, Go	Good, HSG C					
	545	98	Paved parki	ng, HSG C	C					
•	4,445	78	Weighted Average							
	1,587		35.70% Per	vious Area	a					
	2,858		64.30% Imp	ervious Ar	ırea					
Тс	Length	Slop	e Velocity	Capacity	Description					
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment TH1: TOWN HOUSE #1

Runoff = 0.84 cfs @ 12.09 hrs, Volume=

3,034 cf, Depth> 8.57"

Routed to Pond DE61: DRIP #61

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description							
	3,936	98	Roofs, HSG C							
	311	74	>75% Grass cover, Good, HSG C							
	4,247	96	Weighted Average							
	311		7.32% Pervious Area							
	3,936		92.68% lmp	pervious Ar	rea					
Tc	Length	Slope	,	Capacity	Description					
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment TH10: TOWN HOUSE #10

Runoff = 0.69 cfs @ 12.09 hrs, Volume= 2,483 cf, Depth> 8.57"

Routed to Pond DE70: DRIP #70

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 363

A	rea (sf)	CN	Description								
	3,184	98	Roofs, HSG C								
	292	74	>75% Grass cover, Good, HSG C								
	3,476	96	Weighted Average								
	292		8.40% Perv	ious Area							
	3,184		91.60% lmp	pervious Ar	ea						
_											
Tc	9	Slope	,	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
6.0					Direct Entry						

Summary for Subcatchment TH11: TOWN HOUSE #11

Runoff = 0.84 cfs @ 12.09 hrs, Volume=

3,008 cf, Depth> 8.57"

Routed to Pond DE71: DRIP #71

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Α	rea (sf)	CN	Description							
•	3,899	98	Roofs, HSG C							
	311	74	>75% Gras	s cover, Go	Good, HSG C					
	4,210 311 3,899		Weighted Average 7.39% Pervious Area 92.61% Impervious Area							
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	•					
6.0					Direct Entry,					

Summary for Subcatchment TH2: TOWN HOUSE #2

Runoff = 0.84 cfs @ 12.09 hrs, Volume=

3,034 cf, Depth> 8.57"

Routed to Pond DE62: DRIP #62

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description						
	3,936	98	Roofs, HSG C						
	311	74	>75% Grass cover, Good, HSG C						
	4,247	96	Weighted Average						
	311		7.32% Pervious Area						
	3,936		92.68% lmp	pervious Ar	ea				
Tc	Length	Slope	,	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					

6.0

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 364

Summary for Subcatchment TH3: TOWN HOUSE #3

Runoff = 0.60 cfs @ 12.09 hrs, Volume= 2,122 cf, Depth> 8.45"

Routed to Pond DE63: DRIP #63

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description							
	2,672	98	Roofs, HSG C							
	341	74	>75% Grass cover, Good, HSG C							
	3,013	95	Weighted Average							
	341		11.32% Pei	rvious Area	a					
	2,672		88.68% lmp	pervious Ar	rea					
Tc	Length	Slope	•	Capacity	•					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.0					Direct Entry,					

Summary for Subcatchment TH4: TOWN HOUSE #4

Runoff = 0.69 cfs @ 12.09 hrs, Volume= 2,479 cf, Depth> 8.57"

Routed to Pond DE64: DRIP #64

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description								
	3,178	98	Roofs, HSG C								
	292	74	>75% Grass cover, Good, HSG C								
	3,470	96	Veighted Average								
	292		8.41% Pervious Area								
	3,178	!	91.59% Imp	pervious Ar	rea						
_		0.1			5						
Tc	Length	Slope	,	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
6.0					Direct Entry,						

Summary for Subcatchment TH5: TOWN HOUSE #5

Runoff = 0.60 cfs @ 12.09 hrs, Volume= 2,124 cf, Depth> 8.45"

Routed to Pond DE65: DRIP #65

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 365

	rea (sf)	CN	Description				
	2,675	98	Roofs, HSG C				
	341	74	>75% Grass cover, Good, HSG C				
	3,016	95	Weighted A	verage			
	341		11.31% Pe	rvious Area			
	2,675		88.69% Impervious Area				
_							
Tc	9	Slope	,	Capacity	Description		
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)			
6.0					Direct Entry.		

Summary for Subcatchment TH6: TOWN HOUSE #6

Runoff = 0.68 cfs @ 12.09 hrs, Volume=

2,434 cf, Depth> 8.57"

Routed to Pond DE66: DRIP #66

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Aı	rea (sf)	CN	Description			
	3,116	98	Roofs, HSC	G C		
	291	74	>75% Gras	s cover, Go	ood, HSG C	
	3,407	96	Weighted A	verage		
	291		8.54% Perv	ious Area		
	3,116		91.46% lmp	pervious Ar	rea	
Tc	Length	Slope	e Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	,	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment TH7: TOWN HOUSE #7

Runoff = 0.69 cfs @ 12.09 hrs, Volume= 2,487 cf, Depth> 8.57"

Routed to Pond DE67: DRIP #67

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

Area (sf) CN	Description					
3,1	89 98	Roofs, HSG C					
2	92 74	>75% Grass cover, Good, HSG C					
3,4 2 3,1	92	Weighted Average 8.39% Pervious Area 91.61% Impervious Area					
Tc Ler (min) (fe	gth Slo						

6.0 Direct Entry,

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 366

Summary for Subcatchment TH8: TOWN HOUSE #8

Runoff = 0.84 cfs @ 12.09 hrs, Volume= 3,009 cf, Depth> 8.57"

Routed to Pond DE68: DRIP #68

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description			
	3,901	98	Roofs, HSC	G C		
	311	74	>75% Grass cover, Good, HSG C			
	4,212	96	Weighted A	verage		
	311		7.38% Perv	ious Area		
	3,901	!	92.62% lmp	pervious Ar	rea	
_				_		
Tc	Length	Slope	,	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Summary for Subcatchment TH9: TOWN HOUSE #9

Runoff = 0.69 cfs @ 12.09 hrs, Volume= 2,486 cf, Depth> 8.57"

Routed to Pond DE69: DRIP #69

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

A	rea (sf)	CN	Description			
	3,188	98	Roofs, HSC	G C		
	292	74	>75% Gras	s cover, Go	lood, HSG C	
	3,480	96	96 Weighted Average			
	292		8.39% Pervious Area			
	3,188	!	91.61% Imp	pervious Ar	rea	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	,	(cfs)	2 cccinpueri	
6.0					Direct Entry,	

Summary for Reach 8R: OVERLAND FLOW

Inflow Area = 11,975 sf, 92.37% Impervious, Inflow Depth > 7.60" for 100YR event

Inflow = 1.97 cfs @ 12.14 hrs, Volume= 7,579 cf

Outflow = 0.70 cfs @ 12.48 hrs, Volume= 7,164 cf, Atten= 64%, Lag= 20.2 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.12 fps, Min. Travel Time= 75.2 min

Avg. Velocity = 0.05 fps, Avg. Travel Time= 175.9 min

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 367

Peak Storage= 3,174 cf @ 12.48 hrs
Average Depth at Peak Storage= 0.11' S

Average Depth at Peak Storage= 0.11', Surface Width= 51.12' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 28.09 cfs

50.00' x 1.00' deep channel, n= 0.400 Side Slope Z-value= 5.0 '/' Top Width= 60.00' Length= 563.0' Slope= 0.0213 '/'

Inlet Invert= 208.00', Outlet Invert= 196.00'



Summary for Reach 9R: OVERLAND FLOW

Inflow Area = 32,665 sf, 94.81% Impervious, Inflow Depth = 0.85" for 100YR event

Inflow = 1.22 cfs @ 12.42 hrs, Volume= 2,322 cf

Outflow = 0.97 cfs @ 12.60 hrs, Volume= 2,322 cf, Atten= 21%, Lag= 10.5 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.27 fps, Min. Travel Time= 13.0 min Avg. Velocity = 0.06 fps, Avg. Travel Time= 56.1 min

Peak Storage= 752 cf @ 12.60 hrs Average Depth at Peak Storage= 0.16', Surface Width= 23.29' Bank-Full Depth= 1.00' Flow Area= 30.0 sf, Capacity= 23.45 cfs

20.00' x 1.00' deep channel, n= 0.400 Side Slope Z-value= 10.0 '/' Top Width= 40.00' Length= 211.0' Slope= 0.0652 '/' Inlet Invert= 201.75', Outlet Invert= 188.00'



Summary for Reach 10R: OVERLAND FLOW

Inflow Area = 129,716 sf, 63.13% Impervious, Inflow Depth = 2.92" for 100YR event

Inflow = 10.24 cfs @ 12.28 hrs, Volume= 31,579 cf

Outflow = 8.88 cfs @ 12.37 hrs, Volume= 31,578 cf, Atten= 13%, Lag= 5.7 min

Routed to Link AP4: ANALYSIS POINT #4

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 368

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.47 fps, Min. Travel Time= 5.8 min Avg. Velocity = 0.13 fps, Avg. Travel Time= 20.8 min

Peak Storage= 3,056 cf @ 12.37 hrs

Average Depth at Peak Storage= 0.69', Surface Width= 33.85'

Bank-Full Depth= 1.00' Flow Area= 30.0 sf, Capacity= 17.57 cfs

20.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 40.00'

Length= 164.0' Slope= 0.0366 '/'

Inlet Invert= 192.00', Outlet Invert= 186.00'



Summary for Reach 11R: 4x4 Open Bottom Culvert

[52] Hint: Inlet/Outlet conditions not evaluated

[62] Hint: Exceeded Reach 20R OUTLET depth by 1.03' @ 12.45 hrs

[64] Warning: Exceeded Reach 20R outlet bank by 0.42' @ 12.47 hrs

[62] Hint: Exceeded Reach R211 OUTLET depth by 0.56' @ 12.45 hrs

[64] Warning: Exceeded Reach R211 outlet bank by 0.42' @ 12.47 hrs

Inflow Area = 424,818 sf, 45.99% Impervious, Inflow Depth > 4.09" for 100YR event

Inflow = 24.22 cfs @ 12.47 hrs, Volume= 144,775 cf

Outflow = 24.22 cfs @ 12.47 hrs, Volume= 144,761 cf, Atten= 0%, Lag= 0.1 min

Routed to Reach 23R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 2.73 fps, Min. Travel Time= 0.2 min

Avg. Velocity = 0.95 fps, Avg. Travel Time= 0.5 min

Peak Storage= 266 cf @ 12.47 hrs

Average Depth at Peak Storage= 2.22', Surface Width= 4.00'

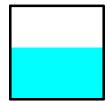
Bank-Full Depth= 4.00' Flow Area= 16.0 sf, Capacity= 42.20 cfs

48.0" W x 48.0" H Box Pipe n= 0.069 Riprap, 6-inch

II- 0.009 Kiprap, 0-ilicii

Length= 30.0' Slope= 0.0150 '/'

Inlet Invert= 194.00', Outlet Invert= 193.55'



Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 369

Summary for Reach 12R: OVERLAND FLOW

Inflow Area = 12,906 sf, 90.20% Impervious, Inflow Depth > 7.64" for 100YR event

Inflow = 2.22 cfs @ 12.14 hrs, Volume= 8,218 cf

Outflow = 1.36 cfs @ 12.29 hrs, Volume= 8,061 cf, Atten= 39%, Lag= 9.1 min

Routed to Link AP2: ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity = 0.17 fps, Min. Travel Time = 24.9 min Avg. Velocity = 0.06 fps, Avg. Travel Time = 74.2 min

Peak Storage= 2,026 cf @ 12.29 hrs

Average Depth at Peak Storage= 0.16', Surface Width= 51.60' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 29.80 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'

Length= 250.0' Slope= 0.0240 '/'

Inlet Invert= 202.00', Outlet Invert= 196.00'

‡

Summary for Reach 14R: OVERLAND FLOW

Inflow Area = 52,768 sf, 0.60% Impervious, Inflow Depth > 4.74" for 100YR event

Inflow = 4.27 cfs @ 12.33 hrs, Volume= 20,864 cf

Outflow = 1.64 cfs @ 12.80 hrs, Volume= 19,373 cf, Atten= 62%, Lag= 28.4 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.18 fps, Min. Travel Time= 79.0 min

Avg. Velocity = 0.08 fps, Avg. Travel Time= 169.1 min

Peak Storage= 7,796 cf @ 12.80 hrs

Average Depth at Peak Storage= 0.18', Surface Width= 53.54'

Bank-Full Depth= 1.00' Flow Area= 60.0 sf, Capacity= 31.55 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 70.00'

Length= 852.0' Slope= 0.0246 '/'

Inlet Invert= 207.00', Outlet Invert= 186.00'

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 370



Summary for Reach 15R: OVERLAND FLOW

Inflow Area = 62,582 sf, 52.00% Impervious, Inflow Depth > 3.97" for 100YR event

Inflow = 3.51 cfs @ 12.40 hrs, Volume= 20,698 cf

Outflow = 1.89 cfs @ 12.67 hrs, Volume= 19,826 cf, Atten= 46%, Lag= 16.2 min

Routed to Link AP2: ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.18 fps, Min. Travel Time= 27.8 min

Avg. Velocity = 0.09 fps, Avg. Travel Time= 57.7 min

Peak Storage= 3,139 cf @ 12.67 hrs

Average Depth at Peak Storage= 0.21', Surface Width= 52.05' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 27.21 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'

Length= 300.0' Slope= 0.0200 '/'

Inlet Invert= 202.00', Outlet Invert= 196.00'



Summary for Reach 18R: OVERLAND FLOW

Inflow Area = 88,676 sf, 39.42% Impervious, Inflow Depth = 3.01" for 100YR event

Inflow = 10.22 cfs @ 12.18 hrs, Volume= 22,242 cf

Outflow = 3.56 cfs @ 12.56 hrs, Volume= 21,983 cf, Atten= 65%, Lag= 22.9 min

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.24 fps, Min. Travel Time= 42.7 min

Avg. Velocity = 0.09 fps, Avg. Travel Time= 116.4 min

Peak Storage= 9,121 cf @ 12.56 hrs

Average Depth at Peak Storage= 0.26', Surface Width= 63.23'

Bank-Full Depth= 1.00' Flow Area= 75.0 sf, Capacity= 38.42 cfs

Prepared by Howard Stein Hudson Associates

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023 Page 371

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 100.00'

Length= 609.0' Slope= 0.0279 '/'

Inlet Invert= 203.00', Outlet Invert= 186.00'



Summary for Reach 20R: OVERLAND FLOW

Inflow Area = 72,222 sf, 68.72% Impervious, Inflow Depth > 5.95" for 100YR event

Inflow = 8.98 cfs @ 12.18 hrs, Volume= 35,788 cf

Outflow = 3.75 cfs @ 12.53 hrs, Volume= 34,661 cf, Atten= 58%, Lag= 20.8 min

Routed to Reach 11R: 4x4 Open Bottom Culvert

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.19 fps, Min. Travel Time= 50.3 min

Avg. Velocity = 0.08 fps, Avg. Travel Time= 118.6 min

Peak Storage= 11,298 cf @ 12.53 hrs

Average Depth at Peak Storage= 0.39', Surface Width= 53.88' Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 18.54 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'

Length= 560.0' Slope= 0.0093 '/'

Inlet Invert= 200.00', Outlet Invert= 194.80'



Summary for Reach 23R: OVERLAND FLOW

[61] Hint: Exceeded Reach 11R outlet invert by 0.28' @ 12.60 hrs

Inflow Area = 424,818 sf, 45.99% Impervious, Inflow Depth > 4.09" for 100YR event

Inflow = 24.22 cfs @ 12.47 hrs, Volume= 144,761 cf

Outflow = 22.58 cfs @ 12.60 hrs, Volume= 143,931 cf, Atten= 7%, Lag= 8.0 min

Routed to Link AP4: ANALYSIS POINT #4

Type III 24-hr 100YR Rainfall=9.06"

19097 Post-Development

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 372

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.41 fps, Min. Travel Time= 9.6 min

Avg. Velocity = 0.13 fps, Avg. Travel Time= 29.8 min

Peak Storage= 13,052 cf @ 12.60 hrs

Average Depth at Peak Storage= 0.83', Surface Width= 83.10'

Bank-Full Depth= 1.00' Flow Area= 70.0 sf, Capacity= 31.93 cfs

50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 20.0 '/' Top Width= 90.00'

Length= 237.0' Slope= 0.0211 '/'

Inlet Invert= 193.00', Outlet Invert= 188.00'

‡

Summary for Reach R202: OVERLAND FLOW

[55] Hint: Peak inflow is 105% of Manning's capacity

[62] Hint: Exceeded Reach SC1 OUTLET depth by 0.41' @ 12.90 hrs

Inflow Area = 432,269 sf, 42.08% Impervious, Inflow Depth > 6.11" for 100YR event

Inflow = 44.78 cfs @ 12.32 hrs, Volume= 219,947 cf

Outflow = 25.82 cfs @ 12.63 hrs, Volume= 212,555 cf, Atten= 42%, Lag= 18.6 min

Routed to Link AP2: ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.29 fps, Min. Travel Time= 40.5 min

Avg. Velocity = 0.11 fps, Avg. Travel Time= 106.8 min

Peak Storage = 62,638 cf @ 12.63 hrs

Average Depth at Peak Storage= 0.75', Surface Width= 137.65'

Bank-Full Depth= 1.00' Flow Area= 125.0 sf, Capacity= 42.56 cfs

100.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 150.00'

Length= 700.0' Slope= 0.0107 '/'

Inlet Invert= 205.50', Outlet Invert= 198.00'

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 373

Summary for Reach R211: OVERLAND FLOW

[55] Hint: Peak inflow is 191% of Manning's capacity

Inflow Area = 241,078 sf, 59.10% Impervious, Inflow Depth = 2.84" for 100YR event

Inflow = 27.69 cfs @ 12.16 hrs, Volume= 57,014 cf

Outflow = 11.38 cfs @ 12.54 hrs, Volume= 56,631 cf, Atten= 59%, Lag= 22.7 min

Routed to Reach 11R: 4x4 Open Bottom Culvert

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 0.27 fps, Min. Travel Time= 37.1 min Avg. Velocity = 0.09 fps, Avg. Travel Time= 117.0 min

Peak Storage= 25,331 cf @ 12.54 hrs

Average Depth at Peak Storage= 0.88', Surface Width= 61.30' Bank-Full Depth= 1.00' Flow Area= 50.0 sf, Capacity= 14.51 cfs

35.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 15.0 '/' Top Width= 65.00'

Length= 600.0' Slope= 0.0087 '/'

‡

Inlet Invert= 200.00', Outlet Invert= 194.80'

Summary for Reach SC1: Stream Crossing #1

[52] Hint: Inlet/Outlet conditions not evaluated

Inflow Area = 432,269 sf, 42.08% Impervious, Inflow Depth > 6.11" for 100YR event

Inflow = 44.78 cfs @ 12.31 hrs, Volume= 219,981 cf

Outflow = 44.78 cfs @ 12.32 hrs, Volume= 219,947 cf, Atten= 0%, Lag= 0.1 min

Routed to Reach R202: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Max. Velocity= 4.71 fps, Min. Travel Time= 0.2 min

Avg. Velocity = 1.47 fps, Avg. Travel Time= 0.5 min

Peak Storage= 409 cf @ 12.32 hrs

Average Depth at Peak Storage= 0.59', Surface Width= 16.00' Bank-Full Depth= 5.00' Flow Area= 69.8 sf, Capacity= 722.91 cfs

192.0" W x 60.0" H, R=207.0" Arch Pipe

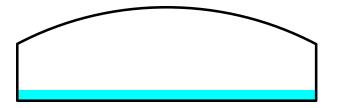
n= 0.030 Stream, clean & straight

Length= 43.1' Slope= 0.0200 '/'

Inlet Invert= 206.37', Outlet Invert= 205.51'

