## SUPPLEMENTAL DATA REPORT

## Proposed Multi-family Development

51-53-55 Summer Street
Walpole, Massachusetts


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## Existing Conditions

The subject site consists of three parcels totaling 54.73 acres in the Limited Manufacturing - LM zone. The addresses of record for the parcels are 51-53-55 Summer Street. Summer Street has a 50-foot-wide right-of-way along the frontage of the existing lots. There are no existing buildings or improvements on site. The site extends to the east side of the railroad tracks. On the north side there is a Park, School, Recreation \& Conservation (PSRC) zoned area encompassing a large wetland area. On the west and south sides there is Residence B (RB) zoned properties.

There is a railroad yard located on the abutting property on the east side of our project site in the LM zone, across from the railroad right-of-way. The PSRC zone does not contain any existing buildings. An RB zone exists to the west and south of the parcel and contain residential dwellings with associated improvements (such as stand-alone garages, pools, sheds, driveways, etc).

The site contains a mix of woodlands, isolated vegetated wetlands, bordering vegetated wetlands, vernal pools, and open grassed areas. All three vernal pools are denoted as potential vernal pools at this time. This property is located within the Area 3 - Primary Recharge Area Water Resource Protection Overlay District and partially within the Large-Scale Ground-Mounted Solar Photovoltaic Overlay District (SPOD). The terrain ranges on site from elevation $186^{\prime}$ to $228^{\prime}$ Mean Sea Level, with the lower areas generally being wetlands and the higher elevations being upland areas. The site topography decreases from south to north starting at Summer Street and ending at Cedar Swamp Brook at the rear of the site. The site currently accepts direct runoff from abutters on the south and west sides. This runoff flows into a wetland on the northern side of the property. Existing flow patterns are generally from the south and west towards north, with localized flow in other directions due to the site terrain.

The site hydrology consists of upland areas flowing to both isolated and bordering vegetated wetlands existing across the entirety of the site. The entire site drains to four analysis points. The first (AP1) is a small portion of the entrance to the site that drains back onto Summer Street and into the drainage system located within Summer Street. This takes up a very small portion of the site drainage. The second analysis point (AP2) for the site is an isolated wetland which is located adjacent to the existing train tracks on the eastern side of the property. This depression has an outlet which flows under the railroad tracks, but it is currently completely blocked; water collects here and slowly infiltrates into the soil. The third analysis point is another wetland area (AP3). This isolated pocket is located adjacent to the eastern train tracks and the other depressed wetland pocket, AP2, and collects and infiltrates water. The final analysis point (AP4) is Cedar Swamp Brook which runs along the entirety of the northern part of the property.

The only drainage infrastructure located onsite is the blocked outlet pipe which runs under the existing railroad tracks. There is an existing storm drain system in Summer Street with a catch basin located along the site's frontage.

Soil conditions on site are mainly Fine Sandy Loam (Canton, Ridgebury, Whitman, Scituate, and Merrimac) with a smaller area of Hollis-Rock Outcrop-Charlton Complex. The hydrologic soil group for these soils area $\mathrm{A}, \mathrm{B}, \mathrm{C} \& \mathrm{D}$ with a majority belonging to groups $\mathrm{B}, \mathrm{C} \& \mathrm{D}$.

Both town and private sewer, water, electricity, gas, and communications are currently located within the Summer Street right-of-way, which is the preferred source of utilities to service the project.

## Proposed Conditions

This project proposed to construct a multifamily housing development consisting of apartment buildings and townhouses for rent. This project is to be serviced by municipal utilities. An easement was purchased from the abutter located at 87 Summer Street to facilitate a second means of emergency access and looped water service for the development.

The existing site is proposed to be improved with the addition of stormwater best management practices which are designed to treat, detain, and infiltrate the proposed impervious areas on the developed site, directing stormwater to the same four (4) analysis points.

There are eight (8) main stormwater treatment trains proposed within the new development. The first main treatment train drains to Pond P204 which is the proposed Stormtech infiltration system located to the east of the proposed multi-family building \#2000. This treatment train takes the clean roof runoff from multi-family building \#2000, the adjacent townhouse unit and some of the pavement runoff to the north of multi-family building \#2000. This treatment train outlets to the adjacent wetland and flows to Analysis Point \#4.

Treatment train \#2 drains to Pond P205 which is located adjacent to the emergency access to Summer Street. This infiltration basin accepts runoff from the emergency access road. This treatment train outlets to the adjacent wetland and flows to Analysis Point \#4.

Treatment train \#3 drains to Pond P206, the second Stormtech chamber system onsite, which is located behind multi-family building \#1000. This Stormtech system accepts all the clean roof runoff from multi-family building \#1000 as well as the associated pretreated street drainage on the northern side of the building. This drainage is piped into the system, treated, and infiltrated onsite prior to out-letting to the adjacent wetland system and flowing to Analysis Point \#4

Treatment train \#4 drains to Pond P207 which is located on the western side of multi-family building \#1000 and accepts most of the street and open-space drainage located adjacent to multi-family buildings \#1000 and \#2000. This treatment train outlets to the adjacent wetland and flows to Analysis Point \#4.

Treatment train \#5 drains to Pond P210 which is located north of Driveway A, on the eastern side of the property adjacent to the railroad tracks. This Pocket Wetland accepts all the associated street drainage from the beginning section of Driveway A until the mail kiosk. This treatment train outlets to the adjacent wetland and flows to Analysis Point \#2.

Treatment train \#6 drains to Pond P212 which is located between Driveway C, Driveway D, and wetland system C, in the center of the development. This Infiltration Pond takes all the street drainage from the high points of both crossings to more than halfway down Driveway C and D. The three townhouses adjacent to the open space in the center of the development are directed to this infiltration pond and treated. This treatment train outlets to the adjacent wetland and flows to Analysis Point \#4.

Treatment train \#7 drains to Pond P213, the third Stormtech chamber system onsite, which is located in the south parking area adjacent to multi-family building \#13000. This Stormtech system accepts the southern half of Driveway D and all roof runoff from multi-family building \#13000. This drainage is piped into the system, treated, and infiltrated prior to discharging to Pond P212, and later to the adjacent wetland system, Analysis Point \#4.

Treatment train \#8 drains to Pond P214, the fourth Stormtech chamber system onsite, which is located in the north parking area adjacent to multi-family building \#13000. This Stormtech system accepts the parking area to the north of multi-family building \#13000. This drainage is piped into the system, treated, and infiltrated prior to discharging to Analysis Point \#4.

The remainder of the clean roof runoff from the townhouse units are handled with individual drip edge systems which outlet to either Analysis Point \#2 or 4 via overland flow.

As of February 28, 2023, tree clearing has occurred as depicted on previously approved plans dated January 10, 2020, revised through April 13, 2021. Tree clearing which has occurred outside of the newly proposed limit of disturbance will grow back to its natural state.

The remainder of the land, which was untouched will flow, as it currently does, to the existing analysis points.

## Stormwater Management Standards

## Standard 1: No new untreated discharges

The Massachusetts Stormwater Handbook requires that the project demonstrates that no new stormwater conveyances (e.g. outfalls) discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

The proposed project will not discharge stormwater directly to, or cause erosion in, wetlands or water of the Commonwealth and will treat stormwater prior to discharge or infiltration.

BMP's have been proposed to treat stormwater collected from the newly paved areas. Each treatment chain consists of a deep sump hooded catch basin, grassed channel and a sediment forebay which is sized to accommodate the water quality volume per the Massachusetts Stormwater Handbook.

The new discharges have been designed to outlet to flared end sections with riprap to minimize any erosion to the isolated vegetated wetland. The table below shows the average flow rate for the 2 -year storm event in feet per second (fps).

| Storm Event | 2-year |
| :--- | :---: |
| Flared End Section (Pond 204) (fps) | 2.71 |
| Flared End Section (Pond 205) (fps) | 0.00 |
| Flared End Section (Pond 206) (fps) | 0.00 |
| Flared End Section (Pond 207) (fps) | 1.10 |
| Flared End Section (Pond 210) (fps) | 3.62 |
| Flared End Section (Pond 212) (fps) | 0.00 |
| Flared End Section (Pond 213) (fps) | 0.00 |
| Flared End Section (Pond 214) (fps) | 0.00 |

## Standard 2: Post-development peak discharge rates not to exceed pre-development peak discharge rates.

Post-development peak discharge rates do not exceed the pre-development peak discharge rates and total runoff volumes for all storm events except for a runoff volume increase to Analysis Point \#4 during the 100-year storm event (Cedar Brook). The proposed condition reduces rates by collecting and controlling the stormwater runoff within the stormwater management system.

| Storm Event | 2 -year | 10 -year | $25-$-year | 100 -year |
| :--- | :---: | :---: | :---: | :---: |
| Pre-Development Rates (cfs) AP1 | 0.74 | 1.28 | 1.71 | 2.59 |
| Volume (cf) (Summer St) | 2,360 | 4,159 | 5,619 | 8,714 |
| Post-Development Rates (cfs) AP1 | 0.67 | 1.05 | 1.35 | 1.96 |
| Volume (cf) (Summer St) | 2,247 | 3,627 | 4,720 | 7,004 |
| Rate Reductions (cfs) | $-\mathbf{0 . 0 7}$ | $-\mathbf{- 0 . 2 3}$ | $-\mathbf{- 0 . 3 6}$ | $\mathbf{- 0 . 6 3}$ |
| Volume Reductions (cf) | $\mathbf{- 1 1 3}$ | $\mathbf{- 5 3 2}$ | $\mathbf{- 8 9 9}$ | $\mathbf{- 1 , 7 1 0}$ |
|  |  |  |  |  |
| Pre-Development Rates (cfs) AP2 | 12.30 | 27.75 | 41.12 | 70.33 |
| Volume (cf) (Wetland at track) | 85,349 | 184,006 | 270,829 | 464,971 |
| Post-Development Rates (cfs) AP2 | 7.31 | 18.24 | 28.46 | 52.47 |
| Volume (cf) (Wetland at track) | 75,144 | 157,893 | 229,211 | 394,820 |
| Rate Reductions (cfs) | $\mathbf{- 4 . 9 9}$ | $\mathbf{- 9 . 5 1}$ | $\mathbf{- 1 2 . 6 6}$ | $\mathbf{- 1 7 . 8 6}$ |
| Volume Reductions (cf) | $\mathbf{- 1 0 , 2 0 5}$ | $\mathbf{- 2 6 , 1 1 3}$ | $\mathbf{- 4 1 , 6 1 8}$ | $\mathbf{- 7 0 , 1 5 1}$ |
|  |  |  |  |  |
| Pre-Development Rates (cfs) AP3 | 2.52 | 5.96 | 8.96 | 15.56 |
| Volume (cf) (Wetland at track) | 8,514 | 18,960 | 28,279 | 49,317 |
| Post-Development Rates (cfs) AP3 | 1.15 | 2.89 | 4.43 | 7.86 |
| Volume (cf) (Wetland at track) | 4,009 | 9,258 | 14,013 | 24,862 |
| Rate Reductions (cfs) | $\mathbf{- 1 . 3 7}$ | $\mathbf{- 3 . 0 7}$ | $\mathbf{- 4 . 5 3}$ | $\mathbf{- 7 . 7 0}$ |
| Volume Reductions (cf) | $\mathbf{- 4 , 5 0 5}$ | $\mathbf{- 9 , 7 0 2}$ | $\mathbf{- 1 4 , 2 6 6}$ | $\mathbf{- 2 4 , 4 5 5}$ |
|  |  |  |  |  |
| Pre-Development Rates (cfs) AP4 | 10.77 | 33.90 | 56.04 | 107.72 |
| Volume (cf) (Cedar Brook) | 73,247 | $\mathbf{1 9 2 , 7 0 8}$ | 306,701 | 576,512 |
| Post-Development Rates (cfs) AP4 | 7.79 | 25.19 | 43.69 | 98.54 |
| Volume (cf) (Cedar Brook) | 65,174 | $\mathbf{1 7 8 , 8 6 0}$ | 300,250 | 593,282 |
|  |  |  |  |  |


| Rate Reductions (cfs) | -2.98 | -8.71 | -12.35 | -9.18 |
| :--- | :---: | :---: | :---: | :---: |
| Volume Reductions (cf) | $-8,073$ | $-13,848$ | $-6,451$ | 16,770 |

## Standard 3: Minimize or eliminate loss of annual recharge to groundwater.

Groundwater recharge will be accomplished using the surface infiltration and subsurface practices. As shown in the table summary for Standard 2, the project decreases the total volume of runoff for all storm events except for a runoff volume increase to Analysis Point \#4 (Cedar Brook) during the 100 -year storm event. This reduction in volume is generated by collecting and infiltrating a significant portion of the impervious surfaces created on site.

## Recharge Volume Requirement:

$R v=F x$ impervious area
$R v=$ Required Recharge Volume, expressed in $\mathrm{Ft}^{3}$, cubic yards, or acre-feet
$F=$ Target Depth Factor associated with each Hydrologic Soil Group
Impervious Area = pavement and rooftop area on site
Recharge volume for the entire site:

## Soil A:

$\mathrm{Rv}=0.60$ in * $55,549 \mathrm{sf}$ * $1 \mathrm{ft} / 12$ in $=\mathbf{2 , 7 7 8} \mathbf{~ c f ~ r e c h a r g e ~}$
Soil B:
$\mathrm{Rv}=0.35 \mathrm{in}$ * $71,802 \mathrm{sf}$ * $1 \mathrm{ft} / 12$ in $=\mathbf{2 , 0 9 5} \mathbf{c f}$ recharge

## Soil C:

$\operatorname{Rv}=0.25 \mathrm{in}$ * $285,304 \mathrm{sf}$ * $1 \mathrm{ft} / 12$ in $=\mathbf{5 , 9 4 4} \mathbf{~ c f ~ r e c h a r g e ~}$
Soil D:
$\mathrm{Rv}=0.1 \mathrm{in}$ * $45,142 \mathrm{sf}$ * $1 \mathrm{ft} / 12 \mathrm{in}=377$ cf recharge

## Total Recharge Required:

$\mathrm{Rv}=(2,778 \mathrm{cf})+(2,095 \mathrm{cf})+(5,944 \mathrm{cf})+(377 \mathrm{cf})=\mathbf{1 1 , 1 9 4} \mathbf{c f}$ total recharge required

## Total recharge provided:

Townhouse Drip Edges (4 Unit - Type I) = 96 cf below outlet $=(96$ cf) $*(5$ buildings $)=480 c f$
Townhouse Drip Edges (4 Unit - Type II) $=82$ cf below outlet $=(82 c f) *(2$ buildings $)=164 c f$
Townhouse Drip Edges $(6$ Unit $)=136$ cf below outlet $=(136 ~ c f) *(4$ buildings $)=544 c f$
Club house drip edge $=130$ cf below outlet
Pond P204 = 2,117 cf below outlet (Stormtech System)
Pond P205 $=7,623$ cf below outlet
Pond P206 = 4,970 cf below outlet (Stormtech System)
Pond P207 = 6,345 cf below outlet

Pond P210 $=0$ cf below outlet
Pond P212 $=22,538$ cf below outlet
Pond P213 = 5,440 cf below outlet (Stormtech System)
Pond P214 = 5,178 cf below outlet (Stormtech System)
Total site recharge provided $=55,529$ cf recharge volume $>11,194$ cf required
Recharge per Pond

Pond P204

Soil A:
$\mathrm{Rv}=0.60$ in * $8,041 \mathrm{sf}$ * $1 \mathrm{ft} / 12 \mathrm{in}=403 \mathbf{c f}$ recharge
Soil C:
$\mathrm{Rv}=0.25$ in * $41,586 \mathrm{sf} * 1 \mathrm{ft} / 12 \mathrm{in}=867 \mathbf{c f}$ recharge
Soil D:
$\mathrm{Rv}=0.1 \mathrm{in}$ * 3 sf * $1 \mathrm{ft} / 12 \mathrm{in}=1 \mathbf{c f}$ recharge

## Total Weighted Average Recharge:

$R_{v}=(403 c f)+(867 c f)+(1 c f)=1,271 \mathbf{c f}$ recharge required

Recharge provided (including drip edges) $=2,383 \mathrm{cf}>1,270 \mathrm{cf}$ required

Pond P205:

Soil B:
$\mathrm{Rv}=0.35$ in * $16,376 \mathrm{sf} * 1 \mathrm{ft} / 12 \mathrm{in}=478$ cf recharge

Recharge provided $=7,623 \mathbf{c f}>478 \mathbf{c f}$ required

Pond P206:

Soil A:
$\mathrm{Rv}=0.60$ in * 131 sf * $1 \mathrm{ft} / 12$ in $=7$ cf recharge
Soil C:
$\mathrm{Rv}=0.25$ in * $27,225 \mathrm{sf} * 1 \mathrm{ft} / 12 \mathrm{in}=568 \mathbf{c f}$ recharge
Soil D:
$\mathrm{Rv}=0.1$ in * $20,862 \mathrm{sf} * 1 \mathrm{ft} / 12 \mathrm{in}=\mathbf{1 7 4} \mathbf{c f}$ recharge

## Total Weighted Average Recharge:

$\mathrm{Rv}=(7 \mathrm{cf})+(568 \mathrm{cf})+(174 \mathrm{cf})=749 \mathrm{cf}$ total recharge required

Recharge provided $=4,970 \mathrm{cf}>749 \mathrm{cf}$ required

## Pond P207

Soil A:
$\mathrm{Rv}=0.60$ in * $35,976 \mathrm{sf}$ * $1 \mathrm{ft} / 12$ in $=1,799 \mathbf{c f}$ recharge
Soil C:
$\mathrm{Rv}=0.25$ in * $25,566 \mathrm{sf}$ * $1 \mathrm{ft} / 12$ in $=533 \mathbf{c f}$ recharge
Soil D:
$\mathrm{Rv}=0.1$ in * $20,350 \mathrm{sf}$ * $1 \mathrm{ft} / 12$ in $=170$ cf recharge
Total Weighted Average Recharge:
$R v=(1,799 \mathrm{cf})+(533 \mathrm{cf})+(170 \mathrm{cf})=\mathbf{2 , 5 0 2} \mathbf{c f}$ recharge required

Recharge provided $=6,345 \mathrm{cf}>2,502 \mathrm{cf}$ required

Pond P210

Soil B:
$\mathrm{Rv}=0.35$ in * $30,443 \mathrm{sf} * 1 \mathrm{ft} / 12$ in $=888$ cf recharge
Soil C:
$\mathrm{Rv}=0.25$ in * 911 sf * $1 \mathrm{ft} / 12$ in $=19 \mathbf{c f}$ recharge
Soil D:
Rv=0.1 in * $1,186 \mathrm{sf}$ * $1 \mathrm{ft} / 12$ in = 10 cf recharge
Total Weighted Average Recharge:
$\mathrm{Rv}=(888 \mathrm{cf})+(19 \mathrm{cf})+(10 \mathrm{cf})=\mathbf{9 1 7} \mathbf{c f}$ recharge required

Recharge provided $=0$ cf $\neq 917$ cf required
(Overall recharge provided on site still greater than overall recharge required)

Pond P212

Soil B:
Rv=0.35 in * 772 sf * $1 \mathrm{ft} / 12$ in $=\mathbf{2 3} \mathbf{~ c f}$ recharge

## Soil C:

$\mathrm{Rv}=0.25$ in * $109,962 \mathrm{sf}$ * $1 \mathrm{ft} / 12 \mathrm{in}=\mathbf{2 , 2 9 1} \mathbf{c f}$ recharge
Soil D:
Rv=0.1 in * $1,920 \mathrm{sf}$ * $1 \mathrm{ft} / 12$ in = 16 cf recharge
Total Weighted Average Recharge:
$R v=(23 c f)+(2,291 c f)+(16 c f)=\mathbf{2 , 3 3 0} \mathbf{c f}$ recharge required

Recharge provided (including drip edges) $=22,906 \mathbf{c f}>2,330 \mathbf{c f}$ required

Pond P213

Soil C:
$\mathrm{Rv}=0.25$ in * $29,819 \mathrm{sf}$ * $1 \mathrm{ft} / 12$ in $=622 \mathbf{c f}$ recharge

Recharge provided $=5,440 \mathrm{cf}>622 \mathrm{cf}$ required

Pond P214

## Soil A:

$\mathrm{Rv}=0.6$ in * $8,071 \mathrm{sf} * 1 \mathrm{ft} / 12 \mathrm{in}=404 \mathbf{c f}$ recharge
Soil C:
$\mathrm{Rv}=0.25 \mathrm{in} * 22,900 \mathrm{sf} * 1 \mathrm{ft} / 12 \mathrm{in}=478 \mathbf{c f}$ recharge
Total Weighted Average Recharge:
$\mathrm{Rv}=(404 \mathrm{cf})+(478 \mathrm{cf})=\mathbf{8 8 2} \mathbf{c f}$ recharge required

## Recharge provided $=5,178 \mathbf{c f}>882 \mathbf{c f}$ required

## Drawdown Within 72 Hours

Townhouse Drip Edge (4 unit - Type I): $96 \mathrm{cf} /[(0.17 \mathrm{in} / \mathrm{hr})(1 \mathrm{ft} / 12 \mathrm{in})(470 \mathrm{sf})]=\mathbf{1 4 . 4}$ hours $<72$ hours, OK

Townhouse Drip Edge (4 unit - Type II): $82 \mathrm{cf} /[(0.17 \mathrm{in} / \mathrm{hr})(1 \mathrm{ft} / 12 \mathrm{in})(404 \mathrm{sf})]=\mathbf{1 4 . 3}$ hours $<7 \boldsymbol{2}$ hours, OK

Townhouse Drip Edge (6 unit): $136 \mathrm{cf} /[) 0.17 \mathrm{in} / \mathrm{hr})(1 \mathrm{ft} / 12 \mathrm{in})(665 \mathrm{sf})]=\mathbf{1 4 . 4}$ hours $<\mathbf{7 2}$ hours, OK Pond P204: 2,117 cf / [(0.66 in/hr)(1 ft/12 in)(5,670 sf) $]=\mathbf{6 . 8}$ hours $<\mathbf{7 2}$ hours, $\mathbf{O K}$ Pond P205: 7,623 cf / [(2.41 in/hr)(1 ft/12 in)(3,939 sf)] = $\mathbf{9 . 7}$ hours $<\mathbf{7 2}$ hours, OK Pond P206: $4,970 \mathrm{cf} /[(3.5 \mathrm{in} / \mathrm{hr})(1 \mathrm{ft} / 12 \mathrm{in})(6,072 \mathrm{sf})]=\mathbf{2 . 8}$ hours $<\mathbf{7 2}$ hours, $\mathbf{O K}$ Pond P207: 6,345 cf / [(3.69 in/hr) $(1 \mathrm{ft} / 12 \mathrm{in})(10,100 \mathrm{sf})]=\mathbf{2 . 1}$ hours $<\mathbf{7 2}$ hours, OK Pond P212: $22,358 \mathrm{cf} /[(5.13 \mathrm{in} / \mathrm{hr})(1 \mathrm{ft} / 12 \mathrm{in})(4,354 \mathrm{sf})]=\mathbf{1 2 . 0}$ hours $<\mathbf{7 2}$ hours, OK Pond P213: 5,440 cf / [(5.13 in/hr)(1 ft/12 in $)(3,317 \mathrm{sf})]=\mathbf{3 . 9}$ hours $<\mathbf{7 2}$ hours, OK Pond P214: 5, $178 \mathrm{cf} /[(8.28 \mathrm{in} / \mathrm{hr})(1 \mathrm{ft} / 12 \mathrm{in})(3,201 \mathrm{sf})]=\mathbf{2} \mathbf{. 4}$ hours $<\mathbf{7 2}$ hours, $\mathbf{O K}$

## Water Quality Volume

Calculated as Vwq $=(\mathrm{Dwq} / 12$ inches/foot $) *(\operatorname{Aimp} * 43,560$ square feet/acre $)$, where:
$\mathrm{V} w q=$ required water quality volume (in cubic feet)
$\mathrm{D} w q=$ water quality depth: one-inch for discharges within a Zone II or Interim Wellhead Protection Area, to or near another critical area, runoff from a LUHPPL, or exfiltration to soils with infiltration rate greater than 2.4 inches/hour or greater; $1 / 2$ inch for discharges near or to other areas.

Aimp $=$ impervious area (in acres)

Aimp $=$ Impervious Area of Subcatchments onsite $=457,797$ SF
$D w q=1$ inch
$V w q=(1$ inch $/ 12$ inches $/$ foot $) *(308,995$ S.F. $)=25,750$ C.F.
$V w q=(1 / 2$ inch $/ 12$ inches $/$ foot $) *(148,802$ S.F. $)=12,401$ C.F.

Total Water Quality Volumes from proposed BMP's $=55,529 \mathbf{c f}>38,151 \mathbf{c f} \mathbf{O K}$

Pretreatment sizing for flow based devices
Calculated as $\mathrm{V} w q=(\mathrm{Dwq} / 12$ inches/foot $) *(\operatorname{Aimp} * 43,560$ square feet/acre $)$, where:
$\mathrm{V} w q=$ required water quality volume (in cubic feet)
$\mathrm{D} w q=$ water quality depth: one-inch for discharges within a Zone II or Interim Wellhead Protection Area, to or near another critical area, runoff from a LUHPPL, or exfiltration to soils with infiltration rate greater than 2.4 inches/hour or greater; $1 / 2$ inch for discharges near or to other areas.

Aimp = impervious area

## Pond P204:

Stormtech Infiltration Chambers $=(1 / 2$ inch $/ 12$ inches $/$ foot $) *(49,630$ S.F. $)=2,068$ C.F.

Designed Infiltration Chambers $=2,117$ C.F. below outlet

## 2,117 CF $>2,068$ CF OK

Stormtech Isolator Row:
$\mathrm{Q}(1 / 2)=(752 \mathrm{csm} / \mathrm{in})(1.14 \mathrm{AC})(0.0015625 \mathrm{mi} 2 / \mathrm{AC})(1 / 2 \mathrm{in})$
$Q(1 / 2)=0.67 \mathrm{cfs}$
For the SC 740 each chamber is rated for 0.14 cfs :
Design calls for 9 SC 740 Isolator Units $=9$ units x $0.14 \mathrm{cfs}=1.26 \mathrm{cfs}$
1.26 cfs $>0.67$ cfs OK

Volume Provided $=1.26 \mathrm{cfs}$

### 1.26 cfs $>0.67$ cfs $0 . K$.

## Pond P205:

Infiltration pond $=(1$ inch $/ 12$ inches $/$ foot $) *(16,376$ S.F. $)=1,365$ C.F.

Designed Infiltration Pond $=7,623$ C.F. below outlet

## 7,623 CF > 1,365 CF OK

Sediment forebay $=0.1$ * 1,365 C.F $=137$ C.F

Designed sediment forebays $=144$ CF

## 144 CF $>137$ CF OK

## Pond P206:

Stormtech Infiltration Chambers $=(1$ inch $/ 12$ inches $/$ foot $) *(48,218$ S.F. $)=4,019$ C.F.

Designed Infiltration Chambers $=4,970$ C.F. below outlet

## 4,970 CF $>4,019$ CF OK

Stormtech Isolator Row:
$Q(1)=(774 \mathrm{csm} / \mathrm{in})(1.11 \mathrm{AC})(0.0015625 \mathrm{mi} 2 / \mathrm{AC})(1 \mathrm{in})$
$Q(1)=1.35 \mathrm{cfs}$
For the SC 740 each chamber is rated for 0.14 cfs:
Design calls for 16 SC 740 Isolator Units $=16$ units $\times 0.14 \mathrm{cfs}=2.24 \mathrm{cfs}$
2.24 cfs > 1.35 cfs OK

Volume Provided $=2.24 \mathrm{cfs}$
$2.24 \mathrm{cfs}>1.35 \mathrm{cfs}$ O.K.

## Pond P207:

Infiltration pond $=(1$ inch $/ 12$ inches $/$ foot $) *(64,794$ S.F. $)=5,400$ C.F.

Designed Infiltration Pond $=6,345$ C.F. below outlet

## 6,345 CF $>5,400$ CF OK

Sediment forebay $=0.1^{*} 5,400$ C.F $=540$ C.F

Designed sediment forebays $=1,257 \mathrm{CF}$

## 1,257 CF > 540 CF OK

## Pond P210:

Pocket Wetland \#1 = (½ inch / 12 inches $/$ foot $) ~ * ~(32,540 ~ S . F) ~=.~ 1,356 ~ C . F . ~$

Micropool and Low / High Marsh Volume $=2,619$ C.F. (See attached design criteria)

2,619 CF $\geq 1,356$ CF OK

Sediment forebay $=0.1$ * 1,356 C.F $=136$ C.F

Designed sediment forebays $=267 \mathrm{CF}$

267 CF > 136 CF OK

## Pond P212:

Infiltration pond $=(1$ inch $/ 12$ inches $/$ foot $) *(112,654$ S.F. $)=9,388$ C.F.

Designed Infiltration Pond $=22,538$ C.F. below outlet

## 22,538 CF > 9,388 CF OK

Sediment forebay $=0.1 * 9,388$ C.F $=939$ C.F

Designed sediment forebays $=2,406 \mathrm{CF}$

2,406 CF > 939 CF OK

## Pond P213:

Stormtech Infiltration Chambers $=(1$ inch $/ 12$ inches $/$ foot $) *(29,819$ S.F. $)=2,485$ C.F.

Designed Infiltration Chambers = 5,440 C.F. below outlet

## 5,440 CF > 2,485 CF OK

Stormtech Isolator Row:
$\mathrm{Q}(1)=(774 \mathrm{csm} / \mathrm{in})(0.69 \mathrm{AC})(0.0015625 \mathrm{mi} 2 / \mathrm{AC})(1 \mathrm{in})$
$Q(1)=0.84 \mathrm{cfs}$
For the SC 740 each chamber is rated for 0.14 cfs :
Design calls for 9 SC 740 Isolator Units $=9$ units $\times 0.14 \mathrm{cfs}=1.26 \mathrm{cfs}$
$1.26 \mathrm{cfs}>0.84 \mathrm{cfs}$ OK
Volume Provided $=1.26 \mathrm{cfs}$

### 1.26 cfs $>0.84 \mathbf{c f s}$ O.K.

## Pond P214:

Stormtech Infiltration Chambers $=(1$ inch $/ 12$ inches $/$ foot $) *(30,971$ S.F. $)=2,581$ C.F.
Designed Infiltration Chambers $=5,178$ C.F. below outlet

## 5,178 CF $>\mathbf{2 , 5 8 1} \mathbf{C F}$

Stormtech Isolator Row:
$\mathrm{Q}(1)=(774 \mathrm{csm} / \mathrm{in})(0.72 \mathrm{AC})(0.0015625 \mathrm{mi} 2 / \mathrm{AC})(1 \mathrm{in})$
$\mathrm{Q}(1)=0.87 \mathrm{cfs}$
For the SC 740 each chamber is rated for 0.14 cfs:
Design calls for 9 SC 740 Isolator Units $=9$ units $\times 0.14 \mathrm{cfs}=1.26 \mathrm{cfs}$
Volume Provided $=1.26 \mathrm{cfs}$

### 1.26 cfs $\mathbf{>} 0.87 \mathbf{c f s}$ O.K.

## Contech Stormceptor STC-900 Water Quality Unit

$\mathrm{Q}(1)=(774 \mathrm{csm} / \mathrm{in})(0.39 \mathrm{AC})(0.0015625 \mathrm{mi} 2 / \mathrm{AC})(1 \mathrm{in})$
$\mathrm{Q}=0.48 \mathrm{cfs}$
STC-900 Water Quality Unit is rated for 0.89 cfs
Volume Provided $=0.89$ cfs
$0.89 \mathrm{cfs}>0.48 \mathrm{cfs}$ O.K.

## Standard 4: Stormwater management system to remove 80\% of the average annual load of Total Suspended Solids (TSS)

The stormwater management system is designed to remove $>80 \%$ annual total suspended solids (TSS) from the proposed roadway, driveways, and sidewalks.

The stormwater management system is designed to remove $80 \%$ of the average annual total suspended solids (TSS) from the proposed development.

## TSS Removal Calculation

Pretreatment Train \#1 to Pond P204

- Deep Sump Hooded Catch Basin:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5 \%} \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Stormtech Isolator Row:

$$
\begin{aligned}
& 75 \% * 25 \%=19 \% \\
& 75 \%-19 \%=56 \%
\end{aligned}
$$

Pretreatment TSS Removal $=\mathbf{2 5 \%}+\mathbf{1 9 \%}=\mathbf{4 4 \%}$

Treatment Train \#1 to Pond P204

- Stormtech Isolator Row:

$$
\begin{aligned}
& 100 \% * 25 \%=25 \% \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Stormtech Infiltration Chambers

$$
\begin{aligned}
& 75 \% * 80 \%=60 \% \\
& 75 \%-60 \%=15 \%
\end{aligned}
$$

TSS Removal of the proposed drainage $=\mathbf{2 5 \%}+\mathbf{6 0 \%}=\mathbf{8 5} \%$
Site impervious percentage $=7 \%$

Pretreatment Train \#2 to Pond P205

- Deep Sump Hooded Catch Basins:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5 \%} \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Sediment Forebay:

$$
\begin{aligned}
& 75 \% * 25 \%=19 \% \\
& 75 \%-19 \%=56 \%
\end{aligned}
$$

## Pretreatment TSS Removal $=\mathbf{2 5} \%+\mathbf{1 9 \%}=\mathbf{4 4 \%}$

Treatment Train \#2 to Pond P205

- Sediment Forebay:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5} \% \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Infiltration Pond

$$
\begin{aligned}
& 75 \% * 80 \%=\mathbf{6 0 \%} \\
& 75 \%-60 \%=15 \%
\end{aligned}
$$

TSS Removal of the proposed drainage $=\mathbf{2 5 \%}+\mathbf{+ 6 0 \%}=\mathbf{8 5 \%}$
Site impervious percentage $=5 \%$

## Pretreatment Train \#3 to Pond P206

- Deep Sump Hooded Catch Basin:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5 \%} \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Stormtech Isolator Row:

$$
\begin{aligned}
& 75 \% * 25 \%=19 \% \\
& 75 \%-19 \%=56 \%
\end{aligned}
$$

Pretreatment TSS Removal = 25\% + 19\% = 44\%

Treatment Train \#3 to Pond P206

- Stormtech Isolator Row:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5 \%} \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Stormtech Infiltration Chambers

$$
\begin{aligned}
& 75 \% * 80 \%=60 \% \\
& 75 \%-60 \%=15 \%
\end{aligned}
$$

TSS Removal of the proposed drainage $=\mathbf{2 5} \%+\mathbf{6 0 \%}=\mathbf{8 5} \%$
Site impervious percentage $=8 \%$

Pretreatment Train \#4 to Pond P207

- Deep Sump Hooded Catch Basins:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5} \% \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Sediment Forebay / CDS Water Quality Unit (Calculation based on minimum treatment from Deep Sump Hooded Catch Basin only. CDS Water Quality unit will see a higher TSS treatment removal rate.):

$$
75 \% * 25 \%=19 \%
$$

$75 \%-19 \%=56 \%$

Pretreatment TSS Removal = 25\% + 19\% = 44\%

Treatment Train \#4 to Pond P207

- Sediment Forebay / CDS Water Quality Unit (Calculations based on minimum treatment from Deep Sump Hooded Catch Basin only. CDS Water Quality Unit will see a higher TSS treatment removal rate.):
$100 \%$ * $25 \%=\mathbf{2 5} \%$
$100 \%-25 \%=75 \%$
- Infiltration Pond:

$$
\begin{aligned}
& 75 \% * 80 \%=\mathbf{6 0 \%} \\
& 75 \%-60 \%=15 \%
\end{aligned}
$$

TSS Removal of the proposed drainage $=\mathbf{2 5} \%+\mathbf{6 0} \%=\mathbf{8 5 \%}$
Site impervious percentage $=\mathbf{2 5 \%}$

Pretreatment Train \#5 to Pond P210

- Deep Sump Hooded Catch Basins:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5} \% \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Sediment Forebay:

$$
\begin{aligned}
& 75 \% * 25 \%=19 \% \\
& 75 \%-19 \%=56 \%
\end{aligned}
$$

Pretreatment TSS Removal = 25\% + 19\% = 44\%

Treatment Train \#5 to Pond P210

- Sediment Forebay:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5} \% \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Pocket Wetland

$$
\begin{aligned}
& 75 \% * 80 \%=\mathbf{6 0 \%} \\
& 75 \%-60 \%=15 \%
\end{aligned}
$$

TSS Removal of the proposed drainage $=\mathbf{2 5 \%}+\mathbf{+ 6 0 \%}=\mathbf{8 5 \%}$
Site impervious percentage $=\mathbf{1 0} \%$

Pretreatment Train \#6 to Pond P212

- Deep Sump Hooded Catch Basin

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5 \%} \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Sediment Forebay:

$$
\begin{aligned}
& 75 \% * 25 \%=19 \% \\
& 75 \%-19 \%=56 \%
\end{aligned}
$$

Pretreatment TSS Removal $=\mathbf{2 5 \%}+\mathbf{1 9 \%}=\mathbf{4 4 \%}$

Treatment Train \#6 to Pond P212

- Sediment Forebay:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5} \% \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Infiltration Pond:

$$
\begin{aligned}
& 75 \% * 80 \%=\mathbf{6 0 \%} \\
& 75 \%-60 \%=15 \%
\end{aligned}
$$

TSS Removal of the proposed drainage $=\mathbf{2 5 \%}+\mathbf{+ 6 0 \%}=\mathbf{8 5 \%}$
Site impervious percentage $=28 \%$

Pretreatment Train \#7 to Pond P213

- Deep Sump Hooded Catch Basin:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5} \% \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Stormtech Isolator Row:

$$
\begin{aligned}
& 75 \% * 25 \%=19 \% \\
& 75 \%-19 \%=56 \%
\end{aligned}
$$

Pretreatment TSS Removal $=\mathbf{2 5 \%}+\mathbf{1 9 \%}=\mathbf{4 4 \%}$

Treatment Train \#7 to Pond P213

- Stormtech Isolator Row:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5} \% \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Stormtech Infiltration Chambers:

$$
\begin{aligned}
& 75 \% * 80 \%=\mathbf{6 0 \%} \\
& 75 \%-60 \%=15 \%
\end{aligned}
$$

TSS Removal of the proposed drainage $=\mathbf{2 5 \%}+\mathbf{6 0 \%}=\mathbf{8 5 \%}$
Site impervious percentage $=8 \%$

Pretreatment Train \#8 to Pond P214

- Deep Sump Hooded Catch Basin:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5 \%} \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Stormtech Isolator Row:

$$
\begin{aligned}
& 75 \% * 25 \%=19 \% \\
& 75 \%-19 \%=56 \%
\end{aligned}
$$

Pretreatment TSS Removal = 25\% + 19\% = 44\%

Treatment Train \#8 to Pond P214

- Stormtech Isolator Row:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5} \% \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

- Stormtech Infiltration Chambers:

$$
\begin{aligned}
& 75 \% * 80 \%=\mathbf{6 0 \%} \\
& 75 \%-60 \%=15 \%
\end{aligned}
$$

TSS Removal of the proposed drainage $=\mathbf{2 5} \%+\mathbf{6 0} \%=\mathbf{8 5} \%$
Site impervious percentage $=\mathbf{3 \%}$
Treatment Train \#9 to Existing Summer Street CB

- Deep Sump Hooded Catch Basins:

$$
\begin{aligned}
& 100 \% * 25 \%=\mathbf{2 5} \% \\
& 100 \%-25 \%=75 \%
\end{aligned}
$$

TSS Removal of the proposed drainage $=\mathbf{2 5 \%}$
Site impervious percentage $=3 \%$

Treatment Train \#10 Overland flow from rooftops and pavement

TSS Removal of the proposed drainage $=0 \%$
Site impervious percentage $=3 \%$

Total weighted TSS Removal rate $=[(7 \% * 85 \%)+(5 \% * 85 \%)+(8 \% * 85 \%)+(25 \% * 85 \%)+$ $(10 \% * 85 \%)+(28 \% * 85 \%)+(8 \% * 85 \%)+(3 \% * 85 \%)+(3 \% * 25 \%)+(3 \% * 0 \%)$
$=5.9+4.3+6.8+21.3+8.5+23.8+6.8+2.6+0.8+0=\underline{80.8 \%>80 \%}, \mathrm{OK}$

## Standard 5: Land uses with higher potential pollutant loads.

The development is not considered a land use that generally produces higher potential pollutant loads.