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 374



Summary for Reach SC2: Stream Crossing #2

[52] Hint: Inlet/Outlet conditions not evaluated

Inflow Area = 52,768 sf, 0.60% Impervious, Inflow Depth > 4.75" for 100YR event

Inflow 4.28 cfs @ 12.32 hrs, Volume= 20.867 cf

4.27 cfs @ 12.33 hrs, Volume= Outflow 20,864 cf, Atten= 0%, Lag= 0.1 min

Routed to Reach 14R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

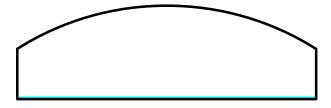
Max. Velocity= 2.01 fps, Min. Travel Time= 0.3 min Avg. Velocity = 1.08 fps, Avg. Travel Time= 0.6 min

Peak Storage= 77 cf @ 12.33 hrs

Average Depth at Peak Storage= 0.13', Surface Width= 16.00' Bank-Full Depth= 5.00' Flow Area= 68.1 sf. Capacity= 768.96 cfs

192.0" W x 60.0" H, R=180.0" Arch Pipe n= 0.030 Stream, clean & straight Length= 36.5' Slope= 0.0241 '/'

Inlet Invert= 208.52', Outlet Invert= 207.64'



Summary for Pond 1P: DMH #33

Inflow Area = 16,111 sf, 93.77% Impervious, Inflow Depth > 8.65" for 100YR event

3.20 cfs @ 12.09 hrs, Volume= Inflow 11,616 cf

Outflow 11,616 cf, Atten= 0%, Lag= 0.0 min

3.20 cfs @ 12.09 hrs, Volume= 3.20 cfs @ 12.09 hrs, Volume= Primary 11.616 cf

Routed to Pond OCS6: OCS #6

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.71' @ 12.09 hrs

Flood Elev= 209.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.50'	12.0" Round Culvert L= 46.7' Ke= 0.500
	_		Inlet / Outlet Invert= 205.50' / 204.33' S= 0.0251 '/' Cc= 0.900

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 375

n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.12 cfs @ 12.09 hrs HW=206.68' TW=203.63' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.12 cfs @ 3.97 fps)

Summary for Pond 3P: OCS #8

Inflow Area = 12,684 sf, 86.64% Impervious, Inflow Depth > 8.12" for 100YR event

Inflow = 2.47 cfs @ 12.09 hrs, Volume= 8,581 cf

Outflow = 2.47 cfs @ 12.09 hrs, Volume= 8,581 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.47 cfs @ 12.09 hrs, Volume= 8,581 cf Routed to Pond P214 : STORMTECH INFILTRATION SYSTEM #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.30' @ 12.41 hrs

Flood Elev= 206.36'

Device Routing Invert Outlet Devices

#1 Primary 200.62' **12.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.40 cfs @ 12.09 hrs HW=202.18' TW=201.77' (Dynamic Tailwater)

1=Orifice/Grate (Orifice Controls 2.40 cfs @ 3.06 fps)

Summary for Pond 5R: TRENCH DRAIN

Inflow Area = 11,173 sf, 75.10% Impervious, Inflow Depth > 8.21" for 100YR event

Inflow = 2.18 cfs @ 12.09 hrs, Volume= 7,644 cf

Outflow = 2.18 cfs @ 12.09 hrs, Volume= 7.644 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.18 cfs @ 12.09 hrs, Volume= 7,644 cf Routed to Pond P206 : STORMTECH INFILTRATION SYSTEM #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 199.24' @ 12.09 hrs

Flood Elev= 199.50'

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 197.22'
 8.0" Round Culvert L= 36.0' Ke= 0.500 Inlet / Outlet Invert= 197.22' / 196.50' S= 0.0200 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=2.13 cfs @ 12.09 hrs HW=199.16' TW=196.52' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.13 cfs @ 6.09 fps)

Summary for Pond 11P: YARD DRAIN

Inflow Area = 21,407 sf, 48.10% Impervious, Inflow Depth > 6.50" for 100YR event

Inflow = 3.59 cfs @ 12.09 hrs, Volume= 11,590 cf

Outflow = 2.82 cfs @ 12.16 hrs, Volume= 11,525 cf, Atten= 22%, Lag= 4.2 min

Primary = 2.82 cfs @ 12.16 hrs, Volume= 11,525 cf

Routed to Pond D13: DMH #13

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 376

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.48' @ 12.16 hrs Surf.Area= 6,408 sf Storage= 1,296 cf

Plug-Flow detention time= 14.4 min calculated for 11,501 cf (99% of inflow)

Center-of-Mass det. time= 10.9 min (811.0 - 800.1)

Volume	Inve	ert Avail.Sto	orage Storag	e Description	
#1	207.2	25' 5,4	75 cf Custo	m Stage Data (Pri	smatic)Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
207.2	25	5,050	0	0	
208.0	00	9,550	5,475	5,475	
Device	Routing	Invert	Outlet Devic	es	
#1	Primary	203.25'	12.0" Roun	d Culvert L= 61.0)' Ke= 0.500
#2	Device 1	207.25'	n= 0.012 Co 4.0" x 4.0" H X 4 rows C=	orrugated PP, smooth Horiz. Orifice/Grate	202.94' S= 0.0051 '/' Cc= 0.900 oth interior, Flow Area= 0.79 sf te X 4.00 columns 4.0" Grate (44% open area) ds

Primary OutFlow Max=2.78 cfs @ 12.16 hrs HW=207.47' TW=203.83' (Dynamic Tailwater) **-1=Culvert** (Passes 2.78 cfs of 6.70 cfs potential flow) **1**—2=Orifice/Grate (Weir Controls 2.78 cfs @ 1.55 fps)

Summary for Pond CB10: CB #10

Inflow Area = 6,961 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

1.39 cfs @ 12.09 hrs, Volume= Inflow = 5.113 cf

1.39 cfs @ 12.09 hrs, Volume= 5,113 cf, Atten= 0%, Lag= 0.0 min Outflow

Primary = 1.39 cfs @ 12.09 hrs, Volume= 5.113 cf

Routed to Pond D5: DMH #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.78' @ 12.09 hrs

Flood Elev= 212.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.76'	12.0" Round Culvert L= 33.8' Ke= 0.500 Inlet / Outlet Invert= 209.76' / 209.59' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.33 cfs @ 12.09 hrs HW=210.75' TW=210.59' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.33 cfs @ 2.13 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 377

Summary for Pond CB11: CB #11

Inflow Area = 7,173 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow = 1.43 cfs @ 12.09 hrs, Volume= 5,269 cf

Outflow = 1.43 cfs @ 12.09 hrs, Volume= 5,269 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.43 cfs @ 12.09 hrs, Volume= 5,269 cf

Routed to Pond D5: DMH #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 210.83' @ 12.09 hrs

Flood Elev= 213.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.94'	12.0" Round Culvert L= 26.3' Ke= 0.500 Inlet / Outlet Invert= 209.94' / 209.67' S= 0.0103 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.38 cfs @ 12.09 hrs HW=210.81' TW=210.59' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.38 cfs @ 2.55 fps)

Summary for Pond CB12: CB #12

Inflow Area = 5,238 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow = 1.04 cfs @ 12.09 hrs, Volume= 3,847 cf

Outflow = 1.04 cfs @ 12.09 hrs, Volume= 3,847 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.04 cfs @ 12.09 hrs, Volume= 3,847 cf

Routed to Pond 1P: DMH #33

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.24' @ 12.09 hrs

Flood Elev= 209.84'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.68'	12.0" Round Culvert L= 41.3' Ke= 0.500 Inlet / Outlet Invert= 206.68' / 205.65' S= 0.0249 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.02 cfs @ 12.09 hrs HW=207.22' TW=206.68' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.02 cfs @ 3.38 fps)

Summary for Pond CB13: CB #13

Inflow Area = 10,873 sf, 90.78% Impervious, Inflow Depth > 8.57" for 100YR event

Inflow = 2.16 cfs @ 12.09 hrs, Volume= 7,768 cf

Outflow = 2.16 cfs @ 12.09 hrs, Volume= 7,768 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.16 cfs @ 12.09 hrs, Volume= 7,768 cf

Routed to Pond 1P: DMH #33

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 378

Peak Elev= 207.53' @ 12.09 hrs Flood Elev= 209.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.70'	12.0" Round Culvert L= 43.7' Ke= 0.500 Inlet / Outlet Invert= 206.70' / 205.61' S= 0.0249 '/' Cc= 0.900 n= 0.013 Corrugated PE. smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=2.10 cfs @ 12.09 hrs HW=207.51' TW=206.68' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.10 cfs @ 3.07 fps)

Summary for Pond CB14: CB #14

Inflow Area = 12,099 sf, 86.22% Impervious, Inflow Depth > 7.97" for 100YR event

Inflow = 2.33 cfs @ 12.09 hrs, Volume= 8,033 cf

Outflow = 2.33 cfs @ 12.09 hrs, Volume= 8,033 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.33 cfs @ 12.09 hrs, Volume= 8,033 cf

Routed to Pond D8: DMH #8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.42' @ 12.09 hrs

Flood Elev= 203.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.79'	12.0" Round Culvert L= 23.2' Ke= 0.500 Inlet / Outlet Invert= 200.79' / 200.67' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=2.27 cfs @ 12.09 hrs HW=203.30' TW=202.93' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.27 cfs @ 2.89 fps)

Summary for Pond CB15: CB #15

Inflow Area = 6,666 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow = 1.33 cfs @ 12.09 hrs, Volume= 4.896 cf

Outflow = 1.33 cfs @ 12.09 hrs, Volume= 4,896 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.33 cfs @ 12.09 hrs, Volume= 4,896 cf

Routed to Pond D8: DMH #8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.16' @ 12.09 hrs

Flood Elev= 203.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.79'	12.0" Round Culvert L= 15.6' Ke= 0.500 Inlet / Outlet Invert= 200.79' / 200.71' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.29 cfs @ 12.09 hrs HW=203.05' TW=202.93' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.29 cfs @ 1.65 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 379

Summary for Pond CB16: CB #16

Inflow Area = 8,516 sf, 64.88% Impervious, Inflow Depth > 6.50" for 100YR event

Inflow = 1.43 cfs @ 12.09 hrs, Volume= 4,611 cf

Outflow = 1.43 cfs @ 12.09 hrs, Volume= 4,611 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.43 cfs @ 12.09 hrs, Volume= 4,611 cf

Routed to Pond D10: DMH #10

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.33' @ 12.09 hrs

Flood Elev= 206.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.47'	12.0" Round Culvert L= 20.9' Ke= 0.500 Inlet / Outlet Invert= 203.47' / 203.33' S= 0.0067 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.40 cfs @ 12.09 hrs HW=204.32' TW=204.09' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.40 cfs @ 2.65 fps)

Summary for Pond CB17: CB #17

Inflow Area = 11,836 sf, 73.87% Impervious, Inflow Depth > 8.09" for 100YR event

Inflow = 2.30 cfs @ 12.09 hrs, Volume= 7,978 cf

Outflow = 2.30 cfs @ 12.09 hrs, Volume= 7,978 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.30 cfs @ 12.09 hrs, Volume= 7,978 cf

Routed to Pond D11: DMH #11

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.31' @ 12.10 hrs

Flood Elev= 208.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.99'	12.0" Round Culvert L= 13.8' Ke= 0.500 Inlet / Outlet Invert= 204.99' / 204.86' S= 0.0094 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.64 cfs @ 12.09 hrs HW=207.15' TW=207.12' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 0.64 cfs @ 0.82 fps)

Summary for Pond CB18: CB #18

Inflow Area = 24,853 sf, 72.99% Impervious, Inflow Depth > 7.05" for 100YR event

Inflow = 3.90 cfs @ 12.09 hrs, Volume= 14,596 cf

Outflow = 3.90 cfs @ 12.09 hrs, Volume= 14,596 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.90 cfs @ 12.09 hrs, Volume= 14,596 cf

Routed to Pond D11: DMH #11

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Page 380

Peak Elev= 207.38' @ 12.11 hrs

Flood Elev= 208.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.72'	15.0" Round Culvert L= 25.1' Ke= 0.500 Inlet / Outlet Invert= 204.72' / 204.59' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.87 cfs @ 12.09 hrs HW=207.25' TW=207.15' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.87 cfs @ 1.52 fps)

Summary for Pond CB20: CB #20

Inflow Area = 11,939 sf, 88.95% Impervious, Inflow Depth > 8.45" for 100YR event

Inflow 2.36 cfs @ 12.09 hrs, Volume= 8,409 cf

2.36 cfs @ 12.09 hrs, Volume= 8,409 cf, Atten= 0%, Lag= 0.0 min Outflow

2.36 cfs @ 12.09 hrs, Volume= Primary = 8.409 cf

Routed to Pond D12: DMH #12

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.83' @ 12.09 hrs

Flood Elev= 207.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.97'	12.0" Round Culvert L= 30.3' Ke= 0.500 Inlet / Outlet Invert= 203.97' / 203.81' S= 0.0053 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=2.30 cfs @ 12.09 hrs HW=205.72' TW=205.35' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.30 cfs @ 2.92 fps)

Summary for Pond CB21: CB #21

Inflow Area = 10,174 sf, 87.04% Impervious, Inflow Depth > 7.85" for 100YR event

1.95 cfs @ 12.09 hrs, Volume= 6,652 cf Inflow =

Outflow 1.95 cfs @ 12.09 hrs, Volume= 6,652 cf, Atten= 0%, Lag= 0.0 min

1.95 cfs @ 12.09 hrs, Volume= Primary = 6,652 cf

Routed to Pond D12: DMH #12

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.70' @ 12.09 hrs

Flood Elev= 208.02'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.32'	12.0" Round Culvert L= 26.0' Ke= 0.500
			Inlet / Outlet Invert= 204.32' / 204.19' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.91 cfs @ 12.09 hrs HW=205.61' TW=205.35' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.91 cfs @ 2.44 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 381

Summary for Pond CB22: CB #22

Inflow Area = 12,001 sf, 91.62% Impervious, Inflow Depth > 8.57" for 100YR event

Inflow = 2.38 cfs @ 12.09 hrs, Volume= 8,574 cf

Outflow = 2.38 cfs @ 12.09 hrs, Volume= 8,574 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.38 cfs @ 12.09 hrs, Volume= 8,574 cf

Routed to Pond D14: DMH #14

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.41' @ 12.09 hrs

Flood Elev= 208.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.33'	12.0" Round Culvert L= 16.1' Ke= 0.500 Inlet / Outlet Invert= 205.33' / 205.25' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.32 cfs @ 12.09 hrs HW=206.39' TW=205.73' (Dynamic Tailwater) 1=Culvert (Barrel Controls 2.32 cfs @ 3.48 fps)

Summary for Pond CB23: CB #23

Inflow Area = 9,694 sf, 61.00% Impervious, Inflow Depth > 7.72" for 100YR event

Inflow = 1.84 cfs @ 12.09 hrs, Volume= 6,239 cf

Outflow = 1.84 cfs @ 12.09 hrs, Volume= 6,239 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.84 cfs @ 12.09 hrs, Volume= 6,239 cf

Routed to Pond D14: DMH #14

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.30' @ 12.09 hrs

Flood Elev= 208.57'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.41'	12.0" Round Culvert L= 16.3' Ke= 0.500 Inlet / Outlet Invert= 205.41' / 205.32' S= 0.0055 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.79 cfs @ 12.09 hrs HW=206.29' TW=205.73' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.79 cfs @ 3.27 fps)

Summary for Pond CB24: CB #24

Inflow Area = 7,930 sf, 72.16% Impervious, Inflow Depth > 8.21" for 100YR event

Inflow = 1.55 cfs @ 12.09 hrs, Volume= 5,426 cf

Outflow = 1.55 cfs @ 12.09 hrs, Volume= 5,426 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.55 cfs @ 12.09 hrs, Volume= 5,426 cf

Routed to Pond D16: DMH #16

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 382

Peak Elev= 206.37' @ 12.09 hrs Flood Elev= 209.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.21'	12.0" Round Culvert L= 12.1' Ke= 0.500 Inlet / Outlet Invert= 205.21' / 205.15' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.45 cfs @ 12.09 hrs HW=206.33' TW=206.19' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.45 cfs @ 1.84 fps)

Summary for Pond CB25: CB #25

Inflow Area = 8,487 sf, 80.92% Impervious, Inflow Depth > 8.33" for 100YR event

Inflow = 1.67 cfs @ 12.09 hrs, Volume= 5,892 cf

Outflow = 1.67 cfs @ 12.09 hrs, Volume= 5,892 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.67 cfs @ 12.09 hrs, Volume= 5.892 cf

Routed to Pond D16: DMH #16

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.30' @ 12.09 hrs

Flood Elev= 208.38'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.22'	15.0" Round Culvert L= 11.4' Ke= 0.500 Inlet / Outlet Invert= 205.22' / 205.16' S= 0.0053 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.52 cfs @ 12.09 hrs HW=206.28' TW=206.19' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.52 cfs @ 1.85 fps)

Summary for Pond CB26: CB #26

Inflow Area = 8,835 sf, 63.75% Impervious, Inflow Depth > 7.97" for 100YR event

Inflow = 1.70 cfs @ 12.09 hrs, Volume= 5,866 cf

Outflow = 1.70 cfs @ 12.09 hrs, Volume= 5,866 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.70 cfs @ 12.09 hrs, Volume= 5,866 cf

Routed to Pond D17: DMH #17

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 202.61' @ 12.09 hrs

Flood Elev= 204.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	201.77'	12.0" Round Culvert L= 42.5' Ke= 0.500
			Inlet / Outlet Invert= 201.77' / 201.55' S= 0.0052 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.66 cfs @ 12.09 hrs HW=202.60' TW=201.61' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.66 cfs @ 3.23 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 383

Summary for Pond CB27: CB #27

Inflow Area = 6,111 sf, 91.90% Impervious, Inflow Depth > 8.57" for 100YR event

Inflow = 1.21 cfs @ 12.09 hrs, Volume= 4,366 cf

Outflow = 1.21 cfs @ 12.09 hrs, Volume= 4,366 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.21 cfs @ 12.09 hrs, Volume= 4,366 cf

Routed to Pond D17: DMH #17

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 201.82' @ 12.09 hrs

Flood Elev= 204.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	201.00'	12.0" Round Culvert L= 18.0' Ke= 0.500 Inlet / Outlet Invert= 201.00' / 200.90' S= 0.0056 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.18 cfs @ 12.09 hrs HW=201.79' TW=201.61' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.18 cfs @ 2.42 fps)

Summary for Pond CB28: CB #28

Inflow Area = 10,372 sf, 51.33% Impervious, Inflow Depth > 7.60" for 100YR event

Inflow = 1.95 cfs @ 12.09 hrs, Volume= 6,570 cf

Outflow = 1.95 cfs @ 12.09 hrs, Volume= 6,570 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.95 cfs @ 12.09 hrs, Volume= 6,570 cf

Routed to Pond D18: DMH #18

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 199.02' @ 12.09 hrs

Flood Elev= 200.92'

Device	Routing	Invert	Outlet Devices
#1	Primary	197.75'	12.0" Round Culvert L= 13.7' Ke= 0.500
			Inlet / Outlet Invert= 197.75' / 197.69' S= 0.0044 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.90 cfs @ 12.09 hrs HW=198.98' TW=198.73' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.90 cfs @ 2.42 fps)

Summary for Pond CB29: CB #29

Inflow Area = 8,495 sf, 84.21% Impervious, Inflow Depth > 8.33" for 100YR event

Inflow = 1.67 cfs @ 12.09 hrs, Volume= 5,898 cf

Outflow = 1.67 cfs @ 12.09 hrs, Volume= 5,898 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.67 cfs @ 12.09 hrs, Volume= 5,898 cf

Routed to Pond D19: DMH #19

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 384

Peak Elev= 206.87' @ 12.09 hrs

Flood Elev= 208.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.38'	12.0" Round Culvert L= 13.5' Ke= 0.500 Inlet / Outlet Invert= 205.38' / 205.31' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.35 cfs @ 12.09 hrs HW=206.79' TW=206.66' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.35 cfs @ 1.71 fps)

Summary for Pond CB30: CB #30

Inflow Area = 8,933 sf, 82.40% Impervious, Inflow Depth > 8.33" for 100YR event

Inflow = 1.76 cfs @ 12.09 hrs, Volume= 6,202 cf

Outflow = 1.76 cfs @ 12.09 hrs, Volume= 6,202 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.76 cfs @ 12.09 hrs, Volume= 6,202 cf

Routed to Pond D19: DMH #19

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.89' @ 12.09 hrs

Flood Elev= 208.54'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.38'	12.0" Round Culvert L= 17.5' Ke= 0.500 Inlet / Outlet Invert= 205.38' / 205.29' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior Flow Area= 0.79 sf

Primary OutFlow Max=1.45 cfs @ 12.09 hrs HW=206.81' TW=206.66' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.45 cfs @ 1.84 fps)

Summary for Pond CB31: CB #31

Inflow Area = 16,365 sf, 68.64% Impervious, Inflow Depth > 7.85" for 100YR event

Inflow = 3.13 cfs @ 12.09 hrs, Volume= 10,699 cf

Outflow = 3.13 cfs @ 12.09 hrs, Volume= 10,699 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.13 cfs @ 12.09 hrs, Volume= 10,699 cf

Routed to Pond D21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.84' @ 12.09 hrs

Flood Elev= 207.36'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.19'	12.0" Round Culvert L= 16.4' Ke= 0.500 Inlet / Outlet Invert= 204.19' / 204.11' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.05 cfs @ 12.09 hrs HW=205.77' TW=205.12' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.05 cfs @ 3.89 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 385

Summary for Pond CB32: CB #32

Inflow Area = 12,710 sf, 70.47% Impervious, Inflow Depth > 7.97" for 100YR event

Inflow = 2.45 cfs @ 12.09 hrs, Volume= 8,439 cf

Outflow = 2.45 cfs @ 12.09 hrs, Volume= 8,439 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.45 cfs @ 12.09 hrs, Volume= 8,439 cf

Routed to Pond D21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.58' @ 12.09 hrs

Flood Elev= 207.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.19'	12.0" Round Culvert L= 16.3' Ke= 0.500 Inlet / Outlet Invert= 204.19' / 204.11' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.39 cfs @ 12.09 hrs HW=205.52' TW=205.12' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.39 cfs @ 3.04 fps)

Summary for Pond CB33: CB #33

Inflow Area = 5,421 sf, 83.90% Impervious, Inflow Depth > 8.33" for 100YR event

Inflow = 1.07 cfs @ 12.09 hrs, Volume= 3,764 cf

Outflow = 1.07 cfs @ 12.09 hrs, Volume= 3,764 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.07 cfs @ 12.09 hrs, Volume= 3,764 cf

Routed to Pond D22: DMH #22

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.39' @ 12.09 hrs

Flood Elev= 208.45'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.28'	12.0" Round Culvert L= 11.7' Ke= 0.500 Inlet / Outlet Invert= 205.28' / 205.22' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.04 cfs @ 12.09 hrs HW=206.35' TW=206.27' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.04 cfs @ 1.33 fps)

Summary for Pond CB34: CB #34

Inflow Area = 8,622 sf, 80.51% Impervious, Inflow Depth > 8.21" for 100YR event

Inflow = 1.69 cfs @ 12.09 hrs, Volume= 5,899 cf

Outflow = 1.69 cfs @ 12.09 hrs, Volume= 5,899 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.69 cfs @ 12.09 hrs, Volume= 5,899 cf

Routed to Pond D22: DMH #22

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Page 386

Peak Elev= 206.50' @ 12.09 hrs

Flood Elev= 208.38'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.21'	12.0" Round Culvert L= 16.5' Ke= 0.500 Inlet / Outlet Invert= 205.21' / 205.13' S= 0.0048 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.64 cfs @ 12.09 hrs HW=206.46' TW=206.27' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.64 cfs @ 2.09 fps)

Summary for Pond CB35: CB #35

Inflow Area = 4,149 sf, 98.10% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow 0.83 cfs @ 12.09 hrs, Volume= 3,048 cf

3,048 cf, Atten= 0%, Lag= 0.0 min 3,048 cf 0.83 cfs @ 12.09 hrs, Volume= Outflow =

Primary = 0.83 cfs @ 12.09 hrs, Volume=

Routed to Pond D23: DMH #23

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.66' @ 12.09 hrs

Flood Elev= 210.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	207.04'	12.0" Round Culvert L= 15.2' Ke= 0.500 Inlet / Outlet Invert= 207.04' / 206.96' S= 0.0053 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior Flow Area= 0.79 sf

Primary OutFlow Max=0.79 cfs @ 12.09 hrs HW=207.65' TW=207.48' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.79 cfs @ 2.27 fps)

Summary for Pond CB36: CB #36

Inflow Area = 6,622 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

1.32 cfs @ 12.09 hrs, Volume= 4,864 cf Inflow =

Outflow 1.32 cfs @ 12.09 hrs, Volume= 4,864 cf, Atten= 0%, Lag= 0.0 min

1.32 cfs @ 12.09 hrs, Volume= Primary = 4,864 cf

Routed to Pond D23: DMH #23

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.78' @ 12.09 hrs

Flood Elev= 210.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	207.04'	12.0" Round Culvert L= 16.1' Ke= 0.500 Inlet / Outlet Invert= 207.04' / 206.96' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE. smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.29 cfs @ 12.09 hrs HW=207.77' TW=207.48' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.29 cfs @ 2.93 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 387

Summary for Pond CB38: CB #38

Inflow Area = 7,637 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow = 1.52 cfs @ 12.09 hrs, Volume= 5,610 cf

Outflow = 1.52 cfs @ 12.09 hrs, Volume= 5,610 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.52 cfs @ 12.09 hrs, Volume= 5,610 cf

Routed to Pond D25: DMH #25

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 212.84' @ 12.09 hrs

Flood Elev= 212.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.69'	12.0" Round Culvert L= 16.7' Ke= 0.500 Inlet / Outlet Invert= 209.69' / 209.61' S= 0.0048 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.48 cfs @ 12.09 hrs HW=212.67' TW=212.52' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.48 cfs @ 1.89 fps)

Summary for Pond CB39: CB #39

Inflow Area = 7,612 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow = 1.52 cfs @ 12.09 hrs, Volume= 5,591 cf

Outflow = 1.52 cfs @ 12.09 hrs, Volume= 5,591 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.52 cfs @ 12.09 hrs, Volume= 5,591 cf

Routed to Pond D25: DMH #25

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 212.84' @ 12.09 hrs

Flood Elev= 212.86'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.69'	12.0" Round Culvert L= 16.4' Ke= 0.500 Inlet / Outlet Invert= 209.69' / 209.61' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.48 cfs @ 12.09 hrs HW=212.67' TW=212.52' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.48 cfs @ 1.88 fps)

Summary for Pond CB40: CB #40

Inflow Area = 4,211 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow = 0.84 cfs @ 12.09 hrs, Volume= 3,093 cf

Outflow = 0.84 cfs @ 12.09 hrs, Volume= 3,093 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.84 cfs @ 12.09 hrs, Volume= 3,093 cf

Routed to Pond D27: DMH #27

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 388

Peak Elev= 214.52' @ 12.09 hrs Flood Elev= 217.04'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.68'	12.0" Round Culvert L= 17.8' Ke= 0.500 Inlet / Outlet Invert= 213.68' / 213.55' S= 0.0073 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.74 cfs @ 12.09 hrs HW=214.50' TW=214.43' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.74 cfs @ 1.47 fps)

Summary for Pond CB41: CB #41

Inflow Area = 5,586 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow = 1.11 cfs @ 12.09 hrs, Volume= 4,103 cf

Outflow = 1.11 cfs @ 12.09 hrs, Volume= 4,103 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.11 cfs @ 12.09 hrs, Volume= 4,103 cf

Routed to Pond D27: DMH #27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 214.63' @ 12.09 hrs

Flood Elev= 217.06'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.89'	12.0" Round Culvert L= 18.4' Ke= 0.500 Inlet / Outlet Invert= 213.89' / 213.80' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.05 cfs @ 12.09 hrs HW=214.62' TW=214.43' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.05 cfs @ 2.39 fps)

Summary for Pond CB43: CB #43

Inflow Area = 3,109 sf, 75.36% Impervious, Inflow Depth > 7.72" for 100YR event

Inflow = 0.59 cfs @ 12.09 hrs, Volume= 2.001 cf

Outflow = 0.59 cfs @ 12.09 hrs, Volume= 2,001 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.59 cfs @ 12.09 hrs, Volume= 2,001 cf

Routed to Pond D29: DMH #29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 220.71' @ 12.09 hrs

Flood Elev= 223.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	220.00'	12.0" Round Culvert L= 14.9' Ke= 0.500
			Inlet / Outlet Invert= 220.00' / 219.93' S= 0.0047 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.57 cfs @ 12.09 hrs HW=220.70' TW=220.64' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.57 cfs @ 1.38 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 389

Summary for Pond CB44: CB #44

Inflow Area = 1,978 sf, 84.43% Impervious, Inflow Depth > 8.09" for 100YR event

Inflow = 0.38 cfs @ 12.09 hrs, Volume= 1,333 cf

Outflow = 0.38 cfs @ 12.09 hrs, Volume= 1,333 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.38 cfs @ 12.09 hrs, Volume= 1,333 cf

Routed to Pond D29: DMH #29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 220.68' @ 12.09 hrs

Flood Elev= 223.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	220.00'	12.0" Round Culvert L= 14.9' Ke= 0.500 Inlet / Outlet Invert= 220.00' / 219.93' S= 0.0047 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.37 cfs @ 12.09 hrs HW=220.67' TW=220.64' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.37 cfs @ 0.95 fps)

Summary for Pond CB45: CB #45

Inflow Area = 2,465 sf, 50.30% Impervious, Inflow Depth > 6.62" for 100YR event

Inflow = 0.42 cfs @ 12.09 hrs, Volume= 1,360 cf

Outflow = 0.42 cfs @ 12.09 hrs, Volume= 1,360 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.42 cfs @ 12.09 hrs, Volume= 1,360 cf

Routed to Pond D30: DMH #30

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 221.73' @ 12.09 hrs

Flood Elev= 224.46'

Device	Routing	Invert	Outlet Devices
#1	Primary	221.29'	12.0" Round Culvert L= 18.2' Ke= 0.500 Inlet / Outlet Invert= 221.29' / 221.20' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.41 cfs @ 12.09 hrs HW=221.72' TW=221.59' (Dynamic Tailwater) 1=Culvert (Outlet Controls 0.41 cfs @ 1.88 fps)

Summary for Pond CB46: CB #46

Inflow Area = 4,397 sf, 50.97% Impervious, Inflow Depth > 6.62" for 100YR event

Inflow = 0.75 cfs @ 12.09 hrs, Volume= 2,426 cf

Outflow = 0.75 cfs @ 12.09 hrs, Volume= 2,426 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.75 cfs @ 12.09 hrs, Volume= 2,426 cf

Routed to Pond D30: DMH #30

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 390

Peak Elev= 222.06' @ 12.09 hrs

Flood Elev= 224.69'

Device	Routing	Invert	Outlet Devices
#1	Primary	221.53'	12.0" Round Culvert L= 15.3' Ke= 0.500 Inlet / Outlet Invert= 221.53' / 221.45' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.73 cfs @ 12.09 hrs HW=222.06' TW=221.59' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.73 cfs @ 2.55 fps)

Summary for Pond CB47: CB#47

Inflow Area = 3,012 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow 0.60 cfs @ 12.09 hrs, Volume= 2.212 cf

0.60 cfs @ 12.09 hrs, Volume= 2,212 cf, Atten= 0%, Lag= 0.0 min 2,212 cf Outflow =

0.60 cfs @ 12.09 hrs, Volume= Primary =

Routed to Pond D31: DMH#31

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 226.92' @ 12.16 hrs

Flood Elev= 230.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	225.05'	12.0" Round Culvert L= 20.9' Ke= 0.500 Inlet / Outlet Invert= 225.05' / 224.95' S= 0.0048 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.60 cfs @ 12.09 hrs HW=226.42' TW=226.40' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.60 cfs @ 0.76 fps)

Summary for Pond CB48: CB#48

Inflow Area = 60,128 sf, 25.16% Impervious, Inflow Depth > 5.38" for 100YR event

7.16 cfs @ 12.16 hrs, Volume= Inflow = 26.943 cf

Outflow 7.16 cfs @ 12.16 hrs, Volume= 26,943 cf, Atten= 0%, Lag= 0.0 min

Primary = 7.16 cfs @ 12.16 hrs, Volume= 26,943 cf

Routed to Pond D31: DMH#31

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 228.37' @ 12.16 hrs

Flood Elev= 230.25'

Device	Routing	Invert	Outlet Devices
#1	Primary	224.82'	15.0" Round Culvert L= 16.9' Ke= 0.500 Inlet / Outlet Invert= 224.82' / 224.74' S= 0.0047 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=7.03 cfs @ 12.16 hrs HW=228.27' TW=226.85' (Dynamic Tailwater) 1=Culvert (Inlet Controls 7.03 cfs @ 5.73 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 391

Summary for Pond CB49: CB #49

Inflow Area = 5,238 sf, 84.59% Impervious, Inflow Depth > 8.33" for 100YR event

Inflow = 1.03 cfs @ 12.09 hrs, Volume= 3,637 cf

Outflow = 1.03 cfs @ 12.09 hrs, Volume= 3,637 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.03 cfs @ 12.09 hrs, Volume= 3,637 cf

Routed to Pond DMH32: DMH #32

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.26' @ 12.09 hrs

Flood Elev= 205.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.76'	12.0" Round Culvert L= 15.5' Ke= 0.500 Inlet / Outlet Invert= 202.76' / 202.68' S= 0.0052 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.00 cfs @ 12.09 hrs HW=204.21' TW=204.14' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.00 cfs @ 1.28 fps)

Summary for Pond CB50: CB #50

Inflow Area = 15,040 sf, 77.20% Impervious, Inflow Depth > 8.21" for 100YR event

Inflow = 2.94 cfs @ 12.09 hrs, Volume= 10,290 cf

Outflow = 2.94 cfs @ 12.09 hrs, Volume= 10,290 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.94 cfs @ 12.09 hrs, Volume= 10,290 cf

Routed to Pond DMH32: DMH #32

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.79' @ 12.09 hrs

Flood Elev= 205.93'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.78'	12.0" Round Culvert L= 15.3' Ke= 0.500
			Inlet / Outlet Invert= 202.78' / 202.70' S= 0.0052 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.86 cfs @ 12.09 hrs HW=204.71' TW=204.14' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.86 cfs @ 3.65 fps)

Summary for Pond CB51: CB #51

Inflow Area = 6,823 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow = 1.36 cfs @ 12.09 hrs, Volume= 5,012 cf

Outflow = 1.36 cfs @ 12.09 hrs, Volume= 5,012 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.36 cfs @ 12.09 hrs, Volume= 5,012 cf

Routed to Pond OCS7: OCS #7

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 392

Peak Elev= 204.55' @ 12.34 hrs

Flood Elev= 212.77'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.35'	12.0" Round Culvert L= 31.4' Ke= 0.500 Inlet / Outlet Invert= 202.35' / 202.19' S= 0.0051 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=1.36 cfs @ 12.09 hrs HW=203.74' TW=203.61' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.36 cfs @ 1.73 fps)

Summary for Pond CB52: CB #52

Inflow Area = 9,052 sf, 87.14% Impervious, Inflow Depth > 8.45" for 100YR event

Inflow = 1.79 cfs @ 12.09 hrs, Volume= 6,376 cf

Outflow = 1.79 cfs @ 12.09 hrs, Volume= 6,376 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.79 cfs @ 12.09 hrs, Volume= 6,376 cf

Routed to Pond OCS7: OCS #7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.56' @ 12.33 hrs

Flood Elev= 205.84'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.68'	12.0" Round Culvert L= 25.5' Ke= 0.500 Inlet / Outlet Invert= 202.68' / 202.55' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior Flow Area= 0.79 sf

Primary OutFlow Max=1.87 cfs @ 12.09 hrs HW=203.85' TW=203.61' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.87 cfs @ 2.38 fps)

Summary for Pond CB53: CB #53

Inflow Area = 7,863 sf, 86.52% Impervious, Inflow Depth > 8.21" for 100YR event

Inflow = 1.54 cfs @ 12.09 hrs, Volume= 5.380 cf

Outflow = 1.54 cfs @ 12.09 hrs, Volume= 5,380 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.54 cfs @ 12.09 hrs, Volume= 5,380 cf

Routed to Pond 3P: OCS #8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.58' @ 12.09 hrs

Flood Elev= 205.95'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.78'	12.0" Round Culvert L= 32.0' Ke= 0.500
			Inlet / Outlet Invert= 202.78' / 202.62' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.50 cfs @ 12.09 hrs HW=203.57' TW=202.18' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.50 cfs @ 3.10 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

Page 393

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Summary for Pond CB54: CB #54

Inflow Area = 4,821 sf, 86.85% Impervious, Inflow Depth > 7.97" for 100YR event

Inflow = 0.93 cfs @ 12.09 hrs, Volume= 3,201 cf

Outflow = 0.93 cfs @ 12.09 hrs, Volume= 3,201 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.93 cfs @ 12.09 hrs, Volume= 3,201 cf

Routed to Pond 3P: OCS #8

Routing by Dyn-Stor-Ind method. Time Span= 0.00-24.00 hrs. dt= 0.05 hrs / 3

Peak Elev= 203.32' @ 12.40 hrs

Flood Elev= 205.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.66'	12.0" Round Culvert L= 36.7' Ke= 0.500 Inlet / Outlet Invert= 202.66' / 202.48' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.91 cfs @ 12.09 hrs HW=203.24' TW=202.18' (Dynamic Tailwater) 1=Culvert (Barrel Controls 0.91 cfs @ 2.74 fps)

Summary for Pond CB7: CB#5

Inflow Area = 4,650 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow = 0.93 cfs @ 12.09 hrs, Volume= 3,416 cf

Outflow = 0.93 cfs @ 12.09 hrs, Volume= 3,416 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.93 cfs @ 12.09 hrs, Volume= 3,416 cf

Routed to Pond D4: DMH#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 215.06' @ 12.14 hrs

Flood Elev= 215.79'

Device	Routing	Invert	Outlet Devices
#1	Primary	212.60'	12.0" Round Culvert L= 15.1' Ke= 0.500
			Inlet / Outlet Invert= 212.60' / 212.45' S= 0.0099 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.92 cfs @ 12.09 hrs HW=214.04' TW=213.78' (Dynamic Tailwater) 1=Culvert (Inlet Controls 1.92 cfs @ 2.44 fps)

Summary for Pond CB8: CB#8

Inflow Area = 5,450 sf, 88.75% Impervious, Inflow Depth > 8.33" for 100YR event
Inflow = 1.07 cfs @ 12.09 hrs, Volume= 3,784 cf
Outflow = 1.07 cfs @ 12.09 hrs, Volume= 3,784 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.07 cfs @ 12.09 hrs, Volume= 3,784 cf

Routed to Pond D4 : DMH#4

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Page 394

Peak Elev= 215.06' @ 12.14 hrs

Flood Elev= 215.79'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.79'	12.0" Round Culvert L= 15.1' Ke= 0.500 Inlet / Outlet Invert= 213.79' / 213.64' S= 0.0099 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=1.30 cfs @ 12.09 hrs HW=214.47' TW=213.79' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.30 cfs @ 3.24 fps)

Summary for Pond CB9: CB #9

Inflow Area = 16,307 sf, 93.95% Impervious, Inflow Depth > 8.69" for 100YR event

Inflow = 3.25 cfs @ 12.09 hrs, Volume= 11,814 cf

Outflow = 3.25 cfs @ 12.09 hrs, Volume= 11,814 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.25 cfs @ 12.09 hrs, Volume= 11,814 cf