## Standard 6: Stormwater discharges to critical areas

There are three potential vernal pools located on the property. Potential vernal pool \#1 is located on the eastern side of the property just north of Pocket Wetland \#1 and adjacent to the railroad. Potential vernal pool \#2 is isolated and located just to the north of PVP\#1, also adjacent to the railroad. Potential vernal pool \#3 is located on the southern side of the parcel to the south of both wetland crossings. A pocket wetland is proposed to outlet more than 180 ' upslope from potential vernal pool \#1. All treatment trains that discharge to critical areas have been designed to meet the pretreatment requirement of $44 \%$ TSS removal prior to entering the treatment system such as an Infiltration Pond or Pocket Wetland.

## Standard 7: Redevelopment projects

The project is not considered a redevelopment project.

## Standard 8: Control construction-related impacts

The project will install erosion and sediment controls prior to any earthwork activity. Erosion control barriers will be placed down slope from the proposed construction to prevent erosion and sedimentation into the surrounding areas. The barriers will be maintained and inspected periodically during construction; sediment buildup will be removed, and any damaged barrier will be replaced as needed. See site plan and SWPPP.

## Standard 9: Long-term operation and maintenance plan

See Appendix A for the operation and maintenance requirements of the stormwater management system.

## Standard 10: No illicit discharges

An illicit discharge compliance statement has been provided by the property owner under separate cover.

## Appendix A: Test Pit Information and Falling Head Permeability Test Results

Test Pits Performed on 12/4/2019, 12/5/2019, 12/6/2019, 01/09/2020, 10/20/2020, 10/21/2020
Test Pits Performed By Kasey Ferreira, E.I.T.
Test Pits Witnessed By Chris Johnson, Town of Walpole

| TP-1 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0^{\prime \prime}-13^{\prime \prime}$ | $A_{p}$ | Loam/Organics |  |
| $13^{\prime \prime}-23^{\prime \prime}$ | $\mathrm{B}_{w}$ | Sandy Loam |  |
| $23^{\prime \prime}-120^{\prime \prime}$ | C | Loamy Sand |  |
| Mottles at 24" |  |  |  |
| HSG C |  |  |  |


| TP-2 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10 "$ | $A_{p}$ | Loam/Organics |  |
| $10 "-118 "$ | C | Loamy Sand |  |
| Weeping at 71", Mottles at 33" |  |  |  |
| HSG B |  |  |  |


| TP-3 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-12 "$ | $A_{p}$ | Loam |  |
| $12 "-20 "$ | $B_{w}$ | Sandy Loam |  |
| $20 "-98 "$ | C | Loamy Sand |  |
| Standing at 94", Mottles at 31" |  |  |  |
| HSG C |  |  |  |


| TP-4 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-11^{\prime \prime}$ | $A_{p}$ | Loam/Organics |  |
| $11^{\prime \prime-96 " ~}$ | C | Loamy Sand |  |
| Standing at 97", Mottles at 49" |  |  |  |
| HSG A |  |  |  |


| TP-5 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0^{\prime \prime-12 "}$ | $A_{p}$ | Loam/Organics |  |
| $12^{\prime \prime}-109 "$ | C | Loamy Sand |  |
| Standing at 85", Mottles at 41" |  |  |  |
| HSG A |  |  |  |


| TP-6 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-11^{\prime \prime}$ | $A_{p}$ | Loam/Organics |  |
| $11^{\prime \prime}-20^{\prime \prime}$ | $\mathrm{B}_{w}$ | Sandy Loam |  |
| $20 "-99^{\prime \prime}$ | C | Loamy Sand |  |
| Standing at 60", Mottles at 26" |  |  |  |
| HSG C |  |  |  |


| TP-7 (Drainage) |  |  |  |
| :---: | :---: | :---: | :---: |
| 0"-7" | $\mathrm{A}_{\mathrm{p}}$ | Loam |  |
| 7"-122" | C | Loamy Sand |  |
| Weeping at 103", Mottles at 40" |  |  |  |
| HSG B |  |  |  |


| TP-8 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-7 "$ | $A_{p}$ | Loam |  |
| $7 "-18 "$ | $B_{w}$ | Sandy Loam |  |
| $18 "-139^{\prime \prime}$ | C | Loamy Sand |  |
| Standing at 130", Weeping at 125", Mottles at 44" |  |  |  |
| HSG B |  |  |  |


| TP-9 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-40^{\prime \prime}$ | Fill |  |  |
| $40 "-117^{\prime \prime}$ | C | Medium Sand |  |
| Weeping at 34", Mottles at 40" |  |  |  |
| HSG A |  |  |  |


| TP-10 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0^{\prime \prime-} 9^{\prime \prime}$ | $A_{p}$ | Loam |  |
| $9^{\prime \prime}-23^{\prime \prime}$ | $B_{w}$ | Sandy Loam |  |
| $23^{\prime \prime-96 " ~}$ | C | Coarse Sand |  |
| Standing at 96", Mottles at 34" |  |  |  |
| HSG C |  |  |  |


| TP-11 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-8 "$ | $A_{p}$ | Loam |  |
| $8 "-17^{\prime \prime}$ | $\mathrm{B}_{w}$ | Loamy Sand |  |
| $17^{\prime \prime}-122^{\prime \prime}$ | C | Medium Sand |  |
| Standing at $115 "$, Weeping at 46", Mottles at 30" |  |  |  |
| HSG B |  |  |  |

MASTER LOG - TEST PIT INFORMATION
55 Summer Street, Walpole MA

| TP-12 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0^{\prime \prime}-9 "$ | $A_{p}$ | Loam |  |
| $9 "-18^{\prime \prime}$ | $\mathrm{B}_{w}$ | Loamy Sand |  |
| $18^{\prime \prime}-120 "$ | C | Medium Sand |  |
| Weeping at 24", Mottles at 29" |  |  |  |
| HSG B |  |  |  |


| TP-13 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-13^{\prime \prime}$ | $A_{p}$ | Loam |  |
| $13^{\prime \prime}-28^{\prime \prime}$ | $\mathrm{B}_{w}$ | Sandy Loam |  |
| $28 "-135 "$ | C | Loamy Sand |  |
| Weeping at 115", Mottles at 43" |  |  |  |
| HSG B |  |  |  |


| TP-14 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-7 "$ | $A_{p}$ | Loam |  |
| $77^{\prime \prime}-13^{\prime \prime}$ | $\mathrm{B}_{w}$ | Loamy Fine Sand |  |
| $13^{\prime \prime}-95^{\prime \prime}$ | $\mathrm{C}_{1}$ | Coarse Sand |  |
| $95^{\prime \prime}-120^{\prime \prime}$ | $\mathrm{C}_{2}$ | Gravel |  |
| Standing at 104", Mottles at 95" |  |  |  |
| HSG A |  |  |  |


| TP-15 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-5 "$ | $A_{p}$ | Loam |  |
| $5 "-22^{\prime \prime}$ | $B_{w}$ | Sandy Loam |  |
| $22^{\prime \prime-120 "}$ | C | Loamy Sand |  |
| Weeping at 30" |  |  |  |
| HSG C |  |  |  |


| TP-16 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0^{\prime \prime-} 5^{\prime \prime}$ | $A_{p}$ | Loam |  |
| $5 "-20^{\prime \prime}$ | $B_{w}$ | Sandy Loam |  |
| $20 "-120 "$ | C | Loamy Sand |  |
| Standing at 96", Mottles at 36" |  |  |  |
| HSG C |  |  |  |


| TP-17 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| 0"-9" | $A_{p}$ | Loam/Organics |  |
| $9 "-108^{\prime \prime}$ | C | Loamy Sand |  |
| Weeping at 18" |  |  |  |
| HSG A/D (D) |  |  |  |


| TP-18 (Drainage) |  |  |  |
| :---: | :---: | :---: | :---: |
| 0"-8" | $\mathrm{A}_{p}$ | Loam |  |
| 8"-17" | Bw | Sandy Loam |  |
| 17"-120" | C | Loamy Sand |  |
| Weeping at 29", Mottles at $26{ }^{\prime \prime}$ |  |  |  |
| HSG C |  |  |  |

MASTER LOG - TEST PIT INFORMATION
55 Summer Street, Walpole MA

| TP-19 (Building) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0^{\prime \prime}-8^{\prime \prime}$ | $A_{p}$ | Loam |  |
| $8 "-20^{\prime \prime}$ | $B_{w}$ | Loamy Sand |  |
| $20 "-120 "$ | C | Sand |  |
| Mottles at 50" |  |  |  |
| HSG A |  |  |  |


| TP-20 (Building) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-7 "$ | $A_{p}$ | Loam |  |
| $7 "-24 "$ | $B_{w}$ | Sandy Loam |  |
| $24 "-103 "$ | C | Loamy Sand |  |
| Mottles at 48" |  |  |  |
| HSG B |  |  |  |


| TP-21 (Exploratory) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-9 "$ | $A_{p}$ | Loam |  |
| $9^{\prime \prime}-24 "$ | $B_{w}$ | Loamy Sand |  |
| $24 "-102 "$ | C | Sand |  |
| Mottles at 43" |  |  |  |
| HSG A |  |  |  |


| TP-22 (Exploratory) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-6 "$ | $A_{p}$ | Loam |  |
| $6 "-23 "$ | $B_{w}$ | Sandy Loam |  |
| $23^{\prime \prime}-66 "$ | C | Loamy Sand |  |
| Mottles at 32" |  |  |  |
| HSG C |  |  |  |


| TP-23 (Building) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0^{\prime \prime-12 "}$ | $A_{p}$ | Loam |  |
| $12^{\prime \prime}-24 "$ | $B_{w}$ | Loamy Sand |  |
| $24 "-118^{\prime \prime}$ | C | Sand |  |
| Standing at 96", Mottles at 36" |  |  |  |
| HSG B |  |  |  |


| TP-24 (Building) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-11^{\prime \prime}$ | $A_{p}$ | Loam |  |
| $11^{\prime \prime}-24 "$ | $\mathrm{~B}_{\mathrm{w}}$ | Loamy Sand |  |
| $24 "-102^{\prime \prime}$ | C | Sand |  |
| Weeping at 100", Mottles at 39", Refusal at 102" |  |  |  |
| HSG B |  |  |  |

MASTER LOG - TEST PIT INFORMATION
55 Summer Street, Walpole MA

| TP-25 (Building) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10 "$ | $A_{p}$ | Loam |  |
| $10 "-20 "$ | $B_{w}$ | Loamy Sand |  |
| $20 "-69 "$ | C | Sand |  |
| Mottles at 41", Refusal at 69" |  |  |  |
| HSG A |  |  |  |


| TP-25A (Exploratory) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0^{\prime \prime-} 8^{\prime \prime}$ | $A_{p}$ | Loam |  |
| $8 "-15^{\prime \prime}$ | $\mathrm{B}_{w}$ | Sandy Loam |  |
| $15^{\prime \prime}-108^{\prime \prime}$ | C | Loamy Sand |  |
| Mottles at 32" |  |  |  |
| HSG C |  |  |  |


| TP-26 (Building) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0^{\prime \prime-10 "}$ | $A_{p}$ | Loam |  |
| $10^{\prime \prime}-18^{\prime \prime}$ | $B_{w}$ | Sandy Loam |  |
| $18 "-75^{\prime \prime}$ | C | Loamy Sand |  |
| Mottles at 49", Refusal at 75" |  |  |  |
| HSG B |  |  |  |


| TP-27 (Building) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-8 "$ | $A_{p}$ | Loam |  |
| $8 "-25 "$ | $\mathrm{~B}_{w}$ | Sandy Loam |  |
| $25 "-48^{\prime \prime}$ | $\mathrm{C}_{1}$ | Loamy Sand |  |
| $48 "-110$ | $\mathrm{C}_{2}$ | Loamy Sand |  |
| Standing at $100 "$, Weeping at $54 "$, Mottles at $30 "$ |  |  |  |
| HSG C |  |  |  |


| TP-28 (Building) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0^{\prime \prime-}-6 "$ | $A_{p}$ | Loam |  |
| $6 "-24 "$ | $B_{w}$ | Sandy Loam |  |
| $24 "-99^{\prime \prime}$ | C | Loamy Sand |  |
| Standing at 90", Weeping at 65", Mottles at 32" |  |  |  |
| HSG C |  |  |  |


| TP-29 (Exploratory) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-13^{\prime \prime}$ | $A_{p}$ | Loam |  |
| $13^{\prime \prime-18 "}$ | $B_{w}$ | Sandy Loam |  |
| $18 "-132^{\prime \prime}$ | C | Loamy Sand |  |
| Mottles at 43" |  |  |  |
| HSG B |  |  |  |


| TP-30 (Exploratory) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-12^{\prime \prime}$ | $A_{p}$ | Loam/Organics |  |
| $12^{\prime \prime-} 30 "$ | $B_{w}$ | Loam |  |
| $30 "-128 "$ | C | Loamy Sand |  |
| Weeping at 102", Mottles at $36 "$ |  |  |  |
| HSG C |  |  |  |


| TP-31 (Exploratory) |  |  |  |
| :--- | :--- | :--- | :--- |
| 0"-32" | Fill |  |  |
| $32^{\prime \prime}-96 "$ | C | Gravelly Loamy sand |  |
| Mottles at 42" |  |  |  |
| HSG A |  |  |  |


| TP-32 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-14 "$ | A | Sandy Loam |  |
| $14 "-20^{\prime \prime}$ | B | Sandy Loam |  |
| $20 "-88^{\prime \prime}$ | C | Sandy Loam |  |
| Mottles at 30", No Standing |  |  |  |
| HSG C |  |  |  |


| TP-33 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-12^{\prime \prime}$ | A | Sandy Loam |  |
| $12^{\prime \prime-}-30$ | B | Sandy Loam |  |
| $30 "-87^{\prime \prime}$ | C | Loamy Sand |  |
| Mottles at 30" |  |  |  |
| HSG C |  |  |  |


| TP-34 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10 "$ | A | Sandy Loam |  |
| $10 "-24 "$ | Bw | Sandy Loam |  |
| $24 "-72 "$ | C | Loamy Sand |  |
| Mottles at 37" |  |  |  |
| HSG C |  |  |  |


| TP-36 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10^{\prime \prime}$ | A | Sandy Loam |  |
| $10^{\prime \prime}-22^{\prime \prime}$ | Bw | Sandy Loam |  |
| $22 "-62^{\prime \prime}$ | C | Sandy Loam |  |
| Mottles at 21" |  |  |  |
| HSG C |  |  |  |


| TP-37 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-11^{\prime \prime}$ | A | Sandy Loam |  |
| $11^{\prime \prime-} 28^{\prime \prime}$ | Bw | Sandy Loam |  |
| $28^{\prime \prime-52 "}$ | C | Sandy Loam |  |
| Seasonal high at 28" |  |  |  |
| HSG C |  |  |  |

MASTER LOG - TEST PIT INFORMATION
55 Summer Street, Walpole MA

| TP-38 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-12^{\prime \prime}$ | A | Sandy Loam |  |
| $12^{\prime \prime}-28^{\prime \prime}$ | B | Sandy Loam |  |
| $28^{\prime \prime-72 "}$ | C | Sand |  |
| Mottles at 42" |  |  |  |
| HSG B |  |  |  |


| TP-39 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-14 "$ | A | Sandy Loam |  |
| $14 "-37^{\prime \prime}$ | B | Sandy Loam |  |
| $37^{\prime \prime-}-66^{\prime \prime}$ | C | Loamy Sand |  |
| Mottles at 36" |  |  |  |
| HSG C |  |  |  |


| TP-40 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| 0"-14" | A | Sandy Loam |  |
| $14 "-30 "$ | Bw | Sandy Loam |  |
| $30 "-59^{\prime \prime}$ | C1 | Sand |  |
| $59 "-98 "$ | C2 | Loamy Sand |  |
| Seasonal high at 28" |  |  |  |
| HSG C |  |  |  |


| TP-40A (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-14 "$ | A | Sandy Loam |  |
| $14 "-23^{\prime \prime}$ | Bw | Sandy Loam |  |
| $23^{\prime \prime-} 80^{\prime \prime}$ | C | Sand |  |
| Seasonal high at 40" |  |  |  |
| HSG C |  |  |  |


| TP-41 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| 0"-9" | A | Sandy Loam |  |
| 9"-20" | B | Sandy Loam |  |
| $20 "-88^{\prime \prime}$ | C | Sand |  |
| Seasonal high at 45" |  |  |  |
| HSG B |  |  |  |


| TP-42 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10 "$ | A | Sandy Loam |  |
| $10 "-28^{\prime \prime}$ | Bw | Sandy Loam |  |
| $28 "-86 "$ | C | Sand |  |
| Seasonal high at 48", presence of color change |  |  |  |
| HSG B |  |  |  |

MASTER LOG - TEST PIT INFORMATION
55 Summer Street, Walpole MA

| TP-43 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10 "$ | A | Sandy Loam |  |
| $10 "-26 "$ | Bw | Sandy Loam |  |
| $26 "-64 "$ | C1 | Sandy Loam |  |
| $64 "-100 "$ | C2 | Loamy Sand |  |
| Mottles at $26 "$ |  |  |  |
| HSG C |  |  |  |


| TP-43A (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10 "$ | A | Sandy Loam |  |
| $10 "-19^{\prime \prime}$ | Bw | Sandy Loam |  |
| $19^{\prime \prime-} 89^{\prime \prime}$ | C | Sandy Loam |  |
| Mottles at 16" |  |  |  |
| HSG B/D (B) |  |  |  |


| TP-44 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10 "$ | A | Sandy Loam |  |
| $10 "-35^{\prime \prime}$ | Bw | Sandy Loam |  |
| $35 "-52 "$ | C1 | Sandy Loam |  |
| $52^{\prime \prime-76 "}$ | C2 | Loamy Sand |  |
| Seasonal high at 35" |  |  |  |
| HSG C |  |  |  |


| TP-45 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| 0 "-12" | A | Sandy Loam |  |
| $12^{\prime \prime-} 27^{\prime \prime}$ | Bw | Sandy Loam |  |
| $27 "-56 "$ | C1 | Sandy Loam |  |
| $56 "-91^{\prime \prime}$ | C2 | Loamy Sand |  |
|  |  |  |  |
| TBD |  |  |  |


| TP-46 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| 0"-12" | A | Sandy Loam |  |
| $12^{\prime \prime}-27^{\prime \prime}$ | Bw | Sandy Loam |  |
| $27^{\prime \prime}-52^{\prime \prime}$ | C1 | Sandy Loam |  |
| $52^{\prime \prime}-100 "$ | C2 | Loamy Sand |  |
| Seasonal high at 18" |  |  |  |
| HSG B/D (B) |  |  |  |


| TP-47 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| 0"-12" | A | Sandy Loam |  |
| $12^{\prime \prime}-34 "$ | Bw | Sandy Loam |  |
| $34 "-48^{\prime \prime}$ | C1 | Sandy Loam |  |
| $48 "-102 "$ | C2 | Loamy Sand |  |
| Seasonal high at $30 "$ |  |  |  |
| HSG C |  |  |  |

MASTER LOG - TEST PIT INFORMATION
55 Summer Street, Walpole MA

| TP-48 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-12^{\prime \prime}$ | A | Sandy Loam |  |
| $12^{\prime \prime}-29 "$ | Bw | Sandy Loam |  |
| $29 "-80 "$ | C | Loamy Sand |  |
| Seasonal high at 36" |  |  |  |
| HSG C |  |  |  |


| TP-49 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10^{\prime \prime}$ | A | Sandy Loam |  |
| $10 "-24 "$ | Bw | Sandy Loam |  |
| $24 "-60 "$ | C | Loamy Sand |  |
| Refusal at 60". No seasonal high present. |  |  |  |
| HSG B |  |  |  |


| TP-50 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-12^{\prime \prime}$ | A | Sandy Loam |  |
| $12^{\prime \prime}-25^{\prime \prime}$ | Bw | Sandy Loam |  |
| $25 "-67^{\prime \prime}$ | C | Loamy Sand |  |
| No seasonal high. |  |  |  |
| HSG B |  |  |  |


| TP-51 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-12^{\prime \prime}$ | A | Sandy Loam |  |
| $12 "-34 "$ | B | Sandy Loam |  |
| $34 "-65 "$ | C | Loamy Sand |  |
| Seasonal high at 21" |  |  |  |
| HSG B/D |  |  |  |


| TP-52 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10 "$ | A | Sandy Loam |  |
| $10^{\prime \prime}-28^{\prime \prime}$ | Bw | Sandy Loam |  |
| $28 "-72^{\prime \prime}$ | C | Loamy Sand |  |
| Seasonal high at 53" |  |  |  |
| HSG B |  |  |  |


| TP-53 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10^{\prime \prime}$ | A | Sandy Loam |  |
| $10^{\prime \prime}-32^{\prime \prime}$ | B | Sandy Loam |  |
| $32 "-78^{\prime \prime}$ | C | Sandy Loam |  |
| Seasonal high at 32" |  |  |  |
| HSG C |  |  |  |

MASTER LOG - TEST PIT INFORMATION
55 Summer Street, Walpole MA

| TP-2A (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-14 "$ | A | Sandy Loam |  |
| $14 "-28 "$ | Bw | Sandy Loam |  |
| $28 "-72^{\prime \prime}$ | C | Sandy Loam |  |
| Mottles at 25" |  |  |  |
| HSG C |  |  |  |


| HSG-1 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10^{\prime \prime}$ | A | Sandy Loam |  |
| $10^{\prime \prime}-28^{\prime \prime}$ | Bw | Sandy Loam |  |
| $28^{\prime \prime}-32^{\prime \prime}$ | C | Loamy Sand |  |
| Refusal at 32". No seasonal high. |  |  |  |
| HSG C |  |  |  |


| HSG-2 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10 "$ | A | Sandy Loam |  |
| $10 "-30^{\prime \prime}$ | Bw | Sandy Loam |  |
| $30 "-42^{\prime \prime}$ | C | Loamy Sand |  |
| Seasonal high at 32" |  |  |  |
| HSG C |  |  |  |


| HSG-3 (Drainage) |  |  |  |
| :--- | :--- | :--- | :--- |
| $0 "-10 "$ | A | Sandy Loam |  |
| $10 "-22^{\prime \prime}$ | Bw | Sandy Loam |  |
| $22^{\prime \prime-41 "}$ | C | Sandy Loam |  |
| Seasonal high at 34" |  |  |  |
| HSG C |  |  |  |

## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 1 <br> Test Date: 28-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 8 " below existing grade

| H (inches) | $\begin{gathered} \mathrm{T} \\ \text { (seconds) } \end{gathered}$ | $\begin{gathered} \mathrm{H}_{1} / \mathrm{H}_{2} \\ \text { (inches) } \end{gathered}$ | $\begin{gathered} \mathbf{t}_{2}-\mathbf{t}_{1} \\ \text { (seconds) } \end{gathered}$ | $\ln \left(\mathrm{H}_{1} / \mathrm{H}_{2}\right)$ | k (in/hr) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 24 | 0 | n/a | n/a |  |  |
| 23 | 600 | 1.04 | 600 | 0.043 | 0.3 |
| 22 | 1320 | 1.05 | 720 | 0.044 | 0.3 |
| 21 | 2340 | 1.05 | 1020 | 0.047 | 0.2 |
| 20 | 3360 | 1.05 | 1020 | 0.049 | 0.2 |
| 19 | 4440 | 1.05 | 1080 | 0.051 | 0.2 |
| 18 | 5460 | 1.06 | 1020 | 0.054 | 0.2 |
|  | Average | $0.22 \mathrm{in} / \mathrm{hr}$ |  |  |  |
|  | Safety Factor | 2 |  |  |  |
|  | Design K | $0.11 \mathrm{in} / \mathrm{hr}$ |  |  |  |

## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 2A <br> Test Date: 28-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 8 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 32 <br> Test Date: 20-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 32 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

```
Project: Summer Street, Walpole
Location: OTH 33
Test Date: 20-Oct-20
By:
Daniel J. Merrikin, P.E./Kasey Ferreira
```



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 37 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 34 <br> Test Date: 20-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 32 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 16 <br> Test Date: 20-Oct-20 <br> By: Daniel J. Merrikin, P.E.

Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 32 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 38 <br> Test Date: 20-Oct-20 <br> By: Daniel J. Merrikin, P.E.

Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 38 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 40 <br> Test Date: 20-Oct-20 <br> By: Daniel J. Merrikin, P.E.

Test apparatus
24 " long x 4" diameter schedule 40 pvc pipe
Bottom of pipe set 32 " below existing grade

| $\begin{gathered} \mathrm{H} \\ \text { (inches) } \end{gathered}$ | $\begin{gathered} \mathrm{T} \\ \text { (seconds) } \end{gathered}$ | $\begin{gathered} \mathrm{H}_{1} / \mathrm{H}_{2} \\ \text { (inches) } \end{gathered}$ | $\begin{gathered} \mathbf{t}_{2}-\mathrm{t}_{1} \\ \text { (seconds) } \end{gathered}$ | $\operatorname{In}\left(\mathrm{H}_{1} / \mathrm{H}_{2}\right)$ | k (in/hr) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 24 | 0 | n/a | n/a |  |  |
| 22 | 33 | 1.09 | 33 | 0.087 | 10.8 |
| 20 | 75 | 1.10 | 42 | 0.095 | 9.3 |
| 18 | 118 | 1.11 | 43 | 0.105 | 10.1 |
| 16 | 166 | 1.13 | 48 | 0.118 | 10.1 |
| 14 | 219 | 1.14 | 53 | 0.134 | 10.4 |
| 12 | 277 | 1.17 | 58 | 0.154 | 10.9 |
|  | Average | $10.3 \mathrm{in} / \mathrm{hr}$ |  |  |  |
|  | Safety Factor | $2$ |  |  |  |
|  | Design K | $5.13 \mathrm{in} / \mathrm{hr}$ |  |  |  |

## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 40A <br> Test Date: 20-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24" long x 4" diameter schedule 40 pvc pipe
Bottom of pipe set 42 " below existing grade

| H (inches) | $\begin{gathered} \mathrm{T} \\ \text { (seconds) } \end{gathered}$ | $\begin{gathered} \mathrm{H}_{1} / \mathrm{H}_{2} \\ \text { (inches) } \end{gathered}$ | $\begin{gathered} \mathbf{t}_{2}-\mathbf{t}_{1} \\ \text { (seconds) } \end{gathered}$ | $\ln \left(\mathrm{H}_{1} / \mathrm{H}_{2}\right)$ | k (in/hr) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 24 | 0 | n/a | n/a |  |  |
| 22 | 34 | 1.09 | 34 | 0.087 | 10.5 |
| 20 | 60 | 1.10 | 26 | 0.095 | 15.1 |
| 18 | 93 | 1.11 | 33 | 0.105 | 13.1 |
| 16 | 129 | 1.13 | 36 | 0.118 | 13.4 |
| 14 | 171 | 1.14 | 42 | 0.134 | 13.1 |
| 12 | 217 | 1.17 | 46 | 0.154 | 13.8 |
|  | Average | $13.2 \mathrm{in} / \mathrm{hr}$ |  |  |  |
|  | Safety Factor | 2 |  |  |  |
|  | Design K | $6.58 \mathrm{in} / \mathrm{hr}$ |  |  |  |

## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 41 <br> Test Date: 20-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4" diameter schedule 40 pvc pipe
Bottom of pipe set 42 " below existing grade

| $\begin{gathered} \mathrm{H} \\ \text { (inches) } \end{gathered}$ | $\begin{gathered} \mathrm{T} \\ \text { (seconds) } \end{gathered}$ | $\begin{gathered} \mathrm{H}_{1} / \mathrm{H}_{2} \\ \text { (inches) } \end{gathered}$ | $\begin{gathered} \mathrm{t}_{2}-\mathrm{t}_{1} \\ \text { (seconds) } \end{gathered}$ | $\operatorname{In}\left(\mathrm{H}_{1} / \mathrm{H}_{2}\right)$ | k (in/hr) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 24 | 0 | n/a | n/a |  |  |
| 22 | 15 | 1.09 | 15 | 0.087 | 23.8 |
| 20 | 37 | 1.10 | 22 | 0.095 | 17.8 |
| 18 | 63 | 1.11 | 26 | 0.105 | 16.7 |
| 16 | 102 | 1.13 | 39 | 0.118 | 12.4 |
| 14 | 145 | 1.14 | 43 | 0.134 | 12.8 |
| 12 | 185 | 1.17 | 40 | 0.154 | 15.8 |
|  | Average | $16.6 \mathrm{in} / \mathrm{hr}$ |  |  |  |
|  | Safety Factor | $2$ |  |  |  |
|  | Design K | $8.28 \mathrm{in} / \mathrm{hr}$ |  |  |  |

## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 42 (in C1) <br> Test Date: 20-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 38 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole location: OTH 42 (in C2) (remove C1) <br> Test Date: 21-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 52 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole location: OTH 43 (in C2) (remove C1) Test Date: 21-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 74 " below existing grade

| $\begin{gathered} \mathrm{H} \\ \text { (inches) } \end{gathered}$ | $\begin{gathered} \mathrm{T} \\ \text { (seconds) } \end{gathered}$ | $\begin{gathered} \mathrm{H}_{1} / \mathrm{H}_{2} \\ \text { (inches) } \end{gathered}$ | $\begin{gathered} t_{2}-t_{1} \\ \text { (seconds) } \end{gathered}$ | $\ln \left(\mathrm{H}_{1} / \mathrm{H}_{2}\right)$ | k (in/hr) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 24 | 0 | n/a | n/a |  |  |
| 23 | 75 | 1.04 | 75 | 0.043 | 2.3 |
| 22 | 164 | 1.05 | 89 | 0.044 | 2.1 |
| 21 | 390 | 1.05 | 226 | 0.047 | 0.8 |
| 20 | 600 | 1.05 | 210 | 0.049 | 1.0 |
| 19 | 870 | 1.05 | 270 | 0.051 | 0.8 |
| 18 | 1110 | 1.06 | 240 | 0.054 | 0.9 |
|  | Average | $1.3 \mathrm{in} / \mathrm{hr}$ |  |  |  |
|  | Safety Factor | 2 |  |  |  |
|  | Design K | $0.66 \mathrm{in} / \mathrm{hr}$ |  |  |  |

## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 44 (in C2) (remove C1) Test Date: 21-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 56 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 45 (in C2) (remove C1) Test Date: 21-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 62 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

Project: Summer Street, Walpole
Location: OTH 46 (in C2) (remove C1)
Test Date: 21-Oct-20
By: Daniel J. Merrikin, P.E.


Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 64 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole location: OTH 47 (in C2) (remove C1) <br> Test Date: 21-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 72 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole <br> Location: OTH 48 <br> Test Date: 21-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 48 " below existing grade

| $\begin{gathered} \mathrm{H} \\ \text { (inches) } \end{gathered}$ | $\begin{gathered} \mathrm{T} \\ \text { (seconds) } \end{gathered}$ | $\begin{gathered} \mathrm{H}_{1} / \mathrm{H}_{2} \\ \text { (inches) } \end{gathered}$ | $\begin{gathered} \mathbf{t}_{2}-\mathrm{t}_{1} \\ \text { (seconds) } \end{gathered}$ | $\operatorname{In}\left(\mathrm{H}_{1} / \mathrm{H}_{2}\right)$ | k (in/hr) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 24 | 0 | n/a | n/a |  |  |
| 23 | 7 | 1.04 | 7 | 0.043 | 25.0 |
| 22 | 15 | 1.05 | 8 | 0.044 | 22.8 |
| 21 | 35 | 1.05 | 20 | 0.047 | 9.6 |
| 20 | 59 | 1.05 | 24 | 0.049 | 8.4 |
| 19 | 105 | 1.05 | 46 | 0.051 | 4.6 |
| 18 | 145 | 1.06 | 40 | 0.054 | 5.6 |
|  | Average | $12.6 \mathrm{in} / \mathrm{hr}$ |  |  |  |
|  | Safety Factor | $2$ |  |  |  |
|  | Design K | $6.32 \mathrm{in} / \mathrm{hr}$ |  |  |  |

## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole <br> Location: OTH 49 <br> Test Date: 21-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 32 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole <br> Location: OTH 50 <br> Test Date: 21-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 42 " below existing grade


## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole Location: OTH 51 (in C2) (remove C1) Test Date: 21-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 58 " below existing grade

| $\begin{gathered} \mathrm{H} \\ \text { (inches) } \end{gathered}$ | $\begin{gathered} \mathrm{T} \\ \text { (seconds) } \end{gathered}$ | $\begin{gathered} \mathrm{H}_{1} / \mathrm{H}_{2} \\ \text { (inches) } \end{gathered}$ | $\begin{gathered} \mathbf{t}_{2}-\mathrm{t}_{1} \\ \text { (seconds) } \end{gathered}$ | $\ln \left(\mathrm{H}_{1} / \mathrm{H}_{2}\right)$ | k (in/hr) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 24 | 0 | n/a | n/a |  |  |
| 23 | 45 | 1.04 | 45 | 0.043 | 3.9 |
| 22 | 85 | 1.05 | 40 | 0.044 | 4.6 |
| 21 | 150 | 1.05 | 65 | 0.047 | 2.9 |
| 20 | 240 | 1.05 | 90 | 0.049 | 2.2 |
| 19 | 375 | 1.05 | 135 | 0.051 | 1.6 |
| 18 | 510 | 1.06 | 135 | 0.054 | 1.6 |
|  | Average | $2.8 \mathrm{in} / \mathrm{hr}$ |  |  |  |
|  | Safety Factor |  |  |  |  |
|  | Design K | $1.40 \mathrm{in} / \mathrm{hr}$ |  |  |  |

## PERMEABILITY TESTING RESULTS

Falling Head Permeability Test

## Project: Summer Street, Walpole <br> Location: OTH 52 <br> Test Date: 21-Oct-20 <br> By: Daniel J. Merrikin, P.E.



Test apparatus
24 " long x 4 " diameter schedule 40 pvc pipe
Bottom of pipe set 42 " below existing grade


## Appendix B: Mounding Analysis

Groundwater Mounding Analysis (Hantush's Method using Glover's Solution)


|  |  |  |
| :--- | :--- | :--- |
| COMPANY: Howard Stein Hudson | MODEL RESULTS |  |

Groundwater Mounding Analysis (Hantush's Method using Glover's Solution)


|  | MODEL RESULTS |  |
| :---: | :---: | :---: |
| COMPANY: Howard Stein Hudson |  |  |
|  |  | Mound |
| PROJECT: Infiltration Pond \#2 | Time | Height |
| ANALYST: Matthew Baker |  |  |
|  | 0 | 0 |
| DATE: 6/12/2023 TIME: 1:04:22 PM | 0 | 0.1 |
|  | 0 | 0.3 |
| INPUT PARAMETERS | 0.1 | 0.53 |
|  | 0.2 | 0.73 |
| Application rate: $3.13 \mathrm{c.ft} / \mathrm{day} / \mathrm{sq}$. ft | 0.2 | 0.92 |
| Duration of application: 1 day | 0.3 | 1.1 |
| Total simulation time: 1 day | 0.4 | 1.28 |
| Fillable porosity: 0.4 | 0.5 | 1.48 |
| Hydraulic conductivity: $90 \mathrm{ft} / \mathrm{day}$ | 0.7 | 1.69 |
| Initial saturated thickness: 20 ft | 1 | 1.97 |

Groundwater Mounding Analysis (Hantush's Method using Glover's Solution)


|  |  |  |
| :--- | :--- | :--- |
| COMPANY: Howard Stein Hudson |  |  |
| MODEL RESULTS |  |  |

Groundwater Mounding Analysis (Hantush's Method using Glover's Solution)


| COMPANY: Howard Stein Hudson | MODEL RESULTS |  |
| :---: | :---: | :---: |
|  |  |  |
|  |  | Mound |
| PROJECT: Stormtech \#1 | Time (day) | Height <br> (ft) |
| ANALYST: Matthew Baker |  |  |
|  | 0 | 0 |
| DATE: 6/7/2023 TIME: 12:17:27 PM | 0 | 0.03 |
|  | 0 | 0.11 |
| INPUT PARAMETERS | 0.1 | 0.24 |
|  | 0.2 | 0.37 |
| Application rate: $0.99 \mathrm{c.ft} / \mathrm{day} / \mathrm{sq}$. ft | 0.2 | 0.52 |
| Duration of application: 1 day | 0.3 | 0.67 |
| Total simulation time: 1 day | 0.4 | 0.84 |
| Fillable porosity: 0.4 | 0.5 | 1.02 |
| Hydraulic conductivity: $12 \mathrm{ft} / \mathrm{day}$ | 0.7 | 1.23 |
| Initial saturated thickness: 35 ft | 1 | 1.52 |

Groundwater Mounding Analysis (Hantush's Method using Glover's Solution)


|  | MODEL RESULTS |  |
| :---: | :---: | :---: |
| COMPANY: Howard Stein Hudson | Mound |  |
|  |  |  |
| PROJECT: Stormtech \#2 | Time | Height |
| ANALYST: Matthew Baker |  |  |
|  | 0 | 0 |
| DATE: 6/7/2023 TIME: 12:16:03 PM | 0 | 0.12 |
|  | 0 | 0.37 |
| INPUT PARAMETERS | 0.1 | 0.63 |
|  | 0.2 | 0.83 |
| Application rate: $3.86 \mathrm{c.ft/day/sq}$. ft | 0.2 | 0.99 |
| Duration of application: 1 day | 0.3 | 1.14 |
| Total simulation time: 1 day | 0.4 | 1.28 |
| Fillable porosity: 0.4 | 0.5 | 1.42 |
| Hydraulic conductivity: 70 ft /day | 0.7 | 1.57 |
| Initial saturated thickness: 35 ft | 1 | 1.76 |

Groundwater Mounding Analysis (Hantush's Method using Glover's Solution)


|  | MODEL RESULTS |  |
| :---: | :---: | :---: |
| COMPANY: Howard Stein Hudson |  |  |
|  |  | Mound |
| PROJECT: Stormtech \#3 | Time | Height |
| ANALYST: Matthew Baker |  |  |
|  | 0 | 0 |
| DATE: 6/7/2023 TIME: 12:21:05 PM | 0 | 0.15 |
|  | 0 | 0.41 |
| INPUT PARAMETERS | 0.1 | 0.64 |
|  | 0.2 | 0.8 |
| Application rate: $4.61 \mathrm{c.ft} / \mathrm{day} / \mathrm{sq}$. ft | 0.2 | 0.94 |
| Duration of application: 1 day | 0.3 | 1.05 |
| Total simulation time: 1 day | 0.4 | 1.16 |
| Fillable porosity: 0.4 | 0.5 | 1.27 |
| Hydraulic conductivity: $85 \mathrm{ft} /$ day | 0.7 | 1.39 |
| Initial saturated thickness: 35 ft | 1 | 1.53 |

Groundwater Mounding Analysis (Hantush's Method using Glover's Solution)


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| :--- | :--- | :--- |
| COMPANY: Howard Stein Hudson | MODEL RESULTS |  |

## Appendix C: Operation and Maintenance Plan

# Operation and Maintenance Plan and Long-Term Pollution Prevention Plan 

## 51-53-55 Summer Street

Walpole, Massachusetts

Prepared by:
Howard Stein Hudson
114 Turnpike Road, Suite 2C
Chelmsford, MA 01824

# OPERATIONS AND MAINTENENACE PLAN 

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## Operation and Maintenance Plan

## Deep Sump Hooded Catch Basins

System Owner: Fairfield Residential (until ownership is transferred) Robert Hewitt rhewitt@ffres.com<br>Estimated Annual Maintenance: \$10,200.00-\$15,300.00<br>(Per DEP Stormwater Structural BMP's Vol 2)

Inspect or clean deep sump basins at least four times per year and at the end of the foliage and snow removal seasons. Sediments must also be removed four times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin. If handling runoff from land uses with higher potential pollutant loads or discharging runoff near or to a critical area, more frequent cleaning may be necessary. Clamshell buckets are typically used to remove sediment in Massachusetts. However, vacuum trucks are preferable because they remove more trapped sediment and supernatant than clamshells. Vacuuming is also a speedier process and is less likely to snap the cast iron hood within the deep sump catch basin.