Routed to Pond D5: DMH #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 211.34' @ 12.09 hrs

Flood Elev= 213.27'

Device	Routing	Invert	Outlet Devices
#1	Primary	210.10'	12.0" Round Culvert L= 19.9' Ke= 0.500
			Inlet / Outlet Invert= 210.10' / 209.71' S= 0.0196 '/' Cc= 0.900
			n= 0.013 Corrugated PF smooth interior Flow Area= 0.79 sf

Primary OutFlow Max=3.16 cfs @ 12.09 hrs HW=211.30' TW=210.59' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.16 cfs @ 4.03 fps)

Summary for Pond D10: DMH #10

Inflow Area = 8,516 sf, 64.88% Impervious, Inflow Depth > 6.50" for 100YR event

Inflow = 1.43 cfs @ 12.09 hrs, Volume= 4.611 cf

Outflow = 1.43 cfs @ 12.09 hrs, Volume= 4,611 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.43 cfs @ 12.09 hrs, Volume= 4,611 cf

Routed to Pond P207: INFILTRATION POND #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.11' @ 12.09 hrs

Flood Elev= 206.49'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.33'	12.0" Round Culvert L= 15.6' Ke= 0.500 Inlet / Outlet Invert= 203.33' / 203.25' S= 0.0051 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.40 cfs @ 12.09 hrs HW=204.09' TW=198.46' (Dynamic Tailwater) 1=Culvert (Barrel Controls 1.40 cfs @ 3.00 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 395

Summary for Pond D11: DMH #11

Inflow Area = 36,689 sf, 73.28% Impervious, Inflow Depth > 7.38" for 100YR event

Inflow = 6.20 cfs @ 12.09 hrs, Volume= 22,574 cf

Outflow = 6.20 cfs @ 12.09 hrs, Volume= 22,574 cf, Atten= 0%, Lag= 0.0 min

Primary = 6.20 cfs @ 12.09 hrs, Volume= 22,574 cf

Routed to Pond OCS3: OCS#3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.27' @ 12.10 hrs

Flood Elev= 208.33'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.25'	18.0" Round Culvert L= 44.6' Ke= 0.500 Inlet / Outlet Invert= 204.25' / 204.03' S= 0.0049 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=6.09 cfs @ 12.09 hrs HW=207.14' TW=206.62' (Dynamic Tailwater) 1=Culvert (Inlet Controls 6.09 cfs @ 3.44 fps)

Summary for Pond D12: DMH #12

Inflow Area = 22,113 sf, 88.07% Impervious, Inflow Depth > 8.17" for 100YR event

Inflow = 4.31 cfs @ 12.09 hrs, Volume= 15,061 cf

Outflow = 4.31 cfs @ 12.09 hrs, Volume= 15,061 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.31 cfs @ 12.09 hrs, Volume= 15,061 cf

Routed to Pond D13: DMH #13

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.44' @ 12.09 hrs

Flood Elev= 207.78'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.21'	12.0" Round Culvert L= 41.9' Ke= 0.500
	-		Inlet / Outlet Invert= 203.21' / 203.00' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=4.20 cfs @ 12.09 hrs HW=205.35' TW=204.10' (Dynamic Tailwater) 1=Culvert (Outlet Controls 4.20 cfs @ 5.35 fps)

Summary for Pond D13: DMH #13

Inflow Area = 81,632 sf, 72.61% Impervious, Inflow Depth > 7.75" for 100YR event

Inflow = 14.18 cfs @ 12.09 hrs, Volume= 52,717 cf

Outflow = 14.18 cfs @ 12.09 hrs, Volume= 52,717 cf, Atten= 0%, Lag= 0.0 min

Primary = 14.18 cfs @ 12.09 hrs, Volume= 52,717 cf

Routed to Pond P207: INFILTRATION POND #2

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 396

Peak Elev= 204.14' @ 12.09 hrs

Flood Elev= 208.12'

Device	Routing	Invert	Outlet Devices
#1	Primary	201.95'	24.0" Round Culvert L= 60.1' Ke= 0.500 Inlet / Outlet Invert= 201.95' / 201.65' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=14.01 cfs @ 12.09 hrs HW=204.12' TW=198.49' (Dynamic Tailwater) 1=Culvert (Barrel Controls 14.01 cfs @ 5.11 fps)

Summary for Pond D14: DMH #14

Inflow Area = 38,112 sf, 77.40% Impervious, Inflow Depth > 8.23" for 100YR event

Inflow = 7.44 cfs @ 12.09 hrs, Volume= 26,131 cf

Outflow = 7.44 cfs @ 12.09 hrs, Volume= 26,131 cf, Atten= 0%, Lag= 0.0 min

Primary = 7.44 cfs @ 12.09 hrs, Volume= 26,131 cf

Routed to Pond d13: DMH #13

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.77' @ 12.09 hrs

Flood Elev= 208.78'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.13'	18.0" Round Culvert L= 256.3' Ke= 0.500
			Inlet / Outlet Invert= 204.13' / 202.85' S= 0.0050 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior. Flow Area= 1.77 sf

Primary OutFlow Max=7.26 cfs @ 12.09 hrs HW=205.73' TW=204.10' (Dynamic Tailwater) 1=Culvert (Outlet Controls 7.26 cfs @ 4.79 fps)

Summary for Pond D16: DMH #16

Inflow Area = 16,417 sf, 76.69% Impervious, Inflow Depth > 8.27" for 100YR event

Inflow = 3.22 cfs @ 12.09 hrs, Volume= 11,318 cf

Outflow = 3.22 cfs @ 12.09 hrs, Volume= 11,318 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.22 cfs @ 12.09 hrs, Volume= 11,318 cf

Routed to Pond D14: DMH #14

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.22' @ 12.09 hrs

Flood Elev= 208.59'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.90'	15.0" Round Culvert L= 103.5' Ke= 0.500 Inlet / Outlet Invert= 204.90' / 204.38' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.14 cfs @ 12.09 hrs HW=206.19' TW=205.73' (Dynamic Tailwater) 1=Culvert (Outlet Controls 3.14 cfs @ 3.09 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 397

Summary for Pond D17: DMH #17

Inflow Area = 14,946 sf, 75.26% Impervious, Inflow Depth > 8.22" for 100YR event

Inflow = 2.92 cfs @ 12.09 hrs, Volume= 10,232 cf

Outflow = 2.92 cfs @ 12.09 hrs, Volume= 10,232 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.92 cfs @ 12.09 hrs, Volume= 10,232 cf

Routed to Pond D18: DMH #18

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 201.64' @ 12.09 hrs

Flood Elev= 204.84'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.55'	12.0" Round Culvert L= 91.6' Ke= 0.500 Inlet / Outlet Invert= 200.55' / 197.69' S= 0.0312 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.84 cfs @ 12.09 hrs HW=201.61' TW=198.73' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.84 cfs @ 3.62 fps)

Summary for Pond D18: DMH #18

Inflow Area = 25,318 sf, 65.46% Impervious, Inflow Depth > 7.96" for 100YR event

Inflow = 4.87 cfs @ 12.09 hrs, Volume= 16,802 cf

Outflow = 4.87 cfs @ 12.09 hrs, Volume= 16,802 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.87 cfs @ 12.09 hrs, Volume= 16,802 cf

Routed to Pond OCS1: OCS#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 198.76' @ 12.09 hrs

Flood Elev= 201.13'

Device	Routing	Invert	Outlet Devices
#1	Primary	197.44'	15.0" Round Culvert L= 46.3' Ke= 0.500 Inlet / Outlet Invert= 197.44' / 196.98' S= 0.0099 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=4.74 cfs @ 12.09 hrs HW=198.73' TW=196.91' (Dynamic Tailwater) 1=Culvert (Barrel Controls 4.74 cfs @ 4.65 fps)

Summary for Pond D19: DMH #19

Inflow Area = 17,428 sf, 83.29% Impervious, Inflow Depth > 8.33" for 100YR event

Inflow = 3.43 cfs @ 12.09 hrs, Volume= 12,100 cf

Outflow = 3.43 cfs @ 12.09 hrs, Volume= 12,100 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.43 cfs @ 12.09 hrs, Volume= 12,100 cf

Routed to Pond d20: DMH #20

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 398

Peak Elev= 206.75' @ 12.09 hrs

Flood Elev= 208.57'

Device	Routing	Invert	Outlet Devices
#1	Primary	205.19'	12.0" Round Culvert L= 82.5' Ke= 0.500
			Inlet / Outlet Invert= 205.19' / 204.43' S= 0.0092 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.31 cfs @ 12.09 hrs HW=206.66' TW=205.53' (Dynamic Tailwater) 1=Culvert (Outlet Controls 3.31 cfs @ 4.22 fps)

Summary for Pond D2: DMH#2

Inflow Area = 73,240 sf, 37.72% Impervious, Inflow Depth > 5.96" for 100YR event

Inflow = 9.15 cfs @ 12.14 hrs, Volume= 36,354 cf

Outflow = 9.15 cfs @ 12.14 hrs, Volume= 36,354 cf, Atten= 0%, Lag= 0.0 min

Primary = 9.15 cfs @ 12.14 hrs, Volume= 36,354 cf

Routed to Pond P205: INFILTRATION POND #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 209.92' @ 12.14 hrs

Flood Elev= 212.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.90'	15.0" Round Culvert L= 38.2' Ke= 0.500 Inlet / Outlet Invert= 206.90' / 206.52' S= 0.0099 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior Flow Area= 1.23 sf

Primary OutFlow Max=9.07 cfs @ 12.14 hrs HW=209.88' TW=207.03' (Dynamic Tailwater) 1=Culvert (Inlet Controls 9.07 cfs @ 7.39 fps)

Summary for Pond D20: DMH #20

Inflow Area = 17,428 sf, 83.29% Impervious, Inflow Depth > 8.33" for 100YR event

Inflow = 3.43 cfs @ 12.09 hrs, Volume= 12.100 cf

Outflow = 3.43 cfs @ 12.09 hrs, Volume= 12,100 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.43 cfs @ 12.09 hrs, Volume= 12,100 cf

Routed to Pond D21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.57' @ 12.09 hrs

Flood Elev= 207.68'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.19'	15.0" Round Culvert L= 63.5' Ke= 0.500 Inlet / Outlet Invert= 204.19' / 203.87' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.34 cfs @ 12.09 hrs HW=205.53' TW=205.12' (Dynamic Tailwater) 1=Culvert (Outlet Controls 3.34 cfs @ 3.16 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 399

Summary for Pond D21: DMH #21

Inflow Area = 71,317 sf, 79.77% Impervious, Inflow Depth > 8.21" for 100YR event

Inflow = 13.91 cfs @ 12.09 hrs, Volume= 48,812 cf

Outflow = 13.91 cfs @ 12.09 hrs, Volume= 48,812 cf, Atten= 0%, Lag= 0.0 min

Primary = 13.91 cfs @ 12.09 hrs, Volume= 48,812 cf

Routed to Pond p212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.17' @ 12.09 hrs

Flood Elev= 207.55'

Device	Routing	Invert	Outlet Devices
#1	Primary	203.02'	24.0" Round Culvert L= 72.4' Ke= 0.500 Inlet / Outlet Invert= 203.02' / 202.66' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=13.55 cfs @ 12.09 hrs HW=205.12' TW=202.97' (Dynamic Tailwater) 1=Culvert (Barrel Controls 13.55 cfs @ 5.10 fps)

Summary for Pond D22: DMH #22

Inflow Area = 24,814 sf, 89.39% Impervious, Inflow Depth > 8.50" for 100YR event

Inflow = 4.90 cfs @ 12.09 hrs, Volume= 17,574 cf

Outflow = 4.90 cfs @ 12.09 hrs, Volume= 17,574 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.90 cfs @ 12.09 hrs, Volume= 17,574 cf

Routed to Pond d21: DMH #21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.31' @ 12.09 hrs

Flood Elev= 208.46'

Device	Routing	Invert	Outlet Devices
#1	Primary	204.87'	15.0" Round Culvert L= 134.2' Ke= 0.500 Inlet / Outlet Invert= 204.87' / 203.92' S= 0.0071 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=4.78 cfs @ 12.09 hrs HW=206.27' TW=205.12' (Dynamic Tailwater) 1=Culvert (Outlet Controls 4.78 cfs @ 4.34 fps)

Summary for Pond D23: DMH #23

Inflow Area = 10,771 sf, 99.27% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow = 2.15 cfs @ 12.09 hrs, Volume= 7,912 cf

Outflow = 2.15 cfs @ 12.09 hrs, Volume= 7,912 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.15 cfs @ 12.09 hrs, Volume= 7,912 cf

Routed to Pond D22: DMH #22

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 400

Peak Elev= 207.50' @ 12.09 hrs

Flood Elev= 210.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.70'	15.0" Round Culvert L= 173.3' Ke= 0.500 Inlet / Outlet Invert= 206.70' / 204.97' S= 0.0100 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=207.48' TW=206.27' (Dynamic Tailwater) 1=Culvert (Outlet Controls 2.09 cfs @ 3.69 fps)

Summary for Pond D25: DMH #25

Inflow Area = 36,995 sf, 87.96% Impervious, Inflow Depth > 8.28" for 100YR event

Inflow = 7.14 cfs @ 12.09 hrs, Volume= 25,517 cf

Outflow = 7.14 cfs @ 12.09 hrs, Volume= 25,517 cf, Atten= 0%, Lag= 0.0 min

Primary = 7.14 cfs @ 12.09 hrs, Volume= 25,517 cf

Routed to Pond P210: POCKET WETLAND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 212.68' @ 12.09 hrs

Flood Elev= 213.11'

Device	Routing	Invert	Outlet Devices
#1	Primary	209.36'	15.0" Round Culvert L= 237.6' Ke= 0.500 Inlet / Outlet Invert= 209.36' / 208.17' S= 0.0050 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior. Flow Area= 1.23 sf

Primary OutFlow Max=6.95 cfs @ 12.09 hrs HW=212.52' TW=204.08' (Dynamic Tailwater) 1=Culvert (Barrel Controls 6.95 cfs @ 5.67 fps)

Summary for Pond D27: DMH #27

Inflow Area = 21,746 sf, 79.51% Impervious, Inflow Depth > 7.90" for 100YR event

Inflow = 4.10 cfs @ 12.09 hrs, Volume= 14,316 cf

Outflow = 4.10 cfs @ 12.09 hrs, Volume= 14,316 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.10 cfs @ 12.09 hrs, Volume= 14,316 cf

Routed to Pond D35: DMH #35

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 214.46' @ 12.09 hrs

Flood Elev= 217.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	213.34'	15.0" Round Culvert L= 63.9' Ke= 0.500 Inlet / Outlet Invert= 213.34' / 212.38' S= 0.0150 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=4.04 cfs @ 12.09 hrs HW=214.44' TW=213.55' (Dynamic Tailwater) 1=Culvert (Outlet Controls 4.04 cfs @ 4.72 fps)

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 401

Summary for Pond D28: DMH #28

Inflow Area = 11,949 sf, 62.72% Impervious, Inflow Depth > 7.15" for 100YR event

Inflow = 2.14 cfs @ 12.09 hrs, Volume= 7,120 cf

Outflow = 2.14 cfs @ 12.09 hrs, Volume= 7,120 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.14 cfs @ 12.09 hrs, Volume= 7,120 cf

Routed to Pond D27: DMH #27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 218.28' @ 12.09 hrs

Flood Elev= 220.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	217.46'	12.0" Round Culvert L= 158.3' Ke= 0.500 Inlet / Outlet Invert= 217.46' / 214.29' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=218.27' TW=214.44' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.09 cfs @ 3.07 fps)

Summary for Pond D29: DMH #29

Inflow Area = 11,949 sf, 62.72% Impervious, Inflow Depth > 7.15" for 100YR event

Inflow = 2.14 cfs @ 12.09 hrs, Volume= 7,120 cf

Outflow = 2.14 cfs @ 12.09 hrs, Volume= 7,120 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.14 cfs @ 12.09 hrs, Volume= 7,120 cf

Routed to Pond D28: DMH #28

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 220.65' @ 12.09 hrs

Flood Elev= 223.21'

Device	Routing	Invert	Outlet Devices
#1	Primary	219.83'	12.0" Round Culvert L= 150.9' Ke= 0.500 Inlet / Outlet Invert= 219.83' / 217.55' S= 0.0151 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf

Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=220.64' TW=218.27' (Dynamic Tailwater) 1=Culvert (Inlet Controls 2.09 cfs @ 3.07 fps)

Summary for Pond D30: DMH #30

Inflow Area = 6,862 sf, 50.73% Impervious, Inflow Depth > 6.62" for 100YR event

Inflow = 1.17 cfs @ 12.09 hrs, Volume= 3,786 cf

Outflow = 1.17 cfs @ 12.09 hrs, Volume= 3,786 cf, Atten= 0%, Lag= 0.0 min

Primary = 1.17 cfs @ 12.09 hrs, Volume= 3,786 cf

Routed to Pond D29: DMH #29

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 402

Peak Elev= 221.60' @ 12.09 hrs

Flood Elev= 224.95'

Device Routing Invert Outlet Devices

#1 Primary 220.92' **12.0" Round Culvert** L= 184.2' Ke= 0.500

Inlet / Outlet Invert= 220.92' / 220.00' S= 0.005

Inlet / Outlet Invert= 220.92' / 220.00' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.14 cfs @ 12.09 hrs HW=221.59' TW=220.64' (Dynamic Tailwater) 1=Culvert (Outlet Controls 1.14 cfs @ 2.90 fps)

Summary for Pond D31: DMH#31

Inflow Area = 63,140 sf, 28.73% Impervious, Inflow Depth > 5.54" for 100YR event

Inflow = 7.60 cfs @ 12.16 hrs, Volume= 29,155 cf

Outflow = 7.60 cfs @ 12.16 hrs, Volume= 29,155 cf, Atten= 0%, Lag= 0.0 min

Primary = 7.60 cfs @ 12.16 hrs, Volume= 29,155 cf

Routed to Pond D4: DMH#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 226.91' @ 12.16 hrs

Flood Elev= 229.59'

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 224.63'
 15.0" Round Culvert L= 288.5' Ke= 0.500 Inlet / Outlet Invert= 224.63' / 213.09' S= 0.0400 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=7.50 cfs @ 12.16 hrs HW=226.87' TW=214.77' (Dynamic Tailwater) 1=Culvert (Inlet Controls 7.50 cfs @ 6.12 fps)

Summary for Pond D34: DMH #34

Inflow Area = 23,255 sf,100.00% Impervious, Inflow Depth > 8.81" for 100YR event

Inflow = 4.64 cfs @ 12.09 hrs, Volume= 17,082 cf

Outflow = 4.64 cfs @ 12.09 hrs, Volume= 17,082 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.64 cfs @ 12.09 hrs, Volume= 17,082 cf

Routed to Pond OCS1: OCS#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 200.07' @ 12.09 hrs

Flood Elev= 202.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	198.07'	12.0" Round Culvert L= 52.0' Ke= 0.500
	•		Inlet / Outlet Invert= 198.07' / 197.03' S= 0.0200 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=4.52 cfs @ 12.09 hrs HW=200.00' TW=196.90' (Dynamic Tailwater) 1=Culvert (Inlet Controls 4.52 cfs @ 5.75 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 403

Summary for Pond D35: DMH #35

Inflow Area = 21,746 sf, 79.51% Impervious, Inflow Depth > 7.90" for 100YR event

Inflow = 4.10 cfs @ 12.09 hrs, Volume= 14,316 cf

Outflow = 4.10 cfs @ 12.09 hrs, Volume= 14,316 cf, Atten= 0%, Lag= 0.0 min

Primary = 4.10 cfs @ 12.09 hrs, Volume= 14,316 cf

Routed to Pond D25: DMH #25

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 213.61' @ 12.09 hrs

Flood Elev= 215.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	212.28'	15.0" Round Culvert L= 171.5' Ke= 0.500 Inlet / Outlet Invert= 212.28' / 209.71' S= 0.0150 '/' Cc= 0.900 n= 0.012, Flow Area= 1.23 sf