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*Evidence of maintenance (i.e. receipts) must be provided.

## Contech Water Quality Unit - STC 450i

System Owner: Fairfield Residential (until ownership is transferred)
Robert Hewitt
rhewitt@ffres.com
Estimated Annual Maintenance: \$200.00-\$300.00
(Per Stormceptor Inspection and Maintenance Guide)
Inspect or clean pos-construction prior to being put in service. The unit is to be cleaned by a vacuum truck. The unit should be cleaned once the sediment depth reaches $15 \%$ of the storage capacity, or when about 8 inches of sediment has been accumulated. Additional information regarding the maintenance of the unit can be found within the attached product Maintenance Manual.

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*Evidence of maintenance (i.e. receipts) must be provided.

June 20, 2023

## Subsurface Infiltration System

System Owner: Fairfield Residential (until ownership is transferred)
Robert Hewitt
rhewitt@ffres.com
Estimated Annual Maintenance: \$200.00-\$300.00
(Per DEP Stormwater Structural BMP's Vol 2)
For the first 3 months after construction, the subsurface infiltration system should be inspected after every storm greater than 1 " for standing water for periods more than 72 hours. Therein after, the subsurface infiltration system should be inspected biannually. If standing water is observed for longer than 72 hours, a pump should be placed in the basin and discharged through the outlet pipe. After the system is dewatered, it should be observed by a Professional Engineer. A Professional Engineer should provide an opinion as to why the infiltration system is not draining and provide recommendations to restore infiltration capacity to the system.

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*Evidence of maintenance (i.e. receipts) must be provided.

## Isolator Row

System Owner: Fairfield Residential (until ownership is transferred)
Robert Hewitt
rhewitt@ffres.com
Estimated Annual Maintenance: \$600.00-\$900.00
(Per DEP Stormwater Structural BMP's Vol 2)
In the first year of operation, the Isolator Row should be inspected every 6 months for depth of sediment. Therein after, the Isolator Row should be inspected annually. If sediment is present, a stadia rod should be inserted into the inspection port to determine depth of sediment. If/when the depth exceeds 3 inches throughout the length of the Isolator Row, clean out should be performed. Please see the Isolator Row Maintenance Manual for cleanout procedures.

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[^0]June 20, 2023

## Sediment Forebay

System Owner: Fairfield Residential (until ownership is transferred) Robert Hewitt<br>rhewitt@ffres.com<br>Estimated Annual Maintenance: \$1,200.00<br>(Per DEP Stormwater Structural BMP's Vol 2)

In many cases, a landscaping contractor working elsewhere on the site can complete maintenance tasks. Stabilize the floor and sidewalls of the sediment forebay before making it operational, otherwise the practice will discharge excess amounts of suspended sediments.

Inspect and clean out the sediment forebay to assure that sediments and associated pollutants are cleaned out. Frequently removing accumulated sediments will make it less likely that sediments will be resuspended. At a minimum, inspect the sediment forebays monthly and clean them out at least four times a year.

Mow the grass areas and keep the grass height no greater than 6 inches. Check for signs of rilling and gullying and repair as needed. After removing the sediment, replace any vegetation damaged during the clean-out by either reseeding or resodding. When reseeding, incorporate practices such as hydroseeding with a tackifier, blanket, or similar practice to ensure that no scour occurs in the forebay, while the seeds germinate and develop roots.
*Paying careful attention to the pretreatment and operation and maintenance can extend the life of the soil media.

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*Evidence of maintenance (ie. receipts) must be provided.

## Infiltration Ponds

## System Owner: Fairfield Residential (until ownership is transferred) Robert Hewitt <br> rhewitt@ffres.com <br> Estimated Annual Maintenance: $\mathbf{\$ 5 8 0 . 0 0}$ <br> (Per DEP Stormwater Structural BMP's Vol 2)

In many cases, a landscaping contractor working elsewhere on the site can complete maintenance tasks. Inspect the basin and outlet structure to ensure no structural damage has occurred and that they are functioning properly and up to design standards.

Inspection and preventive maintenance are required at least twice per year, and after each major storm event. Note how long water remains standing in the basin after a storm. If water remains standing after 48 to 72 hours after a storm, the infiltration basin may be clogged.

At least twice per year, mow the buffer area, side slopes, and basin bottom. Remove grass clippings, accumulated organic matter, trash and debris at this time.

Remove sediment from the basin as necessary when the basin is dry. Use light equipment when removing the top layer, as to not compact the underlying soil. Use deep tilling to break and remove any clogged surfaces and revegetate immediately.

Important items to check during inspections include:

- Signs of differential settlement
- Cracking
- Erosion
- Leakage in the embankments
- Tree growth on the embankments
- Condition of rip rap
- Sediment accumulation
- Health of vegetation, turf
*Paying careful attention to the pretreatment and operation and maintenance can extend the life of the soil media.

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*Evidence of maintenance (ie. receipts) must be provided.

## Pocket Wetland

System Owner: Fairfield Residential (until ownership is transferred) Robert Hewitt<br>rhewitt@ffres.com<br>Estimated Annual Maintenance: $\mathbf{\$ 5 8 0 . 0 0}$<br>(Per DEP Stormwater Structural BMP's Vol 2)

The constructed stormwater wetland must be observed over time. In the first 3 years after construction, inspect the constructed stormwater wetland twice a year during both the growing and non-growing seasons. Following the maturation of the plant community within the treatment wetland(s), in addition to annual forebay inspection and clean-outs, the plant communities will be assessed to verify that the desired species composition is retained, and that woody or pernicious species do not become substantially established. In the event that undesirable plant specimens become established these will be removed either by hand pulling, puller-bear extraction, or by targeted herbicide application by a Licensed Applicator. A brief, annual photolog (1-2 photos per community type from consistent view positions) will provide documentation under the O\&M plan. During these inspections, record and map the following information:

- The types and distribution of the dominant wetland plants in the marsh;
- The presence and distribution of planted wetland species;
- The presence and distribution of invasive wetland species (invasive species must be removed);
- Indications that other species are replacing the planted wetland species;
- Percentages of standing water that is unvegetated (excluding the deep-water cells which are not suitable for emerging plant growth);
- The maximum elevation and the vegetative condition in this zone if the design elevation of the normal pool is being maintained for wetlands with extended zones;
- Stability of the original depth zones and the micro-topographic features; and
- Accumulation of sediment in the forebay and micro pool; and survival rate of plants (cells with dead plants must be replanted).

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*Evidence of maintenance (ie. receipts) must be provided.

## Outfalls and Riprap

System Owner: Fairfield Residential (until ownership is transferred)
Robert Hewitt
rhewitt@ffres.com
Estimated Annual Maintenance: \$750.00-\$1,000.00
(Per DEP Stormwater Structural BMP's Vol 2)
Inspections should be performed annually and after major storm events. If riprap has been damaged, repairs should be made promptly to prevent a progressive failure. Channel obstructions, such as trees and sediment bars, can change flow patterns and cause erosive forces which may damage riprap and the integrity of the outfall.

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*Evidence of maintenance (ie. receipts) must be provided.

MULTIFAMILY DEVELOPMENT - 51-53-55 SUMMER STREET
June 20, 2023

## Drip-edge System

System Owner: Fairfield Residential (until ownership is transferred)
Robert Hewitt
rhewitt@ffres.com
Estimated Annual Maintenance: \$2,500.00-\$3,000.00
(Per DEP Stormwater Structural BMP's Vol 2)
Inspect the infiltration trench after the first several rainfall events, after all major storms, and on regularly scheduled dates every six months. Inspect the trench 24 hours or several days after a rain event, to look for ponded water. If there is ponded water at the surface of the trench, it is likely that the trench surface is clogged. To address surface clogging, remove and replace the topsoil or first layer of stone aggregate and the filter fabric. If water is ponded inside the trench, it may indicate that the bottom of the trench has failed. To rehabilitate a failed trench, all accumulated sediment must be stripped from the bottom, the bottom of the trench must be scarified and tilled to induce infiltration, and all of the stone aggregate and filter fabric or media must be removed and replaced.

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*Evidence of maintenance (ie. receipts) must be provided.

## Erosion and Sediment Control Notes

A. Erosion and sediment control measures must be installed prior to the start of construction and maintained and upgraded as necessary during construction by the contractor. It is the contractor's responsibility to inspect and install additional control measures as needed during construction.
B. All catch basins receiving drainage from the project site must be provided with a catch basin filter.
C. Stabilization of all re-graded and soil stockpile areas must be maintained during all phases of construction.
D. Sediment removed from erosion and sediment control devices must be properly removed and disposed. All damaged controls must be removed and replaced.
E. The contractor is responsible for implementing the erosion and sediment control plan which will be submitted as part of the SWPPP. This includes the installation and maintenance of control measures, informing all parties engaged on the construction site of the requirements and objectives of the plant, and notifying the proper city agency of any transfer of this responsibility.
F. The contractor shall be responsible for controlling wide erosion and dust throughout the life of his contract. Dust control may include, but is not limited to, sprinkling of water on exposed soils and street sweeping adjacent roadways.
G. If final grading is to be delayed for more than 21 days after land disturbance activates cease, temporary vegetation or mulch shall be used to stabilize soils within 14 days of the last disturbance.
H. If a disturbed area will be exposed for greater than one year, permanent grasses or other approved cover must be installed.
I. The contractor must keep on-site at all times additional silt fence and straw wattle for the installation at the direction of the engineer or the city to mitigate any emergency condition.
J. The constriction fencing and erosion and sediment controls as shown may not be practical during all stages of construction. Earthwork activity on-site must be done in a manner such that runoff is directed to a sediment control device or infiltrated to the ground.
K. Demolition and construction debris must be properly contained and disposed of.
L. Disposal of all demolished materials is the responsibility of the contractor and must be hauled off-site in accordance with all federal, state, and local requirements.

## General Construction Sequence

- Construction sequence to comply with the submitted and approved SWPPP.


## Construction

A seven-step process to prepare a wetland bed prior to planting (Shueler 1992):

1. Prepare final pond-scaping and grading plans for the constructed stormwater wetland. At the same time, order wetland plant stocks from aquatic nurseries.
2. Once the constructed stormwater wetland volume has been excavated, grade the wetland to create the major internal features (pool, aquatic bench, deep water channels, etc.).
3. Because deep subsoils often lack the nutrients and organic matter needed to support vigorous plant growth, add topsoil and/or wetland mulch to the wetland excavation. If available, wetland mulch is preferable to topsoil.
4. After the mulch or topsoil has been added, grade the constructed stormwater wetland to its final elevations. Temporarily stabilize all wetland features above the normal pool. After final grading, close the pool drain to allow the pool to fill. MassDEP recommends evaluating the wetland elevations during a standing period of approximately six months to assess how the constructed stormwater wetland responds to storm flows and inundation, where the pond-scaping zones are located, and whether the final grade and micro-topography will persist over time.
5. Before planting, measure the constructed stormwater wetland depths to the nearest inch to confirm planting depth. If necessary, modify the pond-scape plan at this time to reflect altered depths or availability of plant stock.
6. Aggressively apply erosion controls during the standing and planting periods. Stabilize the vegetation in all areas above the normal pool elevation during the standing period (typically by hydroseeding).
7. Dewater the constructed stormwater wetland at least three days before planting, because a dry wetland is easier to plant than a wet one.

## Invasive Vegetation Control

## A. Wetland Replication Areas

1. Undesirable exotic vegetation, including all species from the Massachusetts Department of Agriculture, "Massachusetts Prohibited Plant List" (MA DAR, 2017) will be removed from areas where compensatory wetland replication will occur, including the adjacent upland work areas if applicable. Throughout the anticipated two (2) growing season monitoring period, undesirable plants will be removed by hand-pulling or mechanical means if necessary. Non-invasive wetland plants that are not desirable to the replication plan may also be controlled manually during the two seasons of maturation for the replication areas.

## B. Upland Project Area Buffer Zone Invasive Species Control Plan

1. The most pernicious invasions of exotic vegetation currently occur within areas of former pasture within uplands that lie within the project area. These materials will be removed during site preparation and either chipped and composted and disposed of offsite or taken off site to a processing facility.
2. Undesirable exotic vegetation will be removed from areas of upland Buffer Zone within the project development areas (buildings, roads and drainage, landscaped areas). In general lawns and public areas with landscaping will be kept free of undesirable vegetation during normal landscape maintenance. Particular attention will be paid to areas not associated with private dwellings to assure that seed sources do not develop within areas beyond exclusive use zones. No management of exotic vegetation will be conducted within areas left in their natural state.

## C. Invasive Plant Species Maintenance Plan

1. Throughout the areas improved beyond exclusive use and public use areas (lawns, gardens, planting islands, sidewalks) annual mowing will be performed, after August 1, for the purpose of controlling invasive plant species, both woody and herbaceous.
2. Areas beyond typical public view or visitation are particularly susceptible to overgrowth by undesirable species; examples of these can be found at:
http://www.massnrc.org/MIPAG/invasive.htm. These species can generally be controlled or eliminated by regular mowing and destruction of propagules prior to ripening. The areas of greatest concern are low/no activity areas such as detention basin berms, and
areas cleared and graded for project construction, but not part of normal habitation and use activities. Example areas include:

### 2.1 East of Basin \#1

2.2 East of Building \#1 and parking
2.3 North and East of Basin
2.4 North of Maintenance Facilities
3. Regular, annual mowing of these areas will discourage establishment and propagation of undesirable plant species. In the event that species on the MIPAG list (or other current recognized lists) become established despite mowing, conventional measures will be employed to eradicate these from herbaceous communities functioning to stabilize areas disturbed in the course of project construction. Hand, or mechanical pulling, or licensed herbicide application, as appropriate will be used to minimize advancement of undesirable plant species in "back areas" not typically used for recreation or enjoyment by the residents.

## Long-Term Pollution Prevention Plan

This Long-Term Pollution Prevention Plan is prepared to comply with the provisions set forth in the Massachusetts Department of Environmental Protection (DEP) Stormwater Management Standards. Structural Best Management Practices (BMP's) require periodic maintenance to ensure proper function and efficiency in pollutant removal from stormwater discharges that would otherwise reach wetland resource areas untreated.

Maintenance schedules found below are as recommended in Department of Environmental Protection's Massachusetts Stormwater Handbook and as recommended in manufacturer's specifications.

- Transfer of Ownership

After the project is completed the site ownership and maintenance responsibilities will be transferred to a from Fairfield Residential. The new owner will be responsible for the operation, maintenance, and inspection of all components of the onsite stormwater management system.

## Trash and Litter Cleanup

The owner (or maintainer) shall perform trash and litter cleanup once per month in and around the site. Trash and litter shall be disposed of in the on-site dumpsters during construction, and after construction shall be collected and properly disposed of.

## Paved Roadway

Impervious surfaces (pavement and sidewalks) shall be swept on an annual basis between April 1 and May 31. Raised sidewalks shall be swept by hand prior to any mechanical (rotary brush) sweeping. Sand and sediment deposits shall be collected from the site and disposed of by a licensed contractor, who must dispose of the material off-site in a manner consistent with all local, state, and federal regulations.

## Deep Sump Hooded Catch Basins

Catch Basins shall be inspected on a bi-annual basis. Any sediment accumulations in excess of half the unit's sump depth shall be removed. Material shall be removed by a licensed contractor, who shall be responsible for disposing of the material off-site in a manner consistent with all local, state, and federal regulations.

## Contech Water Quality Unit - STC 450i

Inspect or clean pos-construction prior to being put in service. The unit is to be cleaned by a vacuum truck. The unit should be cleaned once the sediment depth reaches $15 \%$ of the storage capacity, or when about 8 inches of sediment has been accumulated. Additional information regarding the maintenance of the unit can be found within the attached product Maintenance Manual.

## Landscape Maintenance of Maintained Landscape Areas

Landscaped surfaces in and around the proposed development in maintained areas shall be kept healthy and maintained. All lawn areas, as shown in maintained upland areas, shall be kept cut and watered to maintain groundcover. Clippings shall be contained and disposed of at an offsite location. Care should be taken as to not dump or dispose of any clippings and or plant material into the replication area and/or the Town of Walpole Conservation Areas. All landscaping on the site shall be installed so as to not require ongoing use of fertilizers and pesticides. Fertilizers containing phosphorus shall not be used on the property. All landscape waste accumulated in the course of grounds maintenance, including grass clippings and leaves, shall be disposed of at a proper upland composting or disposal area, and shall not be disposed of in any wetland area or buffer zone without the approval of the conservation commission. Non-organic fertilizers and pesticides and landscape care chemicals within 100 feet of the wetlands are prohibited. Signs must be posted as referenced in condition \#49 of the Decision and dog curbing rules must be implemented (pick up and proper disposal) to further reduce nutrient loading within wetland resource areas.

Shrubs and trees shall be maintained and shall be replaced immediately if the plant has died.

## Snow Plowing and Disposal

All snow shall be plowed and stored within the areas designated on the O\&M and LTPPP Plan. Once these areas can no longer accept additional snow, all excess snow shall be removed from the site. Absolutely no deposition of snow shall enter directly into the wetlands or placed over any stormwater management facility. This shall be maintained in perpetuity.

## Subsurface Infiltration Systems

For the first 3 months after construction, the subsurface infiltration system should be inspected after every storm greater than 1 " for standing water for periods more than 72 hours. Therein after, the subsurface infiltration system should be inspected biannually. If standing water is observed for longer than 72 hours, a pump should be placed in the basin and discharged through the outlet pipe. After the system is dewatered, it should be observed by a Professional Engineer. A Professional

Engineer should provide an opinion as to why the infiltration system is not draining and provide recommendations to restore infiltration capacity to the system.

## Isolator Row

In the first year of operation, the Isolator Row should be inspected every 6 months for depth of sediment. Therein after, the Isolator Row should be inspected annually. If sediment is present, a stadia rod should be inserted into the inspection port to determine depth of sediment. If/when the depth exceeds 3 inches throughout the length of the Isolator Row, clean out should be performed. Please see the Isolator Row Maintenance Manual for cleanout procedures.

## Sediment Forebays

In many cases, a landscaping contractor working elsewhere on the site can complete maintenance tasks. Stabilize the floor and sidewalls of the sediment forebay before making it operational, otherwise the practice will discharge excess amounts of suspended sediments.

Inspect and clean out the sediment forebay to assure that sediments and associated pollutants are cleaned out. Frequently removing accumulated sediments will make it less likely that sediments will be resuspended. At a minimum, inspect the sediment forebays monthly and clean them out at least four times a year.

Mow the grass areas and keep the grass height no greater than 6 inches. Check for signs of rilling and gullying and repair as needed. After removing the sediment, replace any vegetation damaged during the clean-out by either reseeding or resodding. When reseeding, incorporate practices such as hydroseeding with a tackifier, blanket, or similar practice to ensure that no scour occurs in the forebay while the seeds germinate and develop roots.

## Infiltration Pond

In many cases, a landscaping contractor working elsewhere on the site can complete maintenance tasks. Inspect the basin and outlet structure to ensure no damage has occurred and that they are functioning properly and up to design standards.

Inspection and preventive maintenance is required at least twice per year, and after each major storm event. Note how long water remains standing in the basin after a storm. If water remains standing after 48 to 72 hours after a storm, the infiltration basin may be clogged.

At least twice per year, mow the buffer area, side slopes, and basin bottom. Remove grass clippings, accumulated organic matter, trash and debris at this time.

Remove sediment from the basin as necessary when the basin is dry. Use light equipment when removing the top layer, as not to compact the underlying soil. Use deep tilling to break and remove any clogged surfaces and revegetate immediately.

Important items to check during inspections include:

- Signs of differential settlement
- Cracking
- Erosion
- Leakage in the embankments
- Condition of rip rap
- Sediment accumulation
- Health of vegetation, turf


## Pocket Wetlands

Unlike conventional wet basin systems that require large-scale sediment removal at infrequent intervals, constructed stormwater wetlands require small-scale maintenance at regular intervals to evaluate the health and composition of the plant species.

Proponents must carefully observe the constructed stormwater wetland system over time. In the first three years after construction, inspect the constructed stormwater wetlands twice a year during both the growing and non-growing seasons. The sediment forebays should be inspected and cleaned once a year. This requirement must be included in the Operations and Maintenance plan. During these inspections, record and map the following information:

- The types and distribution of the dominant wetland plants in the marsh
- The presence and distribution of planted wetland species
- The presence and distribution of invasive wetland species (invasives must be removed)
- Indications that other species are replacing the planted wetland species
- Percentage of standing water that is unvegetated (excluding the deep water cells which are not suitable for emergent plant growth)
- The maximum elevation and the vegetative condition in this zone, if the design elevation of the normal pool is being maintained for wetlands with extended zones
- Stability of the original depth zones and the micro-topographical features
- Accumulation of sediment in the forebay and micropool; and survival rate of plants (cells with dead plants must be replanted)


## Outfalls and Riprap

Inspections should be performed annually and after major storm events. If riprap has been damaged, repairs should be made promptly to prevent a progressive failure. Channel obstructions, such as
trees and sediment bars, can change flow patterns and cause erosive forces which may damage riprap and the integrity of the outfall.

## Drip-edge System

Inspect the infiltration trench after the first several rainfall events, after all major storms, and on regularly scheduled dates every six months. Inspect the trench 24 hours or several days after a rain event, to look for ponded water. If there is ponded water at the surface of the trench, it is likely that the trench surface is clogged. To address surface clogging, remove and replace the topsoil or first layer of stone aggregate and the filter fabric. If water is ponded inside the trench, it may indicate that the bottom of the trench has failed. To rehabilitate a failed trench, all accumulated sediment must be stripped from the bottom, the bottom of the trench must be scarified and tilled to induce infiltration, and all of the stone aggregate and filter fabric or media must be removed and replaced.

## DEP Standard 4: Water Quality

The Long-Term Pollution Prevention Plan Includes the following:

## Good housekeeping practices:

Prevent or reduce pollutant runoff from reaching the wetland resource areas through street sweeping, stabilizing all disturbed areas with vegetative cover and catch basin cleaning.

## Provisions for storing materials and waste products inside or under cover:

All materials on site are to be stored in a neat and orderly fashion in their appropriate containers and, if possible, under a roof or other secure enclosure. All waste products are to be placed in secure receptacles until they are emptied by a solid waste management company licensed in the commonwealth of Massachusetts.

## Vehicle washing controls:

Vehicle washing will occur on-site as part of standard operations. All contaminants / hazardous waste shall be disposed of in a manner specified by local or state regulations or by the manufacturer. provide an effective means of minimizing the discharge of pollutants from equipment and vehicle washing, wheel wash water, and other types of wash waters. Ensure there is no discharge of soaps, solvents, or detergents in equipment and vehicle wash water; and for storage of soaps, detergents, or solvents, provide either (1) cover (e.g., plastic sheeting, temporary roofs) to minimize the exposure of these detergents to precipitation and to stormwater, or (2) a similarly effective means designed to minimize the discharge of pollutants from these areas. The homeowners shall be instructed in these practices.

## Requirements for routine inspections and maintenance of Stormwater BMP's:

Follow the procedures outlined within the Operations and Maintenance Section of this report.

## Spill prevention and response plan:

Spill Prevention: As mentioned previously, all materials on site are to be stored in a neat and orderly fashion in their appropriate containers and, if possible, under a roof or other secure enclosure. Products shall be kept in their original containers with the original manufacturer's label. Product should not be mixed unless recommended by the manufacturer. The manufacturer's recommendations for proper use, storage and disposal shall be followed at all times and, if possible, all of the product should be used up before proper disposal.

The manufacturer's recommended methods for cleanup must be followed and spills cleaned up immediately after discovery. Spills shall be kept well ventilated and personnel must wear appropriate protective gear to prevent injury from contact with hazardous substances. Spills of toxic or hazardous materials must be reported to the appropriate local and/ or state agency in accordance with the local and/ or Commonwealth of Massachusetts regulations.

## Requirements for storage and use of fertilizers, herbicides and pesticides:

Consult the town of Chelmsford, MA Conservation Commission for any questions regarding these materials.

## Fertilizers:

Fertilizers are to be applied at the minimum amounts recommended by the manufacturer and once applied shall be worked into the soil to limit the possibility of entering the storm drains. Storage procedures are to be followed as previously stated and the contents of any partially used bags should be transferred to a sealable container, either bag or bin to avoid spilling.
Herbicides and Pesticides: Storage of these materials are to be as outlined previously and especially out of the reach of pets and children, away from damp areas where their containers may succumb to moisture or rust and should not be stored near food. These materials must not be placed in the trash or washed down the drain. Handle using rubber gloves and use an appropriate mask when using these products for extensive periods of time.

## Provisions for maintenance of lawns, gardens, and other landscaped areas:

lawns gardens and other landscape areas are to be maintained in a manner that the ground remains stabilized. All dead plants shall be replaced in a timely manner as to prevent erosion and sedimentation control within the resource areas buffer zones.

## Provisions for solid waste management:

All waste products are to be placed in secure receptacles until they are emptied by a solid waste management company licensed in the Commonwealth of Massachusetts.

## Snow disposal and plowing plans relative to Wetland Resource Areas:

Snow disposal/removal shall refer to the locations as depicted on the O\&M and LTPPP Plan attached to this document.

## Winter Road Salt and/or Sand Use and Storage restrictions:

Road Salt use must be in compliance with the Guidelines on Deicing Chemical (Road Salt) Storage effective date December 19, 1997, Guideline No. DWSG97-1 found in the BRP's Drinking Water Program. Sand Use: Encourage the use of environmentally friendly alternatives such as calcium chloride and/or sand instead of road salt for melting ice whenever possible. Use of de-icing agents should be tightly restricted to those absolutely necessary for public safety in consideration of associated vegetated wetlands. Environmentally friendly salt alternatives shall be used for de-icing operations.

## Provisions for prevention of illicit discharges to the stormwater management systems:

 According to Standard 10 in the Massachusetts Stormwater Handbook, Illicit discharges to the stormwater management system are discharges that are not entirely comprised of stormwater. Notwithstanding the foregoing, an illicit discharge does not include discharges from the following activities or facilities: firefighting, water line flushing, landscape irrigation, uncontaminated groundwater, potable water sources, foundation drains, air conditioning condensation, footing drains, individual resident car washing, flows from riparian habitats and wetlands, dechlorinated water from swimming pools, water used for street washing and water used to clean residential buildings without detergents.
## Training for staff or personnel involved with implementing LTPPP:

This responsibility lies with the owner(s) unless a legally-binding agreement is made with another party to perform such duties for the owner(s).

## List of Emergency contacts for implementing Long-Term Pollution Prevention Plan:

This responsibility lies with the owner(s) unless a legally-binding agreement is made with another party to perform such duties for the owner(s).

## Appendix A: Stormtech Construction Guide

## REQUIRED MATERIALS AND EQUIPMENT LIST

- StormTech solid end caps and pre-cored end caps
- StormTech chambers
- StormTech manifolds and fittings
- ADS Plus and non-woven geotextile fabrics


## IMPORTANT NOTES:

A. This installation guide provides the minimum requirements for proper installation of chambers. Non-adherence to this guide may result in damage to chambers during installation. Replacement of damaged chambers during or after backilling is costly and very time consuming. It is recommended that all installers are familiar with this guide, and that the contractor inspects the chambers for distortion, damage and joint integrity as work progresses.
B. Use of a dozer to push embedment stone between the rows of chambers may cause damage to chambers and is not an acceptable backfill method. Any chambers damaged by using the "dump and push" method are not covered under the StormTech standard warranty.
C. Care should be taken in the handling of chambers and end caps. Avoid dropping, prying or excessive force on chambers during removal from pallet and initial placement.

## Requirements for System Installation



Excavate bed and prepare subgrade per engineer's plans.


Place non-woven geotextile over prepared soils and up excavation walls. Install underdrains if required.


Place clean, crushed, angular stone foundation $6 "(150 \mathrm{~mm}) \mathrm{min}$. Compact to achieve a flat surface.

## Manifold, Scour Fabric and Chamber Assembly



Install manifolds and lay out ADS PLUS fabric at inlet rows [min. $12.5 \mathrm{ft}(3.8 \mathrm{~m})$ ] at each inlet end cap. Place a continuous piece along entire length of Isolator ${ }^{\circledR}$ PLUS Row(s).


Align the first chamber and end cap of each row with inlet pipes. Contractor may choose to postpone stone placement around end chambers and leave ends of rows open for easy inspection of chambers during the backfill process.

## Prefabricated End Caps



24 " (600 mm) inlets are the maximum size that can fit into a SC-740/DC-780 end cap and must be prefabricated with a 24 " ( 600 mm ) pipe stub. SC-310 chambers with a 12 " ( 300 mm ) inlet pipe must use a prefabricated end cap with a 12" (300 mm) pipe stub. When used on an Isolator Row PLUS, these end caps will contain a welded FLAMP (flared end ramp) that will lay on top of the ADS PLUS fabric (shown above)


Continue installing chambers by overlapping chamber end corrugations. Chamber joints are labeled "Lower Joint - Overlap Here" and "Build this direction Upper Joint" Be sure that the chamber placement does not exceed the reach of the construction equipment used to place the stone. Maintain minimum 6" (150 mm) spacing between rows.

## Isolator Row PLUS



Place a continuous layer of ADS PLUS fabric between the foundation stone and the Isolator Row PLUS chambers, making sure the fabric lays flat and extends the entire width of the chamber feet. Drape a strip of ADS non-woven geotextile over the row of chambers (not required over DC-780). This is the same type of non-woven geotextile used as a separation layer around the angular stone of the StormTech system.

## Initial Anchoring of Chambers - Embedment Stone



Initial embedment shall be spotted along the centerline of the chamber evenly anchoring the lower portion of the chamber. This is best accomplished with a stone conveyor or excavator reaching along the row.


No equipment shall be operated on the bed at this stage of the installation Excavators must be located off the bed. Dump trucks shall not dump stone directly on to the bed. Dozers or loaders are not allowed on the bed at this time.

## Backfill of Chambers - Embedment Stone



UNEVEN BACKFILL


EVEN BACKFILL

Backfill chambers evenly. Stone column height should never differ by more than 12 " ( 300 mm ) between adjacent chamber rows or between chamber rows and perimeter.


PERIMETER NOT BACKFILLED

Perimeter stone must be brought up evenly with chamber rows. Perimeter must be fully backfilled, with stone extended horizontally to the excavation wall.

## Backfill - Embedment Stone \& Cover Stone



Continue evenly backfilling between rows and around perimeter until embedment stone reaches tops of chambers. Perimeter stone must extend horizontally to the excavation wall for both straight or sloped sidewalls. Only after chambers have been backfilled to top of chamber and with a minimum 6" ( 150 mm ) of cover stone on top of chambers can small dozers be used over the chambers for backfilling remaining cover stone.


Small dozers and skid loaders may be used to finish grading stone backfill in accordance with ground pressure limits in Table 2. They must push material parallel to rows only. Never push perpendicular to rows. StormTech recommends that the contractor inspect chambers before placing final backfill. Any chambers damaged by construction shall be removed and replaced.

Inserta Tee Detail


## Final Backfill of Chambers - Fill Material



Install non-woven geotextile over stone. Geotextile must overlap 24" (600 mm) min. where edges meet. Compact each lift of backfill as specified in the site design engineer's drawings. Roller travel parallel with rows.

## StormTech Isolator Row PLUS Detail



| Material Location | Description | AASHTO M43 Designation ${ }^{1}$ | Compaction/Density Requirement |
| :---: | :---: | :---: | :---: |
| (D) Final Fill: Fill Material for layer ' D ' starts from the top of the 'C' layer to the bottom of flexible pavement or unpaved finished grade above. Note that the pavement subbase may be part of the ' D ' layer. | Any soil/rock materials, native soils or per engineer's plans. Check plans for pavement subgrade requirements. | N/A | Prepare per site design engineer's plans. Paved installations may have stringent material and preparation requirements. |
| (C)Initial Fill: Fill Material for layer ' $C$ ' starts from the top of the embedment stone ('B' layer) to 18 " ( 450 mm ) above the top of the chamber. Note that pavement subbase may be part of the ' $C$ ' layer. | Granular well-graded soil/ aggregate mixtures, $<35 \%$ fines or processed aggregate. Most pavement subbase materials can be used in lieu of this layer. | $\begin{gathered} \text { AASHTO M45 } \\ \text { A-1, A-2-4, A-3 } \\ \text { or } \\ \text { AASHTO M431 } \\ 3,357,4,467,5,56,57,6, \\ 67,68,7,78,8,89,9,10 \end{gathered}$ | Begin compaction after min. 12 " ( 300 mm ) of material over the chambers is reached. Compact additional layers in 6 " ( 150 mm ) max. lifts to a min. 95\% Proctor density for well-graded material and $95 \%$ relative density for processed aggregate materials. Roller gross vehicle weight not to exceed $12,000 \mathrm{lbs}$ ( 53 $\mathrm{kN})$. Dynamic force not to exceed $20,000 \mathrm{lbs}(89 \mathrm{kN})$ |
| (B)Embedment Stone: Embedment Stone surrounding chambers from the foundation stone to the 'C' layer above. | Clean, crushed, angular stone | AASHTO M43 ${ }^{1}$ $3,357,4,467,5,56,57$ | No compaction required. |
| (A)Foundation Stone: Foundation Stone below the chambers from the subgrade up to the foot (bottom) of the chamber. | Clean, crushed, angular stone, | AASHTO M43 ${ }^{1}$ $3,357,4,467,5,56,57$ | Place and compact in 6 " $(150 \mathrm{~mm})$ lifts using two full coverages with a vibratory compactor. ${ }^{2,3}$ |

## PLEASE NOTE:

1. The listed AASHTO designations are for gradations only, The stone must also be clean, crushed, angular: For example, a specification for \#4 stone would state: "clean, crushed, angular no. 4 (AASHTO M43) stone".
2. StormTech compaction requirements are met for 'A' location materials when placed and compacted in 6" (150 mm) (max) lifts using two full coverages with a vibratory compactor.
3. Where infiltration surfaces may be comprised by compaction, for standard installations and standard design load conditions, a flat surface may be achieved by raking or dragging without compaction equipment. For special load designs, contact StormTech for compaction requirements.

Figure 1- Inspection Port Detail


NOTE:
INSPECTION PORTS MAY BE CONNECTED THROUGH ANY CHAMBER CORRUGATION CREST.

Figure 2 - Fill Material Locations


NOTES:

1. 36 " (900 mm) of stabilized cover materials over the chambers is required for full dump truck travel and dumping.
2. During paving operations, dump truck axle loads on 18" (450 mm) of cover may be necessary. Precautions should be taken to avoid rutting of the road base layer, to ensure that compaction requirements have been met, and that a minimum of 18" (450 mm ) of cover exists over the chambers. Contact StormTech for additional guidance on allowable axle loads during paving.
3. Ground pressure for track dozers is the vehicle operating weight divided by total ground contact area for both tracks. Excavators will exert higher ground pressures based on loaded bucket weight and boom extension.
4. Mini-excavators (<8,000lbs/3,628 kg) can be used with at least $12 "(300 \mathrm{~mm})$ of stone over the chambers and are limited by the maximum ground pressures in Table 2 based on a full bucket at maximum boom extension.
5. Storage of materials such as construction materials, equipment, spoils, etc. should not be located over the StormTech system. The use of equipment over the StormTech system not covered in Table 2 (ex. soil mixing equipment, cranes, etc) is limited. Please contact StormTech for more information.
6. Allowable track loads based on vehicle travel only. Excavators shall not operate on chamber beds until the total backfill reaches 3 feet ( 900 mm ) over the entire bed.

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Tech, Inc
\#11010 09/20 CS
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Table 2 - Maximum Allowable Construction Vehicle Loads ${ }^{5}$

| Material Location | Fill Depth over Chambers in. $[\mathrm{mm}]$ <br> in. [mm] | Maximum Allowable Wheel Loads |  | Maximum Allowable Track Loads ${ }^{6}$ |  | Maximum Allowable Roller LoadsMax Drum Weightor Dynamic Forcelos [kN] |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Max Axle Load for Trucks lbs [kN] | Max Wheel Load for Loaders lbs [kN] | Track Width in. [mm] | Max Ground Pressure psf [kPa] |  |
| (D) Final Fill Material | $\begin{aligned} & 36 "[900] \\ & \text { Compacted } \end{aligned}$ | 32,000 [142] | 16,000 [71] | $12 "$ " 305$]$ $18 "[457]$ $244^{"}[610]$ $30 "$ " 762$]$ $36 "[914]$ | $\begin{gathered} 3420[164] \\ 2350[113] \\ 1850[89] \\ 1510[72] \\ 1310[63] \end{gathered}$ | 38,000 [169] |
| (C) Initial Fill Material | $\begin{gathered} 24 "[600] \\ \text { Compacted } \end{gathered}$ | 32,000 [142] | 16,000 [71] | $12 "$ " 305$]$ $18 "[457]$ $244 "[610]$ $30 "$ " 762$]$ $36 "[914]$ | $\begin{gathered} 2480[119] \\ 1770[85] \\ 1430[68] \\ 1210[58] \\ 1070[51] \end{gathered}$ | 20,000 [89] |
|  | $\begin{gathered} 24 "[600] \\ \text { Loose/Dumped } \end{gathered}$ | 32,000 [142] | 16,000 [71] | $12 "$ " 305$]$ $18 "[457]$ $244 "[610]$ $30 "$ " 762$]$ $36 "[914]$ | $\begin{gathered} 2245[107] \\ 1625[78] \\ 1325[63] \\ 1135[54] \\ 1010[48] \end{gathered}$ | Roller gross vehicle weight not to exceed 12,000 lbs. [ 53 kN$]$ |
|  | 18" [450] | 32,000 [142] | 16,000 [71] | $12 "$ " 305$]$ $18 "[457]$ $24 "$ " 610$]$ $30 "$ " 762$]$ $36 "[914]$ | $\begin{gathered} 2010[96] \\ 1480[71] \\ 1220[58] \\ 1060[51] \\ 950[45] \end{gathered}$ | 20,000 [89] <br> Roller gross vehicle weight not to exceed 12,000 lbs. [ 53 kN ] |
| (B)Embedment Stone | 12 " 300$]$ | 16,000 [71] | NOT ALLOWED | $12 "$ " 305$]$ $18 "[457]$ $24 "$ " 610$]$ $30 "$ " 762$]$ $36 "[914]$ | $\begin{gathered} 1540[74] \\ 1190[57] \\ 1010[48] \\ 910[43] \\ 840[40] \end{gathered}$ | 20,000 [89] <br> Roller gross vehicle weight not to exceed 12,000 lbs. [ 53 kN ] |
|  | $6 "$ [150] | 8,000 [35] | NOT ALLOWED | $12 "$ " 305$]$ 18 " $[457]$ 244 " $[610]$ $30 "$ " 762$]$ 36 " | $\begin{gathered} 1070[51] \\ 900[43] \\ 800[38] \\ 760[36] \\ 720[34] \end{gathered}$ | NOT ALLOWED |

Table 3 - Placement Methods and Descriptions

| Material Location | Placement Methods/ Restrictions | Wheel Load Restrictions | Track Load Restrictions | Roller Load Restrictions |
| :---: | :---: | :---: | :---: | :---: |
|  |  | See Table 2 for Maximum Construction Loads |  |  |
| (D) Final Fill Material | A variety of placement methods may be used. All construction loads must not exceed the maximum limits in Table 2. | $36^{\prime \prime}(900 \mathrm{~mm})$ minimum cover required for dump trucks to dump over chambers. | Dozers to push parallel to rows until 36 " ( 900 mm ) compaced cover is reached. ${ }^{4}$ | Roller travel parallel to rows only until $36^{\prime \prime}$ ( 900 mm ) compacted cover is reached. |
| (C) Initial Fill Material | Excavator positioned off bed recommended. Small excavator allowed over chambers. Small dozer allowed. | Asphalt can be dumped into paver when compacted pavement subbase reaches $18 "(450 \mathrm{~mm})$ above top of chambers. | Small LGP track dozers \& skid loaders allowed to grade cover stone with at least 6 " $(150 \mathrm{~mm})$ stone under tracks at all times. Equipment must push parallel to rows at all times. | Use dynamic force of roller only after compacted fill depth reaches 12 " ( 300 mm ) over chambers. Roller travel parallel to chamber rows only. |
| (B) Embedment Stone | No equipment allowed on bare chambers. Use excavator or stone conveyor positioned off bed or on foundation stone to evenly fill around all chambers to at least the top of chambers. | No wheel loads allowed. Material must be placed outside the limits of the chamber bed. | No tracked equipment is allowed on chambers until a min. 6" ( 150 mm ) cover stone is in place. | No rollers allowed. |
| (A) Foundation Stone | № StormTech restrictions. Contractor responsible for any conditions or requirements by others relative to subgrade bearing capacity, dewatering or protection of subgrade. |  |  |  |

## Appendix B: Stormtech Isolator Row Operation and Maintenance Manual

## Isolator ${ }^{\oplus}$ Row 0\&M Manual



## THE ISOLATOR ${ }^{\circledR}$ ROW

## INTRODUCTION

An important component of any Stormwater Pollution Prevention Plan is inspection and maintenance. The StormTech Isolator Row is a technique to inexpensively enhance Total Suspended Solids (TSS) removal and provide easy access for inspection and maintenance.

## THE ISOLATOR ROW

The Isolator Row is a row of StormTech chambers, either SC-160LP, SC-310, SC-310-3, SC-740, DC-780, MC-3500 or MC-4500 models, that is surrounded with filter fabric and connected to a closely located manhole for easy access. The fabric-wrapped chambers provide for settling and filtration of sediment as storm water rises in the Isolator Row and ultimately passes through the filter fabric. The open bottom chambers and perforated sidewalls (SC-310, SC- 310-3 and SC-740 models) allow storm water to flow both vertically and horizontally out of the chambers. Sediments are captured in the Isolator Row protecting the storage areas of the adjacent stone and chambers from sediment accumulation.
Two different fabrics are used for the Isolator Row. A woven geotextile fabric is placed between the stone and the Isolator Row chambers. The tough geotextile provides a media for storm water filtration and provides a durable surface for maintenance operations. It is also designed to prevent scour of the underlying stone and remain intact during high pressure jetting. A non-woven fabric is placed over the chambers to provide a filter media for flows passing through the perforations in the sidewall of the chamber. The non-woven fabric is not required over the SC-160LP, DC-780, MC-3500 or MC-4500 models as these chambers do not have perforated side walls.
The Isolator Row is typically designed to capture the "first flush" and offers the versatility to be sized on a volume basis or flow rate basis. An upstream manhole not only provides access to the Isolator Row but typically includes a high flow weir such that storm water flowrates or volumes that exceed the capacity of the Isolator Row overtop the over flow weir and discharge through a manifold to the other chambers.
The Isolator Row may also be part of a treatment train. By treating storm water prior to entry into the chamber system, the service life can be extended and pollutants such as hydrocarbons can be captured. Pre-treatment best management practices can be as simple as deep sump catch basins, oil-water separators or can be innovative storm water treatment devices. The design of the treatment train and selection of pretreatment devices by the design engineer is often driven by regulatory requirements. Whether pretreatment is used or not, the Isolator Row is recommended by StormTech as an effective means to minimize maintenance requirements and maintenance costs.
Note: See the StormTech Design Manual for detailed information on designing inlets for a StormTech system, including the Isolator Row.


Looking down the Isolator Row from the manhole opening, woven geotextile is shown between the chamber and stone base.


StormTech Isolator Row with Overflow Spillway (not to scale)



## ISOLATOR ROW INSPECTION/MAINTENANCE

## INSPECTION

The frequency of inspection and maintenance varies by location. A routine inspection schedule needs to be established for each individual location based upon site specific variables. The type of land use (i.e. industrial, commercial, residential), anticipated pollutant load, percent imperviousness, climate, etc. all play a critical role in determining the actual frequency of inspection and maintenance practices.
At a minimum, StormTech recommends annual inspections. Initially, the Isolator Row should be inspected every 6 months for the first year of operation. For subsequent years, the inspection should be adjusted based upon previous observation of sediment deposition.
The Isolator Row incorporates a combination of standard manhole(s) and strategically located inspection ports (as needed). The inspection ports allow for easy access to the system from the surface, eliminating the need to perform a confined space entry for inspection purposes.
If upon visual inspection it is found that sediment has accumulated, a stadia rod should be inserted to determine the depth of sediment. When the average depth of sediment exceeds 3 inches throughout the length of the Isolator Row, clean-out should be performed.

## MAINTENANCE

The Isolator Row was designed to reduce the cost of periodic maintenance. By "isolating" sediments to just one row, costs are dramatically reduced by eliminating the need to clean out each row of the entire storage bed. If inspection indicates the potential need for maintenance, access is provided via a manhole(s) located on the end(s) of the row for cleanout. If entry into the manhole is required, please follow local and OSHA rules for a confined space entries.
Maintenance is accomplished with the JetVac process. The JetVac process utilizes a high pressure water nozzle to propel itself down the Isolator Row while scouring and suspending sediments. As the nozzle is retrieved, the captured pollutants are flushed back into the manhole for vacuuming. Most sewer and pipe maintenance companies have vacuum/JetVac combination vehicles. Selection of an appropriate JetVac nozzle will improve maintenance efficiency. Fixed nozzles designed for culverts or large diameter pipe cleaning are preferable. Rear facing jets with an effective spread of at least 45 " are best. Most JetVac reels have 400 feet of hose allowing maintenance of an Isolator Row up to 50 chambers long. The JetVac process shall only be performed on StormTech Isolator Rows that have AASHTO class 1 woven geotextile (as specified by StormTech) over their angular base stone.

## StormTech Isolator Row (not to scale)

Note: Non-woven fabric is only required over the inlet pipe connection into the end cap for SC-160LP, DC-780, MC-3500 and MC-4500 chamber models and is not required over the entire Isolator Row.


## ISOLATOR ROW STEP BY STEP MAINTENANCE PROCEDURES

## STEP 1

Inspect Isolator Row for sediment.
A) Inspection ports (if present)
i. Remove lid from floor box frame
ii. Remove cap from inspection riser
iii. Using a flashlight and stadia rod,measure depth of sediment and record results on maintenance log.
iv. If sediment is at or above 3 inch depth, proceed to Step 2. If not, proceed to Step 3.
B) All Isolator Rows
i. Remove cover from manhole at upstream end of Isolator Row
ii. Using a flashlight, inspect down Isolator Row through outlet pipe

1. Mirrors on poles or cameras may be used to avoid a confined space entry
2. Follow OSHA regulations for confined space entry if entering manhole
iii. If sediment is at or above the lower row of sidewall holes (approximately 3 inches), proceed to Step 2. If not, proceed to Step 3.

STEP 2
Clean out Isolator Row using the JetVac process.
A) A fixed floor cleaning nozzle with rear facing nozzle spread of 45 inches or more is preferable
B) Apply multiple passes of JetVac until backflush water is clean
C) Vacuum manhole sump as required

## STEP 3

Replace all caps, lids and covers, record observations and actions.

## STEP 4

Inspect \& clean catch basins and manholes upstream of the StormTech system.


SAMPLE MAINTENANCE LOG

| Date | Stadia Rod Readings |  | Sediment Depth$(1)-(2)$ | Observations/Actions | Inspector |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Fixed point to chamber bottom (1) | Fixed point to top of sediment (2) |  |  |  |
| 3/15/11 | 6.3 ft | none |  | New installation. Fixed point is CI frame at grade | $D J M$ |
| 9/24/11 |  | 6.2 | 0.1 ft | Some grit felt | SM |
| 6/20/13 |  | 5.8 | 0.5 ft | Mucky feel, debris visible in manhole and in Isolator Row, maintenance due | NV |
| 7/7/13 | 6.3 ft |  | $\bigcirc$ | System jetted and vacuumed | DJM |

## Appendix C: Stormceptor STC Operation and Maintenance Guide

## Stormceptor ${ }^{\circledR}$ STC Operation and Maintenance Guide



## Stormceptor Design Notes

- Only the STC 450i is adaptable to function with a catch basin inlet and/or inline pipes.
- Only the Stormceptor models STC 450i to STC 7200 may accommodate multiple inlet pipes.


## Inlet and outlet invert elevation differences are as follows:

| Inlet and Outlet Pipe Invert Elevations Differences |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inlet Pipe Configuration | STC 450i | STC 900 to STC 7200 | STC 11000 to STC 16000 |  |
| Single inlet pipe | $3 \mathrm{in} .(75 \mathrm{~mm})$ | $1 \mathrm{in} .(25 \mathrm{~mm})$ | $3 \mathrm{in} .(75 \mathrm{~mm})$ |  |
| Multiple inlet pipes | $3 \mathrm{in} .(75 \mathrm{~mm})$ | $3 \mathrm{in} .(75 \mathrm{~mm})$ | Only one inlet pipe. |  |

## Maximum inlet and outlet pipe diameters:

| Inlet/Outlet Configuration | Inlet Unit <br> STC 450 i | In-Line Unit <br> STC 900 to STC 7200 | Series* <br> STC 11000 to STC 16000 |
| :---: | :---: | :---: | :---: |
| Straight Through | 24 inch $(600 \mathrm{~mm})$ | 42 inch $(1050 \mathrm{~mm})$ | 60 inch $(1500 \mathrm{~mm})$ |
| Bend $(90$ degrees $)$ | 18 inch $(450 \mathrm{~mm})$ | 33 inch $(825 \mathrm{~mm})$ | 33 inch $(825 \mathrm{~mm})$ |

- The inlet and in-line Stormceptor units can accommodate turns to a maximum of 90 degrees.
- Minimum distance from top of grade to crown is 2 feet $(0.6 \mathrm{~m})$
- Submerged conditions. A unit is submerged when the standing water elevation at the proposed location of the Stormceptor unit is greater than the outlet invert elevation during zero flow conditions. In these cases, please contact your local Stormceptor representative and provide the following information:
- Top of grade elevation
- Stormceptor inlet and outlet pipe diameters and invert elevations
- Standing water elevation
- Stormceptor head loss, $K=1.3$ (for submerged condition, $K=4$ )


## Stormceptor: OPERATION AND MAINTENANCE GUIDE Table of Content

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## 1. About Stormceptor

The Stormceptor® STC (Standard Treatment Cell) was developed by Imbrium ${ }^{\top M}$ Systems to address the growing need to remove and isolate pollution from the storm drain system before it enters the environment. The Stormceptor STC targets hydrocarbons and total suspended solids (TSS) in stormwater runoff. It improves water quality by removing contaminants through the gravitational settling of fine sediments and floatation of hydrocarbons while preventing the re-suspension or scour of previously captured pollutants.

The development of the Stormceptor STC revolutionized stormwater treatment, and created an entirely new category of environmental technology. Protecting thousands of waterways around the world, the Stormceptor System has set the standard for effective stormwater treatment.

### 1.1. Patent Information

The Stormceptor technology is protected by the following patents:

- Australia Patent No. 693,164 • 693,164•707,133 • 729,096•779401
- Austrian Patent No. 289647
- Canadian Patent No 2,009,208 •2,137,942•2,175,277 • 2,180,305 • 2,180,383 • 2,206,338 • 2,327,768 (Pending)
- China Patent No 1168439
- Denmark DK 711879
- German DE 69534021
- Indonesian Patent No 16688
- Japan Patent No 9-11476 (Pending)
- Korea 10-2000-0026101 (Pending)
- Malaysia Patent No PI9701737 (Pending)
- New Zealand Patent No 314646
- United States Patent No 4,985,148 • 5,498,331 • 5,725,760 • 5,753,115 • 5,849,181 • 6,068,765 • 6,371,690
- Stormceptor OSR Patent Pending • Stormceptor LCS Patent Pending


## 2. Stormceptor Design Overview

### 2.1. Design Philosophy

The patented Stormceptor System has been designed to focus on the environmental objective of providing long-term pollution control. The unique and innovative Stormceptor design allows for continuous positive treatment of runoff during all rainfall events, while ensuring that all captured pollutants are retained within the system, even during intense storm events.

An integral part of the Stormceptor design is PCSWMM for Stormceptor - sizing software developed in conjunction with Computational Hydraulics Inc. (CHI) and internationally acclaimed expert, Dr. Bill James. Using local historical rainfall data and continuous simulation modeling, this software allows a Stormceptor unit to be designed for each individual site and the corresponding water quality objectives.

By using PCSWMM for Stormceptor, the Stormceptor System can be designed to remove a wide range of particles (typically from 20 to 2,000 microns), and can also be customized to remove a specific particle size distribution (PSD). The specified PSD should accurately reflect what is in the stormwater runoff to ensure the device is achieving the desired water quality objective. Since stormwater runoff contains small particles (less than 75 microns), it is important to design a treatment system to remove smaller particles in addition to coarse particles.

### 2.2. Benefits

The Stormceptor System removes free oil and suspended solids from stormwater, preventing spills and non-point source pollution from entering downstream lakes and rivers. The key benefits, capabilities and applications of the Stormceptor System are as follows:

- Provides continuous positive treatment during all rainfall events
- Can be designed to remove over $80 \%$ of the annual sediment load
- Removes a wide range of particles
- Can be designed to remove a specific particle size distribution (PSD)
- Captures free oil from stormwater
- Prevents scouring or re-suspension of trapped pollutants
- Pre-treatment to reduce maintenance costs for downstream treatment measures (ponds, swales, detention basins, filters)
- Groundwater recharge protection
- Spills capture and mitigation
- Simple to design and specify
- Designed to your local watershed conditions
- Small footprint to allow for easy retrofit installations
- Easy to maintain (vacuum truck)
- Multiple inlets can connect to a single unit
- Suitable as a bend structure
- Pre-engineered for traffic loading (minimum AASHTO HS-20)
- Minimal elevation drop between inlet and outlet pipes
- Small head loss
- Additional protection provided by an $18^{\prime \prime}(457 \mathrm{~mm})$ fiberglass skirt below the top of the insert, for the containment of hydrocarbons in the event of a spill.


### 2.3. Environmental Benefit

Freshwater resources are vital to the health and welfare of their surrounding communities. There is increasing public awareness, government regulations and corporate commitment to reducing the pollution entering our waterways. A major source of this pollution originates from stormwater runoff from urban areas. Rainfall runoff carries oils, sediment and other contaminants from roads and parking lots discharging directly into our streams, lakes and coastal waterways.

The Stormceptor System is designed to isolate contaminants from getting into the natural environment. The Stormceptor technology provides protection for the environment from spills that occur at service stations and vehicle accident sites, while also removing contaminated sediment in runoff that washes from roads and parking lots.

## 3. Key Operation Features

### 3.1. Scour Prevention

A key feature of the Stormceptor System is its patented scour prevention technology. This innovation ensures pollutants are captured and retained during all rainfall events, even extreme storms. The Stormceptor System provides continuous positive treatment for all rainfall events, including intense storms. Stormceptor slows incoming runoff, controlling and reducing velocities in the lower chamber to create a non-turbulent environment that promotes free oils and floatable debris to rise and sediment to settle.

The patented scour prevention technology, the fiberglass insert, regulates flows into the lower chamber through a combination of a weir and orifice while diverting high energy flows away through the upper chamber to prevent scouring. Laboratory testing demonstrated no scouring when tested up to $125 \%$ of the unit's operating rate, with the unit loaded to $100 \%$ sediment capacity (NJDEP, 2005). Second, the depth of the lower chamber ensures the sediment storage zone is adequately separated from the path of flow in the lower chamber to prevent scouring.

### 3.2. Operational Hydraulic Loading Rate

Designers and regulators need to evaluate the treatment capacity and performance of manufactured stormwater treatment systems. A commonly used parameter is the "operational hydraulic loading rate" which originated as a design methodology for wastewater treatment devices.

Operational hydraulic loading rate may be calculated by dividing the flow rate into a device by its settling area. This represents the critical settling velocity that is the prime determinant to quantify the influent particle size and density captured by the device. PCSWMM for Stormceptor uses a similar parameter that is calculated by dividing the hydraulic detention time in the device by the fall distance of the sediment.

$$
v_{\mathrm{SC}}=\frac{H}{\sigma_{\mathrm{H}}} \quad=\quad \frac{\mathrm{Q}}{A_{\mathrm{S}}}
$$

Where:
$\mathrm{v}_{\mathrm{SC}}=$ critical settling velocity, $\mathrm{ft} / \mathrm{s}(\mathrm{m} / \mathrm{s})$
$\mathrm{H}=$ tank depth, $\mathrm{ft}(\mathrm{m})$
$\varnothing_{\mathrm{H}}=$ hydraulic detention time, $\mathrm{ft} / \mathrm{s}(\mathrm{m} / \mathrm{s})$
$\mathrm{Q}=$ volumetric flow rate, $\mathrm{ft} 3 / \mathrm{s}(\mathrm{m} 3 / \mathrm{s})$
$\mathrm{A}_{\mathrm{s}}=$ surface area, $\mathrm{ft}^{2}\left(\mathrm{~m}^{2}\right)$
(Tchobanoglous, G. and Schroeder, E.D. 1987. Water Quality. Addison Wesley.)

Unlike designing typical wastewater devices, stormwater systems are designed for highly variable flow rates including intense peak flows. PCSWMM for Stormceptor incorporates all of the flows into its calculations, ensuring that the operational hydraulic loading rate is considered not only for one flow rate, but for all flows including extreme events.

### 3.3. Double Wall Containment

The Stormceptor System was conceived as a pollution identifier to assist with identifying illicit discharges. The fiberglass insert has a continuous skirt that lines the concrete barrel wall for a depth of 18 inches ( 457 mm ) that provides double wall containment for hydrocarbons storage. This protective barrier ensures that toxic floatables do not migrate through the concrete wall into the surrounding soils.

## 4. Stormceptor Product Line

### 4.1. Stormceptor Models

A summary of Stormceptor models and capacities are listed in Table 1.

Table 1. Stormceptor Models
$\left.\begin{array}{|c|c|c|c|}\hline \text { Stormceptor Model } & \begin{array}{c}\text { Total Storage Volume } \\ \text { U.S. Gal (L) }\end{array} & \begin{array}{c}\text { Hydrocarbon Storage } \\ \text { Capacity U.S. Gal (L) }\end{array} & \begin{array}{c}\text { Maximum Sediment } \\ \text { Capacity ft }\end{array} \\ \hline \text { (L) }\end{array}\right]$

NOTE: Storage volumes may vary slightly from region to region. For detailed information, contact your local Stormceptor representative.

### 4.2. Inline Stormceptor

The Inline Stormceptor, Figure 1, is the standard design for most stormwater treatment applications. The patented Stormceptor design allows the Inline unit to maintain continuous positive treatment of total suspended solids (TSS) year-round, regardless of flow rate. The Inline Stormceptor is composed of a precast concrete tank with a fiberglass insert situated at the invert of the storm sewer pipe, creating an upper chamber above the insert and a lower chamber below the insert.


Figure 1. Inline Stormceptor

## Operation

As water flows into the Stormceptor unit, it is slowed and directed to the lower chamber by a weir and drop tee. The stormwater enters the lower chamber, a non-turbulent environment, allowing free oils to rise and sediment to settle. The oil is captured underneath the fiberglass insert and shielded from exposure to the concrete walls by a fiberglass skirt. After the pollutants separate, treated water continues up a riser pipe, and exits the lower chamber on the downstream side of the weir before leaving the unit. During high flow events, the Stormceptor System's patented scour prevention technology ensures continuous pollutant removal and prevents re-suspension of previously captured pollutants.


Figure 2. Inlet Stormceptor

### 4.3. Inlet Stormceptor

The Inlet Stormceptor System, Figure 2, was designed to provide protection for parking lots, loading bays, gas stations and other spill-prone areas. The Inlet Stormceptor is designed to remove sediment from stormwater introduced through a grated inlet, a storm sewer pipe, or both.

The Inlet Stormceptor design operates in the same manner as the Inline unit, providing continuous positive treatment, and ensuring that captured material is not re-suspended.

### 4.4. Series Stormceptor

Designed to treat larger drainage areas, the Series Stormceptor System, Figure 3, consists of two adjacent Stormceptor models that function in parallel. This design eliminates the need for additional structures and piping to reduce installation costs.


Figure 3. Series System
The Series Stormceptor design operates in the same manner as the Inline unit, providing continuous positive treatment, and ensuring that captured material is not re-suspended.

## 5. Sizing the Stormceptor System

The Stormceptor System is a versatile product that can be used for many different aspects of water quality improvement. While addressing these needs, there are conditions that the designer needs to be aware of in order to size the Stormceptor model to meet the demands of each individual site in an efficient and cost-effective manner.

PCSWMM for Stormceptor is the support tool used for identifying the appropriate Stormceptor model. In order to size a unit, it is recommended the user follow the seven design steps in the program. The steps are as follows:

## STEP 1 - Project Details

The first step prior to sizing the Stormceptor System is to clearly identify the water quality objective for the development. It is recommended that a level of annual sediment (TSS) removal be identified and defined by a particle size distribution.

## STEP 2 - Site Details

Identify the site development by the drainage area and the level of imperviousness. It is recommended that imperviousness be calculated based on the actual area of imperviousness based on paved surfaces, sidewalks and rooftops.

## STEP 3 - Upstream Attenuation

The Stormceptor System is designed as a water quality device and is sometimes used in conjunction with onsite water quantity control devices such as ponds or underground detention systems. When possible, a greater benefit is typically achieved when installing a Stormceptor unit upstream of a detention facility. By placing the Stormceptor unit upstream of a detention structure, a benefit of less maintenance of the detention facility is realized.

## STEP 4 - Particle Size Distribution

It is critical that the PSD be defined as part of the water quality objective. PSD is critical for the design of treatment system for a unit process of gravity settling and governs the size of a treatment system. A range of particle sizes has been provided and it is recommended that clays and silt-sized particles be considered in addition to sand and gravel-sized particles. Options and sample PSDs are provided in PCSWMM for Stormceptor. The default particle size distribution is the Fine Distribution, Table 2, option.

Table 2. Fine Distribution

| Particle Size | Distribution | Specific Gravity |
| :---: | :---: | :---: |
| 20 | $20 \%$ | 1.3 |
| 60 | $20 \%$ | 1.8 |
| 150 | $20 \%$ | 2.2 |
| 400 | $20 \%$ | 2.65 |
| 2000 | $20 \%$ | 2.65 |

If the objective is the long-term removal of $80 \%$ of the total suspended solids on a given site, the PSD should be representative of the expected sediment on the site. For example, a system designed to remove $80 \%$ of coarse particles (greater than 75 microns) would provide relatively poor removal efficiency of finer particles that may be naturally prevalent in runoff from the site.

Since the small particle fraction contributes a disproportionately large amount of the total available particle surface area for pollutant adsorption, a system designed primarily for coarse particle capture will compromise water quality objectives.

## STEP 5 - Rainfall Records

Local historical rainfall has been acquired from the U.S. National Oceanic and Atmospheric Administration, Environment Canada and regulatory agencies across North America. The rainfall data provided with PCSMM for Stormceptor provides an accurate estimation of small storm hydrology by modeling actual historical storm events including duration, intensities and peaks.

## STEP 6 - Summary

At this point, the program may be executed to predict the level of TSS removal from the site. Once the simulation has completed, a table shall be generated identifying the TSS removal of each Stormceptor unit.

## STEP 7 - Sizing Summary

Performance estimates of all Stormceptor units for the given site parameters will be displayed in a tabular format. The unit that meets the water quality objective, identified in Step 1, will be highlighted.

### 5.1. PCSWMM for Stormceptor

The Stormceptor System has been developed in conjunction with PCSWMM for Stormceptor as a technological solution to achieve water quality goals. Together, these two innovations model, simulate, predict and calculate the water quality objectives desired by a design engineer for TSS removal.

PCSWMM for Stormceptor is a proprietary sizing program which uses site specific inputs to a computer model to simulate sediment accumulation, hydrology and long-term total suspended solids removal. The model has been calibrated to field monitoring results from Stormceptor units that have been monitored in North America. The sizing methodology can be described by three processes:

1. Determination of real time hydrology
2. Buildup and wash off of TSS from impervious land areas
3. TSS transport through the Stormceptor (settling and discharge). The use of a calibrated model is the preferred method for sizing stormwater quality structures for the following reasons:
» The hydrology of the local area is properly and accurately incorporated in the sizing (distribution of flows, flow rate ranges and peaks, back-to-back storms, inter-event times)
» The distribution of TSS with the hydrology is properly and accurately considered in the sizing
» Particle size distribution is properly considered in the sizing
» The sizing can be optimized for TSS removal
» The cost benefit of alternate TSS removal criteria can be easily assessed
» The program assesses the performance of all Stormceptor models. Sizing may be selected based on a specific water quality outcome or based on the Maximum Extent Practicable
For more information regarding PCSWMM for Stormceptor, contact your local Stormceptor representative, or visit www.imbriumsystems.com to download a free copy of the program.

### 5.2. Sediment Loading Characteristics

The way in which sediment is transferred to stormwater can have a considerable effect on which type of system is implemented. On typical impervious surfaces (e.g. parking lots) sediment will build over time and wash off with the next rainfall. When rainfall patterns are examined, a short intense storm will have a higher concentration of sediment than a long slow drizzle. Together with rainfall data representing the site's typical rainfall patterns, sediment loading characteristics play a part in the correct sizing of a stormwater quality device.

## Typical Sites

For standard site design of the Stormceptor System, PCSWMM for Stormceptor is utilized to accurately assess the unit's performance. As an integral part of the product's design, the program can be used to meet local requirements for total suspended solid removal. Typical installations of manufactured stormwater treatment devices would occur on areas such as paved parking lots or paved roads. These are considered "stable" surfaces which have non - erodible surfaces.

## Unstable Sites

While standard sites consist of stable concrete or asphalt surfaces, sites such as gravel parking lots, or maintenance yards with stockpiles of sediment would be classified as "unstable". These types of sites do not exhibit first flush characteristics, are highly erodible and exhibit atypical sediment loading characteristics and must therefore be sized more carefully. Contact your local Stormceptor representative for assistance in selecting a proper unit sized for such unstable sites.

## 6. Spill Controls

When considering the removal of total petroleum hydrocarbons (TPH) from a storm sewer system there are two functions of the system: oil removal, and spill capture.
'Oil Removal' describes the capture of the minute volumes of free oil mobilized from impervious surfaces. In this instance relatively low concentrations, volumes and flow rates are considered. While the Stormceptor unit will still provide an appreciable oil removal function during higher flow events and/or with higher TPH concentrations, desired effluent limits may be exceeded under these conditions.
'Spill Capture' describes a manner of TPH removal more appropriate to recovery of a relatively high volume of a single phase deleterious liquid that is introduced to the storm sewer system over a relatively short duration. The two design criteria involved when considering this manner of introduction are overall volume and the specific gravity of the material. A standard Stormceptor unit will be able to capture and retain a maximum spill volume and a minimum specific gravity.

For spill characteristics that fall outside these limits, unit modifications are required. Contact your local Stormceptor Representative for more information.

One of the key features of the Stormceptor technology is its ability to capture and retain spills. While the standard Stormceptor System provides excellent protection for spill control, there are additional options to enhance spill protection if desired.

### 6.1. Oil Level Alarm

The oil level alarm is an electronic monitoring system designed to trigger a visual and audible alarm when a pre-set level of oil is reached within the lower chamber. As a standard, the oil
level alarm is designed to trigger at approximately $85 \%$ of the unit's available depth level for oil capture. The feature acts as a safeguard against spills caused by exceeding the oil storage capacity of the separator and eliminates the need for manual oil level inspection.

The oil level alarm installed on the Stormceptor insert is illustrated in Figure 4.


Figure 4. Oil level alarm

### 6.2. Increased Volume Storage Capacity

The Stormceptor unit may be modified to store a greater spill volume than is typically available. Under such a scenario, instead of installing a larger than required unit, modifications can be made to the recommended Stormceptor model to accommodate larger volumes. Contact your local Stormceptor representative for additional information and assistance for modifications.

## 7. Stormceptor Options

The Stormceptor System allows flexibility to incorporate to existing and new storm drainage infrastructure. The following section identifies considerations that should be reviewed when installing the system into a drainage network. For conditions that fall outside of the recommendations in this section, please contact your local Stormceptor representative for further guidance.

### 7.1. Installation Depth Minimum Cover

The minimum distance from the top of grade to the crown of the inlet pipe is 24 inches ( 600 mm ). For situations that have a lower minimum distance, contact your local Stormceptor representative.

### 7.2. Maximum Inlet and Outlet Pipe Diameters

Maximum inlet and outlet pipe diameters are illustrated in Figure 5. Contact your local Stormceptor representative for larger pipe diameters


Figure 5. Maximum pipe diameters for straight through and bend applications
*The bend should only be incorporated into the second structure (downstream structure) of the Series Stormceptor System

### 7.3. Bends

The Stormceptor System can be used to change horizontal alignment in the storm drain network up to a maximum of 90 degrees. Figure 6 illustrates the typical bend situations of the Stormceptor System. Bends should only be applied to the second structure (downstream structure) of the Series Stormceptor System.

| Stormceptor <br> System <br> Inlet Stormceptor <br> Inline | Maximum Bend Configurations |
| :---: | :---: | :---: |
| Stormceptor |  |
| Inlet Pipe |  |
| Stormceptor |  |
| Inlet Pipe |  |

Figure 6. Maximum bend angles

### 7.4. Multiple Inlet Pipes

The Inlet and Inline Stormceptor System can accommodate two or more inlet pipes. The maximum number of inlet pipes that can be accommodated into a Stormceptor unit is a function of the number, alignment and diameter of the pipes and its effects on the structural integrity of the precast concrete. When multiple inlet pipes are used for new developments, each inlet pipe shall have an invert elevation 3 inches ( 75 mm ) higher than the outlet pipe invert elevation.

### 7.5. Inlet/Outlet Pipe Invert Elevations

Recommended inlet and outlet pipe invert differences are listed in Table 3.
Table 3. Recommended Drops Between Inlet and Outlet Pipe Inverts

| Number of Inlet <br> Pipes | Inlet System |  | In-Line System |  | Series System |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 3 inches $(75 \mathrm{~mm})$ | 1 inch $(25 \mathrm{~mm})$ | 3 inches $(75 \mathrm{~mm})$ |  |  |
| $>1$ | 3 inches $(75 \mathrm{~mm})$ | 3 inches $(75 \mathrm{~mm})$ | Not Applicable |  |  |

### 7.6. Shallow Stormceptor

In cases where there may be restrictions to the depth of burial of storm sewer systems. In this situation, for selected Stormceptor models, the lower chamber components may be increased in diameter to reduce the overall depth of excavation required.

### 7.7. Customized Live Load

The Stormceptor system is typically designed for local highway truck loading (AASHTO HS- 20). When the project requires live loads greater than HS-20, the Stormceptor System may be customized structurally for a pre-specified live load. Contact your local Stormceptor representative for customized loading conditions.

### 7.8. Pre-treatment

The Stormceptor System may be sized to remove sediment and for spills control in conjunction with other stormwater BMPs to meet the water quality objective. For pretreatment applications, the Stormceptor System should be the first unit in a treatment train. The benefits of pre-treatment include the extension of the operational life (extension of maintenance frequency) of large stormwater management facilities, prevention of spills and lower total life- cycle maintenance cost.

### 7.9. Head loss

The head loss through the Stormceptor System is similar to a 60 degree bend at a manhole. The $K$ value for calculating minor losses is approximately 1.3 (minor loss $=k^{*} 1.3 \mathrm{v} 2 / 2 \mathrm{~g}$ ).

However, when a Submerged modification is applied to a Stormceptor unit, the corresponding K value is 4 .

### 7.10. Submerged

The Submerged modification, Figure 7, allows the Stormceptor System to operate in submerged or partially submerged storm sewers. This configuration can be installed on all models of the Stormceptor System by modifying the fiberglass insert. A customized weir height and a secondary drop tee are added.

Submerged instances are defined as standing water in the storm drain system during zero flow conditions. In these instances, the following information is necessary for the proper design and application of submerged modifications:

- Stormceptor top of grade elevation
- Stormceptor outlet pipe invert elevation
- Standing water elevation


Figure 7. Submerged Stormceptor

## 8. Comparing Technologies

Designers have many choices available to achieve water quality goals in the treatment of stormwater runoff. Since many alternatives are available for use in stormwater quality treatment it is important to consider how to make an appropriate comparison between "approved alternatives". The following is a guide to assist with the accurate comparison of differing technologies and performance claims.

### 8.1. Particle Size Distribution (PSD)

The most sensitive parameter to the design of a stormwater quality device is the selection of the design particle size. While it is recommended that the actual particle size distribution (PSD) for sites be measured prior to sizing, alternative values for particle size should be selected to represent what is likely to occur naturally on the site. A reasonable estimate of a particle size distribution likely to be found on parking lots or other impervious surfaces should consist of a wide range of particles such as 20 microns to 2,000 microns (Ontario MOE, 1994).

There is no absolute right particle size distribution or specific gravity and the user is cautioned to review the site location, characteristics, material handling practices and regulatory requirements when selecting a particle size distribution. When comparing technologies, designs using different PSDs will result in incomparable TSS removal efficiencies. The PSD of the TSS removed needs to be standard between two products to allow for an accurate comparison.

### 8.2. Scour Prevention

In order to accurately predict the performance of a manufactured treatment device, there must be confidence that it will perform under all conditions. Since rainfall patterns cannot be predicted, stormwater quality devices placed in storm sewer systems must be able to withstand extreme events, and ensure that all pollutants previously captured are retained in the system.

In order to have confidence in a system's performance under extreme conditions, independent validation of scour prevention is essential when examining different technologies. Lack of independent verification of scour prevention should make a designer wary of accepting any product's performance claims.

### 8.3. Hydraulics

Full scale laboratory testing has been used to confirm the hydraulics of the Stormceptor System. Results of lab testing have been used to physically design the Stormceptor System and the sewer pipes entering and leaving the unit. Key benefits of Stormceptor are:

- Low head loss (typical $k$ value of 1.3 )
- Minimal inlet/outlet invert elevation drop across the structure
- Use as a bend structure
- Accommodates multiple inlets

The adaptability of the treatment device to the storm sewer design infrastructure can affect the overall performance and cost of the site.

### 8.4. Hydrology

Stormwater quality treatment technologies need to perform under varying climatic conditions. These can vary from long low intensity rainfall to short duration, high intensity storms. Since a treatment device is expected to perform under all these conditions, it makes sense that any system's design should accommodate those conditions as well.

Long-term continuous simulation evaluates the performance of a technology under the varying conditions expected in the climate of the subject site. Single, peak event design does not provide this information and is not equivalent to long-term simulation. Designers should request long-term simulation performance to ensure the technology can meet the long-term water quality objective.

## 9. Testing

The Stormceptor System has been the most widely monitored stormwater treatment technology in the world. Performance verification and monitoring programs are completed to the strictest standards and integrity. Since its introduction in 1990, numerous independent field tests and studies detailing the effectiveness of the Stormceptor System have been completed.