Primary OutFlow Max=4.02 cfs @ 12.09 hrs HW=213.55' TW=212.53' (Dynamic Tailwater) 1=Culvert (Outlet Controls 4.02 cfs @ 4.00 fps)

Summary for Pond D4: DMH#4

Inflow Area = 73,240 sf, 37.72% Impervious, Inflow Depth > 5.96" for 100YR event

Inflow = 9.15 cfs @ 12.14 hrs, Volume= 36,354 cf

Outflow = 9.15 cfs @ 12.14 hrs, Volume= 36,354 cf, Atten= 0%, Lag= 0.0 min

Primary = 9.15 cfs @ 12.14 hrs, Volume= 36,354 cf

Routed to Pond D2: DMH#2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 215.03' @ 12.14 hrs

Flood Elev= 217.64'

Device	Routing	Invert	Outlet Devices
#1	Primary	210.34'	15.0" Round Culvert L= 222.3' Ke= 0.500 Inlet / Outlet Invert= 210.34' / 207.01' S= 0.0150 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=9.07 cfs @ 12.14 hrs HW=214.90' TW=209.88' (Dynamic Tailwater) 1=Culvert (Outlet Controls 9.07 cfs @ 7.39 fps)

Summary for Pond D5: DMH #5

Inflow Area = 30,441 sf, 96.76% Impervious, Inflow Depth > 8.75" for 100YR event

Inflow = 6.07 cfs @ 12.09 hrs, Volume= 22,196 cf

Outflow = 6.07 cfs @ 12.09 hrs, Volume= 22,196 cf, Atten= 0%, Lag= 0.0 min

Primary = 6.07 cfs @ 12.09 hrs, Volume= 22,196 cf

Routed to Pond D6: DMH #6

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 404

Peak Elev= 210.62' @ 12.09 hrs Flood Elev= 212.97'

Device Routing Invert

Device	Routing	Invert	Outlet Devices
#1	Primary	209.09'	18.0" Round Culvert L= 183.0' Ke= 0.500 Inlet / Outlet Invert= 209.09' / 208.17' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=5.90 cfs @ 12.09 hrs HW=210.59' TW=209.48' (Dynamic Tailwater) 1=Culvert (Outlet Controls 5.90 cfs @ 4.16 fps)

Summary for Pond D6: DMH #6

Inflow Area = 30,441 sf, 96.76% Impervious, Inflow Depth > 8.75" for 100YR event

Inflow = 6.07 cfs @ 12.09 hrs, Volume= 22,196 cf

Outflow = 6.07 cfs @ 12.09 hrs, Volume= 22,196 cf, Atten= 0%, Lag= 0.0 min

Primary = 6.07 cfs @ 12.09 hrs, Volume= 22,196 cf

Routed to Pond D7: DMH #7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 209.51' @ 12.09 hrs

Flood Elev= 214.82'

Device	Routing	Invert	Outlet Devices
#1	Primary	208.07'	18.0" Round Culvert L= 299.7' Ke= 0.500 Inlet / Outlet Invert= 208.07' / 206.57' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 1.77 sf

Primary OutFlow Max=5.90 cfs @ 12.09 hrs HW=209.48' TW=207.71' (Dynamic Tailwater) 1=Culvert (Outlet Controls 5.90 cfs @ 4.42 fps)

Summary for Pond D7: DMH #7

Inflow Area = 30,441 sf, 96.76% Impervious, Inflow Depth > 8.75" for 100YR event

Inflow = 6.07 cfs @ 12.09 hrs, Volume= 22.196 cf

Outflow = 6.07 cfs @ 12.09 hrs, Volume= 22,196 cf, Atten= 0%, Lag= 0.0 min

Primary = 6.07 cfs @ 12.09 hrs, Volume= 22,196 cf

Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.73' @ 12.09 hrs

Flood Elev= 213.17'

Device	Routing	Invert	Outlet Devices
#1	Primary	206.47'	18.0" Round Culvert L= 44.2' Ke= 0.500 Inlet / Outlet Invert= 206.47' / 204.04' S= 0.0550 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=5.90 cfs @ 12.09 hrs HW=207.71' TW=202.97' (Dynamic Tailwater) 1=Culvert (Inlet Controls 5.90 cfs @ 3.79 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 405

Summary for Pond D8: DMH #8

Inflow Area = 18,765 sf, 91.12% Impervious, Inflow Depth > 8.27" for 100YR event

Inflow = 3.66 cfs @ 12.09 hrs, Volume= 12,929 cf

Outflow = 3.66 cfs @ 12.09 hrs, Volume= 12,929 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.66 cfs @ 12.09 hrs, Volume= 12,929 cf

Routed to Pond D9: DMH #9

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 203.04' @ 12.09 hrs

Flood Elev= 204.72'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.57'	12.0" Round Culvert L= 87.7' Ke= 0.500 Inlet / Outlet Invert= 200.57' / 200.13' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.57 cfs @ 12.09 hrs HW=202.93' TW=201.57' (Dynamic Tailwater) 1=Culvert (Outlet Controls 3.57 cfs @ 4.54 fps)

Summary for Pond D9: DMH #9

Inflow Area = 18,765 sf, 91.12% Impervious, Inflow Depth > 8.27" for 100YR event

Inflow = 3.66 cfs @ 12.09 hrs, Volume= 12,929 cf

Outflow = 3.66 cfs @ 12.09 hrs, Volume= 12,929 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.66 cfs @ 12.09 hrs, Volume= 12,929 cf

Routed to Pond P207: INFILTRATION POND #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 201.60' @ 12.09 hrs

Flood Elev= 204.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	200.03'	12.0" Round Culvert L= 11.9' Ke= 0.500 Inlet / Outlet Invert= 200.03' / 199.97' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.57 cfs @ 12.09 hrs HW=201.57' TW=198.45' (Dynamic Tailwater) 1=Culvert (Barrel Controls 3.57 cfs @ 4.54 fps)

Summary for Pond DE61: DRIP #61

Inflow Area =	4,247 sf, 92.68% lm	npervious, Inflow Depth > 8	3.57" for 100YR event
Inflow =	0.84 cfs @ 12.09 hrs, '	Volume= 3,034 cf	
Outflow =	0.69 cfs @ 12.15 hrs,	Volume= 2,884 cf,	Atten= 18%, Lag= 3.6 min
Discarded =	0.00 cfs @ 2.75 hrs,	Volume= 211 cf	_
Primary =	0.69 cfs @ 12.15 hrs,	Volume= 2,673 cf	
D 1 1 D		1 A /	

Routed to Reach 8R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 406

Peak Elev= 213.59' @ 12.15 hrs Surf.Area= 665 sf Storage= 372 cf

Plug-Flow detention time= 56.7 min calculated for 2,884 cf (95% of inflow)

Center-of-Mass det. time= 27.9 min (777.6 - 749.6)

Volume	Inve	ert Ava	il.Storage	Storage Descrip	otion	
#1	212.1	19'	539 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
212.1	19	665	0.0	0	0	
212.2	20	665	40.0	3	3	
214.1	19	665	40.0	529	532	
214.2	20	665	100.0	7	539	
Device	Routing	In	vert Out	let Devices		
#1	Primary	214	.10' 180	.0' long x 0.5' br	eadth Broad-Cre	ested Rectangular Weir
	,			ad (feet) 0.20 0.4		
				ef. (English) 2.80		
#2	Primary	212		' Round Culvert		
						S= 0.0050 '/' Cc= 0.900
						rior, Flow Area= 0.20 sf
#3	Discarde	od 212				area Phase-In= 0.01'
#3	Discalde	5u 212	19 U. I 1	o ili/ili Exilitiati	on over Surface	aita Filast-III- 0.01

Discarded OutFlow Max=0.00 cfs @ 2.75 hrs HW=212.21' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.69 cfs @ 12.15 hrs HW=213.58' TW=208.08' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.69 cfs @ 3.49 fps)

Summary for Pond DE62: DRIP #62

Inflow Area =	4,247 sf,	92.68% Impervious,	Inflow Depth > 8.57" for 100YR event				
Inflow =	0.84 cfs @	12.09 hrs, Volume=	3,034 cf				
Outflow =	0.69 cfs @	12.15 hrs, Volume=	2,884 cf, Atten= 18%, Lag= 3.6 min				
Discarded =	0.00 cfs @	2.75 hrs, Volume=	211 cf				
Primary =	0.69 cfs @	12.15 hrs, Volume=	2,673 cf				
Routed to Reach 8R : OVERLAND FLOW							

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 213.59' @ 12.15 hrs Surf.Area= 665 sf Storage= 372 cf

Plug-Flow detention time= 56.7 min calculated for 2,884 cf (95% of inflow) Center-of-Mass det. time= 27.9 min (777.6 - 749.6)

Volume	Invert	Avail.Storage	Storage Description
#1	212.19'	539 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 407

Elevation	Surf.Area	Voids	Inc.Store	Cum.Store
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)
212.19	665	0.0	0	0
212.20	665	40.0	3	3
214.19	665	40.0	529	532
214.20	665	100.0	7	539

Device	Routing	Invert	Outlet Devices
#1	Primary	214.10'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	212.70'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 212.70' / 212.65' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	212.19'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 2.75 hrs HW=212.21' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.69 cfs @ 12.15 hrs HW=213.58' TW=208.08' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Barrel Controls 0.69 cfs @ 3.49 fps)

Summary for Pond DE63: DRIP #63

Inflow Area =	3,013 sf, 88.68% Impervious,	Inflow Depth > 8.45" for 100YR event							
Inflow =	0.60 cfs @ 12.09 hrs, Volume=	2,122 cf							
Outflow =	0.52 cfs @ 12.14 hrs, Volume=	2,032 cf, Atten= 12%, Lag= 3.0 min							
Discarded =	0.00 cfs @ 2.95 hrs, Volume=	127 cf							
Primary =	0.52 cfs @ 12.14 hrs, Volume=	1,906 cf							
Routed to Reach 12R : OVERLAND FLOW									

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 208.20' @ 12.13 hrs Surf.Area= 404 sf Storage= 196 cf

Plug-Flow detention time= 48.1 min calculated for 2,032 cf (96% of inflow) Center-of-Mass det. time= 23.0 min (777.0 - 754.1)

volume	invert Av	all.Storage	Storage Descrip	otion	
#1	206.99'	327 cf	Custom Stage	Data (Prismatic)Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
206.99	404	0.0	0	0	
207.00	404	40.0	2	2	
208.99	404	40.0	322	323	
209.00	404	100.0	4	327	

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 408

Device	Routing	Invert	Outlet Devices
#1	Primary	208.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	207.50'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 207.50' / 207.45' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	206.99'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 2.95 hrs HW=207.01' (Free Discharge) **1 3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.51 cfs @ 12.14 hrs HW=208.19' TW=202.14' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.51 cfs @ 2.62 fps)

Summary for Pond DE64: DRIP #64

Inflow Area = 3,470 sf, 91.59% Impervious, Inflow Depth > 8.57" for 100YR event

Inflow = 0.69 cfs @ 12.09 hrs, Volume= 2,479 cf

2,374 cf, Atten= 13%, Lag= 3.0 min Outflow

0.60 cfs @ 12.14 hrs, Volume= 0.00 cfs @ 2.55 hrs, Volume= Discarded = 149 cf 0.60 cfs @ 12.14 hrs, Volume= 2.224 cf Primary

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 206.28' @ 12.14 hrs Surf.Area= 470 sf Storage= 242 cf

Plug-Flow detention time= 49.3 min calculated for 2,369 cf (96% of inflow)

Center-of-Mass det. time= 24.3 min (774.0 - 749.6)

470 100.0

207.00

Volume	Invert A	vail.Storage	Storage Descrip	otion		
#1	204.99'	381 cf	Custom Stage	Data (Prismatic)	Listed below (Recalc)	
Elevation (feet)	Surf.Are (sq-f		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)		
204.99	47	0.0	0	0		
205.00	47	0 40.0	2	2		
206.99	47	0 40.0	374	376		

5

381

Device	Routing	Invert	Outlet Devices
#1	Primary	206.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	205.50'	6.0" Round Culvert L= 10.0' Ke= 0.500
	•		Inlet / Outlet Invert= 205.50' / 205.45' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	204.99'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 409

Discarded OutFlow Max=0.00 cfs @ 2.55 hrs HW=205.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.59 cfs @ 12.14 hrs HW=206.27' TW=202.13' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.59 cfs @ 2.98 fps)

Summary for Pond DE65: DRIP #65

Inflow Area = 3,016 sf, 88.69% Impervious, Inflow Depth > 8.45" for 100YR event

Inflow = 0.60 cfs @ 12.09 hrs, Volume= 2,124 cf

Outflow = 0.52 cfs @ 12.14 hrs, Volume= 2,034 cf, Atten= 12%, Lag= 3.0 min

Discarded = 0.00 cfs @ 2.95 hrs, Volume= 127 cf Primary = 0.52 cfs @ 12.14 hrs, Volume= 1,908 cf

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 207.21' @ 12.13 hrs Surf.Area= 404 sf Storage= 196 cf

Plug-Flow detention time= 47.7 min calculated for 2,030 cf (96% of inflow)

Center-of-Mass det. time= 23.0 min (777.0 - 754.1)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	205.9	99'	327 c	Custom Stage	Data (Prismatic	Listed below (Recalc)
Elevation (feet		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
205.99	9	404	0.0	0	0	
206.00	0	404	40.0	2	2	
207.99	9	404	40.0	322	323	
208.00	0	404	100.0	4	327	
Device	Routing	In	ıvert Oı	ıtlet Devices		
#1	Primary	207		_		ested Rectangular Weir
			He	ad (feet) 0.20 0.4	10 060 080 1 <i>0</i>)()

#1	Primary	207.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	206.50'	6.0" Round Culvert L= 10.0' Ke= 0.500
	•		Inlet / Outlet Invert= 206.50' / 206.45' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	205.99'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 2.95 hrs HW=206.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.51 cfs @ 12.14 hrs HW=207.19' TW=202.14' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.51 cfs @ 2.62 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 410

Summary for Pond DE66: DRIP #66

Inflow Area = 3,407 sf, 91.46% Impervious, Inflow Depth > 8.57" for 100YR event

Inflow 0.68 cfs @ 12.09 hrs, Volume= 2.434 cf

Outflow 0.59 cfs @ 12.14 hrs, Volume= 2,329 cf, Atten= 13%, Lag= 3.0 min

Discarded = 0.00 cfs @ 2.10 hrs, Volume= 149 cf 0.58 cfs @ 12.14 hrs, Volume= Primary = 2,180 cf

Routed to Reach 12R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 209.07' @ 12.14 hrs Surf.Area= 470 sf Storage= 240 cf

Plug-Flow detention time= 50.3 min calculated for 2,329 cf (96% of inflow)

Center-of-Mass det. time= 24.6 min (774.2 - 749.6)

Volume Invert Avail.Storage Storage Description	
#1 207.79' 381 cf Custom Stage Data (Prismatic)Listed below (Recal	c)
Elevation Surf.Area Voids Inc.Store Cum.Store	
(feet) (sq-ft) (%) (cubic-feet) (cubic-feet)	
207.79 470 0.0 0 0	
207.80 470 40.0 2 2	
209.79 470 40.0 374 376	
209.80 470 100.0 5 381	
Device Routing Invert Outlet Devices	
#1 Primary 209.70' 180.0' long x 0.5' breadth Broad-Crested Rectangular	Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00	
Coef. (English) 2.80 2.92 3.08 3.30 3.32	
#2 Primary 208.30' 6.0" Round Culvert L= 10.0' Ke= 0.500	
Inlet / Outlet Invert= 208.30' / 208.25' S= 0.0050 '/' Cc=	0.900
n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.2	
#3 Discarded 207.79' 0.170 in/hr Exfiltration over Surface area Phase-In= 0.	

Discarded OutFlow Max=0.00 cfs @ 2.10 hrs HW=207.80' (Free Discharge) **T_3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.57 cfs @ 12.14 hrs HW=209.05' TW=202.14' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.57 cfs @ 2.93 fps)

Summary for Pond DE67: DRIP #67

Inflow Area = 3,481 sf, 91.61% Impervious, Inflow Depth > 8.57" for 100YR event Inflow 0.69 cfs @ 12.09 hrs, Volume= 2,487 cf 0.60 cfs @ 12.14 hrs, Volume= Outflow 2,382 cf, Atten= 13%, Lag= 3.0 min 0.00 cfs @ 2.55 hrs, Volume= Discarded = 149 cf Primary = 0.60 cfs @ 12.14 hrs, Volume= 2,232 cf

Routed to Reach 8R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 411

Peak Elev= 209.28' @ 12.14 hrs Surf.Area= 470 sf Storage= 242 cf

Plug-Flow detention time= 49.2 min calculated for 2,377 cf (96% of inflow)

Center-of-Mass det. time= 24.3 min (773.9 - 749.6)

Volume	Inve	ert Avai	il.Storage	Storage Descrip	otion	
#1	207.9	9'	381 cf	Custom Stage	Data (Prismatic)	isted below (Recalc)
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
207.9	99	470	0.0	0	0	
208.0	00	470	40.0	2	2	
209.9	99	470	40.0	374	376	
210.0	00	470	100.0	5	381	
Device	Routing	In	vert Out	let Devices		
#1	Primary	209	9.90' 180	.0' long x 0.5' br	eadth Broad-Cre	sted Rectangular Weir
	,			_	0 0.60 0.80 1.00	
				` '	2.92 3.08 3.30	
#2	Primary	208		` ,	L= 10.0' Ke= 0.	
₩2	1 minary	200				S= 0.0050 '/'
110	D:			•		or, Flow Area= 0.20 sf
#3	Discarde	ea 207	'.99' 0.1 7	(U in/nr Exfiltration	on over Surface	area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 2.55 hrs HW=208.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.59 cfs @ 12.14 hrs HW=209.27' TW=208.08' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.59 cfs @ 2.99 fps)

Summary for Pond DE68: DRIP #68

Inflow Area =	4,212 sf	, 92.62% Impervious,	Inflow Depth > 8.57" for 100YR event
Inflow =	0.84 cfs @	12.09 hrs, Volume=	3,009 cf
Outflow =	0.68 cfs @	12.15 hrs, Volume=	2,863 cf, Atten= 19%, Lag= 3.7 mir
Discarded =	0.00 cfs @	2.75 hrs, Volume=	211 cf
Primary =	0.68 cfs @	12.15 hrs, Volume=	2,652 cf
Routed to Pond	d OCS4 : OCS	S#4	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 208.26' @ 12.15 hrs Surf.Area= 665 sf Storage= 338 cf

Plug-Flow detention time= 54.9 min calculated for 2,863 cf (95% of inflow) Center-of-Mass det. time= 26.5 min (776.1 - 749.6)

Volume	Invert	Avail.Storage	Storage Description
#1	206.99'	539 cf	Custom Stage Data (Prismatic)Listed below (Recalc)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 412

Elevation	Surf.Area	Voids	Inc.Store	Cum.Store
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)
206.99	665	0.0	0	0
207.00	665	40.0	3	3
208.99	665	40.0	529	532
209.00	665	100.0	7	539

Device	Routing	Invert	Outlet Devices
#1	Primary	208.90'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	207.50'	6.0" Round Culvert L= 20.0' Ke= 0.500
			Inlet / Outlet Invert= 207.50' / 206.00' S= 0.0750 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	206.99'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 2.75 hrs HW=207.01' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.67 cfs @ 12.15 hrs HW=208.26' TW=205.72' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Inlet Controls 0.67 cfs @ 3.44 fps)