- Coventry University, UK - 97\% removal of oil, $83 \%$ removal of sand and $73 \%$ removal of peat
- National Water Research Institute, Canada, - scaled testing for the development of the Stormceptor System identifying both TSS removal and scour prevention.
- New Jersey TARP Program - full scale testing of an STC 900 demonstrating $75 \%$ TSS removal of particles from 1 to 1000 microns. Scour testing completed demonstrated that the system does not scour. The New Jersey Department of Environmental Protection was followed.
- City of Indianapolis - full scale testing of an STC 900 demonstrating over $80 \%$ TSS removal of particles from 50 microns to 300 microns at $130 \%$ of the unit's operating rate. Scour testing completed demonstrated that the system does not scour.
- Westwood Massachusetts (1997), demonstrated >80\% TSS removal
- Como Park (1997), demonstrated 76\% TSS removal
- Ontario MOE SWAMP Program - 57\% removal of 1 to 25 micron particles
- Laval Quebec - 50\% removal of 1 to 25 micron particles


## 10. Installation

The installation of the concrete Stormceptor should conform in general to state highway, or local specifications for the installation of manholes. Selected sections of a general specification that are applicable are summarized in the following sections.

### 10.1. Excavation

Excavation for the installation of the Stormceptor should conform to state highway, or local specifications. Topsoil removed during the excavation for the Stormceptor should be stockpiled in designated areas and should not be mixed with subsoil or other materials.

Topsoil stockpiles and the general site preparation for the installation of the Stormceptor should conform to state highway or local specifications.

The Stormceptor should not be installed on frozen ground. Excavation should extend a minimum of 12 inches ( 300 mm ) from the precast concrete surfaces plus an allowance for shoring and bracing where required. If the bottom of the excavation provides an unsuitable foundation additional excavation may be required.

In areas with a high water table, continuous dewatering may be required to ensure that the excavation is stable and free of water.

### 10.2. Backfilling

Backfill material should conform to state highway or local specifications. Backfill material should be placed in uniform layers not exceeding 12 inches $(300 \mathrm{~mm}$ ) in depth and compacted to state highway or local specifications.

## 11. Stormceptor Construction Sequence

The concrete Stormceptor is installed in sections in the following sequence:

1. Aggregate base
2. Base slab
3. Lower chamber sections
4. Upper chamber section with fiberglass insert
5. Connect inlet and outlet pipes
6. Assembly of fiberglass insert components (drop tee, riser pipe, oil cleanout port and orifice plate
7. Remainder of upper chamber
8. Frame and access cover

The precast base should be placed level at the specified grade. The entire base should be in contact with the underlying compacted granular material. Subsequent sections, complete with joint seals, should be installed in accordance with the precast concrete manufacturer's recommendations.

Adjustment of the Stormceptor can be performed by lifting the upper sections free of the excavated area, re-leveling the base and reinstalling the sections. Damaged sections and gaskets should be repaired or replaced as necessary. Once the Stormceptor has been constructed, any lift holes must be plugged with mortar.

## 12. Maintenance

### 12.1 $\quad$ Health and Safety

The Stormceptor System has been designed considering safety first. It is recommended that confined space entry protocols be followed if entry to the unit is required. In addition, the fiberglass insert has the following health and safety features:

- Designed to withstand the weight of personnel
- A safety grate is located over the 24 inch ( 600 mm ) riser pipe opening
- Ladder rungs can be provided for entry into the unit, if required


### 12.2. Maintenance Procedures

Maintenance of the Stormceptor system is performed using vacuum trucks. No entry into the unit is required for maintenance (in most cases). The vacuum service industry is a well- established sector of the service industry that cleans underground tanks, sewers and catch basins. Costs to clean a Stormceptor will vary based on the size of unit and transportation distances.

The need for maintenance can be determined easily by inspecting the unit from the surface. The depth of oil in the unit can be determined by inserting a dipstick in the oil inspection/cleanout port.

Similarly, the depth of sediment can be measured from the surface without entry into the Stormceptor via a dipstick tube equipped with a ball valve. This tube would be inserted through the riser pipe. Maintenance should be performed once the sediment depth exceeds the guideline values provided in the Table 4.

Table 4. Sediment Depths Indicating Required Servicing*

| Particle Size |  |
| :---: | :---: |
| Model | Specific Gravity |
| 450 i | $8(200)$ |
| 900 | $8(200)$ |
| 1200 | $10(250)$ |
| 1800 | $15(381)$ |
| 2400 | $12(300)$ |
| 3600 | $17(430)$ |
| 4800 | $15(380)$ |
| 6000 | $18(460)$ |
| 7200 | $15(381)$ |
| 11000 | $17(380)$ |
| 13000 | $20(500)$ |
| 16000 | $17(380)$ |
| * based on 15\% of the Stormceptor unit's total storage |  |

Although annual servicing is recommended, the frequency of maintenance may need to be increased or reduced based on local conditions (i.e. if the unit is filling up with sediment more quickly than projected, maintenance may be required semi-annually; conversely once the site has stabilized maintenance may only be required every two or three years).

Oil is removed through the oil inspection/cleanout port and sediment is removed through the riser pipe. Alternatively oil could be removed from the 24 inches $(600 \mathrm{~mm}$ ) opening if water is removed from the lower chamber to lower the oil level below the drop pipes.

The following procedures should be taken when cleaning out Stormceptor:

1. Check for oil through the oil cleanout port
2. Remove any oil separately using a small portable pump
3. Decant the water from the unit to the sanitary sewer, if permitted by the local regulating authority, or into a separate containment tank
4. Remove the sludge from the bottom of the unit using the vacuum truck
5. Re-fill Stormceptor with water where required by the local jurisdiction

### 12.3. Submerged Stormceptor

Careful attention should be paid to maintenance of the Submerged Stormceptor System. In cases where the storm drain system is submerged, there is a requirement to plug both the inlet and outlet pipes to economically clean out the unit.

### 12.4. Hydrocarbon Spills

The Stormceptor is often installed in areas where the potential for spills is great. The Stormceptor System should be cleaned immediately after a spill occurs by a licensed liquid waste hauler.

### 12.5. Disposal

Requirements for the disposal of material from the Stormceptor System are similar to that of any other stormwater Best Management Practice (BMP) where permitted. Disposal options for the sediment may range from disposal in a sanitary trunk sewer upstream of a sewage treatment plant, to disposal in a sanitary landfill site. Petroleum waste products collected in the Stormceptor (free oil/chemical/fuel spills) should be removed by a licensed waste management company.

### 12.6. Oil Sheens

With a steady influx of water with high concentrations of oil, a sheen may be noticeable at the Stormceptor outlet. This may occur because a rainbow or sheen can be seen at very small oil concentrations ( $<10 \mathrm{mg} / \mathrm{L}$ ). Stormceptor will remove over $98 \%$ of all free oil spills from storm sewer systems for dry weather or frequently occurring runoff events.

The appearance of a sheen at the outlet with high influent oil concentrations does not mean the unit is not working to this level of removal. In addition, if the influent oil is emulsified the Stormceptor will not be able to remove it. The Stormceptor is designed for free oil removal and not emulsified conditions.


## SUPPORT

Drawings and specifications are available at www.ContechES.com.
Site-specific design support is available from our engineers.

[^1]
## Appendix D: O\&M and LTPPP Plan



## Appendix D: Pre and Post Drainage Maps










## Appendix E: HydroCAD, Stage Storage, and Pocket Wetland Calculations



## 19097 Pre-Development

Prepared by Howard Stein Hudson Associates
Printed 6/16/2023
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## Project Notes

Rainfall events imported from "19097 Post-Development.hcp"

## 19097 Pre-Development

Prepared by Howard Stein Hudson Associates
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## Rainfall Events Listing

| Event\# | Event <br> Name | Storm Type | Curve | Mode | Duration <br> (hours) | B/B | Depth <br> (inches) |
| :---: | :--- | :--- | :--- | :--- | ---: | ---: | ---: |
| 1 | 2 YR | Type III 24-hr |  | Default | 24.00 | 1 | 3.27 |
| 2 | 10YR | Type III 24-hr | Default | 24.00 | 1 | 4.96 | 2 |
| 3 | $25 Y R$ | Type III 24-hr | Default | 24.00 | 1 | 6.29 | 2 |
| 4 | 100YR | Type III 24-hr | Default | 24.00 | 1 | 9.06 | 2 |

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## Area Listing (all nodes)

| Area <br> $(\mathrm{sq}-\mathrm{ft})$ | CN | Description <br> (subcatchment-numbers) |
| ---: | :--- | :--- |
| 143,648 | 68 | 1 acre lots, 20\% imp, HSG B (102S, 104S) |
| 549 | 79 | 1 acre lots, 20\% imp, HSG C (104S) |
| 179,555 | 61 | $>75 \%$ Grass cover, Good, HSG B (101S, 102S) |
| 15,945 | 74 | $>75 \%$ Grass cover, Good, HSG C (102S) |
| 3,192 | 80 | $>75 \%$ Grass cover, Good, HSG D (102S) |
| 89,402 | 30 | Brush, Good, HSG A (103S, 104S) |
| 2,920 | 65 | Brush, Good, HSG C (104S) |
| 4,643 | 73 | Brush, Good, HSG D (103S) |
| 1,262 | 96 | Gravel surface, HSG B (101S) |
| 33,283 | 98 | Paved parking, HSG B (101S, 102S) |
| 448,007 | 98 | Water Surface, O\% imp, HSG D (102S, 103S, 104S) |
| 212,938 | 30 | Woods, Good, HSG A (103S, 104S) |
| 358,427 | 55 | Woods, Good, HSG B (102S, 104S) |
| 866,259 | 70 | Woods, Good, HSG C (102S, 103S, 104S) |
| 213,890 | 77 | Woods, Good, HSG D (102S, 103S, 104S) |
| $\mathbf{2 , 5 7 3 , 9 2 0}$ | $\mathbf{6 8}$ | TOTAL AREA |

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## Soil Listing (all nodes)

| Area <br> $(\mathrm{sq-ft})$ | Soil <br> Group | Subcatchment <br> Numbers |
| ---: | :--- | :--- |
| 302,340 | HSG A | $103 \mathrm{~S}, 104 \mathrm{~S}$ |
| 716,175 | HSG B | $101 \mathrm{~S}, 102 \mathrm{~S}, 104 \mathrm{~S}$ |
| 885,673 | HSG C | $102 \mathrm{~S}, 103 \mathrm{~S}, 104 \mathrm{~S}$ |
| 669,732 | HSG D | 102S,103S,104S |
| 0 | Other |  |
| $\mathbf{2 , 5 7 3 , 9 2 0}$ |  | TOTAL AREA |

## 19097 Pre-Development

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## Ground Covers (all nodes)

| $\begin{array}{r} \text { HSG-A } \\ (\mathrm{sq-ft}) \end{array}$ | $\begin{array}{r} \text { HSG-B } \\ \text { (sq-ft) } \end{array}$ | HSG-C $(\mathrm{sq}-\mathrm{ft})$ | $\begin{aligned} & \text { HSG-D } \\ & \text { (sq-ft) } \end{aligned}$ | $\begin{aligned} & \text { Other } \\ & \text { (sq-ft) } \end{aligned}$ | $\begin{array}{r} \text { Total } \\ \text { (sq-ft) } \end{array}$ | Ground <br> Cover |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 143,648 | 549 | 0 | 0 | 144,197 | 1 acre lots, 20\% imp |
| 0 | 179,555 | 15,945 | 3,192 | 0 | 198,692 | $>75 \%$ Grass cover, Good |
| 89,402 | 0 | 2,920 | 4,643 | 0 | 96,965 | Brush, Good |
| 0 | 1,262 | 0 | 0 | 0 | 1,262 | Gravel surface |
| 0 | 33,283 | 0 | 0 | 0 | 33,283 | Paved parking |
| 0 | 0 | 0 | 448,007 | 0 | 448,007 | Water Surface, 0\% imp |
| 212,938 | 358,427 | 866,259 | 213,890 | 0 | 1,651,514 | Woods, Good |
| 302,340 | 716,175 | 885,673 | 669,732 | 0 | 2,573,920 | TOTAL AREA |

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method
Subcatchment101S: SUMMER STREET Runoff Area=13,756 sf $64.57 \%$ Impervious Runoff Depth>2.06" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=88$ Runoff=0.74 cfs $2,360 \mathrm{cf}$

Subcatchment 102S: OVERLANDTO Runoff Area=956,300 sf $3.58 \%$ Impervious Runoff Depth>1.07" Flow Length=1,531' Tc=44.5 $\mathrm{min} \quad \mathrm{CN}=74$ Runoff=12.30 cfs $85,349 \mathrm{cf}$

## Subcatchment103S: ISOLATED

Runoff Area=105,094 sf $0.00 \%$ Impervious Runoff Depth $>0.97{ }^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=72$ Runoff= $2.52 \mathrm{cfs} 8,514 \mathrm{cf}$

Subcatchment104S: REMAININGLAND Runoff Area=1,498,770 sf 1.27\% Impervious Runoff Depth>0.59" Flow Length=987' Tc=28.6 min CN=64 Runoff=10.77 cfs 73,247 cf

Link AP1: ANALYSISPOINT 1

Link AP2: ANALYSISPOINT 2

Link AP3: ANALYSISPOINT 3

## Link AP4: ANALYSISPOINT 4

Inflow=0.74 cfs 2,360 cf Primary $=0.74$ cfs 2,360 cf Inflow=12.30 cfs $85,349 \mathrm{cf}$ Primary $=12.30$ cfs 85,349 cf

Inflow=2.52 cfs 8,514 cf Primary $=2.52$ cfs 8,514 cf

Inflow=10.77 cfs 73,247 cf Primary $=10.77$ cfs 73,247 cf

> Total Runoff Area $=2,573,920$ sf Runoff Volume $=169,469$ cf Average Runoff Depth $=\mathbf{0 . 7 9 "}$ 97.59\% Pervious $=\mathbf{2 , 5 1 1 , 7 9 8} \mathbf{~ s f} \quad 2.41 \%$ Impervious $=\mathbf{6 2 , 1 2 2} \mathbf{~ s f}$

## Summary for Subcatchment 101S: SUMMER STREET (SOUTH)

Runoff $=\quad 0.74$ cfs @ 12.09 hrs, Volume= 2,360 cf, Depth> 2.06"

Routed to Link AP1 : ANALYSIS POINT 1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG B |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,612 | $61>$ |  |  |  |
|  | 8,882 | 98 P | Paved parking, HSG B |  |  |
|  | 1,262 | 96 G |  |  |  |
|  | 13,756 | 88 | Weighted Average |  |  |
|  | 4,874 |  | 35.43\% Pervious Area |  |  |
|  | 8,882 |  | 64.57\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entr |

## Summary for Subcatchment 102S: OVERLAND TO TRAIN TRACKS (EAST)

Runoff $=\quad 12.30$ cfs @ 12.66 hrs, Volume= 85,349 cf, Depth> 1.07"
Routed to Link AP2 : ANALYSIS POINT 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 175,943 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 184,742 | 55 | Woods, Good, HSG B |
| 24,401 | 98 | Paved parking, HSG B |
| 49,311 | 68 | 1 acre lots, 20\% imp, HSG B |
| 15,945 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 235,274 | 70 | Woods, Good, HSG C |
| 3,192 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 1,550 | 77 | Woods, Good, HSG D |
| 265,942 | 98 | Water Surface, O\% imp, HSG D |
| 956,300 | 74 | Weighted Average |
| 922,037 |  | 96.42\% Pervious Area |
| 34,263 |  | $3.58 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |
| 3.2 | 50 | 0.0800 | 0.26 | Sheet Flow, <br> Grass: Short $\mathrm{n}=0.150 \quad \mathrm{P} 2=3.27 "$ <br> 0.3 |
| 77 | 0.0780 | 4.50 | Shallow Concentrated Flow, |  |
| 41.0 | 1,404 | 0.0130 | 0.57 | Unpaved Kv=16.1 fps <br> Shallow Concentrated Flow, <br> Woodland Kv=5.0 fps |
| 44.5 | 1,531 | Total |  |  |

## Summary for Subcatchment 103S: ISOLATED WETLAND (NORTHEAST)

Runoff $=\quad 2.52$ cfs @ 12.10 hrs, Volume= 8,514 cf, Depth> 0.97"

Routed to Link AP3 : ANALYSIS POINT 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment 104S: REMAINING LAND (NORTH - RIVER)

Runoff $=\quad 10.77$ cfs @ 12.50 hrs, Volume= $\quad 73,247$ cf, Depth> 0.59"
Routed to Link AP4 : ANALYSIS POINT 4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- | :--- |
| 83,589 | 30 | Brush, Good, HSG A |
| 211,292 | 30 | Woods, Good, HSG A |
| 173,685 | 55 | Woods, Good, HSG B |
| 94,337 | 68 | 1 acre lots, 20\% imp, HSG B |
| 549 | 79 | 1 acre lots, 20\% imp, HSG C |
| 2,920 | 65 | Brush, Good, HSG C |
| 582,787 | 70 | Woods, Good, HSG C |
| 173,260 | 98 | Water Surface, 0\% imp, HSG D |
| 176,351 | 77 | Woods, Good, HSG D |

## Summary for Link AP1: ANALYSIS POINT 1

| Inflow Area $=$ | $13,756 \mathrm{sf}, 64.57 \%$ | Impervious, | Inflow Depth > 2.06" | for 2 YR event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.74 \mathrm{cfs} @$ | 12.09 hrs , Volume | $2,360 \mathrm{cf}$ |
| Primary | $=$ | $0.74 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $2,360 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP2: ANALYSIS POINT 2

| In | 0 s | perviou | Inflow Depth > 1.07" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflo | 12.30 cfs @ | 12.66 hrs , Volume= | 85,349 cf |
| Primary | 12.30 cfs @ | 12.66 hrs, Volume= | $85,349 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP3: ANALYSIS POINT 3

| Inflow Area $=$ | $105,094 \mathrm{sff}$ | $0.00 \%$ Impervious, | Inflow Depth $>0.97 "$ | for 2 YR event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $2.52 \mathrm{cfs} @$ | 12.10 hrs , Volume= | $8,514 \mathrm{cf}$ |
| Primary | $=$ | $2.52 \mathrm{cfs} @$ | 12.10 hrs , Volume $=$ | $8,514 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP4: ANALYSIS POINT 4

| Inflow Area | 1,498,770 sf, | 1.27\% Impervious, | w Depth > 0.59" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 10.77 cfs @ | 12.50 hrs , Volume= | 73,247 cf |
| Primary | 10.77 cfs @ | 12.50 hrs , Volume= | 73,247 cf, Atten= 0\%, Lag= 0.0 m |

Primary outflow $=$ Inflow, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method
Subcatchment101S: SUMMER STREET Runoff Area=13,756 sf $64.57 \%$ Impervious Runoff Depth $>3.63$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=88$ Runoff=1.28 cfs $4,159 \mathrm{cf}$

Subcatchment102S: OVERLANDTO Runoff Area=956,300 sf $3.58 \%$ Impervious Runoff Depth>2.31" Flow Length=1,531' Tc=44.5 min CN=74 Runoff=27.75 cfs 184,006 cf

Subcatchment 103S: ISOLATED
Runoff Area=105,094 sf $0.00 \%$ Impervious Runoff Depth $>2.16$ " Tc=6.0 min CN=72 Runoff=5.96 cfs $18,960 \mathrm{cf}$

Subcatchment104S: REMAININGLAND Runoff Area=1,498,770 sf 1.27\% Impervious Runoff Depth>1.54" Flow Length=987' Tc=28.6 min CN=64 Runoff=33.90 cfs 192,708 cf

## Link AP1: ANALYSISPOINT 1

Link AP2: ANALYSISPOINT 2

Link AP3: ANALYSISPOINT 3

Link AP4: ANALYSISPOINT 4

Inflow=1.28 cfs 4,159 cf Primary=1.28 cfs 4,159 cf

Inflow=27.75 cfs 184,006 cf Primary=27.75 cfs 184,006 cf

Inflow=5.96 cfs 18,960 cf Primary=5.96 cfs 18,960 cf

Inflow=33.90 cfs 192,708 cf Primary $=33.90$ cfs 192,708 cf

> Total Runoff Area $=2,573,920$ sf Runoff Volume $=399,833$ cf Average Runoff Depth $=1.86$ "
> $\mathbf{9 7 . 5 9 \%}$ Pervious $=\mathbf{2 , 5 1 1 , 7 9 8} \mathbf{~ s f} \quad 2.41 \%$ Impervious $=\mathbf{6 2 , 1 2 2} \mathbf{~ s f}$

## Summary for Subcatchment 101S: SUMMER STREET (SOUTH)

Runoff =
$=\quad 1.28$ cfs @
12.09 hrs , Volume=

4,159 cf, Depth> 3.63"
Routed to Link AP1 : ANALYSIS POINT 1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG B |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,612 | $\begin{aligned} & \hline 61 \\ & 98 \end{aligned}$ |  |  |  |
|  | 8,882 |  | $>75 \%$ Grass cover, Good, HSG B Paved parking, HSG B |  |  |
|  | 1,262 | 96 | Gravel surf | ce, HSG B |  |
|  | 13,756 | 88 | Weighted Average |  |  |
|  | 4,874 |  | 35.43\% Pervious Area |  |  |
|  | 8,882 |  | 64.57\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{r} \text { Length } \\ \text { (feet) } \\ \hline \end{array}$ | Slope $(\mathrm{ft} / \mathrm{ft})$ | $\begin{array}{r} \text { Velocity } \\ (\mathrm{ft} / \mathrm{sec}) \\ \hline \end{array}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment 102S: OVERLAND TO TRAIN TRACKS (EAST)

Runoff $=\quad 27.75$ cfs @ 12.62 hrs, Volume= 184,006 cf, Depth> 2.31"
Routed to Link AP2 : ANALYSIS POINT 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 175,943 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 184,742 | 55 | Woods, Good, HSG B |
| 24,401 | 98 | Paved parking, HSG B |
| 49,311 | 68 | 1 acre lots, 20\% imp, HSG B |
| 15,945 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 235,274 | 70 | Woods, Good, HSG C |
| 3,192 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 1,550 | 77 | Woods, Good, HSG D |
| 265,942 | 98 | Water Surface, 0\% imp, HSG D |
| 956,300 | 74 | Weighted Average |
| 922,037 |  | $96.42 \%$ Pervious Area |
| 34,263 |  | $3.58 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |
| 3.2 | 50 | 0.0800 | 0.26 | Sheet Flow, <br> Grass: Short $\mathrm{n}=0.150 \quad \mathrm{P} 2=3.27 "$ <br> 0.3 |
| 77 | 0.0780 | 4.50 | Shallow Concentrated Flow, |  |
| 41.0 | 1,404 | 0.0130 | 0.57 | Unpaved Kv=16.1 fps <br> Shallow Concentrated Flow, <br> Woodland Kv=5.0 fps |
| 44.5 | 1,531 | Total |  |  |

## Summary for Subcatchment 103S: ISOLATED WETLAND (NORTHEAST)

Runoff = 5.96 cfs @ 12.10 hrs, Volume= 18,960 cf, Depth> 2.16"
Routed to Link AP3 : ANALYSIS POINT 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |  |  |
| ---: | ---: | :--- | :---: | :---: |
| 5,813 | 30 | Brush, Good, HSG A |  |  |
| 1,646 | 30 | Woods, Good, HSG A |  |  |
| 48,198 | 70 | Woods, Good, HSG C |  |  |
| 4,643 | 73 | Brush, Good, HSG D |  |  |
| 35,989 | 77 | Woods, Good, HSG D |  |  |
| 8,805 | 98 | Water Surface, 0\% imp, HSG D |  |  |
| 105,094 | 72 | Weighted Average |  |  |
| 105,094 | $100.00 \%$ Pervious Area |  |  |  |
| TcLength Slope Velocity <br> (ft/ft) (ft/sec) Capacity <br> (cfs) Description  <br> 6.0 (feet) Direct Entry, |  |  |  |  |

## Summary for Subcatchment 104S: REMAINING LAND (NORTH - RIVER)

Runoff = 33.90 cfs @ 12.44 hrs, Volume= 192,708 cf, Depth> 1.54"
Routed to Link AP4 : ANALYSIS POINT 4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |  |
| ---: | ---: | :--- | :--- |
| 83,589 | 30 | Brush, Good, HSG A |  |
| 211,292 | 30 | Woods, Good, HSG A |  |
| 173,685 | 55 | Woods, Good, HSG B |  |
| 94,337 | 68 | 1 acre lots, 20\% imp, HSG B |  |
| 549 | 79 | 1 acre lots, 20\% imp, HSG C |  |
| 2,920 | 65 | Brush, Good, HSG C |  |
| 582,787 | 70 | Woods, Good, HSG C |  |
| 173,260 | 98 | Water Surface, 0\% imp, HSG D |  |
| 176,351 | 77 | Woods, Good, HSG D |  |

28.6987 Total

## Summary for Link AP1: ANALYSIS POINT 1



Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP2: ANALYSIS POINT 2

| Inflow Area = | 956,300 sf, | 3.58\% Impervious, | Inflow Depth > 2.31" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 27.75 cfs @ | 12.62 hrs , Volume= | 184,006 cf |
| Primary | 27.75 cfs @ | 12.62 hrs , Volume= | 184,006 cf, |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP3: ANALYSIS POINT 3

| Inflow Area | 105,094 sf, | 0.00\% Impervious, | Inflow Depth > 2.16" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 5.96 cfs @ | 12.10 hrs , Volume= | 18,960 cf |
| Primary | 5.96 cfs @ | 12.10 hrs , Volume= | 18,960 cf, Atten= 0\%, Lag= 0.0 min |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP4: ANALYSIS POINT 4

Inflow Area $=1,498,770$ sf, $1.27 \%$ Impervious, Inflow Depth > 1.54" for 10YR event
Inflow $=33.90 \mathrm{cfs} @ 12.44 \mathrm{hrs}$, Volume $=192,708 \mathrm{cf}$
Primary $=33.90$ cfs @ 12.44 hrs , Volume $=192,708 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$
Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method
Subcatchment101S: SUMMER STREET Runoff Area=13,756 sf 64.57\% Impervious Runoff Depth>4.90" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=88$ Runoff=1.71 cfs $5,619 \mathrm{cf}$

Subcatchment102S: OVERLANDTO Runoff Area=956,300 sf $3.58 \%$ Impervious Runoff Depth $>3.40$ " Flow Length=1,531' Tc=44.5 min CN=74 Runoff=41.12 cfs 270,829 cf

Subcatchment 103S: ISOLATED
Runoff Area=105,094 sf $0.00 \%$ Impervious Runoff Depth $>3.23$ " Tc=6.0 min CN=72 Runoff=8.96 cfs 28,279 cf

Subcatchment104S: REMAININGLAND Runoff Area=1,498,770 sf 1.27\% Impervious Runoff Depth>2.46" Flow Length=987' Tc=28.6 min CN=64 Runoff=56.04 cfs 306,701 cf

## Link AP1: ANALYSISPOINT 1

## Link AP2: ANALYSISPOINT 2

## Link AP3: ANALYSISPOINT 3

## Link AP4: ANALYSISPOINT 4

Inflow=1.71 cfs 5,619 cf Primary=1.71 cfs 5,619 cf

Inflow=41.12 cfs 270,829 cf
Primary=41.12 cfs 270,829 cf
Inflow=8.96 cfs 28,279 cf Primary $=8.96$ cfs 28,279 cf

Inflow=56.04 cfs 306,701 cf Primary=56.04 cfs 306,701 cf

> Total Runoff Area $=2,573,920$ sf Runoff Volume $=\mathbf{6 1 1 , 4 2 8}$ cf Average Runoff Depth $=2.85$ " 97.59\% Pervious $=\mathbf{2 , 5 1 1 , 7 9 8} \mathbf{s f} \quad 2.41 \%$ Impervious $=\mathbf{6 2 , 1 2 2} \mathbf{~ s f}$

## Summary for Subcatchment 101S: SUMMER STREET (SOUTH)

Runoff $=1.71$ cfs @ 12.09 hrs, Volume= 5,619 cf, Depth> 4.90"

Routed to Link AP1 : ANALYSIS POINT 1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG B |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,612 | $61>$ |  |  |  |
|  | 8,882 | 98 P | Paved parking, HSG B |  |  |
|  | 1,262 | 96 G |  |  |  |
|  | 13,756 | 88 | Weighted Average |  |  |
|  | 4,874 |  | 35.43\% Pervious Area |  |  |
|  | 8,882 |  | 64.57\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entr |

## Summary for Subcatchment 102S: OVERLAND TO TRAIN TRACKS (EAST)

Runoff $=\quad 41.12$ cfs @ 12.61 hrs, Volume= 270,829 cf, Depth> 3.40"
Routed to Link AP2 : ANALYSIS POINT 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 175,943 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 184,742 | 55 | Woods, Good, HSG B |
| 24,401 | 98 | Paved parking, HSG B |
| 49,311 | 68 | 1 acre lots, 20\% imp, HSG B |
| 15,945 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 235,274 | 70 | Woods, Good, HSG C |
| 3,192 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 1,550 | 77 | Woods, Good, HSG D |
| 265,942 | 98 | Water Surface, 0\% imp, HSG D |
| 956,300 | 74 | Weighted Average |
| 922,037 |  | $96.42 \%$ Pervious Area |
| 34,263 |  | $3.58 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |
| 3.2 | 50 | 0.0800 | 0.26 | Sheet Flow, <br> Grass: Short $\mathrm{n}=0.150 \quad \mathrm{P} 2=3.27 "$ <br> 0.3 |
| 77 | 0.0780 | 4.50 | Shallow Concentrated Flow, |  |
| 41.0 | 1,404 | 0.0130 | 0.57 | Unpaved Kv=16.1 fps <br> Shallow Concentrated Flow, <br> Woodland Kv=5.0 fps |
| 44.5 | 1,531 | Total |  |  |

## Summary for Subcatchment 103S: ISOLATED WETLAND (NORTHEAST)

Runoff = 8.96 cfs @ 12.09 hrs, Volume= 28,279 cf, Depth> 3.23"
Routed to Link AP3 : ANALYSIS POINT 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"


## Summary for Subcatchment 104S: REMAINING LAND (NORTH - RIVER)

Runoff $=56.04$ cfs @ 12.42 hrs, Volume= 306,701 cf, Depth> 2.46"
Routed to Link AP4 : ANALYSIS POINT 4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 83,589 | 30 | Brush, Good, HSG A |
| 211,292 | 30 | Woods, Good, HSG A |
| 173,685 | 55 | Woods, Good, HSG B |
| 94,337 | 68 | 1 acre lots, 20\% imp, HSG B |
| 549 | 79 | 1 acre lots, 20\% imp, HSG C |
| 2,920 | 65 | Brush, Good, HSG C |
| 582,787 | 70 | Woods, Good, HSG C |
| 173,260 | 98 | Water Surface, 0\% imp, HSG D |
| 176,351 | 77 | Woods, Good, HSG D |

## Summary for Link AP1: ANALYSIS POINT 1



Primary outflow $=$ Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP2: ANALYSIS POINT 2

| Inflow | 956,300 sf, | 3.58\% Impervious, | " for |
| :---: | :---: | :---: | :---: |
| Inflow | 41.12 cfs @ | 12.61 hrs , Volume= | 270,829 cf |
| Primary | 41.12 cfs @ | 12.61 hrs, Volume= | 270,829 cf, Atten=0\%, Lag= 0.0 |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP3: ANALYSIS POINT 3

| Inflow Area $=$ | $105,094 \mathrm{sff}$ | $0.00 \%$ Impervious, | Inflow Depth > $3.23 "$ | for 25 YR event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $8.96 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $28,279 \mathrm{cf}$ |
| Primary | $=$ | $8.96 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $28,279 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP4: ANALYSIS POINT 4

Inflow Area $=1,498,770$ sf, $1.27 \%$ Impervious, Inflow Depth > 2.46" for 25YR event
Inflow $=56.04$ cfs @ 12.42 hrs, Volume $=\quad 306,701 \mathrm{cf}$
Primary $=56.04$ cfs @ 12.42 hrs , Volume $=306,701 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$
Primary outflow $=$ Inflow, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method
Subcatchment101S: SUMMER STREET Runoff Area=13,756 sf 64.57\% Impervious Runoff Depth>7.60" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=88$ Runoff $=2.59 \mathrm{cfs} 8,714 \mathrm{cf}$

Subcatchment 102S: OVERLANDTO Runoff Area=956,300 sf $3.58 \%$ Impervious Runoff Depth $>5.83$ " Flow Length=1,531' Tc=44.5 min CN=74 Runoff=70.33 cfs 464,971 cf

Subcatchment 103S: ISOLATED
Runoff Area $=105,094$ sf $0.00 \%$ Impervious Runoff Depth $>5.63$ " Tc=6.0 $\mathrm{min} \mathrm{CN}=72$ Runoff=15.56 cfs 49,317 cf

Subcatchment104S: REMAININGLAND Runoff Area=1,498,770 sf $1.27 \%$ Impervious Runoff Depth>4.62" Flow Length=987' Tc=28.6 min CN=64 Runoff=107.72 cfs 576,512 cf

## Link AP1: ANALYSISPOINT 1

## Link AP2: ANALYSISPOINT 2

Link AP3: ANALYSISPOINT 3

Link AP4: ANALYSISPOINT 4

Total Runoff Area $=2,573,920$ sf Runoff Volume $=1,099,514$ cf Average Runoff Depth $=5.13$ "

$$
97.59 \% \text { Pervious }=2,511,798 \mathrm{sf} \quad 2.41 \% \text { Impervious }=62,122 \mathrm{sf}
$$

Inflow=2.59 cfs 8,714 cf Primary $=2.59$ cfs 8,714 cf

Inflow=70.33 cfs 464,971 cf Primary $=70.33$ cfs 464,971 cf

Inflow=15.56 cfs 49,317 cf Primary $=15.56$ cfs 49,317 cf

Inflow=107.72 cfs 576,512 cf Primary $=107.72$ cfs 576,512 cf

## Summary for Subcatchment 101S: SUMMER STREET (SOUTH)

Runoff =
2.59 cfs @ 12.09 hrs, Volume=

8,714 cf, Depth> 7.60"
Routed to Link AP1 : ANALYSIS POINT 1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG B |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,612 | $\begin{aligned} & \hline 61 \\ & 98 \end{aligned}$ |  |  |  |
|  | 8,882 |  | $>75 \%$ Grass cover, Good, HSG B Paved parking, HSG B |  |  |
|  | 1,262 | 96 | Gravel surf | ce, HSG B |  |
|  | 13,756 | 88 | Weighted Average |  |  |
|  | 4,874 |  | 35.43\% Pervious Area |  |  |
|  | 8,882 |  | 64.57\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{r} \text { Length } \\ \text { (feet) } \\ \hline \end{array}$ | Slope $(\mathrm{ft} / \mathrm{ft})$ | $\begin{array}{r} \text { Velocity } \\ (\mathrm{ft} / \mathrm{sec}) \\ \hline \end{array}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment 102S: OVERLAND TO TRAIN TRACKS (EAST)

Runoff $=\quad 70.33$ cfs @ 12.60 hrs, Volume= 464,971 cf, Depth> 5.83"
Routed to Link AP2 : ANALYSIS POINT 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 175,943 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 184,742 | 55 | Woods, Good, HSG B |
| 24,401 | 98 | Paved parking, HSG B |
| 49,311 | 68 | 1 acre lots, 20\% imp, HSG B |
| 15,945 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 235,274 | 70 | Woods, Good, HSG C |
| 3,192 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 1,550 | 77 | Woods, Good, HSG D |
| 265,942 | 98 | Water Surface, 0\% imp, HSG D |
| 956,300 | 74 | Weighted Average |
| 922,037 |  | $96.42 \%$ Pervious Area |
| 34,263 |  | $3.58 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |
| 3.2 | 50 | 0.0800 | 0.26 | Sheet Flow, <br> Grass: Short $\mathrm{n}=0.150 \quad \mathrm{P} 2=3.27 "$ <br> 0.3 |
| 77 | 0.0780 | 4.50 | Shallow Concentrated Flow, |  |
| 41.0 | 1,404 | 0.0130 | 0.57 | Unpaved Kv=16.1 fps <br> Shallow Concentrated Flow, <br> Woodland Kv=5.0 fps |
| 44.5 | 1,531 | Total |  |  |

## Summary for Subcatchment 103S: ISOLATED WETLAND (NORTHEAST)

Runoff = 15.56 cfs @ 12.09 hrs, Volume= 49,317 cf, Depth> 5.63"
Routed to Link AP3 : ANALYSIS POINT 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


## Summary for Subcatchment 104S: REMAINING LAND (NORTH - RIVER)

Runoff $=107.72$ cfs @ 12.40 hrs, Volume= 576,512 cf, Depth> 4.62"
Routed to Link AP4 : ANALYSIS POINT 4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |  |
| ---: | ---: | :--- | :--- |
| 83,589 | 30 | Brush, Good, HSG A |  |
| 211,292 | 30 | Woods, Good, HSG A |  |
| 173,685 | 55 | Woods, Good, HSG B |  |
| 94,337 | 68 | 1 acre lots, 20\% imp, HSG B |  |
| 549 | 79 | 1 acre lots, 20\% imp, HSG C |  |
| 2,920 | 65 | Brush, Good, HSG C |  |
| 582,787 | 70 | Woods, Good, HSG C |  |
| 173,260 | 98 | Water Surface, 0\% imp, HSG D |  |
| 176,351 | 77 | Woods, Good, HSG D |  |

28.6987 Total

## Summary for Link AP1: ANALYSIS POINT 1



Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP2: ANALYSIS POINT 2

| Inflow | 956,300 sf, | 3.58\% Impervious, | 5.83" |
| :---: | :---: | :---: | :---: |
| Inflow | 70.33 cfs @ | 12.60 hrs , Volume= | 464,971 cf |
| Primary | 70.33 cfs @ | 12.60 hrs , Volume= | $464,971 \mathrm{cf}, \mathrm{Atten}=0 \%, \mathrm{Lag}=0.0$ |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP3: ANALYSIS POINT 3

| In | 105,094 sf, | 0.00\% Impervious, | w Depth > 5.63" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 15.56 cfs @ | 12.09 hrs , Volume= | 49,317 cf |
| Primary | 15.56 cfs @ | 12.09 hrs , Volume= | $49,317 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP4: ANALYSIS POINT 4

| Inflow Area $=$ | $1,498,770 \mathrm{sf}$, | $1.27 \%$ | Impervious, | Inflow Depth > $4.62 "$ | for 100 YR event |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $107.72 \mathrm{cfs} @$ | 12.40 hrs , Volume= | $576,512 \mathrm{cf}$ |  |
| Primary | $=$ | $107.72 \mathrm{cfs} @$ | 12.40 hrs , Volume $=$ | $576,512 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |  |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs


## 19097 Post-Development

Prepared by Howard Stein Hudson Associates
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## Project Notes

Rainfall events imported from "19097 PreDevelopment.hcp"
Rainfall events imported from "19097 PostDevelopment-prelim.hcp"

## 19097 Post-Development

Prepared by Howard Stein Hudson Associates
Printed 6/19/2023
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## Rainfall Events Listing

| Event\# | Event <br> Name | Storm Type | Curve | Mode | Duration <br> (hours) | B/B | Depth <br> (inches) |
| ---: | :--- | :--- | :--- | ---: | ---: | ---: | ---: |
| 1 | 2YR | Type III 24-hr | Default | 24.00 | 1 | 3.27 | 2 |
| 2 | 10YR | Type III 24-hr | Default | 24.00 | 1 | 4.96 | 2 |
| 3 | 25YR | Type III 24-hr | Default | 24.00 | 1 | 6.29 | 2 |
| 4 | 100YR | Type III 24-hr | Default | 24.00 | 1 | 9.06 | 2 |

## 19097 Post-Development

Prepared by Howard Stein Hudson Associates
Printed 6/19/2023
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## Area Listing (all nodes)

| $\begin{array}{r} \text { Area } \\ \text { (sq-ft) } \end{array}$ | CN | Description (subcatchment-numbers) |
| :---: | :---: | :---: |
| 17,568 | 51 | 1 acre lots, $20 \%$ imp, HSG A (S206) |
| 147,453 | 68 | 1 acre lots, 20\% imp, HSG B (C48, S206) |
| 40,742 | 39 | $\begin{aligned} & >75 \% \text { Grass cover, Good, HSG A (C14, C16, C21, C23, C25, C27, C53, C54, } \\ & \text { S206, S208, S209, S213, T2) } \end{aligned}$ |
| 258,351 | 61 | $\begin{aligned} & \text { >75\% Grass cover, Good, HSG B (C43, C44, C45, C46, C8, C9, S201, S202, } \\ & \text { S203, S204, S206, S211, S212) } \end{aligned}$ |
| 271,812 | 74 | >75\% Grass cover, Good, HSG C (C13, C14, C16, C17, C18, C20, C22, C23, C25, C28, C29, C30, C31, C32, C33, C34, C49, C50, C52, C53, C54, C9, CH1, S202, S203, S204, S205, S206, S207, S208, S209, S210, S212, S213, T1, T2, TH1, TH10, TH11, TH2, TH3, TH4, TH5, TH6, TH7, TH8, TH9) |
| 22,374 | 80 | >75\% Grass cover, Good, HSG D (C22, C23, C24, C25, C26, C27, C28, C35, C9, CH1, S202, S205, S206, S212, S213, T1) |
| 41,148 | 30 | Brush, Good, HSG A (S206) |
| 107,179 | 48 | Brush, Good, HSG B (S202, S204, S206, S212) |
| 66,752 | 65 | Brush, Good, HSG C (S202, S204, S205, S206, S210, S212) |
| 6,281 | 73 | Brush, Good, HSG D (S204, S205) |
| 46,900 | 98 | Paved parking, HSG A (C14, C15, C16, C20, C21, C22, C23, C25, C27, C53, C54, S206, S208, S209, S213, T2) |
| 39,770 | 98 | Paved parking, HSG B (C38, C39, C40, C41, C43, C44, C45, C46, S201, S202, S212) |
| 141,055 | 98 | Paved parking, HSG C (C13, C14, C15, C16, C17, C18, C20, C21, C22, C23, C25, C28, C29, C30, C31, C32, C33, C34, C35, C36, C38, C39, C51, C52, C53, C54, C9, S202, S204, S205, S206, S208, S209, S212, S213, T1, T2) |
| 41,121 | 98 | Paved parking, HSG D (C22, C23, C24, C25, C26, C27, C28, C35, C38, C39, C9, S202, S204, S205, S212, S213, T1) |
| 17,148 | 98 | Paved roads w/curbs \& sewers, HSG B (C10, C47, C48, C7, C8, C9) |
| 34,156 | 98 | Paved roads w/curbs \& sewers, HSG C (C10, C11, C12, C49, C50) |
| 919 | 98 | Paved roads w/curbs \& sewers, HSG D (C10) |
| 8,649 | 98 | Roofs, HSG A (B2, B3, S213) |
| 14,884 | 98 | Roofs, HSG B (MB1, S202) |
| 110,093 | 98 | Roofs, HSG C (B1, B2, B3, C13, C29, C30, C31, C32, C33, C34, C51, C9, CH1, S213, TH1, TH10, TH11, TH2, TH3, TH4, TH5, TH6, TH7, TH8, TH9) |
| 3,102 | 98 | Roofs, HSG D (B1, CH1) |
| 1,371 | 98 | Water Surface, 0\% imp, HSG A (S207, S213) |
| 1,060 | 98 | Water Surface, 0\% imp, HSG B (S203) |
| 29,784 | 98 | Water Surface, 0\% imp, HSG C (S203, S207, S210, S213) |
| 171,979 | 98 | Water Surface, 0\% imp, HSG D (S206, S209, S212) |
| 7,328 | 98 | Water Surface, HSG B (S211) |
| 271,794 | 98 | Water Surface, HSG D (S202, S204, S205) |
| 145,962 | 30 | Woods, Good, HSG A (S205, S206, S209) |
| 123,002 | 55 | Woods, Good, HSG B (S202, S204, S206, S212) |

## 19097 Post-Development

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## Area Listing (all nodes) (continued)

| Area <br> $(\mathrm{sq-ft})$ | CN | Description <br> (subcatchment-numbers) |
| ---: | :--- | :--- |
| 232,021 | 70 | Woods, Good, HSG C (S202, S204, S205, S206, S209, S212) |
| 152,162 | 77 | Woods, Good, HSG D (S204, S205, S206) |
| $\mathbf{2 , 5 7 3 , 9 2 0}$ | $\mathbf{7 5}$ | TOTAL AREA |

## 19097 Post-Development

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## Soil Listing (all nodes)

| $\begin{array}{r} \text { Area } \\ \text { (sq-ft) } \end{array}$ | Soil Group | Subcatchment Numbers |
| :---: | :---: | :---: |
| 302,340 | HSG A | B2, B3, C14, C15, C16, C20, C21, C22, C23, C25, C27, C53, C54, S205, S206, S207, S208, S209, S213, T2 |
| 716,175 | HSG B | C10, C38, C39, C40, C41, C43, C44, C45, C46, C47, C48, C7, C8, C9, MB1, S201, S202, S203, S204, S206, S211, S212 |
| 885,673 | HSG C | B1, B2, B3, C10, C11, C12, C13, C14, C15, C16, C17, C18, C20, C21, C22, C23, C25, C28, C29, C30, C31, C32, C33, C34, C35, C36, C38, C39, C49, C50, C51, C52, C53, C54, C9, CH1, S202, S203, S204, S205, S206, S207, S208, S209, S210, S212, S213, T1, T2, TH1, TH10, TH11, TH2, TH3, TH4, TH5, TH6, TH7, TH8, TH9 |
| 669,732 | HSG D | B1, C10, C22, C23, C24, C25, C26, C27, C28, C35, C38, C39, C9, CH1, S202, S204, S205, S206, S209, S212, S213, T1 |
| 0 | Other |  |
| 2,573,920 |  | TOTAL AREA |

Pipe Listing (all nodes)

| Line\# | Node Number | In-Invert (feet) | Out-Invert <br> (feet) | Length (feet) | Slope <br> (ft/ft) | n | Width (inches (inches) | Diam/Height (inches) | Inside-Fill (inches) | Node <br> Name |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 11R | 194.00 | 193.55 | 30.0 | 0.0150 | 0.069 | 48.0 | 48.0 | 0.0 |  |
| 2 | SC1 | 206.37 | 205.51 | 43.1 | 0.0200 | 0.030 | 192.0 | 60.0 | 0.0 |  |
| 3 | SC2 | 208.52 | 207.64 | 36.5 | 0.0241 | 0.030 | 192.0 | 60.0 | 0.0 |  |
| 4 | 1P | 205.50 | 204.33 | 46.7 | 0.0251 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 5 | 5R | 197.22 | 196.50 | 36.0 | 0.0200 | 0.012 | 0.0 | 8.0 | 0.0 |  |
| 6 | 11P | 203.25 | 202.94 | 61.0 | 0.0051 | 0.012 | 0.0 | 12.0 | 0.0 |  |
| 7 | CB10 | 209.76 | 209.59 | 33.8 | 0.0050 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 8 | CB11 | 209.94 | 209.67 | 26.3 | 0.0103 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 9 | CB12 | 206.68 | 205.65 | 41.3 | 0.0249 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 10 | CB13 | 206.70 | 205.61 | 43.7 | 0.0249 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 11 | CB14 | 200.79 | 200.67 | 23.2 | 0.0052 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 12 | CB15 | 200.79 | 200.71 | 15.6 | 0.0051 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 13 | CB16 | 203.47 | 203.33 | 20.9 | 0.0067 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 14 | CB17 | 204.99 | 204.86 | 13.8 | 0.0094 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 15 | CB18 | 204.72 | 204.59 | 25.1 | 0.0052 | 0.013 | 0.0 | 15.0 | 0.0 |  |
| 16 | CB20 | 203.97 | 203.81 | 30.3 | 0.0053 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 17 | CB21 | 204.32 | 204.19 | 26.0 | 0.0050 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 18 | CB22 | 205.33 | 205.25 | 16.1 | 0.0050 | 0.012 | 0.0 | 12.0 | 0.0 |  |
| 19 | CB23 | 205.41 | 205.32 | 16.3 | 0.0055 | 0.012 | 0.0 | 12.0 | 0.0 |  |
| 20 | CB24 | 205.21 | 205.15 | 12.1 | 0.0050 | 0.012 | 0.0 | 12.0 | 0.0 |  |
| 21 | CB25 | 205.22 | 205.16 | 11.4 | 0.0053 | 0.012 | 0.0 | 15.0 | 0.0 |  |
| 22 | CB26 | 201.77 | 201.55 | 42.5 | 0.0052 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 23 | CB27 | 201.00 | 200.90 | 18.0 | 0.0056 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 24 | CB28 | 197.75 | 197.69 | 13.7 | 0.0044 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 25 | CB29 | 205.38 | 205.31 | 13.5 | 0.0052 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 26 | CB30 | 205.38 | 205.29 | 17.5 | 0.0051 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 27 | CB31 | 204.19 | 204.11 | 16.4 | 0.0049 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 28 | CB32 | 204.19 | 204.11 | 16.3 | 0.0049 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 29 | CB33 | 205.28 | 205.22 | 11.7 | 0.0051 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 30 | CB34 | 205.21 | 205.13 | 16.5 | 0.0048 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 31 | CB35 | 207.04 | 206.96 | 15.2 | 0.0053 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 32 | CB36 | 207.04 | 206.96 | 16.1 | 0.0050 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 33 | CB38 | 209.69 | 209.61 | 16.7 | 0.0048 | 0.012 | 0.0 | 12.0 | 0.0 |  |
| 34 | CB39 | 209.69 | 209.61 | 16.4 | 0.0049 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 35 | CB40 | 213.68 | 213.55 | 17.8 | 0.0073 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 36 | CB41 | 213.89 | 213.80 | 18.4 | 0.0049 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 37 | CB43 | 220.00 | 219.93 | 14.9 | 0.0047 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 38 | CB44 | 220.00 | 219.93 | 14.9 | 0.0047 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 39 | CB45 | 221.29 | 221.20 | 18.2 | 0.0049 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 40 | CB46 | 221.53 | 221.45 | 15.3 | 0.0052 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 41 | CB47 | 225.05 | 224.95 | 20.9 | 0.0048 | 0.012 | 0.0 | 12.0 | 0.0 |  |

Pipe Listing (all nodes) (continued)

| Line\# | Node Number | In-Invert (feet) | Out-Invert (feet) | Length (feet) | Slope (ft/ft) | n | Width (ind (inches) | Diam/Height (inches) | Inside-Fill (inches) | Node <br> Name |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 42 | CB48 | 224.82 | 224.74 | 16.9 | 0.0047 | 0.012 | 0.0 | 15.0 | 0.0 |  |
| 43 | CB49 | 202.76 | 202.68 | 15.5 | 0.0052 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 44 | CB50 | 202.78 | 202.70 | 15.3 | 0.0052 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 45 | CB51 | 202.35 | 202.19 | 31.4 | 0.0051 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 46 | CB52 | 202.68 | 202.55 | 25.5 | 0.0051 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 47 | CB53 | 202.78 | 202.62 | 32.0 | 0.0050 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 48 | CB54 | 202.66 | 202.48 | 36.7 | 0.0049 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 49 | CB7 | 212.60 | 212.45 | 15.1 | 0.0099 | 0.012 | 0.0 | 12.0 | 0.0 |  |
| 50 | CB8 | 213.79 | 213.64 | 15.1 | 0.0099 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 51 | CB9 | 210.10 | 209.71 | 19.9 | 0.0196 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 52 | D10 | 203.33 | 203.25 | 15.6 | 0.0051 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 53 | D11 | 204.25 | 204.03 | 44.6 | 0.0049 | 0.013 | 0.0 | 18.0 | 0.0 |  |
| 54 | D12 | 203.21 | 203.00 | 41.9 | 0.0050 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 55 | D13 | 201.95 | 201.65 | 60.1 | 0.0050 | 0.013 | 0.0 | 24.0 | 0.0 |  |
| 56 | D14 | 204.13 | 202.85 | 256.3 | 0.0050 | 0.012 | 0.0 | 18.0 | 0.0 |  |
| 57 | D16 | 204.90 | 204.38 | 103.5 | 0.0050 | 0.012 | 0.0 | 15.0 | 0.0 |  |
| 58 | D17 | 200.55 | 197.69 | 91.6 | 0.0312 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 59 | D18 | 197.44 | 196.98 | 46.3 | 0.0099 | 0.013 | 0.0 | 15.0 | 0.0 |  |
| 60 | D19 | 205.19 | 204.43 | 82.5 | 0.0092 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 61 | D2 | 206.90 | 206.52 | 38.2 | 0.0099 | 0.013 | 0.0 | 15.0 | 0.0 |  |
| 62 | D20 | 204.19 | 203.87 | 63.5 | 0.0050 | 0.013 | 0.0 | 15.0 | 0.0 |  |
| 63 | D21 | 203.02 | 202.66 | 72.4 | 0.0050 | 0.013 | 0.0 | 24.0 | 0.0 |  |
| 64 | D22 | 204.87 | 203.92 | 134.2 | 0.0071 | 0.013 | 0.0 | 15.0 | 0.0 |  |
| 65 | D23 | 206.70 | 204.97 | 173.3 | 0.0100 | 0.013 | 0.0 | 15.0 | 0.0 |  |
| 66 | D25 | 209.36 | 208.17 | 237.6 | 0.0050 | 0.012 | 0.0 | 15.0 | 0.0 |  |
| 67 | D27 | 213.34 | 212.38 | 63.9 | 0.0150 | 0.012 | 0.0 | 15.0 | 0.0 |  |
| 68 | D28 | 217.46 | 214.29 | 158.3 | 0.0200 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 69 | D29 | 219.83 | 217.55 | 150.9 | 0.0151 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 70 | D30 | 220.92 | 220.00 | 184.2 | 0.0050 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 71 | D31 | 224.63 | 213.09 | 288.5 | 0.0400 | 0.012 | 0.0 | 15.0 | 0.0 |  |
| 72 | D34 | 198.07 | 197.03 | 52.0 | 0.0200 | 0.012 | 0.0 | 12.0 | 0.0 |  |
| 73 | D35 | 212.28 | 209.71 | 171.5 | 0.0150 | 0.012 | 0.0 | 15.0 | 0.0 |  |
| 74 | D4 | 210.34 | 207.01 | 222.3 | 0.0150 | 0.012 | 0.0 | 15.0 | 0.0 |  |
| 75 | D5 | 209.09 | 208.17 | 183.0 | 0.0050 | 0.013 | 0.0 | 18.0 | 0.0 |  |
| 76 | D6 | 208.07 | 206.57 | 299.7 | 0.0050 | 0.013 | 0.0 | 18.0 | 0.0 |  |
| 77 | D7 | 206.47 | 204.04 | 44.2 | 0.0550 | 0.013 | 0.0 | 18.0 | 0.0 |  |
| 78 | D8 | 200.57 | 200.13 | 87.7 | 0.0050 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 79 | D9 | 200.03 | 199.97 | 11.9 | 0.0050 | 0.013 | 0.0 | 12.0 | 0.0 |  |
| 80 | DE61 | 212.70 | 212.65 | 10.0 | 0.0050 | 0.013 | 0.0 | 6.0 | 0.0 |  |
| 81 | DE62 | 212.70 | 212.65 | 10.0 | 0.0050 | 0.013 | 0.0 | 6.0 | 0.0 |  |
| 82 | DE63 | 207.50 | 207.45 | 10.0 | 0.0050 | 0.013 | 0.0 | 6.0 | 0.0 |  |

## 19097 Post-Development

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Pipe Listing (all nodes) (continued)

| Line\# | Node <br> Number | In-Invert <br> (feet) | Out-Invert <br> (feet) | Length <br> (feet) | Slope <br> (ft/ft) |  | Width <br> (inches) | Diam/Height <br> (inches) | Inside-Fill <br> (inches) |
| :--- | :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Node <br> Name |  |  |  |  |  |  |  |  |  |
| 83 | DE64 | 205.50 | 205.45 | 10.0 | 0.0050 | 0.013 | 0.0 | 6.0 | 0.0 |
| 84 | DE65 | 206.50 | 206.45 | 10.0 | 0.0050 | 0.013 | 0.0 | 6.0 | 0.0 |
| 85 | DE66 | 208.30 | 208.25 | 10.0 | 0.0050 | 0.013 | 0.0 | 6.0 | 0.0 |
| 86 | DE67 | 208.50 | 208.45 | 10.0 | 0.0050 | 0.013 | 0.0 | 6.0 | 0.0 |
| 87 | DE68 | 207.50 | 206.00 | 20.0 | 0.0750 | 0.013 | 0.0 | 6.0 | 0.0 |
| 88 | DE69 | 206.00 | 205.95 | 10.0 | 0.0050 | 0.013 | 0.0 | 6.0 | 0.0 |
| 89 | DE70 | 206.40 | 206.35 | 10.0 | 0.0050 | 0.013 | 0.0 | 6.0 | 0.0 |
| 90 | DE71 | 207.00 | 206.95 | 10.0 | 0.0050 | 0.013 | 0.0 | 6.0 | 0.0 |
| 91 | DECH | 208.50 | 205.10 | 80.0 | 0.0425 | 0.013 | 0.0 | 4.0 | 0.0 |
| 92 | DMH32 | 202.59 | 201.57 | 19.2 | 0.0531 | 0.013 | 0.0 | 12.0 | 0.0 |
| 93 | P204 | 202.75 | 201.00 | 35.0 | 0.0500 | 0.012 | 0.0 | 15.0 | 0.0 |
| 94 | P206 | 194.65 | 194.50 | 30.0 | 0.0050 | 0.013 | 0.0 | 18.0 | 0.0 |
| 95 | P207 | 194.75 | 194.55 | 40.0 | 0.0050 | 0.012 | 0.0 | 15.0 | 0.0 |
| 96 | P210 | 202.25 | 202.03 | 44.0 | 0.0050 | 0.013 | 0.0 | 12.0 | 0.0 |
| 97 | P213 | 202.30 | 202.00 | 60.0 | 0.0050 | 0.013 | 0.0 | 12.0 | 0.0 |
| 98 | P214 | 201.00 | 200.88 | 25.0 | 0.0048 | 0.013 | 0.0 | 12.0 | 0.0 |

Time span=0.00-24.00 hrs, $\mathrm{dt}=0.05 \mathrm{hrs}, 481$ points $\times 3$
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentB1: MULTIFAMILYBLDG

## SubcatchmentB2: MULTIFAMILYBLDG

SubcatchmentB3: MULTIFAMILY

SubcatchmentC10: CB \#10

SubcatchmentC11: CB \#11

SubcatchmentC12: CB \#12

SubcatchmentC13: CB \#13

SubcatchmentC14: CB \#14

SubcatchmentC15: CB \#15

SubcatchmentC16: CB \#16

SubcatchmentC17: CB \#17

SubcatchmentC18: CB \#18

SubcatchmentC20: CB \#20

SubcatchmentC21: CB \#21

SubcatchmentC22: CB \#22

SubcatchmentC23: CB \#23

Runoff Area=23,255 sf $100.00 \%$ Impervious Runoff Depth $>3.04$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=98$ Runoff $=1.66 \mathrm{cfs} 5,882 \mathrm{cf}$

Runoff Area=17,561 sf $100.00 \%$ Impervious Runoff Depth $>3.04$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=1.25 cfs $4,442 \mathrm{cf}$

Runoff Area=19,981 sf 100.00\% Impervious Runoff Depth>3.04" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=1.42 cfs $5,054 \mathrm{cf}$

Runoff Area $=6,961$ sf $100.00 \%$ Impervious Runoff Depth $>3.04$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.50 \mathrm{cfs} 1,761 \mathrm{cf}$

Runoff Area $=7,173$ sf $100.00 \%$ Impervious Runoff Depth $>3.04$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=0.51 cfs $1,814 \mathrm{cf}$

Runoff Area $=5,238$ sf $100.00 \%$ Impervious Runoff Depth $>3.04$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=0.37cfs $1,325 \mathrm{cf}$

Runoff Area=10,873 sf $90.78 \%$ Impervious Runoff Depth $>2.82$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.75 \mathrm{cfs} 2,552 \mathrm{cf}$

Runoff Area=12,099 sf $86.22 \%$ Impervious Runoff Depth $>2.32$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=91$ Runoff=0.73 cfs $2,341 \mathrm{cf}$

Runoff Area $=6,666$ sf $100.00 \%$ Impervious Runoff Depth $>3.04$ " $\mathrm{T}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.47 \mathrm{cfs} 1,686 \mathrm{cf}$

Runoff Area $=8,516$ sf $64.88 \%$ Impervious Runoff Depth $>1.39$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=79$ Runoff $=0.31 \mathrm{cfs} 985 \mathrm{cf}$

Runoff Area $=11,836$ sf $73.87 \%$ Impervious Runoff Depth $>2.42$ " Tc=6.0 $\mathrm{min} \mathrm{CN}=92$ Runoff=0.73 cfs 2,382 cf

Runoff Area=18,591 sf $66.35 \%$ Impervious Runoff Depth>2.23" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=90$ Runoff $=1.08 \mathrm{cfs} 3,458 \mathrm{cf}$

Runoff Area $=11,939$ sf $88.95 \%$ Impervious Runoff Depth $>2.71^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff $=0.80 \mathrm{cfs} 2,698 \mathrm{cf}$

Runoff Area $=10,174 \mathrm{sf} \mathbf{8 7 . 0 4 \%}$ Impervious Runoff Depth $>2.23$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=90$ Runoff $=0.59 \mathrm{cfs} 1,892 \mathrm{cf}$

Runoff Area $=12,001$ sf $91.62 \%$ Impervious Runoff Depth $>2.82$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.83 \mathrm{cfs} 2,817 \mathrm{cf}$

Runoff Area=9,694 sf $61.00 \%$ Impervious Runoff Depth $>2.14$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=89$ Runoff $=0.54 \mathrm{cfs} 1,732 \mathrm{cf}$

SubcatchmentC24: CB \#24

## SubcatchmentC25: CB \#25

SubcatchmentC26: CB \#26

SubcatchmentC27: CB \#27

SubcatchmentC28: CB \#28

SubcatchmentC29: CB \#29

SubcatchmentC30: CB \#30

SubcatchmentC31: CB \#31

SubcatchmentC32: CB \#32

SubcatchmentC33: CB \#33

SubcatchmentC34: CB \#34

SubcatchmentC35: CB \#35

SubcatchmentC36: CB \#36

SubcatchmentC38: CB \#38

SubcatchmentC39: CB \#39

SubcatchmentC40: CB \#40

SubcatchmentC41: CB \#41

Runoff Area=7,930 sf 72.16\% Impervious Runoff Depth>2.51" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff $=0.51 \mathrm{cfs} 1,660 \mathrm{cf}$

Runoff Area $=8,487$ sf $80.92 \%$ Impervious Runoff Depth $>2.61$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff $=0.56 \mathrm{cfs} 1,846 \mathrm{cf}$

Runoff Area $=8,835$ sf $63.75 \%$ Impervious Runoff Depth $>2.32$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=91$ Runoff $=0.53 \mathrm{cfs} 1,710 \mathrm{cf}$

Runoff Area=6,111 sf $91.90 \%$ Impervious Runoff Depth>2.82" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.42 \mathrm{cfs} 1,434 \mathrm{cf}$

Runoff Area=10,372 sf $51.33 \%$ Impervious Runoff Depth $>2.06$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=88$ Runoff $=0.56 \mathrm{cfs} 1,779 \mathrm{cf}$

Runoff Area=8,495 sf $84.21 \%$ Impervious Runoff Depth $>2.61$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff $=0.56 \mathrm{cfs} 1,848 \mathrm{cf}$

Runoff Area=8,933 sf $82.40 \%$ Impervious Runoff Depth $>2.61$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff=$=0.59 \mathrm{cfs} 1,943 \mathrm{cf}$

Runoff Area=16,365 sf $68.64 \%$ Impervious Runoff Depth>2.23" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=90$ Runoff $=0.95 \mathrm{cfs} 3,044 \mathrm{cf}$

Runoff Area $=12,710$ sf $70.47 \%$ Impervious Runoff Depth $>2.32$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=91$ Runoff $=0.76 \mathrm{cfs} 2,460 \mathrm{cf}$

Runoff Area=5,421 sf $83.90 \%$ Impervious Runoff Depth>2.61" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff $=0.36 \mathrm{cfs} 1,179 \mathrm{cf}$

Runoff Area=8,622 sf $80.51 \%$ Impervious Runoff Depth $>2.51$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff $=0.55 \mathrm{cfs} 1,804 \mathrm{cf}$

Runoff Area=4,149 sf $98.10 \%$ Impervious Runoff Depth $>3.04$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.30 \mathrm{cfs} 1,049 \mathrm{cf}$

Runoff Area $=6,622$ sf $100.00 \%$ Impervious Runoff Depth>3.04" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=$=0.47 \mathrm{cfs} 1,675 \mathrm{cf}$

Runoff Area $=7,637$ sf $100.00 \%$ Impervious Runoff Depth $>3.04$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.54 \mathrm{cfs} 1,932 \mathrm{cf}$

Runoff Area $=7,612$ sf $100.00 \%$ Impervious Runoff Depth $>3.04$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.54 \mathrm{cfs} 1,925 \mathrm{cf}$

Runoff Area $=4,211$ sf $100.00 \%$ Impervious Runoff Depth $>3.04$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.30 \mathrm{cfs} 1,065 \mathrm{cf}$

Runoff Area $=5,586$ sf $100.00 \%$ Impervious Runoff Depth>3.04" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.40 \mathrm{cfs} 1,413 \mathrm{cf}$

SubcatchmentC43: CB \#43

## SubcatchmentC44: CB \#44

SubcatchmentC45: CB \#45

SubcatchmentC46: CB \#46

SubcatchmentC47: CB \#47

SubcatchmentC48: CB \#48

SubcatchmentC49: CB \#49

SubcatchmentC50: CB \#50

SubcatchmentC51: CB \#51

SubcatchmentC52: CB\#52

SubcatchmentC53: CB \#53

SubcatchmentC54: CB \#54

SubcatchmentC7: CB \#5

SubcatchmentC8: CB \#8

SubcatchmentC9: CB \#9

SubcatchmentCH1: CLUBHOUSE

SubcatchmentMB1: MAIL KIOSK

Runoff Area=3,109 sf 75.36\% Impervious Runoff Depth>2.14" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=89$ Runoff=0.17cfs 555 cf

Runoff Area=1,978 sf $84.43 \%$ Impervious Runoff Depth $>2.42$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=92$ Runoff=0.12 cfs 398 cf

Runoff Area=2,465 sf $50.30 \%$ Impervious Runoff Depth $>1.45$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=80$ Runoff=0.09 cfs 299 cf

Runoff Area=4,397 sf $50.97 \%$ Impervious Runoff Depth>1.45" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=80$ Runoff $=0.17 \mathrm{cfs} 533 \mathrm{cf}$

Runoff Area=3,012 sf 100.00\% Impervious Runoff Depth>3.04" Tc=6.0 min CN=98 Runoff=0.21 cfs 762 cf

Runoff Area=60,128 sf 25.16\% Impervious Runoff Depth>0.87" Flow Length=400' Tc=11.8 min CN=70 Runoff=1.03 cfs $4,342 \mathrm{cf}$

Runoff Area=5,238 sf $84.59 \%$ Impervious Runoff Depth $>2.61$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff=0.34 cfs $1,139 \mathrm{cf}$

Runoff Area=15,040 sf $77.20 \%$ Impervious Runoff Depth $>2.51^{\prime \prime}$ Tc=6.0 min CN=93 Runoff=0.96 cfs $3,147 \mathrm{cf}$

Runoff Area=6,823 sf 100.00\% Impervious Runoff Depth>3.04" Tc=6.0 min $\quad \mathrm{CN}=98$ Runoff=0.49 cfs 1,726 cf

Runoff Area=9,052 sf 87.14\% Impervious Runoff Depth>2.71" Tc=6.0 min CN=95 Runoff=0.61 cfs 2,045 cf

Runoff Area=7,863 sf $86.52 \%$ Impervious Runoff Depth $>2.51^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff=0.50 cfs 1,646 cf

Runoff Area=4,821 sf $86.85 \%$ Impervious Runoff Depth>2.32" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=91$ Runoff=0.29 cfs 933 cf

Runoff Area=4,650 sf 100.00\% Impervious Runoff Depth>3.04" Tc=6.0 min $C N=98$ Runoff $=0.33 \mathrm{cfs} 1,176 \mathrm{cf}$

Runoff Area=5,450 sf $88.75 \%$ Impervious Runoff Depth>2.61" Tc=6.0 min CN=94 Runoff=0.36 cfs $1,185 \mathrm{cf}$

Runoff Area=16,307 sf 93.95\% Impervious Runoff Depth>2.92" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=97$ Runoff=1.14 cfs $3,974 \mathrm{cf}$

Runoff Area=6,262 sf 92.70\% Impervious Runoff Depth>2.82" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=96$ Runoff $=0.43 \mathrm{cfs} 1,470 \mathrm{cf}$

Runoff Area=938 sf 100.00\% Impervious Runoff Depth>3.04" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=0.07 cfs 237 cf

SubcatchmentS201: SUMMERSTREET Runoff Area=9,943 sf 92.79\% Impervious Runoff Depth>2.71" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff $=0.67 \mathrm{cfs} 2,247 \mathrm{cf}$

SubcatchmentS202: EXISTING WETLANDRunoff Area=432,269 sf 42.08\% Impervious Runoff Depth>1.19" Flow Length=856' Tc=23.2 $\mathrm{min} \mathrm{CN}=76$ Runoff=8.51 cfs $43,021 \mathrm{cf}$

SubcatchmentS203: POCKET WETLAND\#1 Runoff Area=25,587 sf $0.00 \%$ Impervious Runoff Depth $>0.97$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=72$ Runoff $=0.61 \mathrm{cfs} 2,073 \mathrm{cf}$

## SubcatchmentS204: EXISTING

Runoff Area=308,203 sf $31.07 \%$ Impervious Runoff Depth $>1.14$ " Flow Length=632' Tc=22.6 min CN=75 Runoff=5.78 cfs 29,158 cf

SubcatchmentS205: ISOLATEDWETLANDRunoff Area=55,420 sf $16.57 \%$ Impervious Runoff Depth>0.87" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=70$ Runoff=1.15 cfs $4,009 \mathrm{cf}$

SubcatchmentS206: OVERLANDFLOW Runoff Area=891,295 sf $2.91 \%$ Impervious Runoff Depth>0.63" Flow Length=1,467' Tc=34.5 $\mathrm{min} \quad \mathrm{CN}=65$ Runoff=6.52 cfs $46,602 \mathrm{cf}$

SubcatchmentS207:INFILTRATIONPOND Runoff Area=20,803 sf $0.00 \%$ Impervious Runoff Depth $>2.06$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=88$ Runoff=1.12 cfs $3,568 \mathrm{cf}$

SubcatchmentS208: GRASS AREA Runoff Area=13,760 sf $9.33 \%$ Impervious Runoff Depth>1.14" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=75$ Runoff=0.40 cfs $1,308 \mathrm{cf}$

## SubcatchmentS209: WETLANDC Runoff Area=107,073 sf $0.38 \%$ Impervious Runoff Depth>1.02"

Flow Length=607' Slope=0.0150 '/' Tc=28.9 min CN=73 Runoff=1.60 cfs 9,101 cf
SubcatchmentS210: INFILTRATIONPOND Runoff Area=75,890 sf 0.00\% Impervious Runoff Depth>1.32" Flow Length $=580$ ' Slope $=0.0150$ '/' Tc=16.5 $\mathrm{min} \quad \mathrm{CN}=78$ Runoff $=1.92 \mathrm{cfs} 8,348 \mathrm{cf}$

## SubcatchmentS211: S211

SubcatchmentS212: SWALE

## SubcatchmentS213: COURTYARD

## SubcatchmentT1: Trench Drain 1

## SubcatchmentT2: Drive Under B2

SubcatchmentTH1: TOWN HOUSE\#1

SubcatchmentTH10: TOWN HOUSE \#10

Runoff Area $=15,436$ sf $47.47 \%$ Impervious Runoff Depth $>1.39$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=79$ Runoff=0.56 cfs $1,786 \mathrm{cf}$

Runoff Area=52,768 sf $0.60 \%$ Impervious Runoff Depth $>0.63$ " Flow Length=418' Tc=23.1 min CN=65 Runoff=0.46 cfs $2,770 \mathrm{cf}$

Runoff Area=21,407 sf $48.10 \%$ Impervious Runoff Depth $>1.39$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=79$ Runoff $=0.78 \mathrm{cfs} 2,476 \mathrm{cf}$

Runoff Area $=11,173$ sf $75.10 \%$ Impervious Runoff Depth $>2.51$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff=0.71 cfs 2,338 cf

Runoff Area=4,445 sf $64.30 \%$ Impervious Runoff Depth $>1.32$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=78$ Runoff $=0.15 \mathrm{cfs} 490 \mathrm{cf}$

Runoff Area=4,247 sf $92.68 \%$ Impervious Runoff Depth $>2.82$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.29 \mathrm{cfs} 997 \mathrm{cf}$

Runoff Area=3,476 sf $91.60 \%$ Impervious Runoff Depth>2.82" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.24 \mathrm{cfs} 816 \mathrm{cf}$
Prepared by Howard Stein Hudson Associates Printed 6/19/2023

## Subcatchment TH2: TOWN HOUSE \#2

SubcatchmentTH3: TOWN HOUSE\#3

Subcatchment TH4: TOWN HOUSE\#4

SubcatchmentTH5: TOWN HOUSE\#5

SubcatchmentTH6: TOWN HOUSE \#6

SubcatchmentTH7: TOWN HOUSE\#7

SubcatchmentTH8: TOWN HOUSE \#8

SubcatchmentTH9: TOWN HOUSE \#9

Runoff Area=4,247 sf $92.68 \%$ Impervious Runoff Depth $>2.82$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.29 \mathrm{cfs} 997 \mathrm{cf}$

Runoff Area=3,013 sf $88.68 \%$ Impervious Runoff Depth $>2.71^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff $=0.20 \mathrm{cfs} 681 \mathrm{cf}$

Runoff Area=3,470 sf $91.59 \%$ Impervious Runoff Depth>2.82" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.24 \mathrm{cfs} 814 \mathrm{cf}$

Runoff Area=3,016 sf $88.69 \%$ Impervious Runoff Depth $>2.71^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff $=0.20 \mathrm{cfs} 682 \mathrm{cf}$

Runoff Area=3,407 sf $91.46 \%$ Impervious Runoff Depth $>2.82$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=96$ Runoff $=0.23 \mathrm{cfs} 800 \mathrm{cf}$

Runoff Area=3,481 sf $91.61 \%$ Impervious Runoff Depth $>2.82$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=96$ Runoff $=0.24 \mathrm{cfs} 817 \mathrm{cf}$

Runoff Area=4,212 sf $92.62 \%$ Impervious Runoff Depth $>2.82$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.29 \mathrm{cfs} 989 \mathrm{cf}$

Runoff Area $=3,480$ sf $91.61 \%$ Impervious Runoff Depth $>2.82$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.24 \mathrm{cfs} 817 \mathrm{cf}$

Reach 8R: OVERLANDFLOW Avg. Flow Depth=0.04' Max Vel=0.06 fps Inflow=0.69 cfs $1,940 \mathrm{cf}$ $\mathrm{n}=0.400 \mathrm{~L}=563.0$ ' $\mathrm{S}=0.0213$ '/' Capacity=28.09 cfs Outflow=0.11 cfs $1,734 \mathrm{cf}$

Reach 9R: OVERLANDFLOW Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0 cf $\mathrm{n}=0.400 \mathrm{~L}=211 . \mathbf{0}^{\prime} \quad \mathrm{S}=0.0652^{\prime} / / \quad$ Capacity=23.45 cfs Outflow=$=0.00 \mathrm{cfs} 0 \mathrm{cf}$

Reach 10R: OVERLANDFLOW Avg. Flow Depth=0.02' Max Vel=0.05 fps Inflow=0.04 cfs 101 cf n=0.400 L=164.0' S=0.0366 '// Capacity=17.57 cfs Outflow=0.02 cfs 101 cf

Reach 11R: 4x4 Open Bottom Culvert Avg. Flow Depth=0.36' Max Vel=1.19 fps Inflow=1.70 cfs $14,965 \mathrm{cf}$ 48.0 " $48.0^{\prime \prime}$ Box Pipe $n=0.069$ L=30.0' $S=0.0150$ '/' Capacity=42.20 cfs Outflow=1.70 cfs $14,959 \mathrm{cf}$

Reach 12R: OVERLANDFLOW Avg. Flow Depth=0.06' Max Vel=0.09 fps Inflow=0.77 cfs 2,134 cf $\mathrm{n}=0.400 \mathrm{~L}=250.0^{\prime} \quad \mathrm{S}=0.0240 \mathrm{I} / \mathrm{Capacity=} 29.80 \mathrm{cfs}$ Outflow=0.30 cfs $2,085 \mathrm{cf}$

Reach 14R: OVERLANDFLOW Avg. Flow Depth=0.03' Max Vel=0.05 fps Inflow=0.46 cfs 2,770 cf $\mathrm{n}=0.400 \mathrm{~L}=852.0^{\prime} \quad \mathrm{S}=0.0246$ '/' Capacity $=31.55 \mathrm{cfs} \quad$ Outflow=0.07 cfs $2,124 \mathrm{cf}$