Summary for Pond DE69: DRIP #69

Inflow Area =	3,480 sf	, 91.61% Impervious,	Inflow Depth > 8.57"	for 100YR event		
Inflow =	0.69 cfs @	12.09 hrs, Volume=	2,486 cf			
Outflow =	0.60 cfs @	12.14 hrs, Volume=	2,381 cf, Atte	n= 13%, Lag= 3.0 min		
Discarded =	0.00 cfs @	2.55 hrs, Volume=	149 cf	_		
Primary =	0.60 cfs @	12.14 hrs, Volume=	2,232 cf			
Routed to Pond P212: INFILTRATION POND #1						

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 206.78' @ 12.14 hrs Surf.Area= 470 sf Storage= 242 cf

Plug-Flow detention time= 49.2 min calculated for 2,376 cf (96% of inflow) Center-of-Mass det. time= 24.3 min (773.9 - 749.6)

Volume	Invert Ava	il.Storage	Storage Descrip	tion	
#1	205.49'	381 cf	Custom Stage	Data (Prismatic)	_isted below (Recalc)
Elevation	Surf.Area	Voids	Inc.Store	Cum.Store	
(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
205.49	470	0.0	0	0	
205.50	470	40.0	2	2	
207.49	470	40.0	374	376	
207.50	470	100.0	5	381	

#3

Discarded

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 413

Device	Routing	Invert	Outlet Devices
#1	Primary	207.40'	180.0' long x 0.5' breadth Broad-Crested Rectangular Weir
	•		Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#2	Primary	206.00'	6.0" Round Culvert L= 10.0' Ke= 0.500
			Inlet / Outlet Invert= 206.00' / 205.95' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf
#3	Discarded	205.49'	0.170 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 2.55 hrs HW=205.51' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.59 cfs @ 12.14 hrs HW=206.77' TW=203.04' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.59 cfs @ 2.99 fps)

Summary for Pond DE70: DRIP #70

Inflow Area = 3,476 sf, 91.60% Impervious, Inflow Depth > 8.57" for 100YR event Inflow 0.69 cfs @ 12.09 hrs, Volume= 2,483 cf 0.60 cfs @ 12.14 hrs, Volume= Outflow 2,378 cf, Atten= 13%, Lag= 3.0 min 2.05 hrs, Volume= Discarded = 0.00 cfs @ 149 cf 0.60 cfs @ 12.14 hrs, Volume= Primary 2.229 cf Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.18' @ 12.14 hrs Surf.Area= 470 sf Storage= 242 cf

Plug-Flow detention time= 49.2 min calculated for 2,373 cf (96% of inflow) Center-of-Mass det. time= 24.3 min (773.9 - 749.6)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	205.8	39'	381 cf	Custom Stage	Data (Prismatic	Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
205.8	89	470	0.0	0	0	
205.9	0	470	40.0	2	2	
207.8	89	470	40.0	374	376	
207.9	00	470	100.0	5	381	
Device	Routing	In	vert Ou	tlet Devices		
#1	Primary	207	7.80' 180).0' long x 0.5' br	eadth Broad-Cr	ested Rectangular Weir
	-		He	ad (feet) 0.20 0.4	0 0.60 0.80 1.0	00
			Co	ef. (English) 2.80	2.92 3.08 3.30	3.32
#2	Primary	206		" Round Culvert t / Outlet Invert= 2		0.500 S= 0.0050 '/' Cc= 0.900

n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

205.89' **0.170** in/hr Exfiltration over Surface area Phase-In= 0.01'

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 414

Discarded OutFlow Max=0.00 cfs @ 2.05 hrs HW=205.90' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.59 cfs @ 12.14 hrs HW=207.17' TW=203.04' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.59 cfs @ 2.99 fps)

Summary for Pond DE71: DRIP #71

Inflow Area = 4,210 sf, 92.61% Impervious, Inflow Depth > 8.57" for 100YR event
Inflow = 0.84 cfs @ 12.09 hrs, Volume= 3,008 cf
Outflow = 0.69 cfs @ 12.15 hrs, Volume= 2,858 cf, Atten= 18%, Lag= 3.6 min
Discarded = 0.00 cfs @ 3.10 hrs, Volume= 211 cf
Primary = 0.68 cfs @ 12.15 hrs, Volume= 2,647 cf
Routed to Pond P212 : INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.88' @ 12.15 hrs Surf.Area= 665 sf Storage= 370 cf

Plug-Flow detention time= 56.6 min calculated for 2,852 cf (95% of inflow) Center-of-Mass det. time= 28.0 min (777.7 - 749.6)

Volume	Inv	ert Ava	il.Storage	Storage Descrip	otion	
#1	206.4	49'	805 cf	Custom Stage	Data (Prismatic)Lis	ted below (Recalc)
Elevation	n	Surf.Area	Voids	Inc.Store	Cum.Store	
(fee		(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
206.4	19	665	0.0	0	0	
206.5	50	665	40.0	3	3	
209.4	19	665	40.0	795	798	
209.5	50	665	100.0	7	805	
Device	Routing	In	vert Out	tlet Devices		
#1	Primary	209	9.40' 180	0.0' long x 0.5' br	eadth Broad-Crest	ted Rectangular Weir
			Hea	ad (feet) 0.20 0.4	0 0.60 0.80 1.00	
			Coe	ef. (English) 2.80	2.92 3.08 3.30 3.3	32
#2	Primary	207	7.00' 6.0 '	" Round Culvert	L= 10.0' Ke= 0.50	00

Inlet / Outlet Invert= 207.00' / 206.95' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

206.49' **0.170 in/hr Exfiltration over Surface area** Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 3.10 hrs HW=206.52' (Free Discharge)

3=Exfiltration (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.68 cfs @ 12.15 hrs HW=207.88' TW=203.05' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Barrel Controls 0.68 cfs @ 3.47 fps)

#3

Discarded

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 415

Summary for Pond DECH: DRIP #CH

Inflow Area = 6,262 sf, 92.70% Impervious, Inflow Depth > 8.57" for 100YR event

Inflow = 1.24 cfs @ 12.09 hrs, Volume= 4,474 cf

Outflow = 1.08 cfs @ 12.20 hrs, Volume= 4,473 cf, Atten= 13%, Lag= 6.6 min

Discarded = 0.04 cfs @ 8.85 hrs, Volume= 2,032 cf Primary = 1.05 cfs @ 12.20 hrs, Volume= 2,441 cf

Routed to Pond CB18: CB #18

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 210.92' @ 12.18 hrs Surf.Area= 636 sf Storage= 746 cf

Plug-Flow detention time= 23.0 min calculated for 4,464 cf (100% of inflow)

Avail Chanana Chanana Daganindian

Center-of-Mass det. time= 22.8 min (772.5 - 749.6)

Volume	Inv	ert Ava	II.Storage	Storage Descrip	otion		
#1	207.9	99'	770 cf	Custom Stage	Data (Prismatic)Li	sted below (Recalc)	
Elevation	on	Surf.Area	Voids	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)		
207.9	99	636	0.0	0	0		
208.0	00	636	40.0	3	3		
210.9	99	636	40.0	761	763		
211.0	00	636	100.0	6	770		
Device	Routing	In	vert Out	let Devices			
#1	Primary	210).90' 160	.0' long x 0.5' br	eadth Broad-Cres	sted Rectangular Weir	
	,		Hea	ad (feet) 0.20 0.4	0 0.60 0.80 1.00 2.92 3.08 3.30 3	•	
#2	Primary	208		`	L= 80.0' Ke= 0.5		
	, ,					S= 0.0425 '/' Cc= 0.900	
			n= (0.013 Corrugated	PE, smooth interio	or, Flow Area= 0.09 sf	
#3	Discarde	ed 207	7.99' 2.4 '	10 in/hr Exfiltration	on over Surface a	rea Phase-In= 0.01'	

Discarded OutFlow Max=0.04 cfs @ 8.85 hrs HW=208.02' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.04 cfs)

Primary OutFlow Max=1.03 cfs @ 12.20 hrs HW=210.91' TW=206.78' (Dynamic Tailwater)

1=Broad-Crested Rectangular Weir (Weir Controls 0.62 cfs @ 0.31 fps)

-2=Culvert (Outlet Controls 0.40 cfs @ 4.64 fps)

Summary for Pond DMH32: DMH #32

Inflow Area = 20,278 sf, 79.11% Impervious, Inflow Depth > 8.24" for 100YR event

Inflow = 3.97 cfs @ 12.09 hrs, Volume= 13,927 cf

Outflow = 3.97 cfs @ 12.09 hrs, Volume= 13,927 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.97 cfs @ 12.09 hrs, Volume= 13,927 cf

Routed to Pond P212: INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Type III 24-hr 100YR Rainfall=9.06"

19097 Post-Development

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 416

Peak Elev= 204.19' @ 12.09 hrs Flood Elev= 206.16'

Device	Routing	Invert	Outlet Devices
#1	Primary	202.59'	12.0" Round Culvert L= 19.2' Ke= 0.500 Inlet / Outlet Invert= 202.59' / 201.57' S= 0.0531 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.87 cfs @ 12.09 hrs HW=204.14' TW=202.97' (Dynamic Tailwater) 1=Culvert (Inlet Controls 3.87 cfs @ 4.92 fps)

Summary for Pond OCS1: OCS#1

Inflow Area = 48,573 sf, 81.99% Impervious, Inflow Depth > 8.37" for 100YR event

Inflow = 9.51 cfs @ 12.09 hrs, Volume= 33,884 cf

Outflow = 9.51 cfs @ 12.09 hrs, Volume= 33,884 cf, Atten= 0%, Lag= 0.0 min

Primary = 9.51 cfs @ 12.09 hrs, Volume= 33,884 cf Routed to Pond P206 : STORMTECH INFILTRATION SYSTEM #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 196.96' @ 12.11 hrs

Flood Elev= 201.48'

Device Routing Invert Outlet Devices

#1 Primary 195.00' **24.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=9.29 cfs @ 12.09 hrs HW=196.91' TW=196.52' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 9.29 cfs @ 3.01 fps)

Summary for Pond OCS3: OCS#3

Inflow Area = 54,250 sf, 81.93% Impervious, Inflow Depth > 7.85" for 100YR event

Inflow = 9.71 cfs @ 12.09 hrs, Volume= 35,473 cf

Outflow = 9.71 cfs @ 12.09 hrs, Volume= 35,473 cf, Atten= 0%, Lag= 0.0 min

Primary = 9.71 cfs @ 12.09 hrs, Volume= 35,473 cf Routed to Pond p204 : STORMTECH INFILTRATION SYSTEM #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 206.75' @ 12.11 hrs

Flood Elev= 209.00'

Device Routing Invert Outlet Devices

#1 Primary 203.10' **18.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=9.49 cfs @ 12.09 hrs HW=206.62' TW=205.37' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 9.49 cfs @ 5.37 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 417

Summary for Pond OCS4: OCS#4

Inflow Area = 17,972 sf, 28.85% Impervious, Inflow Depth > 6.37" for 100YR event

Inflow = 2.79 cfs @ 12.10 hrs, Volume= 9,535 cf

Outflow = 2.79 cfs @ 12.10 hrs, Volume= 9,535 cf, Atten= 0%, Lag= 0.0 min

Primary = 2.79 cfs @ 12.10 hrs, Volume= 9,535 cf Routed to Pond P204 : STORMTECH INFILTRATION SYSTEM #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 205.75' @ 12.17 hrs

Flood Elev= 208.00'

Device Routing Invert Outlet Devices

#1 Primary 203.10' **18.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=2.77 cfs @ 12.10 hrs HW=205.54' TW=205.44' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 2.77 cfs @ 1.57 fps)

Summary for Pond OCS6: OCS #6

Inflow Area = 16,111 sf, 93.77% Impervious, Inflow Depth > 8.65" for 100YR event

Inflow = 3.20 cfs @ 12.09 hrs, Volume= 11,616 cf

Outflow = 3.20 cfs @ 12.09 hrs, Volume= 11,616 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.20 cfs @ 12.09 hrs, Volume= 11,616 cf

Routed to Pond P213: Stormtech Infiltration System #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.53' @ 12.34 hrs

Flood Elev= 206.96'

Device Routing Invert Outlet Devices

#1 Primary 201.20' **12.0" Vert. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=3.12 cfs @ 12.09 hrs HW=203.63' TW=202.95' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 3.12 cfs @ 3.97 fps)

Summary for Pond OCS7: OCS #7

Inflow Area = 15,875 sf, 92.67% Impervious, Inflow Depth > 8.61" for 100YR event

Inflow = 3.15 cfs @ 12.09 hrs, Volume= 11,388 cf

Outflow = 3.15 cfs @ 12.09 hrs, Volume= 11,388 cf, Atten= 0%, Lag= 0.0 min

Primary = 3.15 cfs @ 12.09 hrs, Volume= 11,388 cf

Routed to Pond P213: Stormtech Infiltration System #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Peak Elev= 204.53' @ 12.34 hrs

Flood Elev= 206.47'

Device	Routing	Invert	Outlet Devices		
#1	Drimary	201 78'	12 0" Vert Orifice/Grate	C= 0.600	Limited to weir flow at low heads

Primary OutFlow Max=3.07 cfs @ 12.09 hrs HW=203.61' TW=202.95' (Dynamic Tailwater) 1=Orifice/Grate (Orifice Controls 3.07 cfs @ 3.90 fps)

Summary for Pond P204: STORMTECH INFILTRATION SYSTEM #1

72,222 sf, 68.72% Impervious, Inflow Depth > 7.48" for 100YR event Inflow Area = 12.49 cfs @ 12.09 hrs, Volume= 45,008 cf Inflow = 9.06 cfs @ 12.18 hrs, Volume= Outflow 42,342 cf, Atten= 27%, Lag= 5.6 min 0.09 cfs @ 6.70 hrs, Volume= Discarded = 6,554 cf Primary = 8.98 cfs @ 12.18 hrs, Volume= 35,788 cf Routed to Reach 20r: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 205.68' @ 12.18 hrs Surf.Area= 5,670 sf Storage= 11,436 cf Flood Elev= 208.75' Surf.Area= 5,670 sf Storage= 13,379 cf

Plug-Flow detention time= 78.8 min calculated for 42,342 cf (94% of inflow) Center-of-Mass det. time= 46.3 min (812.3 - 766.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	202.50'	5,923 cf	77.50'W x 67.70'L x 4.08'H STORMTECH SC-740
			21,423 cf Overall - 6,615 cf Embedded = 14,808 cf x 40.0% Voids
#2A	203.08'	6,615 cf	ADS_StormTech SC-740 +Cap x 144 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			144 Chambers in 16 Rows
#3B	202.50'	427 cf	6.25'W x 67.70'L x 3.50'H ISOLATOR ROW
			1,481 cf Overall - 413 cf Embedded = 1,067 cf x 40.0% Voids
#4B	203.00'	413 cf	ADS_StormTech SC-740 +Cap x 9 Inside #3
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
		40.070 - 6	Tatal Assailable Otenana

13,379 cf Total Available Storage

Storage Group A created with Chamber Wizard Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	202.75'	15.0" Round Culvert L= 35.0' Ke= 0.500
	•		Inlet / Outlet Invert= 202.75' / 201.00' S= 0.0500 '/' Cc= 0.900
			n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf
#2	Device 1	204.75'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Device 1	203.25'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	202.50'	0.660 in/hr Exfiltration over Surface area Phase-In= 0.01'

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Printed 6/19/2023

Page 419

Discarded OutFlow Max=0.09 cfs @ 6.70 hrs HW=202.56' (Free Discharge) **4=Exfiltration** (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=8.95 cfs @ 12.18 hrs HW=205.67' TW=200.25' (Dynamic Tailwater)

-1=Culvert (Inlet Controls 8.95 cfs @ 7.29 fps)

2=Broad-Crested Rectangular Weir (Passes < 11.63 cfs potential flow)

-3=Orifice/Grate (Passes < 2.43 cfs potential flow)

Summary for Pond P205: INFILTRATION POND #3

Inflow Area = 88,676 sf, 39.42% Impervious, Inflow Depth > 6.05" for 100YR event

Inflow = 11.50 cfs @ 12.12 hrs, Volume= 44,711 cf

Outflow = 10.54 cfs @ 12.18 hrs, Volume= 38,922 cf, Atten= 8%, Lag= 3.6 min

Discarded = 0.32 cfs @ 12.18 hrs, Volume= 16,679 cf Primary = 10.22 cfs @ 12.18 hrs, Volume= 22,242 cf

Routed to Reach 18R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 207.05' @ 12.18 hrs Surf.Area= 5,673 sf Storage= 9,818 cf

Plug-Flow detention time= 106.9 min calculated for 38,841 cf (87% of inflow)

Center-of-Mass det. time= 49.0 min (852.1 - 803.0)

Volume	Inve	rt Avail.Sto	rage Storage	Description	
#1	205.00	0' 15,6	30 cf Custom	Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
205.0	00	3,939	0	0	
206.0	00	4,756	4,348	4,348	
207.0	00	5,627	5,192	9,539	
208.0	00	6,555	6,091	15,630	
Device	Routing	Invert	Outlet Device	S	
#1	Primary	206.65'	15.0' long x	15.0' breadth Bi	road-Crested Rectangular Weir
			Head (feet) 0	0.20 0.40 0.60 0).80 1.00 1.20 1.40 1.60
			Coef. (English	n) 2.68 2.70 2.7	70 2.64 2.63 2.64 2.64 2.63
#2	Discarded	205.00'	2.410 in/hr E	xfiltration over	Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.32 cfs @ 12.18 hrs HW=207.05' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.32 cfs)

Primary OutFlow Max=10.09 cfs @ 12.18 hrs HW=207.05' TW=203.11' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 10.09 cfs @ 1.70 fps)

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 420

Summary for Pond P206: STORMTECH INFILTRATION SYSTEM #2

Inflow Area = 59,746 sf, 80.70% Impervious, Inflow Depth > 8.34" for 100YR event

Inflow = 11.69 cfs @ 12.09 hrs, Volume= 41,528 cf

Outflow = 9.22 cfs @ 12.16 hrs, Volume= 41,523 cf, Atten= 21%, Lag= 4.2 min

Discarded = 0.49 cfs @ 10.15 hrs, Volume= 26,156 cf Primary = 8.72 cfs @ 12.16 hrs, Volume= 15,367 cf

Routed to Link AP4: ANALYSIS POINT #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 196.64' @ 12.16 hrs Surf.Area= 6,072 sf Storage= 8,490 cf

Plug-Flow detention time= 54.2 min calculated for 41,523 cf (100% of inflow)

Center-of-Mass det. time= 54.1 min (808.9 - 754.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	194.67'	1,786 cf	39.50'W x 53.46'L x 3.33'H FIELD A
			7,038 cf Overall - 2,573 cf Embedded = 4,466 cf x 40.0% Voids
#2A	195.00'	2,573 cf	ADS_StormTech SC-740 +Cap x 56 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			56 Chambers in 8 Rows
#3B	194.67'	3,296 cf	58.50'W x 67.70'L x 3.33'H FIELD B
			13,201 cf Overall - 4,962 cf Embedded = 8,239 cf x 40.0% Voids
#4B	195.00'	4,962 cf	
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			108 Chambers in 12 Rows
		10010 6	T () A ()) O(

12,616 cf Total Available Storage

Storage Group A created with Chamber Wizard Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	194.65'	18.0" Round Culvert L= 30.0' Ke= 0.200
	-		Inlet / Outlet Invert= 194.65' / 194.50' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.77 sf
#2	Device 1	195.85'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Discarded	194.67'	3.500 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.49 cfs @ 10.15 hrs HW=194.71' (Free Discharge) **3=Exfiltration** (Exfiltration Controls 0.49 cfs)

Primary OutFlow Max=8.69 cfs @ 12.16 hrs HW=196.63' TW=0.00' (Dynamic Tailwater)

1=Culvert (Passes 8.69 cfs of 8.71 cfs potential flow)

²⁼Sharp-Crested Rectangular Weir (Weir Controls 8.69 cfs @ 2.89 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 421