Reach 15R: OVERLANDFLOW Avg. Flow Depth=0.04' Max Vel=0.06 fps Inflow=0.12 cfs $4,736 \mathrm{cf}$ $\mathrm{n}=0.400 \mathrm{~L}=300.0^{\prime} \quad \mathrm{S}=0.0200$ '/' Capacity=27.21 cfs Outflow=0.12 cfs $4,228 \mathrm{cf}$

Reach 18R: OVERLANDFLOW
Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0 cf $\mathrm{n}=0.400 \mathrm{~L}=609.0$ S=0.0279 '/' Capacity=38.42 cfs Outflow=0.00 cfs 0 cf

Reach 20R: OVERLANDFLOW Avg. Flow Depth=0.09' Max Vel=0.07 fps Inflow=0.93 cfs $5,781 \mathrm{cf}$ $\mathrm{n}=0.400 \mathrm{~L}=560.0$ ' $\mathrm{S}=0.0093$ '/' Capacity=18.54 cfs Outflow=0.34 cfs $5,373 \mathrm{cf}$

Reach 23R: OVERLANDFLOW Avg. Flow Depth=0.16' Max Vel=0.15 fps Inflow=1.70 cfs 14,959 cf $\mathrm{n}=0.400 \mathrm{~L}=237 . \mathbf{O}^{\prime} \quad \mathrm{S}=0.0211^{\prime} / / \quad$ Capacity=31.93 cfs Outflow=1.24 cfs $14,613 \mathrm{cf}$

Reach R202: OVERLANDFLOW Avg. Flow Depth=0.21' Max Vel=0.13 fps Inflow=8.50 cfs $43,012 \mathrm{cf}$ $\mathrm{n}=0.400 \mathrm{~L}=700.0$ S=0.0107 '/' Capacity=42.56 cfs Outflow=2.85 cfs 39,436 cf

Reach R211: OVERLANDFLOW Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0 cf $\mathrm{n}=0.400 \mathrm{~L}=600.0$ ' $\mathrm{S}=0.0087$ '/' Capacity= 14.51 cfs Outflow=0.00 cfs 0 cf

Reach SC1: Stream Crossing\#1 Avg. Flow Depth=0.21' Max Vel=2.48 fps Inflow=8.51 cfs $43,021 \mathrm{cf}$ $192.0^{\prime \prime} \times 60.0$ ", R=207.0" Arch Pipe $\mathrm{n}=0.030 \mathrm{~L}=43.1$ ' $\mathrm{S}=0.0200$ '/' Capacity=722.91 cfs Outflow=8.50 cfs $43,012 \mathrm{cf}$

Reach SC2: Stream Crossing\#2 Avg. Flow Depth=0.03' Max Vel=1.04 fps Inflow=0.46 cfs 2,770 cf $192.0^{\prime \prime} \times 60.0^{\prime \prime}, R=180.0$ " Arch Pipe $n=0.030 \quad L=36.5^{\prime} \quad S=0.0241$ '/' Capacity=768.96 cfs Outflow=0.46 cfs $2,770 \mathrm{cf}$

Pond 1P: DMH \#33

Pond 3P: OCS \#8

Pond 5R: TRENCH DRAIN

Pond 11P: YARD DRAIN

Pond CB10: CB \#10

Pond CB11: CB \#11

Pond CB12: CB \#12

Pond CB13: CB \#13

Pond CB14: CB \#14

Pond CB15: CB \#15

Pond CB16: CB \#16

Peak Elev=206.05' Inflow=1.12 cfs 3,877 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=46.7^{\prime} \mathrm{S}=0.0251$ '//' Outflow=1.12 cfs $3,877 \mathrm{cf}$

Peak Elev=201.07' Inflow=0.79 cfs 2,578 cf Outflow=0.79 cfs 2,578 cf

Peak Elev=197.74' Inflow=0.71 cfs 2,338 cf 8.0" Round Culvert n=0.012 L=36.0' S=0.0200 '/' Outflow=0.71 cfs 2,338 cf

Peak Elev=207.32' Storage=383 cf Inflow=0.78 cfs 2,476 cf Outflow=0.51 cfs 2,448 cf

Peak Elev=210.18' Inflow=0.50 cfs $1,761 \mathrm{cf}$ 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=33.8^{\prime} \mathrm{S}=0.0050$ '//' Outflow=0.50 cfs $1,761 \mathrm{cf}$

Peak Elev=210.32' Inflow=0.51 cfs 1,814 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=26.3^{\prime} \mathrm{S}=0.0103$ '/' Outflow=0.51 cfs $1,814 \mathrm{cf}$

Peak Elev=206.98' Inflow=0.37 cfs $1,325 \mathrm{cf}$ 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=41.3^{\prime} \mathrm{S}=0.0249$ '/' Outflow=0.37 cfs $1,325 \mathrm{cf}$

Peak Elev=207.14' Inflow=0.75 cfs 2,552 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=43.7^{\prime} \mathrm{S}=0.0249$ '/' Outflow=0.75 cfs $2,552 \mathrm{cf}$

Peak Elev=201.41' Inflow=0.73 cfs 2,341 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=23.2^{\prime} \mathrm{S}=0.0052$ '//' Outflow=0.73 cfs $2,341 \mathrm{cf}$

Peak Elev=201.34' Inflow=0.47 cfs 1,686 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=15.6^{\prime} \mathrm{S}=0.0051$ '/' Outflow=0.47 cfs $1,686 \mathrm{cf}$

Peak Elev=203.82' Inflow=0.31 cfs 985 cf 12.0" Round Culvert n=0.013 L=20.9' S=0.0067 '// Outflow=0.31 cfs 985 cf



| 19097 Post-Development Type III 24-hr 2YR Rainfall=3.27" |  |
| :---: | :---: |
| Prepared by Howard Stein Hudson Associates Printed 6/19/2023 |  |
| HydroCAD® 10.20-3c s | © 2023 HydroCAD Software Solutions LLC Page 18 |
| Pond CB54: CB \#54 | Peak Elev=202.98' Inflow=0.29 cfs 933 cf |
|  | 12.0" Round Culvert n=0.013 L=36.7' S=0.0049'/' Outflow=0.29 cfs 933 cf |
| Pond CB7: CB\#5 | Peak Elev=212.90' Inflow=0.33 cfs 1,176 cf |
|  | 12.0" Round Culvert n=0.012 L=15.1' S=0.0099 '/' Outfow=0.33 cfs 1,176 cf |
| Pond CB8: CB\#8 | Peak Elev=214.11' Inflow=0.36 cfs 1,185 cf |
|  | 12.0" Round Culvert n=0.013 L=15.1' S=0.0099 '/' Outflow=0.36 cfs 1,185 cf |
| Pond CB9: CB \#9 | Peak Elev=210.66' Inflow=1.14 cfs 3,974 cf |
|  | 12.0" Round Culvert n=0.013 L=19.9' S=0.0196 '/' Outflow=1.14 cfs 3,974 cf |
| Pond D10: DMH \#10 | Peak Elev=203.66' Inflow=0.31 cfs 985 cf |
|  | 12.0" Round Culvert n=0.013 L=15.6' S=0.0051 '/' Outflow=0.31 cfs 985 cf |
| Pond D11: DMH \#11 | Peak Elev=205.02' Inflow=2.02 cfs 6,256 cf |
|  | 18.0" Round Culvert n=0.013 L=44.6' S=0.0049 '/' Outflow=2.02 cfs 6,256 cf |
| Pond D12: DMH \#12 | Peak Elev=203.96' Inflow=1.40 cfs 4,590 cf |
|  | 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=41.9$ ' S=0.0050 '/' Outflow=1.40 cfs 4,590 cf |
| Pond D13: DMH \#13 | Peak Elev=202.97' Inflow=4.21 cfs 15,091 cf |
|  | 24.0" Round Culvert n=0.013 L=60.1' S=0.0050 '/' Outflow=4.21 cfs 15,091 cf |
| Pond D14: DMH \#14 | Peak Elev=204.91' Inflow=2.43 cfs 8,054 cf |
|  | 18.0" Round Culvert n=0.012 L=256.3' S=0.0050 '/' Outflow=2.43 cfs 8,054 cf |
| Pond D16: DMH \#16 | Peak Elev=205.47' Inflow=1.06 cfs 3,505 cf |
|  | 15.0" Round Culvert n=0.012 L=103.5' S=0.0050 '/' Outflow=1.06 cfs 3,505 cf |
| Pond D17: DMH \#17 | Peak Elev=201.05' Inflow=0.95 cfs 3,144 cf |
|  | 12.0" Round Culvert n=0.013 L=91.6' S=0.0312 '/' Outflow=0.95 cfs 3,144 cf |
| Pond D18: DMH \#18 | Peak Elev=198.06' Inflow=1.51 cfs 4,923 cf |
|  | 15.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=46.3$ ' S=0.0099 '/' Outflow=1.51 cfs 4,923 cf |
| Pond D19: DMH \#19 | Peak Elev=205.76' Inflow=1.15 cfs 3,791 cf |
|  | 12.0" Round Culvert n=0.013 L=82.5' S=0.0092 '/' Outflow=1.15 cfs 3,791 cf |
| Pond D2: DMH\#2 | Peak Elev=207.57' Inflow=1.72 cfs 7,465 cf |
|  | 15.0" Round Culvert n=0.013 L=38.2' S=0.0099 '/' Outflow=1.72 cfs 7,465 cf |
| Pond D20: DMH \#20 | Peak Elev=204.78' Inflow=1.15 cfs 3,791 cf |
|  | 15.0" Round Culvert n=0.013 L=63.5' S=0.0050 '/' Outflow=1.15 cfs 3,791 cf |
| Pond D21: DMH \#21 | Peak Elev=204.08' Inflow=4.53 cfs 15,002 cf |
|  | 24.0" Round Culvert n=0.013 L=72.4' S=0.0050 '/' Outfow=4.53 cfs 15,002 cf |
| Pond D22: DMH \#22 | Peak Elev=205.54' Inflow=1.67 cfs 5,708 cf |
|  | 15.0" Round Culvert n=0.013 L=134.2' S=0.0071 '/' Outfow=1.67 cfs 5,708 cf |

Peak Elev=210.19' Inflow=2.34 cfs 8,121 cf

Pond D25: DMH \#25

Pond D27: DMH \#27

Pond D28: DMH \#28

Pond D29: DMH \#29

Pond D30: DMH \#30

Pond D31: DMH\#31

Pond D34: DMH \#34

Pond D35: DMH \#35

Pond D4: DMH\#4

Pond D5: DMH \#5

Pond D6: DMH \#6

Pond D7: DMH \#7

Pond D8: DMH \#8

Pond D9: DMH \#9

Pond DE61: DRIP \#61

Pond DE62: DRIP \#62

Peak Elev=207.11' Inflow=0.77 cfs 2,724 cf 15.0" Round Culvert $n=0.012$ L=237.6' $S=0.0050$ '/' Outflow=2.34 cfs 8,121 cf

Peak Elev=213.88' Inflow=1.26 cfs 4,263 cf 15.0" Round Culvert $n=0.012$ L=63.9' $S=0.0150$ '/' Outflow=1.26 cfs 4,263 cf

Peak Elev=217.83' Inflow=0.56 cfs 1,785 cf 12.0" Round Culvert $n=0.013$ L=158.3' $S=0.0200$ '/' Outflow=0.56 cfs 1,785 cf

Peak Elev=220.20' Inflow=0.56 cfs 1,785 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=150.9$ ' $\mathrm{S}=0.0151$ '/' Outflow=0.56 cfs 1,785 cf

Peak Elev=221.21' Inflow=0.26 cfs 832 cf
12.0" Round Culvert $n=0.013$ L=184.2' $S=0.0050$ '/' Outflow=0.26 cfs 832 cf

Peak Elev=225.15' Inflow=1.18 cfs 5,104 cf 15.0" Round Culvert $n=0.012$ L=288.5' $S=0.0400$ '/' Outflow=1.18 cfs 5,104 cf

Peak Elev=198.77' Inflow=1.66 cfs 5,882 cf 12.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=52.0^{\prime} \mathrm{S}=0.0200$ '/' Outflow=1.66 cfs $5,882 \mathrm{cf}$

Peak Elev=212.82' Inflow=1.26 cfs 4,263 cf 15.0" Round Culvert n=0.012 L=171.5' S=0.0150 '/' Outflow=1.26 cfs 4,263 cf

Peak Elev=210.98' Inflow=1.72 cfs 7,465 cf 15.0" Round Culvert $n=0.012$ L=222.3' $S=0.0150$ '/' Outflow=1.72 cfs 7,465 cf

Peak Elev=209.86' Inflow=2.15 cfs 7,549 cf 18.0" Round Culvert $n=0.013$ L=183.0' $S=0.0050$ '/' Outflow=2.15 cfs 7,549 cf

Peak Elev=208.82' Inflow=2.15 cfs 7,549 cf 18.0" Round Culvert $n=0.013$ L=299.7' $S=0.0050$ '/' Outflow=2.15 cfs 7,549 cf

Peak Elev=207.14' Inflow=2.15 cfs 7,549 cf 18.0" Round Culvert n=0.013 L=44.2' S=0.0550 '/' Outflow=2.15 cfs 7,549 cf

Peak Elev=201.27' Inflow=1.20 cfs 4,028 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=87.7^{\prime} \mathrm{S}=0.0050$ '/' Outflow=1.20 cfs $4,028 \mathrm{cf}$

Peak Elev=200.73' Inflow=1.20 cfs 4,028 cf 12.0" Round Culvert n=0.013 L=11.9' S=0.0050 '/' Outflow=1.20 cfs 4,028 cf

Peak Elev=213.08' Storage=238 cf Inflow=0.29 cfs 997 cf Discarded $=0.00$ cfs 181 cf Primary $=0.24$ cfs 676 cf Outflow=0.24 cfs 857 cf

Peak Elev=213.08' Storage=238 cf Inflow=0.29 cfs 997 cf Discarded $=0.00$ cfs 181 cf Primary $=0.24$ cfs 676 cf Outflow=0.24 cfs 857 cf


Pond P204: STORMTECHINFILTRATION Peak Elev=203.89' Storage=5,017 cf Inflow=3.90 cfs 12,675 cf Discarded=0.09 cfs 5,215 cf Primary=0.93 cfs 5,781 cf Outflow=1.02 cfs 10,996 cf
Prepared by Howard Stein Hudson Associates Printed 6/19/2023

Pond P205: INFILTRATIONPOND \#3 Peak Elev=205.79' Storage=3,362 cf Inflow=2.27 cfs 9,251 cf Discarded=0.26 cfs 9,244 cf Primary=0.00 cfs 0 cf Outflow=0.26 cfs 9,244 cf

Pond P206: STORMTECHINFILTRATION Peak Elev=195.67' Storage=4,118 cf Inflow=3.88 cfs $13,143 \mathrm{cf}$ Discarded $=0.49$ cfs 13,142 cf Primary=0.00 cfs 0 cf Oufflow=0.49 cfs 13,142 cf

Pond P207: INFILTRATIONPOND \#2 Peak Elev=197.50' Storage=7,511 cf Inflow=6.85 cfs 23,672 cf Discarded $=0.96$ cfs 23,563 cf Primary $=0.04$ cfs 101 cf Outflow $=1.00$ cfs 23,663 cf

Pond P210: POCKET WETLAND\#1 Peak Elev=202.97' Storage=7,606 cf Inflow=2.95 cfs 10,193 cf Outflow=0.12 cfs 4,736 cf

Pond P212: INFILTRATIONPOND \#1 Peak Elev=201.74' Storage=14,075 cf Inflow=9.79 cfs 37,028 cf Discarded=1.25 cfs 37,018 cf Primary=0.00 cfs 0 cf Oufflow=1.25 cfs 37,018 cf

Pond P213: Stormtech Infiltration System\#3 Peak Elev=201.93' Storage=1,973 cf Inflow=2.22 cfs 7,648 cf Discarded=0.39 cfs 7,647 cf Primary=0.00 cfs 0 cf Outflow=0.39 cfs 7,647 cf

Pond P214: STORMTECHINFILTRATION Peak Elev=200.78' Storage=1,357 cf Inflow=2.21 cfs 7,633 cf Discarded=0.61 cfs 7,642 cf Primary=0.00 cfs 0 cf Outflow=0.61 cfs 7,642 cf

## Link AP1: ANALYSISPOINT 1

Link AP2: ANALYSISPOINT 2

## Link AP4: ANALYSISPOINT \#4

Inflow=0.67 cfs 2,247 cf Primary=0.67 cfs 2,247 cf

Inflow=7.31 cfs 75,144 cf Primary=7.31 cfs 75,144 cf

## Link AP3: ANALYSISPOINT 3

Inflow=1.15 cfs 4,009 cf Primary=1.15 cfs 4,009 cf

Inflow=7.79 cfs 65,174 cf Primary $=7.79$ cfs 65,174 cf

Total Runoff Area $=2,573,920$ sf Runoff Volume $=266,846$ cf Average Runoff Depth $=1.24$ " 70.09\% Pervious $=1,803,997$ sf $29.91 \%$ Impervious $=769,923$ sf

## Summary for Subcatchment B1: MULTIFAMILY BLDG \#1

Runoff $=\quad 1.66$ cfs @ 12.09 hrs, Volume $=\quad 5,882$ cf, Depth> 3.04"
Routed to Pond D34 : DMH \#34
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 20,156 | 98 R | Roofs, HSG CRoofs, HSG D |  |  |
|  | 3,099 | 98 R |  |  |  |
|  | 23,255 | 98 V | Weighted Average |  |  |
|  | 23,255 |  | 100.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{r} \text { c } \begin{array}{r} \text { Length } \\ \text { (feet) } \end{array} \\ \hline \end{array}$ | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |

## Summary for Subcatchment B2: MULTIFAMILY BLDG \#2

```
Runoff = 1.25 cfs @ 12.09 hrs, Volume= 4,442 cf, Depth> 3.04"
```

Routed to Pond OCS3 : OCS\#3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | Roofs, HSG A Roofs, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & \hline 7,873 \\ & 9,688 \end{aligned}$ | $\begin{aligned} & 98 \\ & 98 \end{aligned}$ |  |  |  |
|  | $\begin{aligned} & 17,561 \\ & 17,561 \end{aligned}$ | 98 | Weighted Average 100.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | $\begin{array}{rrr} \begin{aligned} \text { Slope } \\ \text { (ft/ft) } \end{aligned} & \begin{array}{r} \text { Velocity } \\ \text { (ft/sec) } \end{array} & \begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array} \\ \hline \end{array}$ |  |  | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment B3: MULTIFAMILY BUILDING \#3

Runoff $=1.42$ cfs @ 12.09 hrs, Volume= 5,054 cf, Depth> 3.04"
Routed to Pond P214 : STORMTECH INFILTRATION SYSTEM \#4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 608 | 98 | Roofs, HSG A |
| 19,373 | 98 | Roofs, HSG C |
| 19,981 | 98 | Weighted Average |
| 19,981 |  | $100.00 \%$ Impervious Area |



Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment C12: CB \#12

Runoff $=\quad 0.37$ cfs @ 12.09 hrs, Volume= $1,325 \mathrm{cf}$, Depth> 3.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment C13: CB \#13

Runoff $=\quad 0.75$ cfs @ 12.09 hrs, Volume= $\quad 2,552 \mathrm{cf}$, Depth> 2.82"
Routed to Pond CB13 : CB \#13
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,003 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 7,547 | 98 | Paved parking, HSG C |
| 2,323 | 98 | Roofs, HSG C |

Summary for Subcatchment C14: CB \#14
Runoff $=\quad 0.73$ cfs @ 12.09 hrs, Volume= 2,341 cf, Depth> 2.32"
Routed to Pond CB14 : CB \#14
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment C15: CB \#15

Runoff = 0.47 cfs @ 12.09 hrs, Volume= 1,686 cf, Depth> 3.04"

Routed to Pond CB15 : CB \#15
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 5,000 | 98 | Paved parking, HSG A |
| 1,666 | 98 | Paved parking, HSG C |
| 6,666 | 98 | Weighted Average |
| 6,666 |  | 100.00\% Impervious Area |
| Tc | Length <br> $(\mathrm{min})$ | Slope <br> (feet) |
| (ft/ft) | Velocity <br> (ft/sec) | Capacity <br> (cfs) |

Direct Entry,

## Summary for Subcatchment C16: CB \#16

```
Runoff = 0.31 cfs @ 12.10 hrs, Volume=
985 cf, Depth> 1.39"
```

Routed to Pond CB16 : CB \#16
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 2,467 | 39 | $>75 \%$ Grass cover, Good, HSG A |
| 4,380 | 98 | Paved parking, HSG A |
| 524 | 74 | $>75 \%$ Grass cover, Good, HSG C <br> 1,145 |
|  | 98 | Paved parking, HSG C |

Direct Entry,
Summary for Subcatchment C17: CB \#17
Runoff $=\quad 0.73 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume= $\quad 2,382 \mathrm{cf}$, Depth> 2.42"
Routed to Pond CB17 : CB \#17
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | escription |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & \hline 3,093 \\ & 8,743 \end{aligned}$ | $\begin{array}{ll} \hline 74 & > \\ 98 & P \end{array}$ | >75\% Grass cover, Good, HSG C Paved parking, HSG C |  |  |
|  | $\begin{array}{r} \hline 11,836 \\ 3,093 \\ 8,743 \end{array}$ | $92 \begin{array}{r}\text { W } \\ \\ \\ 7\end{array}$ | Weighted Average 26.13\% Pervious Area 73.87\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Ent |
| Summary for Subcatchment C18: CB \#18 |  |  |  |  |  |

Runoff $=\quad 1.08$ cfs @ 12.09 hrs, Volume= $\quad 3,458$ cf, Depth> 2.23"
Routed to Pond CB18 : CB \#18
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 6,255 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 12,336 | 98 | Paved parking, HSG C |
| 18,591 | 90 | Weighted Average |
| 6,255 |  | $33.65 \%$ Pervious Area |
| 12,336 |  | $66.35 \%$ Impervious Area |
| Tc | Length | Slope <br> (ft/ft) |
| Velocity  <br> (ft/sec) Capacity <br> (min) (cfs) |  |  |

6.0

Direct Entry,
Summary for Subcatchment C20: CB \#20
Runoff $=\quad 0.80 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume= $\quad 2,698 \mathrm{cf}$, Depth> 2.71"
Routed to Pond CB20 : CB \#20
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,319 | 98 | Paved parking, HSG A |
| 1,319 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 7,301 | 98 | Paved parking, HSG C |
| 11,939 | 95 | Weighted Average |
| 1,319 |  | 11.05\% Pervious Area |
| 10,620 |  | $88.95 \%$ Impervious Area |


| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C21: CB \#21

Runoff $=\quad 0.59$ cfs @ 12.09 hrs, Volume= 1,892 cf, Depth> 2.23"
Routed to Pond CB21 : CB \#21
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,319 |  | >75\% Grass cover, Good, HSG A |  |  |
|  | 7,301 | 98 P | Paved parking, HSG A |  |  |
|  | 1,554 | 98 P | Paved parking, HSG C |  |  |
|  | 10,174 | $90 \quad 1$ | Weighted Average |  |  |
|  | 1,319 |  | 12.96\% Pervious Area |  |  |
|  | 8,855 |  | 87.04\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C22: CB \#22

Runoff $=\quad 0.83$ cfs @ 12.09 hrs, Volume $=\quad 2,817 \mathrm{cf}$, Depth> 2.82"
Routed to Pond CB22 : CB \#22
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment C23: CB \#23

Runoff $=\quad 0.54$ cfs @ 12.09 hrs, Volume= 1,732 cf, Depth> 2.14"
Routed to Pond CB23 : CB \#23
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG A |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 242 | 39 > |  |  |  |
|  | 3,016 | 98 P | Paved parking, HSG A |  |  |
|  | 1,267 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 218 | 98 P | Paved parking, HSG C |  |  |
|  | 2,272 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 2,679 | 98 P | Paved parking, HSG D |  |  |
|  | 9,694 | 89 | Weighted Average |  |  |
|  | 3,781 |  | 39.00\% Pervious Area |  |  |
|  | 5,913 |  | 61.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{rr} \text { c } \begin{array}{r} \text { Length } \\ \text { (feet) } \end{array} \\ \hline \end{array}$ | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C24: CB \#24

Runoff $=\quad 0.51$ cfs @ 12.09 hrs, Volume $=1,660 \mathrm{cf}$, Depth> 2.51"

Routed to Pond CB24 : CB \#24
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG A |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 211 | 39 > |  |  |  |
|  | 519 | 98 P | Paved parking, HSG A |  |  |
|  | 15 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 300 | 98 P | Paved parking, HSG C |  |  |
|  | 1,393 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 6,049 | 98 P | Paved parking, HSG D |  |  |
|  | 8,487 | 94 | Weighted Average |  |  |
|  | 1,619 |  | 19.08\% Pervious Area |  |  |
|  | 6,868 |  | 80.92\% Im | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C26: CB \#26

Runoff $=\quad 0.53$ cfs @ 12.09 hrs, Volume= 1,710 cf, Depth> 2.32"

Routed to Pond CB26 : CB \#26
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,203 | 80 | >75\% Grass cover, Good, HSG D |
| 5,632 | 98 | Paved parking, HSG D |

## Summary for Subcatchment C27: CB \#27

Runoff $=\quad 0.42$ cfs @ 12.09 hrs, Volume= $\quad 1,434 \mathrm{cf}$, Depth> 2.82"
Routed to Pond CB27 : CB \#27
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 98 | 39 | $>75 \%$ Grass cover, Good, HSG A |
| 131 | 98 | Paved parking, HSG A |
| 397 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 5,485 | 98 | Paved parking, HSG D |
| 6,111 | 96 | Weighted Average |
| 495 |  | 8.10\% Pervious Area |
| 5,616 |  | $91.90 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ |
| ---: | ---: | ---: | ---: | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description |  |
| :--- |
| 6.0 |

## Summary for Subcatchment C28: CB \#28

Runoff $=\quad 0.56$ cfs @ 12.09 hrs, Volume= $1,779 \mathrm{cf}$, Depth> $2.06{ }^{\prime \prime}$
Routed to Pond CB28 : CB \#28
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | >75\% Grass cover, Good, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,751 | 74 |  |  |  |
|  | 2,841 | 98 | Paved parking, HSG C |  |  |
|  | 2,297 | 80 | >75\% Grass cover, Good, HSG D |  |  |
|  | 2,483 | 98 | Paved parking, HSG D |  |  |
|  | 10,372 | 88 | Weighted Average |  |  |
|  | 5,048 |  | 48.67\% Pervious Area |  |  |
|  | 5,324 |  | 51.33\% Impervious Area |  |  |
| Tc (min) | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C29: CB \#29

Runoff $=\quad 0.56$ cfs @ 12.09 hrs, Volume $=1,848 \mathrm{cf}$, Depth> 2.61"
Routed to Pond CB29: CB \#29
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

| $\begin{array}{r} \text { Area (sf) } \\ \hline 1,341 \end{array}$ |  | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 74 |  |  |  |
|  | 5,330 | 98 P | >75\% Grass cover, Good, HSG C Paved parking, HSG C |  |  |
|  | 1,824 | 98 R | Roofs, HSG |  |  |
|  | 8,495 | 94 | Weighted | verage |  |
|  | 1,341 |  | 15.79\% Pe | vious Area |  |
|  | 7,154 |  | 84.21\% Im | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |

## Summary for Subcatchment C30: CB \#30

Runoff $=\quad 0.59$ cfs @ 12.09 hrs, Volume= 1,943 cf, Depth> 2.61"

Routed to Pond CB30 : CB \#30
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,572 | 74 | >75\% Grass cover, Good, HSG C |  |  |
|  | 6,310 | 98 P | Paved parking, HSG C |  |  |
|  | 1,051 | 98 | Roofs, HSG C |  |  |
|  | 8,933 | 94 | Weighted Average |  |  |
|  | 1,572 |  | 17.60\% Pervious Area |  |  |
|  | 7,361 |  | 82.40\% Im | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C31: CB \#31

Runoff $=\quad 0.95$ cfs @ 12.09 hrs, Volume= 3,044 cf, Depth> 2.23"
Routed to Pond CB31 : CB \#31
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 5,132 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 9,132 | 98 P | Paved parking, HSG C |  |  |
|  | 2,101 | 98 R | Roofs, HSG C |  |  |
|  | 16,365 | 90 | Weighted Average |  |  |
|  | 5,132 |  | 31.36\% Pervious Area |  |  |
|  | 11,233 |  | 68.64\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ (\mathrm{cfs}) \\ \hline \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment C32: CB \#32

Runoff $=\quad 0.76$ cfs @ 12.09 hrs, Volume $=\quad 2,460 \mathrm{cf}$, Depth> 2.32"
Routed to Pond CB32 : CB \#32
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment C34: CB \#34

Runoff $=\quad 0.55$ cfs @ 12.09 hrs, Volume= 1,804 cf, Depth> 2.51"
Routed to Pond CB34 : CB \#34
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,680 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 5,115 | 98 | Paved parking, HSG C |
| 1,827 | 98 | Roofs, HSG C |
| 8,622 | 93 | Weighted Average |
| 1,680 |  | $19.49 \%$ Pervious Area |
| 6,942 |  | $80.51 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ |
| ---: | ---: | ---: | :--- |
| 6.0 | Capacity <br> $(\mathrm{cfs})$ | Description |  |
|  |  | Direct Entry, |  |
|  |  | Summary for Subcatchment C35: CB \#35 |  |

Runoff $=\quad 0.30$ cfs @ 12.09 hrs, Volume= 1,049 cf, Depth> 3.04"
Routed to Pond CB35 : CB \#35
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,578 | 98 P | Paved parking, HSG C |  |  |
|  | 79 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 492 | 98 P | Paved parking, HSG D |  |  |
|  | 4,149 | $98 \quad 1$ | Weighted Average |  |  |
|  | 79 |  | 1.90\% Pervious Area |  |  |
|  | 4,070 |  | 98.10\% Im | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C36: CB \#36

Runoff $=\quad 0.47$ cfs @ 12.09 hrs, Volume $=\quad 1,675 \mathrm{cf}$, Depth> 3.04"
Routed to Pond CB36 : CB \#36
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment C38: CB \#38

Runoff = 0.54 cfs @ 12.09 hrs, Volume= 1,932 cf, Depth> 3.04"
Routed to Pond CB38 : CB \#38
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 6,647 | 98 P | Paved parking, HSG B |  |  |
|  | 392 | 98 P | Paved parking, HSG C |  |  |
|  | 598 | 98 P | Paved parking, HSG D |  |  |
|  | 7,637 | 98 V | Weighted Average |  |  |
|  | 7,637 |  | 100.00\% Impervious Area |  |  |
| Tc (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C39: CB \#39

Runoff $=\quad 0.54$ cfs @ 12.09 hrs, Volume= 1,925 cf, Depth> 3.04"
Routed to Pond CB39 : CB \#39
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

6.0

Direct Entry,

## Summary for Subcatchment C40: CB \#40

Runoff $=\quad 0.30$ cfs @ 12.09 hrs, Volume $=1,065 \mathrm{cf}$, Depth> 3.04"
Routed to Pond CB40 : CB \#40
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment C41: CB \#41

Runoff $=\quad 0.40$ cfs @ 12.09 hrs, Volume= 1,413 cf, Depth> 3.04"

Routed to Pond CB41 : CB \#41
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment C43: CB \#43

Runoff $=\quad 0.17$ cfs @ 12.09 hrs, Volume= 555 cf, Depth> 2.14"
Routed to Pond CB43 : CB \#43
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,343 | 98 P | Paved parking, HSG B $>75 \%$ Grass cover, Good, HSG B |  |  |
|  | 766 | $61>$ |  |  |  |
|  | 3,109 | $89 \quad \begin{array}{r} \\ \\ \\ 7 \\ \\ \end{array}$ | Weighted Average <br> 24.64\% Pervious Area <br> 75.36\% Impervious Area |  |  |
|  | 766 |  |  |  |  |
|  | 2,343 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

Summary for Subcatchment C44: CB \#44
Runoff $=\quad 0.12$ cfs @ 12.09 hrs, Volume= 398 cf, Depth> 2.42"
Routed to Pond CB44 : CB \#44
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,670 | 98 | Paved parking, HSG B |
| 308 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 1,978 | 92 | Weighted Average |
| 308 |  | 15.57\% Pervious Area |
| 1,670 |  | $84.43 \%$ Impervious Area |



Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"


Summary for Subcatchment C47: CB \#47
Runoff = 0.21 cfs @ 12.09 hrs, Volume= 762 cf, Depth> 3.04"
Routed to Pond CB47 : CB\#47
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |  |
| ---: | ---: | :--- | :--- |
|  | 3,012 | 98 | Paved roads w/curbs \& sewers, HSG B |
|  | 3,012 |  | $100.00 \%$ Impervious Area |
| Tc | Length <br> (min) | (feet) | Slope <br> (ft/ft) |
| 6.0 |  | Velocity <br> (ft/sec) | Capacity <br> (cfs) |

## Summary for Subcatchment C48: CB \#48

Runoff $=\quad 1.03$ cfs @ 12.19 hrs, Volume= 4,342 cf, Depth> 0.87"
Routed to Pond CB48 : CB\#48
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"
$\left.\begin{array}{rrrll}\text { Area (sf) } & \text { CN } & \text { Description } \\ 3,877 \\ 56,251 & 98 & \text { Paved roads w/curbs \& sewers, HSG B } \\ 68 & \text { 1 acre lots, 20\% imp, HSG B }\end{array}\right]$
11.8400 Total

Summary for Subcatchment C49: CB \#49
Runoff $=\quad 0.34$ cfs @ 12.09 hrs, Volume= 1,139 cf, Depth> 2.61"
Routed to Pond CB49 : CB \#49
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 4,431 | 98 | Paved roads w/curbs \& sewers, HSG C |
| 807 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 5,238 | 94 | Weighted Average |
| 807 |  | 15.41\% Pervious Area |
| 4,431 |  | $84.59 \%$ Impervious Area |



Routed to Pond CB50 : CB \#50
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,429 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 11,611 | 98 | Paved roads w/curbs \& sewers, HSG C |
| 15,040 | 93 | Weighted Average |
| 3,429 |  | $22.80 \%$ Pervious Area |
| 11,611 |  | $77.20 \%$ Impervious Area |
| Tc | Length | Slope <br> (ft/ft) |
| Velocity (ft/sec) Capacity <br> (min) (feet) (cfs) |  |  |

Direct Entry,

## Summary for Subcatchment C51: CB \#51

Runoff $=\quad 0.49$ cfs @ 12.09 hrs, Volume= 1,726 cf, Depth> 3.04"
Routed to Pond CB51 : CB \#51
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,147 | 98 R | Roofs, HSG C Paved parking, HSG C |  |  |
|  | 3,676 | 98 P |  |  |  |
|  | 6,823 | 98 | Weighted Average 100.00\% Impervious Area |  |  |
|  | 6,823 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry, |

Summary for Subcatchment C52: CB\#52
Runoff = 0.61 cfs @ 12.09 hrs, Volume= 2,045 cf, Depth> 2.71"
Routed to Pond CB52 : CB \#52
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & \hline 1,164 \\ & 7,888 \end{aligned}$ | $\begin{array}{ll} \hline 74 & 7 \\ 98 & F \end{array}$ | >75\% Grass cover, Good, HSG C Paved parking, HSG C |  |  |
|  | $\begin{aligned} & 9,052 \\ & 1,164 \\ & 7,888 \end{aligned}$ | $95 \quad 1$ | Weighted Average 12.86\% Pervious Area 87.14\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entr |

## Summary for Subcatchment C53: CB \#53

Runoff $=\quad 0.50 \mathrm{cfs} @ 12.09$ hrs, Volume= $\quad 1,646 \mathrm{cf}$, Depth> 2.51"

Routed to Pond CB53 : CB \#53
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 287 | 39 > | >75\% Grass cover, Good, HSG A |  |  |
|  | 3,287 | 98 P | Paved parking, HSG A |  |  |
|  | 773 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 3,516 | 98 P | Paved parking, HSG C |  |  |
|  | 7,863 | 93 V | Weighted Average |  |  |
|  | 1,060 |  | 13.48\% Pervious Area |  |  |
|  | 6,803 |  | 86.52\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ftft) | Velocity (ft/sec) | Capacity (cfs) | Description |

6.0

Direct Entry,

## Summary for Subcatchment C54: CB \#54

Runoff $=\quad 0.29$ cfs @ 12.09 hrs, Volume= 933 cf, Depth> 2.32"
Routed to Pond CB54 : CB \#54
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 550 | 39 | $>75 \%$ Grass cover, Good, HSG A |
| 4,176 | 98 | Paved parking, HSG A |
| 84 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 11 | 98 | Paved parking, HSG C |
| 4,821 | 91 | Weighted Average |
| 634 |  | $13.15 \%$ Pervious Area |
| 4,187 |  | $86.85 \%$ Impervious Area |


| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 6.0 |  | Direct Entry, |  |  |  |  |  |
|  |  | Summary for Subcatchment C7: CB \#5 |  |  |  |  |  |
| RunoffRout | $=$ | 0.33 cfs @ 12.09 hrs , Volume=CB7 : $\mathrm{CB}+5$ |  |  |  | 1,176 cf, Depth> 3.04" |  |
|  | to Pond |  |  |  |  |  |  |

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment C8: CB \#8

Runoff $=\quad 0.36$ cfs @ 12.09 hrs, Volume= $1,185 \mathrm{cf}$, Depth> 2.61"
Routed to Pond CB8 : CB\#8
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{array}{r} 4,837 \\ 613 \\ \hline \end{array}$ | $\begin{array}{ll} 98 \\ 61 & = \end{array}$ | Paved roads w/curbs \& sewers, HSG B $>75 \%$ Grass cover, Good, HSG B |  |  |
|  | $\begin{array}{r} 5,450 \\ 613 \\ 4,837 \end{array}$ | 94 Weighted Average 11.25\% Pervious Area 88.75\% Impervious Area |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | $\begin{array}{rrr}\begin{array}{r}\text { Slope } \\ \text { (ft/ft) }\end{array} & \begin{array}{r}\text { Velocity } \\ (\mathrm{ft} / \mathrm{sec})\end{array} & \begin{array}{r}\text { Capacity } \\ (\mathrm{cfs})\end{array}\end{array}$ |  |  | Description |
| 6.0 |  |  |  |  | Direct Entry, |

## Summary for Subcatchment C9: CB \#9

Runoff $=\quad 1.14$ cfs @ 12.09 hrs, Volume= 3,974 cf, Depth> 2.92"
Routed to Pond CB9 : CB \#9
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment CH1: CLUBHOUSE

Runoff $=\quad 0.43$ cfs @ 12.09 hrs, Volume= 1,470 cf, Depth> 2.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


| Tc <br> $(\mathrm{min})$ | Length <br> (feet) | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