Summary for Pond P207: INFILTRATION POND #2

Inflow Area = 129,716 sf, 63.13% Impervious, Inflow Depth > 7.72" for 100YR event

Inflow 23.17 cfs @ 12.09 hrs, Volume= 83.435 cf

11.39 cfs @ 12.28 hrs, Volume= Outflow 83,408 cf, Atten= 51%, Lag= 11.2 min

Discarded = 1.15 cfs @ 12.28 hrs, Volume= 51,829 cf 10.24 cfs @ 12.28 hrs, Volume= Primary 31,579 cf

Routed to Reach 10R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 198.98' @ 12.28 hrs Surf.Area= 13,476 sf Storage= 25,795 cf

Plug-Flow detention time= 81.6 min calculated for 83,235 cf (100% of inflow)

Center-of-Mass det. time= 81.2 min (853.3 - 772.1)

<u>Volume</u>	Inver	t Avail.Sto	<u>rage Storage</u>	Description	
#1	196.80)' 40,26	60 cf Custom	Stage Data (Pi	rismatic)Listed below (Recalc)
Elevation	on S	Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
196.8	30	10,100	0	0	
198.0	00	12,000	13,260	13,260	
200.0	00	15,000	27,000	40,260	
Device	Routing	Invert	Outlet Device	S	
#1	Primary	198.80'	20.0' long x	21.0' breadth B	Broad-Crested Rectangular Weir
			Head (feet) (0.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60
					70 2.64 2.63 2.64 2.64 2.63
#2	Primary	194.75'		d Culvert L= 40	
					194.55' S= 0.0050 '/' Cc= 0.900
				,	ooth interior, Flow Area= 1.23 sf
#3	Device 2	198.80'			ate X 6.00 columns
					48.0" Grate (56% open area)
				ir flow at low hea	
#4	Device 2	197.40'			0.600 Limited to weir flow at low heads
#5	Discarded	196.80'	3.690 in/hr E	xfiltration over	Surface area Phase-In= 0.01'

Discarded OutFlow Max=1.15 cfs @ 12.28 hrs HW=198.98' (Free Discharge) **5=Exfiltration** (Exfiltration Controls 1.15 cfs)

Primary OutFlow Max=10.13 cfs @ 12.28 hrs HW=198.98' TW=192.58' (Dynamic Tailwater)

-1=Broad-Crested Rectangular Weir (Weir Controls 4.17 cfs @ 1.14 fps)

-2=Culvert (Passes 5.95 cfs of 11.22 cfs potential flow)

3=Orifice/Grate (Weir Controls 4.07 cfs @ 1.40 fps)
4=Orifice/Grate (Orifice Controls 1.88 cfs @ 5.38 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 422

Summary for Pond P210: POCKET WETLAND #1

Inflow Area = 62,582 sf, 52.00% Impervious, Inflow Depth > 7.20" for 100YR event

Inflow 10.93 cfs @ 12.09 hrs, Volume= 37.524 cf

Outflow 3.51 cfs @ 12.40 hrs, Volume= 20,698 cf, Atten= 68%, Lag= 18.8 min

3.51 cfs @ 12.40 hrs, Volume= Primary 20,698 cf

Routed to Reach 15R: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3

Starting Elev= 201.00' Surf.Area= 376 sf Storage= 591 cf

Peak Elev= 204.66' @ 12.40 hrs Surf.Area= 9,782 sf Storage= 20,706 cf (20,115 cf above start)

Plug-Flow detention time= 286.6 min calculated for 20,065 cf (53% of inflow)

Avail.Storage Storage Description

Center-of-Mass det. time= 160.9 min (933.1 - 772.2)

Invert

Volume

#1	199.0	00' 43,1	90 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevation		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
199.0	00	218	0	0	
200.0	00	294	256	256	
201.0	00	376	335	591	
202.0	00	3,991	2,184	2,775	
204.0	00	8,073	12,064	14,839	
206.0	00	13,272	21,345	36,184	
206.5	50	14,753	7,006	43,190	
Device	Routing	Invert	Outlet Device	S	
#1	Primary	205.10	20.0' long x	15.0' breadth B	road-Crested Rectangular Weir
	-		Head (feet) 0	.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60
			Coef. (English	n) 2.68 2.70 2.	70 2.64 2.63 2.64 2.64 2.63
#2	Primary	202.25'	12.0" Round	Culvert L= 44	.0' Ke= 0.500
			Inlet / Outlet I	nvert= 202.25' /	202.03' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Cor	rugated PE, sm	ooth interior, Flow Area= 0.79 sf
#3	Device 2	202.30	2.5" Vert. Ori	fice/Grate C=	0.600 Limited to weir flow at low heads
#4	Device 2	204.50	6.0" x 6.0" He	oriz. Orifice/Gra	ate X 6.00 columns
			X 6 rows C= 0	0.600 in 48.0" x	48.0" Grate (56% open area)

Primary OutFlow Max=3.50 cfs @ 12.40 hrs HW=204.66' TW=202.13' (Dynamic Tailwater)

Limited to weir flow at low heads

-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-2=Culvert (Passes 3.50 cfs of 4.74 cfs potential flow)

-3=Orifice/Grate (Orifice Controls 0.25 cfs @ 7.23 fps) -4=Orifice/Grate (Weir Controls 3.26 cfs @ 1.30 fps)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 423

Summary for Pond P212: INFILTRATION POND #1

Inflow Area = 241,078 sf, 59.10% Impervious, Inflow Depth > 6.77" for 100YR event

Inflow = 32.18 cfs @ 12.10 hrs, Volume= 135,950 cf

Outflow = 29.18 cfs @ 12.16 hrs, Volume= 131,516 cf, Atten= 9%, Lag= 3.6 min

Discarded = 1.49 cfs @ 12.16 hrs, Volume= 74,502 cf Primary = 27.69 cfs @ 12.16 hrs, Volume= 57,014 cf

Routed to Reach R211: OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 203.05' @ 12.16 hrs Surf.Area= 12,518 sf Storage= 29,225 cf

Plug-Flow detention time= 101.4 min calculated for 131,516 cf (97% of inflow)

Center-of-Mass det. time= 81.9 min (855.2 - 773.3)

Volume	Inver	t Avail.Sto	rage Storage	Description		
#1	200.00)' 41,7 ⁷	74 cf Custom	Stage Data (Coni	ic) Listed below (Re	ecalc)
Elevatio	-	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
200.0 201.0 202.0	00	4,354 9,360 10,993	0 6,699 10,166	0 6,699 16,865	4,354 9,368 11,040	
204.0		13,976 Invert	24,909 Outlet Devices	41,774	14,126	
Device #1	Routing Primary	202.50'	25.0' long x 2 Head (feet) 0	20.0' breadth Broa .20 0.40 0.60 0.8 a) 2.68 2.70 2.70	0 1.00 1.20 1.40	1.60
#2	Discarded	200.00'	, ,	filtration over Su		

Discarded OutFlow Max=1.49 cfs @ 12.16 hrs HW=203.05' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 1.49 cfs)

Primary OutFlow Max=27.29 cfs @ 12.16 hrs HW=203.05' TW=200.43' (Dynamic Tailwater) 1=Broad-Crested Rectangular Weir (Weir Controls 27.29 cfs @ 2.00 fps)

Summary for Pond P213: Stormtech Infiltration System #3

Inflow Area = 31,986 sf, 93.23% Impervious, Inflow Depth > 8.63" for 100YR event Inflow = 6.35 cfs @ 12.09 hrs, Volume= 23,003 cf

Outflow = 2.27 cfs @ 12.36 hrs, Volume= 23,001 cf, Atten= 64%, Lag= 16.5 min Discarded = 0.39 cfs @ 10.95 hrs, Volume= 19,325 cf

Primary = 1.88 cfs @ 12.36 hrs, Volume= 3,677 cf

Routed to Pond P212 : INFILTRATION POND #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 204.44' @ 12.36 hrs Surf.Area= 3,317 sf Storage= 7,117 cf

Plug-Flow detention time= 91.4 min calculated for 23,001 cf (100% of inflow) Center-of-Mass det. time= 91.3 min (838.2 - 746.9)

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 424

Volume	Invert	Avail.Storage	Storage Description
#1A	200.95'	2,990 cf	49.00'W x 67.70'L x 3.50'H Field A
			11,610 cf Overall - 4,135 cf Embedded = 7,475 cf x 40.0% Voids
#2A	201.45'	4,135 cf	ADS_StormTech SC-740 +Cap x 90 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			90 Chambers in 10 Rows

7,125 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	202.30'	12.0" Round Culvert L= 60.0' Ke= 0.500
			Inlet / Outlet Invert= 202.30' / 202.00' S= 0.0050 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	200.95'	5.130 in/hr Exfiltration over Surface area Phase-In= 0.01'
#3	Device 1	204.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Device 1	203.35'	6.0" W x 4.0" H Vert. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Discarded OutFlow Max=0.39 cfs @ 10.95 hrs HW=200.99' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.39 cfs)

Primary OutFlow Max=1.83 cfs @ 12.36 hrs HW=204.44' TW=202.93' (Dynamic Tailwater)

-1=Culvert (Passes 1.83 cfs of 4.11 cfs potential flow)

-3=Sharp-Crested Rectangular Weir (Weir Controls 1.06 cfs @ 1.42 fps)

-4=Orifice/Grate (Orifice Controls 0.77 cfs @ 4.62 fps)

Summary for Pond P214: STORMTECH INFILTRATION SYSTEM #4

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=2)

Inflow Area =	32,665 sf, 94.81% Impervious,	Inflow Depth > 8.54" for 100YR event
Inflow =	6.45 cfs @ 12.09 hrs, Volume=	23,257 cf
Outflow =	1.84 cfs @ 12.42 hrs, Volume=	23,256 cf, Atten= 72%, Lag= 20.2 min
Discarded =	0.61 cfs @ 11.50 hrs, Volume=	20,934 cf
Primary =	1.22 cfs @ 12.42 hrs, Volume=	2,322 cf
Routed to Read	ch 9R : OVERLAND FLOW	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3 Peak Elev= 203.26' @ 12.42 hrs Surf.Area= 3,201 sf Storage= 6,493 cf

Plug-Flow detention time= 52.8 min calculated for 23,208 cf (100% of inflow) Center-of-Mass det. time= 52.6 min (801.2 - 748.6)

Prepared by Howard Stein Hudson Associates

Printed 6/19/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 425

Volume	Invert	Avail.Storage	Storage Description
#1A	200.00'	1,851 cf	30.00'W x 67.70'L x 3.50'H Field A
			7,108 cf Overall - 2,481 cf Embedded = 4,627 cf x 40.0% Voids
#2A	200.50'	2,481 cf	ADS_StormTech SC-740 +Cap x 54 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			54 Chambers in 6 Rows
#3B	200.00'	1,087 cf	25.25'W x 46.34'L x 3.50'H Field B
			4,095 cf Overall - 1,378 cf Embedded = 2,717 cf x 40.0% Voids
#4B	200.50'	1,378 cf	ADS_StormTech SC-740 +Cap x 30 Inside #3
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			30 Chambers in 5 Rows
·		6 707 cf	Total Available Storage

6,797 cf Total Available Storage

Storage Group A created with Chamber Wizard Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	201.00'	12.0" Round Culvert L= 25.0' Ke= 0.500
	-		Inlet / Outlet Invert= 201.00' / 200.88' S= 0.0048 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Discarded	200.00'	8.280 in/hr Exfiltration over Surface area Phase-In= 0.01'
#3	Device 1	203.40'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#4	Device 1	202.40'	8.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.61 cfs @ 11.50 hrs HW=200.04' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.61 cfs)

Primary OutFlow Max=1.22 cfs @ 12.42 hrs HW=203.26' TW=201.88' (Dynamic Tailwater)

-1=Culvert (Passes 1.22 cfs of 4.44 cfs potential flow)

-3=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)

-4=Orifice/Grate (Orifice Controls 1.22 cfs @ 3.48 fps)

Summary for Link AP1: ANALYSIS POINT 1

9,943 sf, 92.79% Impervious, Inflow Depth > 8.45" for 100YR event Inflow Area =

7.004 cf Inflow 1.96 cfs @ 12.09 hrs, Volume=

7,004 cf, Atten= 0%, Lag= 0.0 min Primary 1.96 cfs @ 12.09 hrs, Volume=

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP2: ANALYSIS POINT 2

816,898 sf, 39.51% Impervious, Inflow Depth > 5.80" for 100YR event Inflow Area =

52.47 cfs @ 12.41 hrs, Volume= Inflow 394,820 cf

Primary 52.47 cfs @ 12.41 hrs, Volume= 394,820 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Type III 24-hr 100YR Rainfall=9.06"

Prepared by Howard Stein Hudson Associates HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Printed 6/19/2023

Page 426

Summary for Link AP3: ANALYSIS POINT 3

55,420 sf, 16.57% Impervious, Inflow Depth > 5.38" for 100YR event Inflow Area =

Inflow 7.86 cfs @ 12.09 hrs, Volume= 24.862 cf

Primary 7.86 cfs @ 12.09 hrs, Volume= 24,862 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Summary for Link AP4: ANALYSIS POINT #4

1,691,659 sf, 25.34% Impervious, Inflow Depth > 4.21" for 100YR event Inflow Area =

Inflow 98.54 cfs @ 12.49 hrs, Volume= 593,282 cf

98.54 cfs @ 12.49 hrs, Volume= 593,282 cf, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Storage (cubic-feet) 13,379 13,379 13,379 13,379 13,379 13,379 13,379 13,379 13,379 13,379 13,379 Printed 6/16/2023

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 1

Stage-Area-Storage for Pond P204: STORMTECH INFILTRATION SYSTEM #1

Elevation	Surface	Storage	Elevation	Surface
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)
202.50	5,670	0	207.70	5,670
202.60	5,670	227	207.80	5,670
202.70	5,670	454	207.90	5,670
202.80	5,670	680	208.00	5,670
202.90	5,670	907	208.10	5,670
203.00	5,670	1,134	208.20	5,670
203.10	5,670	1,413	208.30	5,670
203.20	5,670	1,883	208.40	5,670
203.30	5,670	2,350	208.50	5,670
203.40	5,670	2,815	208.60	5,670
203.50	5,670	3,277	208.70	5,670
203.60	5,670	3,735		
203.70	5,670	4,189		
203.80	5,670	4,638		
203.90	5,670	5,083		
204.00	5,670	5,524		
204.10	5,670	5,958		
204.20	5,670	6,387		
204.30	5,670	6,810		
204.40	5,670	7,225		
204.50	5,670	7,633		
204.60	5,670	8,033		
204.70	5,670	8,425		
204.80	5,670	8,805		
204.90	5,670	9,174		
205.00	5,670	9,531		
205.10	5,670	9,873		
205.20	5,670	10,199		
205.30	5,670	10,500		
205.40	5,670	10,770		
205.50	5,670	11,016		
205.60	5,670	11,248		
205.70	5,670	11,474		
205.80	5,670	11,701		
205.90	5,670	11,928		
206.00	5,670	12,155		
206.10	5,670	12,365		
206.20	5,670	12,574		
206.30	5,670	12,784		
206.40	5,670 5,670	12,994		
206.50 206.60		13,204 13,379		
206.70	5,670 5,670	13,379		
206.70	5,670	13,379		
206.90	5,670	13,379		
207.00	5,670	13,379		
207.10	5,670	13,379		
207.10	5,670	13,379		
207.30	5,670	13,379		
207.40	5,670	13,379		
207.50	5,670	13,379		
207.60	5,670	13,379		
201.00	0,010	10,010		

Page 2

Stage-Area-Storage for Pond P205: INFILTRATION POND #3

Elevation	Surface	Storage (cubic-feet)	Ele
(feet) 205.00 205.05 205.10 205.15 205.20 205.25 205.30 205.35 205.40 205.45 205.50 205.55 205.60 205.65 205.70 205.75 205.80 205.85 205.90 205.95 206.00 206.15 206.20 206.25 206.30 206.35 206.40 206.45 206.50 206.55 206.60 206.65 206.70 206.75 206.80 206.65 206.70 206.75 206.80 206.85 206.90 206.95 207.00 207.05 207.10 207.15 207.20 207.25 207.30 207.35 207.40 207.55	(sq-ft) 3,939 3,980 4,021 4,062 4,102 4,143 4,184 4,225 4,266 4,307 4,348 4,388 4,429 4,470 4,511 4,552 4,593 4,633 4,674 4,715 4,756 4,800 4,843 4,887 4,930 4,974 5,017 5,061 5,104 5,148 5,192 5,235 5,279 5,322 5,366 5,409 5,453 5,496 5,540 5,583 5,627 5,673 5,720 5,766 5,813 5,859 5,905 5,952 5,998 6,045 6,091 6,137	0 198 398 600 804 1,010 1,218 1,429 1,641 1,855 2,072 2,290 2,510 2,733 2,957 3,184 3,413 3,643 3,876 4,111 4,348 4,586 4,827 5,071 5,316 5,564 5,813 6,065 6,320 6,576 6,834 7,095 7,358 7,623 7,890 8,159 9,539 9,539 9,539 9,539 9,539 9,539 9,822 10,106 10,393 10,683 10,975 11,269 11,565 11,864 12,165 12,469 12,774	

Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)
207.60	6,184	13,082
207.65	6,230	13,393
207.70	6,277	13,705
207.75	6,323	14,020
207.80	6,369	14,338
207.85	6,416	14,657
207.90	6,462	14,979
207.95	6,509	15,303
208.00	6,555	15,630

Printed 6/16/2023

Page 3

Stage-Area-Storage for Pond P206: STORMTECH INFILTRATION SYSTEM #2

	· ·	· ·			
Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
194.67	6,072	0	197.27	6,072	10,788
194.72	6,072	121	197.32	6,072	10,930
194.77	6,072	243	197.37	6,072	11,064
194.82	6,072	364	197.42	6,072	11,194
194.87	6,072	486	197.47	6,072	11,320
194.92	6,072	607	197.52	6,072	11,442
194.97	6,072	729	197.57	6,072	11,564
195.02	6,072	893	197.62	6,072	11,685
195.07	6,072	1,145	197.67	6,072	11,807
195.12	6,072	1,397	197.72	6,072	11,928
195.17	6,072	1,648	197.77	6,072	12,050
195.22	6,072	1,898	197.82	6,072	12,171
195.27	6,072	2,147	197.87	6,072	12,292
195.32	6,072	2,396	197.92	6,072	12,414
195.37	6,072	2,644	197.97	6,072	12,535
195.42	6,072	2,891			
195.47	6,072	3,137			
195.52	6,072	3,382			
195.57	6,072	3,626			
195.62	6,072	3,868			
195.67	6,072	4,110			
195.72	6,072	4,350			
195.77	6,072	4,590			
195.82	6,072	4,827			
195.87	6,072	5,064			
195.92	6,072	5,299			
195.97	6,072	5,533			
196.02	6,072	5,765			
196.07	6,072	5,996			
196.12	6,072	6,225			
196.17	6,072	6,453			
196.22	6,072	6,678			
196.27	6,072	6,902			
196.32	6,072	7,124			
196.37	6,072	7,343			
196.42	6,072	7,561			
196.47	6,072	7,777			
196.52	6,072	7,990			
196.57	6,072	8,201			
196.62	6,072	8,410			
196.67	6,072	8,616			
196.72	6,072	8,818			
196.77	6,072	9,018			
196.82	6,072	9,214			
196.87	6,072	9,407			
196.92	6,072	9,597			
196.97	6,072	9,783			
197.02	6,072	9,965			
197.07	6,072	10,142			
197.12	6,072	10,315			
197.17	6,072	10,481			
197.22	6,072	10,639			

Page 4

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Storage (cubic-feet)

31,530

32,237

32,947

33,662

34,380

35,102

35,828

36,557

37,290

38,027

38,767

39,512

40,260

Stage-Area-Storage for Pond P207: INFILTRATION POND #2

Surface

(sq-ft) 14,100

14,175

14,250

14,325

14,400

14,475

14,550

14,625

14,700

14,775

14,850

14,925

15,000

Elevation	Surface	Storage	Elevation
(feet)	(sq-ft)	(cubic-feet)	(feet)
196.80	10,100	0	199.40
196.85	10,179	507	199.45
196.90	10,258	1,018	199.50
196.95	10,338	1,533	199.55
197.00	10,417	2,052	199.60
197.05	10,496	2,574	199.65
197.10	10,575	3,101	199.70
197.15	10,654	3,632	199.75
197.20	10,733	4,167	199.80
197.25	10,812	4,705	199.85
197.30	10,892	5,248	199.90
197.35	10,971	5,794	199.95
197.40	11,050	6,345	200.00
197.45	11,129	6,899	
197.50	11,208	7,458	
197.55	11,288	8,020	
197.60	11,367	8,587	
197.65	11,446	9,157	
197.70	11,525	9,731	
197.75	11,604	10,309	
197.80	11,683	10,892	
197.85	11,763	11,478	
197.90	11,842	12,068	
197.95	11,921	12,662	
198.00	12,000	13,260	
198.05	12,075	13,862	
198.10	12,150	14,468	
198.15	12,225	15,077	
198.20	12,300	15,690	
198.25	12,375	16,307	
198.30	12,450	16,928	
198.35	12,525	17,552 18,180	
198.40 198.45	12,600 12,675	18,812	
	12,750	19,447	
198.50 198.55	12,730	20,087	
198.60	12,900	20,730	
198.65	12,975	21,377	
198.70	13,050	22,028	
198.75	13,125	22,682	
198.80	13,200	23,340	
198.85	13,275	24,002	
198.90	13,350	24,667	
198.95	13,425	25,337	
199.00	13,500	26,010	
199.05	13,575	26,687	
199.10	13,650	27,368	
199.15	13,725	28,052	
199.20	13,800	28,740	
199.25	13,875	29,432	
199.30	13,950	30,128	
199.35	14,025	30,827	
	,	<i>'</i>	
		•	

Page 5

Stage-Area-Storage for Pond P210: POCKET WETLAND #1

Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
199.00	218	0	204.20	8,593	16,505
199.10	226	22	204.30	8,853	17,377
199.20	233	45	204.40	9,113	18,276
199.30	241	69	204.50	9,373	19,200
199.40	248	93	204.60	9,633	20,150
199.50	256	119	204.70	9,893	21,126
199.60	264	144	204.80	10,153	22,129
199.70	271	171	204.90	10,413	23,157
199.80	279	199	205.00	10,413	24,211
199.90	286	227	205.00		25,291
200.00	294	256	205.10	10,932	
	302			11,192	26,398 27,530
200.10		286	205.30	11,452	27,530
200.20	310	316	205.40	11,712	28,688
200.30	319	348	205.50	11,972	29,872
200.40	327	380	205.60	12,232	31,083
200.50	335	413	205.70	12,492	32,319
200.60	343	447	205.80	12,752	33,581
200.70	351	482	205.90	13,012	34,869
200.80	360	517	206.00	13,272	36,184
200.90	368	554	206.10	13,568	37,526
201.00	376	591	206.20	13,864	38,897
201.10	737	647	206.30	14,161	40,298
201.20	1,099	738	206.40	14,457	41,729
201.30	1,461	866	206.50	14,753	43,190
201.40	1,822	1,031			
201.50	2,184	1,231			
201.60	2,545	1,467			
201.70	2,906	1,740			
201.80	3,268	2,049			
201.90	3,630	2,393			
202.00	3,991	2,775			
202.10	4,195	3,184			
202.20	4,399	3,614			
202.30	4,603	4,064			
202.40	4,807	4,534			
202.50	5,012	5,025			
202.60	5,216	5,536			
202.70	5,420	6,068			
202.80	5,624	6,620			
202.90	5,828	7,193			
203.00	6,032	7,786			
203.10	6,236	8,399			
203.20	6,440	9,033			
203.30	6,644	9,687			
203.40	6,848	10,362			
203.50	7,053	11,057			
203.60	7,257	11,773			
203.70	7,461	12,508			
203.80	7,665	13,265			
203.90	7,869	14,041			
204.00	8,073	14,839			
204.10	8,333	15,659			

Page 6

Stage-Area-Storage for Pond P212: INFILTRATION POND #1

Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
200.00	4,354	0	202.60	11,850	23,716
200.05	4,559	223	202.65	11,923	24,311
200.10	4,769	456	202.70	11,996	24,909
200.15	4,984	700	202.75	12,070	25,510
200.20	5,204	955	202.80	12,143	26,116
200.25	5,428	1,220	202.85	12,217	26,725
200.30	5,657	1,497	202.90	12,291	27,337
200.35	5,891 6.120	1,786	202.95	12,365	27,954 28,574
200.40 200.45	6,129 6,372	2,087 2,399	203.00 203.05	12,440 12,514	28,574
200.43	6,620	2,399 2,724	203.03	12,514	29,198 29,825
200.55	6,873	3,061	203.10	12,665	30,457
200.60	7,130	3,411	203.13	12,740	31,092
200.65	7,130	3,774	203.25	12,815	31,731
200.70	7,659	4,151	203.20	12,891	32,373
200.75	7,931	4,540	203.35	12,967	33,020
200.73	8,207	4,944	203.40	13,044	33,670
200.85	8,488	5,361	203.45	13,120	34,324
200.90	8,774	5,793	203.50	13,197	34,982
200.95	9,065	6,239	203.55	13,274	35,644
201.00	9,360	6,699	203.60	13,351	36,309
201.05	9,439	7,169	203.65	13,428	36,979
201.10	9,517	7,643	203.70	13,506	37,652
201.15	9,597	8,121	203.75	13,584	38,329
201.20	9,676	8,603	203.80	13,662	39,011
201.25	9,756	9,089	203.85	13,740	39,696
201.30	9,836	9,578	203.90	13,818	40,385
201.35	9,917	10,072	203.95	13,897	41,077
201.40	9,997	10,570	204.00	13,976	41,774
201.45	10,079	11,072			
201.50	10,160	11,578			
201.55	10,242	12,088			
201.60	10,324	12,602			
201.65	10,407	13,120			
201.70	10,489	13,643			
201.75	10,572	14,169			
201.80	10,656	14,700			
201.85	10,740	15,235			
201.90	10,824	15,774			
201.95	10,908	16,317			
202.00	10,993	16,865			
202.05	11,063	17,416			
202.10	11,134	17,971			
202.15	11,204	18,530			
202.20	11,275	19,092			
202.25	11,346	19,657			
202.30	11,418	20,226			
202.35	11,489 11 561	20,799 21,375			
202.40 202.45	11,561 11,633	21,375 21,055			
202.45	11,705	21,955 22,538			
202.55	11,778	23,126			
202.00	11,770	20,120			

HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 7

Stage-Area-Storage for Pond P213: Stormtech Infiltration System #3

Elevation Surface (sq-ft) (cubic-feet) (feet) (sq-ft) (cubic-feet) (feet) (sq-ft) (cubic-feet) (cubic-feet) (sq-ft) (cubic-feet) (sq-ft) (cubic-feet) (sq-ft) (sq-		· ·	· ·			•
200.95 3.317 0 203.55 3.317 5,836 201.05 3,317 133 203.65 3,317 5,928 201.05 3,317 133 203.65 3,317 6,016 201.15 3,317 199 203.70 3,317 6,099 201.15 3,317 332 203.80 3,317 6,252 201.25 3,317 398 203.85 3,317 6,252 201.30 3,317 398 203.85 3,317 6,324 201.30 3,317 398 203.85 3,317 6,394 201.35 3,317 597 204.00 3,317 6,461 201.40 3,317 597 204.00 3,317 6,594 201.45 3,317 663 204.05 3,317 6,594 201.55 3,317 1,76 204.20 3,317 6,727 201.66 3,317 1,24 204.25 3,317 6,924						
201.00						
201.05						
201.10						
201.15						
201.20						
201.25						
20130						
201.35						
201.40						
201.50						
201.50						
201.55						
201.60						
201.65						
201.70						
201.75						
201.80 3,317 1,623 204.40 3,317 7,058 201.85 3,317 1,758 204.45 3,317 7,125 201.90 3,317 1,893 201.95 3,317 2,027 202.00 3,317 2,161 202.10 3,317 2,427 202.10 3,317 2,427 202.15 3,317 2,558 202.20 3,317 2,950 202.35 3,317 3,079 202.40 3,317 3,950 202.45 3,317 3,461 202.50 3,317 3,461 202.55 3,317 3,587 202.60 3,317 3,587 202.60 3,317 3,587 202.65 3,317 3,836 202.75 3,317 4,080 202.80 3,317 4,320 202.80 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,787 203.10 3,317 5,012 203.20 3,317 5,012 203.20 3,317 5,121 203.20 3,317 <						
201.85 3,317 1,758 204.45 3,317 7,125 201.90 3,317 1,893 201.95 3,317 2,027 202.00 3,317 2,161 202.05 3,317 2,294 202.10 3,317 2,2558 202.20 3,317 2,690 202.25 3,317 2,950 202.30 3,317 2,950 202.30 3,317 3,079 202.40 3,317 3,307 202.40 3,317 3,587 202.50 3,317 3,461 202.55 3,317 3,587 202.60 3,317 3,712 202.60 3,317 3,958 202.75 3,317 4,080 202.75 3,317 4,080 202.80 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,787 203.10 3,317 4,787 203.10 3,317 5,012 203.20 3,317 5,012 203.20 3,317 5,012 203.25 3,317 5,229 203.30 3,317						
201.90 3,317 1,893 201.95 3,317 2,027 202.00 3,317 2,161 202.05 3,317 2,294 202.10 3,317 2,427 202.15 3,317 2,558 202.20 3,317 2,690 202.25 3,317 2,950 202.30 3,317 3,079 202.40 3,317 3,355 202.45 3,317 3,355 202.50 3,317 3,587 202.55 3,317 3,587 202.60 3,317 3,587 202.55 3,317 3,958 202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,439 202.90 3,317 4,439 202.95 3,317 4,787 203.00 3,317 4,672 203.05 3,317 5,012 203.20 3,317 5,121 203.25 3						
201.95 3,317 2,027 202.00 3,317 2,161 202.05 3,317 2,294 202.10 3,317 2,427 202.15 3,317 2,558 202.20 3,317 2,690 202.25 3,317 2,820 202.30 3,317 2,950 202.35 3,317 3,079 202.40 3,317 3,207 202.45 3,317 3,358 202.50 3,317 3,461 202.55 3,317 3,587 202.60 3,317 3,712 202.65 3,317 3,836 202.70 3,317 3,958 202.75 3,317 4,080 202.85 3,317 4,201 202.85 3,317 4,439 202.90 3,317 4,490 202.95 3,317 4,787 203.05 3,317 4,787 203.15 3,317 5,012 203.20 3,317 5,229 203.30 3				204.45	3,317	7,125
202.00 3,317 2,161 202.05 3,317 2,294 202.10 3,317 2,427 202.15 3,317 2,558 202.20 3,317 2,690 202.25 3,317 2,950 202.30 3,317 3,079 202.40 3,317 3,207 202.45 3,317 3,335 202.50 3,317 3,461 202.55 3,317 3,587 202.60 3,317 3,712 202.65 3,317 3,958 202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,787 203.10 3,317 5,012 203.20 3,317 5,229 203.30 3,317 5,440 203.40 3,317 5,542 203.45 3						
202.05 3,317 2,294 202.10 3,317 2,427 202.15 3,317 2,558 202.20 3,317 2,690 202.25 3,317 2,950 202.35 3,317 3,079 202.40 3,317 3,207 202.45 3,317 3,587 202.50 3,317 3,587 202.60 3,317 3,712 202.65 3,317 3,836 202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 5,012 203.20 3,317 5,229 203.30 3,317 5,440 203.40 3,317 5,643						
202.10 3,317 2,427 202.15 3,317 2,658 202.20 3,317 2,690 202.25 3,317 2,950 202.30 3,317 3,079 202.40 3,317 3,207 202.45 3,317 3,335 202.50 3,317 3,461 202.55 3,317 3,587 202.60 3,317 3,712 202.65 3,317 3,958 202.70 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.25 3,317 5,229 203.30 3,317 5,440 203.40 3,317 5,643						
202.15 3,317 2,558 202.20 3,317 2,690 202.25 3,317 2,820 202.30 3,317 2,950 202.35 3,317 3,079 202.40 3,317 3,207 202.45 3,317 3,3461 202.50 3,317 3,587 202.60 3,317 3,712 202.65 3,317 3,958 202.70 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 5,012 203.25 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,440 203.40 3,317 5,643						
202.20 3,317 2,690 202.25 3,317 2,820 202.30 3,317 2,950 202.35 3,317 3,079 202.40 3,317 3,207 202.45 3,317 3,385 202.50 3,317 3,461 202.55 3,317 3,587 202.60 3,317 3,712 202.65 3,317 3,958 202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,787 203.10 3,317 4,787 203.10 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,440 203.40 3,317 5,643						
202.25 3,317 2,820 202.30 3,317 2,950 202.40 3,317 3,079 202.45 3,317 3,335 202.50 3,317 3,461 202.55 3,317 3,587 202.60 3,317 3,836 202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,672 203.00 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,440 203.40 3,317 5,643						
202.30 3,317 2,950 202.35 3,317 3,079 202.40 3,317 3,207 202.45 3,317 3,335 202.50 3,317 3,461 202.55 3,317 3,587 202.60 3,317 3,712 202.65 3,317 3,958 202.70 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,787 203.10 3,317 4,787 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.35 3,317 5,440 203.40 3,317 5,642 203.45 3,317 5,643						
202.35 3,317 3,079 202.40 3,317 3,207 202.45 3,317 3,335 202.50 3,317 3,461 202.55 3,317 3,587 202.60 3,317 3,712 202.65 3,317 3,836 202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,340 203.40 3,317 5,643						
202.40 3,317 3,207 202.45 3,317 3,335 202.50 3,317 3,461 202.55 3,317 3,587 202.60 3,317 3,712 202.65 3,317 3,836 202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,439 202.90 3,317 4,4556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.20 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,35 203.35 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
202.45 3,317 3,335 202.50 3,317 3,461 202.55 3,317 3,587 202.60 3,317 3,712 202.65 3,317 3,836 202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,439 202.90 3,317 4,456 203.00 3,317 4,787 203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
202.50 3,317 3,461 202.55 3,317 3,587 202.60 3,317 3,712 202.65 3,317 3,836 202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.35 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
202.55 3,317 3,587 202.60 3,317 3,712 202.65 3,317 3,836 202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,439 202.90 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
202.60 3,317 3,712 202.65 3,317 3,836 202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
202.65 3,317 3,836 202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.40 3,317 5,542 203.45 3,317 5,643						
202.70 3,317 3,958 202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.40 3,317 5,542 203.45 3,317 5,643						
202.75 3,317 4,080 202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.40 3,317 5,542 203.45 3,317 5,643						
202.80 3,317 4,201 202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643			·			
202.85 3,317 4,320 202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.40 3,317 5,542 203.45 3,317 5,643		0.04=				
202.90 3,317 4,439 202.95 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.40 3,317 5,542 203.45 3,317 5,643						
202.95 3,317 4,556 203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.35 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
203.00 3,317 4,672 203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.35 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
203.05 3,317 4,787 203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.35 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
203.10 3,317 4,900 203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.35 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
203.15 3,317 5,012 203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.35 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
203.20 3,317 5,121 203.25 3,317 5,229 203.30 3,317 5,335 203.35 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
203.25 3,317 5,229 203.30 3,317 5,335 203.35 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
203.30 3,317 5,335 203.35 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
203.35 3,317 5,440 203.40 3,317 5,542 203.45 3,317 5,643						
203.40 3,317 5,542 203.45 3,317 5,643						
203.45 3,317 5,643						
203.50 3,317 5,741						
	203.50	3,317	5,741			

Prepared by Howard Stein Hudson Associates
HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC

Page 8

Stage-Area-Storage for Pond P214: STORMTECH INFILTRATION SYSTEM #4

	_				
Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
200.00	3,201	Ó	202.60	3,201	5,556
200.05	3,201	64	202.65	3,201	5,644
200.10	3,201	128	202.70	3,201	5,728
200.15	3,201	192	202.75	3,201	5,808
200.20	3,201	256	202.80	3,201	5,884
200.25	3,201	320	202.85	3,201	5,955
200.30	3,201	384	202.90	3,201	6,024
200.35	3,201	448	202.95	3,201	6,091
200.40	3,201	512	203.00	3,201	6,156
200.45	3,201	576	203.05	3,201	6,220
200.50	3,201	640	203.10	3,201	6,284
200.55	3,201	771	203.15	3,201	6,349
200.60	3,201	902	203.20	3,201	6,413
200.65	3,201	1,032	203.25	3,201	6,477
200.70	3,201	1,162	203.30	3,201	6,541
200.75	3,201	1,292	203.35	3,201	6,605
200.80	3,201	1,421	203.40	3,201	6,669
200.85	3,201	1,550	203.45	3,201	6,733
200.90	3,201	1,679	203.50	3,201	6,797
200.95	3,201	1,807	200.00	0,201	0,707
201.00	3,201	1,934			
201.05	3,201	2,061			
201.10	3,201	2,187			
201.15	3,201	2,313			
201.20	3,201	2,438			
201.25	3,201	2,563			
201.30	3,201	2,687			
201.35	3,201	2,810			
201.40	3,201	2,932			
201.45	3,201	3,054			
201.50	3,201	3,175			
201.55	3,201	3,296			
201.60	3,201	3,415			
201.65	3,201	3,534			
201.70	3,201	3,651			
201.75	3,201	3,768			
201.80	3,201	3,884			
201.85	3,201	3,998			
201.90	3,201	4,112			
201.95	3,201	4,225			
202.00	3,201	4,336			
202.05	3,201	4,447			
202.10	3,201	4,556			
202.15	3,201	4,664			
202.20	3,201	4,770			
202.25	3,201	4,874			
202.30	3,201	4,977			
202.35	3,201	5,078			
202.40	3,201	5,178			
202.45	3,201	5,276			
202.50	3,201	5,371			
202.55	3,201	5,465			

Pocket Wetland Sizing

P210

Site Location: 55 Summer Street - Walpole, MA

Design Criteria	Pocket Wetland (req.)	Pocket Wetland (P205)
Minimum Drainage Area (Ac.)	≥ 1 to 10	1.4
Constructed Wetland Surface	≥ 0.01	0.14
Area/Watershed Ratio		
Length to Width Ratio (min.)	≥ 2:1	12:1
Extended Detention (ED)	OPTIONAL	NO
Allocation of WQv Volume (wet pools/low and high	20/80/02	24/76/0
marsh/ED) in %		
Allocation of Surface Area		
(wet pools/low marsh/high	10/45/40/5	9/43/43/5
marsh/semi-wet) in %		
Sediment Forebay	REQUIRED	YES
Micropool	REQUIRED	YES
Oulet Configuration	Hooded Broad-Crested Weir	Multi-Stage Discharge Outlet Structure
Target Allocations	Pocket Wetland	Pocket Wetland
% Surface Are	a (Req.)	8,465
Sediment Forebay	5%	5%
Micropool	5%	4%
Deep Water Channel	0%	0%
Lo Marsh	45%	43%
High Marsh	40%	43%
Semi-Wet	5%	5%
% WQv Volum	2,619	
Sediment Forebay	10%	10%
Micropool	10%	14%
Deep Water Channel	0%	0%
Lo Marsh and High Marsh	80%	76%



Date: 06/20/2023

By: PB Checked: KE

Designed Surface Area (sf)			
Sediment Forebay	392		
Micropool	376		
Deep Water	0		
Lo Marsh	3615		
High Marsh	3674		
Semi-Wet	408		

Designed Water Quali	ty Volume (cf)
Sediment Forebay	267
Micropool	376