## Summary for Subcatchment S201: SUMMER STREET ACCESS APRON

Runoff $=\quad 0.67$ cfs @ 12.09 hrs, Volume= $\quad 2,247$ cf, Depth> 2.71"
Routed to Link AP1 : ANALYSIS POINT 1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 717 | 61 | >75\% Grass cover, Good, HSG B |
| 9,226 | 98 | Paved parking, HSG B |

6.0 Direct Entry,

## Summary for Subcatchment S202: EXISTING WETLAND

Runoff $=\quad 8.51$ cfs @ 12.34 hrs, Volume= 43,021 cf, Depth> 1.19"
Routed to Reach SC1 : Stream Crossing \#1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 136,496 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 83,935 | 55 | Woods, Good, HSG B |
| 29 | 98 | Paved parking, HSG B |
| 13,946 | 98 | Roofs, HSG B |
| 9,038 | 48 | Brush, Good, HSG B |
| 2,573 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 17,121 | 70 | Woods, Good, HSG C |
| 98 | 98 | Paved parking, HSG C |
| 1,097 | 65 | Brush, Good, HSG C |
| 126 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 132 | 98 | Paved parking, HSG D |
| 167,678 | 98 | Water Surface, HSG D |
| 432,269 | 76 | Weighted Average |
| 250,386 |  | 57.92\% Pervious Area |
| 181,883 |  | $42.08 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ | Description |
| ---: | ---: | ---: | ---: | :--- | :--- |
| 5.6 | 50 | 0.0200 | 0.15 | Sheet Flow, <br> Grass: Short $\mathrm{n}=0.150 \quad \mathrm{P} 2=3.27 "$ <br> 1.4 | 118 |
| 0.0400 | 1.40 | Shallow Concentrated Flow, <br> Short Grass Pasture Kv=7.0 fps <br> Shallow Concentrated Flow, <br> Woodland Kv=5.0 fps |  |  |  |
| 16.2 | 688 | 0.0200 | 0.71 |  |  |

## Summary for Subcatchment S203: POCKET WETLAND \#1

Runoff $=\quad 0.61$ cfs @ 12.10 hrs, Volume $=\quad 2,073 \mathrm{cf}$, Depth> 0.97"
Routed to Pond p210 : POCKET WETLAND \#1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 12,682 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 1,060 | 98 | Water Surface, 0\% imp, HSG B |
| 7,785 | 74 | >75\% Grass cover, Good, HSG C |
| 4,060 | 98 | Water Surface, 0\% imp, HSG C |

## Summary for Subcatchment S204: EXISTING WETLANDS

Runoff $=\quad 5.78$ cfs @ 12.34 hrs, Volume $=29,158$ cf, Depth> 1.14"
Routed to Link ap2 : ANALYSIS POINT 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: |
| 53,739 | $61>$ | >75\% Grass cover, Good, HSG B |  |  |
| 17,975 | 55 | Woods, Good, HSG B Brush, Good, HSG B |  |  |
| 20,940 | 48 B |  |  |  |
| 41,421 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
| 68,342 | 70 | Woods, Good, HSG C |  |  |
| 116 | 98 P | Paved parking, HSG C |  |  |
| 1,904 | 65 Brus | Brush, Good, HSG C |  |  |
| 1,528 | 65 Brus | Brush, Good, HSG C |  |  |
| 2,508 | 77 V | Woods, Good, HSG D |  |  |
| 161 | 98 P | Paved parking, HSG D |  |  |
| 4,073 | 73 Brus | Brush, Good, HSG D |  |  |
| 95,496 | 98 V | Water Surface, HSG D |  |  |
| 308,203 | 75 | Weighted Average |  |  |
| 212,430 |  | 68.93\% Pervious Area |  |  |
| 95,773 |  | 31.07\% Impervious Area |  |  |
| Tc Length (min) (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 3.250 | 0.2000 | 0.26 |  | Sheet Flow, <br> Grass: Dense n=0.240 P2=3.27" |
| 19.4582 | 0.0100 | - 0.50 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 22.6632 | Total |  |  |  |

## Summary for Subcatchment S205: ISOLATED WETLAND

Runoff $=1.15$ cfs @ 12.10 hrs , Volume= Routed to Link AP3 : ANALYSIS POINT 3

4,009 cf, Depth> 0.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 10,910 | 30 | Woods, Good, HSG A |
| 3,684 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 2,275 | 70 | Woods, Good, HSG C |
| 171 | 98 | Paved parking, HSG C |
| 1,706 | 65 | Brush, Good, HSG C |
| 1,940 | 80 | >75\% Grass cover, Good, HSG D |
| 23,513 | 77 | Woods, Good, HSG D |
| 393 | 98 | Paved parking, HSG D |
| 2,208 | 73 | Brush, Good, HSG D |
| 8,620 | 98 | Water Surface, HSG D |
| 55,420 | 70 | Weighted Average |
| 46,236 |  | $83.43 \%$ Pervious Area |
| 9,184 |  | $16.57 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ |
| ---: | ---: | ---: | ---: | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description | Direct Entry, |
| :--- |

## Summary for Subcatchment S206: OVERLAND FLOW

Runoff $=\quad 6.52$ cfs @ 12.58 hrs, Volume= 46,602 cf, Depth> 0.63 "

Routed to Link AP4 : ANALYSIS POINT \#4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment S207: INFILTRATION POND \#2

Runoff $=\quad 1.12$ cfs @ 12.09 hrs, Volume $=\quad 3,568$ cf, Depth> 2.06"

Routed to Pond P207 : INFILTRATION POND \#2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 839 | 98 | Water Surface, 0\% imp, HSG A |
| 8,802 | 74 | >75\% Grass cover, Good, HSG C |
| 11,162 | 98 | Water Surface, 0\% imp, HSG C |

## Summary for Subcatchment S208: GRASS AREA

Runoff $=\quad 0.40$ cfs @ 12.10 hrs, Volume= $\quad 1,308$ cf, Depth> 1.14"
Routed to Pond OCS4 : OCS\#4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN D | Description |
| :---: | :---: | :---: |
| 476 | $39>$ | >75\% Grass cover, Good, HSG A |
| 12,000 | $74>$ | >75\% Grass cover, Good, HSG C |
| 168 | 98 P | Paved parking, HSG A |
| 1,116 | 98 P | Paved parking, HSG C |
| 13,760 | 75 | Weighted Average |
| 12,476 |  | 90.67\% Pervious Area |
| 1,284 |  | 9.33\% Impervious Area |
| Tc Length ( min ) (feet) | Slope (ft/ft) | Velocity Capacity Description (ft/sec) (cfs) |

6.0

## Direct Entry,

## Summary for Subcatchment S209: WETLAND C

Runoff $=\quad 1.60$ cfs @ 12.44 hrs, Volume $=\quad 9,101 \mathrm{cf}$, Depth> 1.02"
Routed to Reach 11R : 4x4 Open Bottom Culvert
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment S210: INFILTRATION POND \#1

Runoff $=\quad 1.92$ cfs @ 12.24 hrs, Volume= $\quad 8,348 \mathrm{cf}$, Depth> 1.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment S211: S211

Runoff $=\quad 0.56$ cfs @ 12.10 hrs, Volume= 1,786 cf, Depth> 1.39"

Routed to Pond P205 : INFILTRATION POND \#3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"


### 23.1418 Total

## Summary for Subcatchment S213: COURTYARD

Runoff $=\quad 0.78$ cfs @ 12.10 hrs, Volume= $\quad 2,476 \mathrm{cf}$, Depth> 1.39"
Routed to Pond 11P :YARD DRAIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG A |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 5,047 | 39 > |  |  |  |
|  | 1,678 | 98 P | Paved parking, HSG A |  |  |
|  | 168 | 98 R | Roofs, HSG A |  |  |
|  | 532 | 98 V | Water Surface, 0\% imp, HSG A |  |  |
|  | 4,518 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 7,080 | 98 P | Paved parking, HSG C |  |  |
|  | 878 | 98 R | Roofs, HSG C |  |  |
|  | 718 | 98 | Water Surface, 0\% imp, HSG C |  |  |
|  | 296 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 492 | 98 P | Paved parking, HSG D |  |  |
|  | 21,407 | 79 | Weighted Average |  |  |
|  | 11,111 |  | 51.90\% Pervious Area |  |  |
|  | 10,296 |  | 48.10\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment T1: Trench Drain 1

Runoff $=\quad 0.71 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume $=\quad 2,338 \mathrm{cf}$, Depth> 2.51"
Routed to Pond 5R : TRENCH DRAIN
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | Description $>75 \%$ Grass cover, Good, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,443 | 74 |  |  |  |
|  | 4,228 | 98 | Paved parking, HSG C |  |  |
|  | 1,339 | 80 | >75\% Grass cover, Good, HSG D |  |  |
|  | 4,163 | 98 | Paved parking, HSG D |  |  |
|  | 11,173 | 93 | Weighted Average |  |  |
|  | 2,782 |  | 24.90\% Pervious Area |  |  |
|  | 8,391 |  | 75.10\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{r} \text { Length } \\ \text { (feet) } \\ \hline \end{array}$ | Slope $(\mathrm{ft} / \mathrm{ft})$ | $\begin{array}{rr} \text { e } & \begin{array}{c} \text { Velocity } \\ \text { t) } \\ (\mathrm{ft} / \mathrm{sec}) \end{array} \\ \hline \end{array}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment T2: Drive Under B2

Runoff $=\quad 0.15$ cfs @ 12.10 hrs , Volume= 490 cf , Depth> 1.32"

Routed to Reach 11R : 4x4 Open Bottom Culvert
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,936 | 98 | Roofs, HSG C$>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 311 | 74 |  |  |  |
|  | 4,247 | 96 | Weighted Average |  |  |
|  | 311 |  | 7.32\% Pervious Area |  |  |
|  | 3,936 |  | 92.68\% Im | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{r} \text { c } \begin{array}{r} \text { Length } \\ \text { (feet) } \end{array} \\ \hline \end{array}$ | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment TH10: TOWN HOUSE \#10

Runoff $=\quad 0.24$ cfs @ 12.09 hrs, Volume= 816 cf, Depth> 2.82"
Routed to Pond DE70 : DRIP \#70
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,184 |  | Roofs, HSG C <br> $>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 292 |  |  |  |  |
|  | 3,476 | $96 \begin{array}{r}\text { V } \\ \\ 8 \\ \\ \\ 9\end{array}$ | Weighted Average 8.40\% Pervious Area 91.60\% Impervious Area |  |  |
|  | 292 |  |  |  |  |
|  | 3,184 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH11: TOWN HOUSE \#11

Runoff $=\quad 0.29$ cfs @ 12.09 hrs, Volume $=\quad 988$ cf, Depth> 2.82"
Routed to Pond DE71 : DRIP \#71
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,899 | 98 R | Roofs, HSG C$>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 311 | $74>$ |  |  |  |
|  | 4,210 | 96 | Weighted Average |  |  |
|  | 311 |  | 7.39\% Pervious Area |  |  |
|  | 3,899 |  | 92.61\% Imp | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ (\mathrm{cfs}) \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment TH2: TOWN HOUSE \#2

Runoff $=\quad 0.29$ cfs @ 12.09 hrs, Volume= 997 cf, Depth> 2.82"
Routed to Pond DE62 : DRIP \#62
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,936 | 98 R | Roofs, HSG C |  |  |
|  | 311 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 4,247 | 96 | Weighted Average |  |  |
|  | 311 |  | 7.32\% Pervious Area |  |  |
|  | 3,936 |  | 92.68\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry, |

## Summary for Subcatchment TH3: TOWN HOUSE \#3

Runoff $=\quad 0.20$ cfs @ 12.09 hrs, Volume= 681 cf, Depth> 2.71"
Routed to Pond DE63 : DRIP \#63
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | Rescript, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,672 | 98 |  |  |  |
|  | 341 | 74 | >75\% Gras | s cover, God | od, HSG C |
|  | 3,013 | $95 \quad 1$ | Weighted Average |  |  |
|  | 341 |  | 11.32\% Pervious Area |  |  |
|  | 2,672 |  | 88.68\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity <br> (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH4: TOWN HOUSE \#4

Runoff $=\quad 0.24$ cfs @ 12.09 hrs, Volume= 814 cf, Depth> 2.82"
Routed to Pond DE64 : DRIP \#64
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,178 | 98 | Roofs, HSG C |
| 292 | 74 | >75\% Grass cover, Good, HSG C |
| 3,470 | 96 | Weighted Average |
| 292 |  | $8.41 \%$ Pervious Area |
| 3,178 |  | $91.59 \%$ Impervious Area |
| Tc | Length | Slope  <br> (ft/ft) Velocity <br> (ft/sec) Capacity <br> (min) (cfs) |

6.0

Direct Entry,

## Summary for Subcatchment TH5: TOWN HOUSE \#5

Runoff $=\quad 0.20$ cfs @ 12.09 hrs, Volume $=\quad 682$ cf, Depth> 2.71" Routed to Pond DE65 : DRIP \#65

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"


## Summary for Subcatchment TH6: TOWN HOUSE \#6

Runoff $=\quad 0.23$ cfs @ 12.09 hrs, Volume= 800 cf, Depth> 2.82"
Routed to Pond DE66 : DRIP \#66
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,116 | 98 R | Roofs, HSG C$>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 291 | $74>$ |  |  |  |
|  | 3,407 | 96 | Weighted Average |  |  |
|  | 291 |  | 8.54\% Pervious Area |  |  |
|  | 3,116 |  | 91.46\% Imp | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ (\mathrm{cfs}) \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment TH7: TOWN HOUSE \#7

Runoff $=\quad 0.24$ cfs @ 12.09 hrs, Volume= 817 cf, Depth> 2.82"
Routed to Pond DE67 : DRIP \#67
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,189 | 98 R | Roofs, HSG C |  |  |
|  | 292 | 74 | >75\% Gras | s cover, Go | od, HSG C |
|  | 3,481 | 96 | Weighted Average |  |  |
|  | 292 |  | 8.39\% Pervious Area |  |  |
|  | 3,189 |  | 91.61\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH8: TOWN HOUSE \#8

Runoff $=\quad 0.29$ cfs @ 12.09 hrs, Volume $=\quad 989$ cf, Depth> 2.82"
Routed to Pond DE68 : DRIP \#68
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN | Rescript, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,901 | $\begin{aligned} & 98 \\ & 74 \\ & \hline \end{aligned}$ |  |  |  |
|  | 311 |  | Roofs, HSG C$>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 4,212 | 96 | Weighted Average |  |  |
|  | 311 |  | 7.38\% Pervious Area |  |  |
|  | 3,901 |  | 92.62\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity <br> (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH9: TOWN HOUSE \#9

Runoff $=\quad 0.24$ cfs @ 12.09 hrs, Volume= 817 cf, Depth> 2.82"
Routed to Pond DE69 : DRIP \#69
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 2YR Rainfall=3.27"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,188 | 98 R | Roofs, HSG C |  |  |
|  | 292 | $74>$ | >75\% Gras | s cover, G | od, HSG C |
|  | 3,480 | 96 | Weighted Average |  |  |
|  | 292 |  | 8.39\% Pervious Area |  |  |
|  | 3,188 |  | 91.61\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |

## Summary for Reach 8R: OVERLAND FLOW

Inflow Area $=\quad 11,975$ sf, $92.37 \%$ Impervious, Inflow Depth > 1.94" for 2YR event Inflow $=\quad 0.69 \mathrm{cfs} @ 12.14 \mathrm{hrs}$, Volume $=1,940 \mathrm{cf}$ Outflow $=\quad 0.11 \mathrm{cfs}$ @ 12.77 hrs , Volume $=1,734 \mathrm{cf}$, Atten $=84 \%$, Lag= 37.7 min

Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.06 \mathrm{fps}$, Min. Travel Time $=156.1 \mathrm{~min}$
Avg. Velocity $=0.04 \mathrm{fps}$, Avg. Travel Time $=252.3 \mathrm{~min}$

Peak Storage= 1,031 cf @ 12.77 hrs
Average Depth at Peak Storage= 0.04' , Surface Width= 50.37'
Bank-Full Depth= 1.00' Flow Area $=55.0$ sf, Capacity $=28.09$ cfs
$50.00^{\prime} \times 1.00^{\prime}$ deep channel, $n=0.400$
Side Slope Z-value= 5.0 '/' Top Width=60.00'
Length $=563.0^{\prime}$ Slope $=0.0213$ '/'
Inlet Invert= 208.00', Outlet Invert= 196.00'


## Summary for Reach 9R: OVERLAND FLOW

Inflow Area $=\quad 32,665$ sf, $94.81 \%$ Impervious, Inflow Depth $=0.00$ " for 2 YR event Inflow $=0.00 \mathrm{cfs}$ @ 0.00 hrs , Volume= 0 cf Outflow = $0.00 \mathrm{cfs} @ 0.00 \mathrm{hrs}$, Volume= 0 cf , Atten= 0\%, Lag= 0.0 min

Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.00 \mathrm{fps}$, Min. Travel Time $=0.0 \mathrm{~min}$
Avg. Velocity $=0.00 \mathrm{fps}$, Avg. Travel Time $=0.0 \mathrm{~min}$
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage=0.00'
Bank-Full Depth $=1.00$ Flow Area= 30.0 sf, Capacity $=23.45$ cfs
20.00' x 1.00' deep channel, $n=0.400$

Side Slope Z-value= 10.0 '/' Top Width= 40.00'
Length $=211.0^{\prime}$ Slope $=0.0652{ }^{\prime} / /$
Inlet Invert= 201.75', Outlet Invert= 188.00'


## Summary for Reach 10R: OVERLAND FLOW

Inflow Area = 129,716 sf, 63.13\% Impervious, Inflow Depth = 0.01" for 2YR event Inflow $=0.04$ cfs @ 12.71 hrs, Volume $=101 \mathrm{cf}$ Outflow $=\quad 0.02$ cfs @ 13.19 hrs , Volume= 101 cf , Atten= 53\%, Lag= 29.2 min

Routed to Link AP4 : ANALYSIS POINT \#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.05 \mathrm{fps}, \mathrm{Min}$. Travel Time $=54.7 \mathrm{~min}$
Avg. Velocity $=0.03 \mathrm{fps}$, Avg. Travel Time $=80.0 \mathrm{~min}$
Peak Storage= 60 cf @ 13.19 hrs
Average Depth at Peak Storage=0.02' , Surface Width= 20.36'
Bank-Full Depth=1.00' Flow Area= 30.0 sf, Capacity= 17.57 cfs
20.00' x 1.00' deep channel, $n=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 40.00'
Length= 164.0' Slope= 0.0366 '/'
Inlet Invert= 192.00', Outlet Invert= 186.00'


## Summary for Reach 11R: 4x4 Open Bottom Culvert

[52] Hint: Inlet/Outlet conditions not evaluated
Inflow Area $=\quad 424,818$ sf, $45.99 \%$ Impervious, Inflow Depth > 0.42" for 2YR event
Inflow = 1.70 cfs @ 12.45 hrs , Volume $=\quad 14,965 \mathrm{cf}$
Outflow = $1.70 \mathrm{cfs} @ 12.45 \mathrm{hrs}$, Volume= $14,959 \mathrm{cf}$, Atten= $0 \%$, Lag= 0.3 min
Routed to Reach 23R : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=1.19 \mathrm{fps}$, Min. Travel Time $=0.4 \mathrm{~min}$
Avg. Velocity $=0.55 \mathrm{fps}$, Avg. Travel Time $=0.9 \mathrm{~min}$
Peak Storage= 43 cf @ 12.45 hrs
Average Depth at Peak Storage= 0.36' , Surface Width= 4.00'
Bank-Full Depth= 4.00' Flow Area= 16.0 sf, Capacity= 42.20 cfs
48.0" W x 48.0" H Box Pipe
$\mathrm{n}=0.069$ Riprap, 6 -inch
Length=30.0' Slope= 0.0150 '/'
Inlet Invert= 194.00', Outlet Invert= 193.55'


## Summary for Reach 12R: OVERLAND FLOW

| Inflow Area = | 12,906 sf, 90.20\% Impervious, | Inflow Depth > 1.98" for 2YR event |
| :---: | :---: | :---: |
| Inflow | 0.77 cfs @ 12.13 hrs , Volume= | 2,134 cf |
| Outflow | 0.30 cfs @ 12.43 hrs , Volume= | $2,085 \mathrm{cf}$, Atten= 61\%, Lag= 18.0 min |

Routed to Link AP2 : ANALYSIS POINT 2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.09 \mathrm{fps}, \mathrm{Min}$. Travel Time $=45.1 \mathrm{~min}$
Avg. Velocity $=0.04 \mathrm{fps}$, Avg. Travel Time $=107.5 \mathrm{~min}$
Peak Storage= 811 cf @ 12.43 hrs
Average Depth at Peak Storage $=0.06^{\prime}$, Surface Width= 50.64'
Bank-Full Depth=1.00' Flow Area= 55.0 sf, Capacity $=29.80$ cfs
$50.00^{\prime} \times 1.00$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length $=250.0^{\prime}$ Slope $=0.0240$ '/'
Inlet Invert= 202.00', Outlet Invert= 196.00'


## Summary for Reach 14R: OVERLAND FLOW

| Inflow Area = | 52,768 sf, | 0.60\% Impervious, | " for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.46 cfs @ | 12.41 hrs , Volume= | 2,770 cf |
| Outflow | 0.07 cfs @ | 14.79 hrs , Volume= | 2,124 cf, Atten= 84\%, Lag= 142.6 |

Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.05 \mathrm{fps}$, Min. Travel Time $=266.3 \mathrm{~min}$
Avg. Velocity $=0.04 \mathrm{fps}$, Avg. Travel Time $=316.0 \mathrm{~min}$
Peak Storage= 1,163 cf @ 14.79 hrs
Average Depth at Peak Storage=0.03' , Surface Width= 50.54'
Bank-Full Depth=1.00' Flow Area= 60.0 sf, Capacity= 31.55 cfs
50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 70.00'
Length=852.0' Slope= 0.0246 '/'
Inlet Invert= 207.00', Outlet Invert= 186.00'


## Summary for Reach 15R: OVERLAND FLOW

Inflow Area = 62,582 sf, 52.00\% Impervious, Inflow Depth > 0.91" for 2YR event Inflow = 0.12 cfs @ 15.47 hrs, Volume=

4,736 cf Outflow =
$4,228 \mathrm{cf}$, Atten $=2 \%$, Lag $=92.9 \mathrm{~min}$
Routed to Link AP2 : ANALYSIS POINT 2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.06 \mathrm{fps}, \mathrm{Min}$. Travel Time $=82.1 \mathrm{~min}$
Avg. Velocity $=0.06 \mathrm{fps}$, Avg. Travel Time $=90.4 \mathrm{~min}$
Peak Storage= 595 cf @ 17.02 hrs
Average Depth at Peak Storage $=0.04$ ' , Surface Width $=50.40^{\prime}$
Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 27.21 cfs
$50.00^{\prime} \times 1.00$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length=300.0' Slope= 0.0200 '/'
Inlet Invert= 202.00', Outlet Invert= 196.00'


## Summary for Reach 18R: OVERLAND FLOW

Inflow Area $=\quad 88,676$ sf, $39.42 \%$ Impervious, Inflow Depth $=0.00 "$ for 2 YR event Inflow $=0.00 \mathrm{cfs}$ @ 0.00 hrs , Volume $=0 \mathrm{cf}$
Outflow $=0.00 \mathrm{cfs} @ 0.00 \mathrm{hrs}$, Volume= 0 cf , Atten= $0 \%$, Lag $=0.0 \mathrm{~min}$
Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.00 \mathrm{fps}$, Min. Travel Time $=0.0 \mathrm{~min}$
Avg. Velocity $=0.00 \mathrm{fps}$, Avg. Travel Time $=0.0 \mathrm{~min}$
Peak Storage $=0$ cf @ 0.00 hrs
Average Depth at Peak Storage=0.00'
Bank-Full Depth= 1.00' Flow Area= 75.0 sf, Capacity= 38.42 cfs
50.00' x 1.00' deep channel, $\mathrm{n}=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 100.00'
Length=609.0' Slope= 0.0279 '/'
Inlet Invert= 203.00', Outlet Invert= 186.00'


## Summary for Reach 20R: OVERLAND FLOW

Inflow Area =
72,222 sf, 68.72\% Impervious, Inflow Depth = 0.96" for 2YR event
Inflow =
0.93 cfs @ 12.49 hrs , Volume=

5,781 cf
Outflow $=0.34$ cfs @ 13.75 hrs, Volume=
$5,373 \mathrm{cf}$, Atten $=64 \%, \quad$ Lag $=75.2 \mathrm{~min}$
Routed to Reach 11R : 4x4 Open Bottom Culvert
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.07 \mathrm{fps}$, Min. Travel Time $=128.2 \mathrm{~min}$
Avg. Velocity $=0.04 \mathrm{fps}$, Avg. Travel Time $=210.1 \mathrm{~min}$
Peak Storage= 2,612 cf @ 13.75 hrs
Average Depth at Peak Storage= 0.09' , Surface Width= 50.92'
Bank-Full Depth= 1.00' Flow Area $=55.0$ sf, Capacity= 18.54 cfs
$50.00^{\prime} \times 1.00^{\prime}$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length=560.0' Slope= 0.0093 '/'
Inlet Invert= 200.00', Outlet Invert= 194.80'


## Summary for Reach 23R: OVERLAND FLOW

| Inflow Area = | 424,818 sf, 45.99\% Impervious, | Inflow Depth > 0.42" for 2YR event |
| :---: | :---: | :---: |
| Inflow | 1.70 cfs @ 12.45 hrs, Volume= | 14,959 cf |
| Outflow | 1.24 cfs @ 12.76 hrs, Volume= | 14,613 cf, Atten= 27\%, Lag= 18.2 min |

Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.15 \mathrm{fps}, \mathrm{Min}$. Travel Time $=26.3 \mathrm{~min}$
Avg. Velocity $=0.08 \mathrm{fps}$, Avg. Travel Time $=52.5 \mathrm{~min}$

Peak Storage= 1,960 cf @ 12.76 hrs
Average Depth at Peak Storage= 0.16' , Surface Width= 56.23'
Bank-Full Depth= 1.00' Flow Area= 70.0 sf, Capacity $=31.93$ cfs
$50.00^{\prime} \times 1.00$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 20.0 '/' Top Width= 90.00'
Length $=237.0^{\prime}$ Slope $=0.0211$ '/'
Inlet Invert= 193.00', Outlet Invert= 188.00'


## Summary for Reach R202: OVERLAND FLOW

[62] Hint: Exceeded Reach SC1 OUTLET depth by 0.11 ' @ 13.30 hrs

| Inflow Area | 432,269 sf, 42.08\% Impervious, | 1.19" for 2YR event |
| :---: | :---: | :---: |
| Inflow | 8.50 cfs @ 12.35 hrs , Volume= | 43,012 cf |
| Outflow | 2.85 cfs @ 12.90 hrs , Volume= | $39,436 \mathrm{cf}, \mathrm{Atten}=66 \%, \mathrm{Lag}=33.4 \mathrm{~min}$ |

Routed to Link AP2 : ANALYSIS POINT 2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.13 \mathrm{fps}$, Min. Travel Time $=89.4 \mathrm{~min}$
Avg. Velocity $=0.07 \mathrm{fps}$, Avg. Travel Time $=163.7 \mathrm{~min}$
Peak Storage= 15,282 cf @ 12.90 hrs
Average Depth at Peak Storage= $0.21^{\prime}$, Surface Width= 110.38'
Bank-Full Depth=1.00' Flow Area= 125.0 sf, Capacity= 42.56 cfs
100.00 x 1.00' deep channel, $\mathrm{n}=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 150.00'
Length=700.0' Slope= 0.0107 '/'
Inlet Invert= 205.50', Outlet Invert= 198.00'


## Summary for Reach R211: OVERLAND FLOW

| Inflow Area $=$ | $241,078 \mathrm{sf}$, | $59.10 \%$ Impervious, | Inflow Depth $=0.00 "$ |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.00 \mathrm{cfs} @$ | 0.00 hrs , Volume $=$ |
| Outflow event | $=$ | $0.00 \mathrm{cfs} @$ | 0.00 hrs , Volume $=$ |

Routed to Reach 11R : 4x4 Open Bottom Culvert

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.00 \mathrm{fps}$, Min. Travel Time $=0.0 \mathrm{~min}$
Avg. Velocity $=0.00 \mathrm{fps}$, Avg. Travel Time $=0.0 \mathrm{~min}$
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage=0.00'
Bank-Full Depth= 1.00 Flow Area= 50.0 sf, Capacity= 14.51 cfs
$35.00^{\prime} \times 1.00^{\prime}$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 15.0 '/' Top Width= 65.00'
Length=600.0' Slope= 0.0087 '/'
Inlet Invert= 200.00', Outlet Invert= 194.80'


## Summary for Reach SC1: Stream Crossing \#1

[52] Hint: Inlet/Outlet conditions not evaluated

| In | 432,269 sf, 42.08\% Impervious, | w Depth > 1.19" for 2YR event |
| :---: | :---: | :---: |
| Inflow | 8.51 cfs @ 12.34 hrs , Volume= | 43,021 cf |
| Outflow | 8.50 cfs @ 12.35 hrs , Volume= | $43,012 \mathrm{cf}, \mathrm{Atten}=0 \%, \mathrm{Lag}=0.1 \mathrm{~min}$ |

Routed to Reach R202 : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=2.48 \mathrm{fps}$, Min. Travel Time $=0.3 \mathrm{~min}$
Avg. Velocity $=1.06 \mathrm{fps}$, Avg. Travel Time $=0.7 \mathrm{~min}$
Peak Storage= 148 cf @ 12.35 hrs
Average Depth at Peak Storage= 0.21' , Surface Width= 16.00'
Bank-Full Depth= 5.00' Flow Area= 69.8 sf, Capacity= 722.91 cfs
192.0" W x 60.0" H, R=207.0" Arch Pipe
$\mathrm{n}=0.030$ Stream, clean \& straight
Length= 43.1' Slope= 0.0200 '/'
Inlet Invert= 206.37', Outlet Invert= 205.51'


## Summary for Reach SC2: Stream Crossing \#2

[52] Hint: Inlet/Outlet conditions not evaluated

| Inflow Area $=$ | $52,768 \mathrm{sf}$, | $0.60 \%$ Impervious, | Inflow Depth $>0.63 "$ | for 2 YR event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.46 \mathrm{cfs} @$ | 12.40 hrs , Volume= | $2,770 \mathrm{cf}$ |
| Outflow | $=$ | $0.46 \mathrm{cfs} @$ | 12.41 hrs , Volume= | $2,770 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.6 \mathrm{~min}$ |

Routed to Reach 14R : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity= 1.04 fps , Min. Travel Time $=0.6 \mathrm{~min}$
Avg. Velocity $=1.04 \mathrm{fps}$, Avg. Travel Time $=0.6 \mathrm{~min}$
Peak Storage= 16 cf @ 12.41 hrs
Average Depth at Peak Storage $=0.03^{\prime}$, Surface Width= 16.00'
Bank-Full Depth $=5.00^{\prime}$ Flow Area= 68.1 sf, Capacity= 768.96 cfs
192.0" W x 60.0" H, R=180.0" Arch Pipe
$\mathrm{n}=0.030$ Stream, clean \& straight
Length= 36.5' Slope= 0.0241 '/'
Inlet Invert= 208.52', Outlet Invert= 207.64'


## Summary for Pond 1P: DMH \#33

| Inflow Area | 16,111 sf, | perviou | Inflow Depth > 2.89" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.12 cfs @ | 12.09 hrs , Volume= | 3,877 cf |
| Outflow | 1.12 cfs @ | 12.09 hrs , Volume= | 3,877 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.12 cfs @ | 12.09 hrs , Volume= | 3,877 cf |

Routed to Pond OCS6 : OCS \#6
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.05' @ 12.09 hrs
Flood Elev= 209.64'
Device Routing Invert Outlet Devices
\#1 Primary $\quad 205.50^{\prime} \quad 12.0^{\prime \prime}$ Round Culvert L=46.7' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert= 205.50' / 204.33' S=0.0251 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$
Primary OutFlow Max=1.09 cfs @ 12.09 hrs HW=206.04' TW=201.78' (Dynamic Tailwater)
L1=Culvert (Inlet Controls 1.09 cfs @ 2.51 fps )

## Summary for Pond 3P: OCS \#8

| Inflow Area = | 12,684 sf, | 4\% Impervious, | Inflow Depth > 2.44" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.79 cfs @ | 12.09 hrs , Volume= | 2,578 cf |
| Outflow | 0.79 cfs @ | 12.09 hrs , Volume= | $2,578 \mathrm{cf}$, Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 0.79 cfs @ | 12.09 hrs , Volume= | 2,578 cf |

Routed to Pond P214 : STORMTECH INFILTRATION SYSTEM \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=201.07' @ 12.09 hrs
Flood Elev= 206.36'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | :---: | :---: | :---: | :---: |
| $\# 1$ | Primary | $200.62^{\prime}$ | $12.0^{\prime \prime}$ Vert. Orifice/Grate $\quad$ C= $0.600 \quad$ Limited to weir flow at low heads |  |

Primary OutFlow Max=0.77 cfs @ 12.09 hrs HW=201.07' TW=200.49' (Dynamic Tailwater)
——1=Orifice/Grate (Orifice Controls 0.77 cfs @ 2.28 fps )

## Summary for Pond 5R: TRENCH DRAIN



Primary OutFlow Max=0.70 cfs @ 12.09 hrs HW=197.73' TW=195.23' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 0.70 cfs @ 2.43 fps )

## Summary for Pond 11P: YARD DRAIN

| Inflow Area = | 21,407 sf | \%\% Impervious, | Depth > | .39" for 2YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 0.78 cfs @ | 12.10 hrs , Volume= | 2,476 cf |  |
| Outflow | 0.51 cfs @ | 12.20 hrs , Volume= | 2,448 cf, | Atten $=34 \%, L a g=6.4 \mathrm{~min}$ |
| Primary | 0.51 cfs @ | 12.20 hrs , Volume= | 2,448 cf |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 207.32' @ 12.20 hrs Surf.Area= 5,486 sf Storage= 383 cf
Plug-Flow detention time= 22.4 min calculated for 2,448 cf ( $99 \%$ of inflow)
Center-of-Mass det. time= 15.7 min ( 859.9-844.2)


Primary OutFlow Max=0.51 cfs @ 12.20 hrs HW=207.32' TW=202.75' (Dynamic Tailwater)
L-1=Culvert (Passes 0.51 cfs of 6.55 cfs potential flow)
L2=Orifice/Grate (Weir Controls 0.51 cfs @ 0.88 fps )

## Summary for Pond CB10: CB \#10

 Routed to Pond D5 : DMH \#5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=210.18' @ 12.09 hrs
Flood Elev= 212.93'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | ---: | :--- | :--- |
| $\# 1$ | Primary | $209.76^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=33.8^{\prime} \quad \mathrm{Ke}=0.500$ |  |
|  |  |  | Inlet / Outlet Invert= 209.76' $/ 209.59^{\prime} \quad \mathrm{S}=0.0050$ | $\prime / \prime \quad \mathrm{Cc}=0.900$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |

Primary OutFlow Max=0.48 cfs @ 12.09 hrs HW=210.17' TW=209.85' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.48 cfs @ 2.34 fps )

## Summary for Pond CB11: CB \#11

| Inflow Area = | 7,173 sf | 00\% Impervious, | Inflow Depth > 3.04" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.51 cfs @ | 12.09 hrs , Volume= | 1,814 cf |
| Outflow | 0.51 cfs @ | 12.09 hrs , Volume= | $1,814 \mathrm{cf}$, Atten= $0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | 0.51 cfs @ | 12.09 hrs , Volume= | 1,814 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 210.32' @ 12.09 hrs
Flood Elev= 213.13'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 209.94' | 12.0" Round Culvert L= 26.3' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=209.94' ${ }^{209.67 ' S=0.0103 ~ ' / l ' C c=0.900 ~}$ $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.50 cfs @ 12.09 hrs HW=210.31' TW=209.85' (Dynamic Tailwater)
——1=Culvert (Barrel Controls 0.50 cfs @ 2.79 fps )

## Summary for Pond CB12: CB \#12



Routed to Pond 1P : DMH \#33
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=206.98' @ 12.09 hrs
Flood Elev= 209.84'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 206.68' | 12.0" Round Culvert L= 41.3' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=206.68' $/ 205.65$ ' $\mathrm{S}=0.0249$ '// Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.36 cfs @ 12.09 hrs HW=206.98' TW=206.04' (Dynamic Tailwater)


## Summary for Pond CB13: CB \#13

| Inflow Area = | 10,873 sf, | 90.78\% Impervious, | Inflow Depth > 2.82" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.75 cfs @ | 12.09 hrs , Volume= | 2,552 cf |
| Outflow | 0.75 cfs @ | 12.09 hrs , Volume= | 2,552 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.75 cfs @ | 12.09 hrs , Volume= | 2,552 cf | Routed to Pond 1P : DMH \#33

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.14' @ 12.09 hrs
Flood Elev= 209.86'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | :--- | :--- | :--- |
| $\# 1$ | Primary | $206.70^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert L= 43.7' $\mathrm{Ke}=0.500$ |  |

Inlet / Outlet Invert= 206.70' / 205.61' S=0.0249 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=0.73 cfs @ 12.09 hrs HW=207.13' TW=206.04' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 0.73 cfs @ 2.24 fps )

## Summary for Pond CB14: CB \#14



Primary OutFlow Max=0.71 cfs @ 12.09 hrs HW=201.40' TW=201.26' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.71 cfs @ 2.02 fps )

## Summary for Pond CB15: CB \#15

| Inflow Area $=$ | $6,666 \mathrm{sf}, 100.00 \%$ Impervious, | Inflow Depth > 3.04" | for 2YR event |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.47 \mathrm{cfs} @$ | 12.09 hrs , Volume |
| Outflow | $=$ | $0.47 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |
| Primary | $=$ | $0.47 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | Routed to Pond D8 : DMH \#8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=201.34' @ 12.09 hrs
Flood Elev= 203.95'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $200.79^{\prime}$ | $\mathbf{1 2 . 0 ^ { \prime \prime }}$ Round Culvert $\mathrm{L}=15.6^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 200.79' $/ 200.71^{\prime} \quad \mathrm{S}=0.00511^{\prime \prime \prime} \quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=0.46 cfs @ 12.09 hrs HW=201.33' TW=201.26' (Dynamic Tailwater)
\& 1 = Culvert (Outlet Controls 0.46 cfs @ 1.54 fps )

## Summary for Pond CB16: CB \#16

| Inflow Area = | 8,516 | 64.88\% Imperviou | w Depth > 1.39" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.31 cfs @ | 12.10 hrs , Volume= | 985 cf |
| Outflow | 0.31 cfs @ | 12.10 hrs , Volume= | 985 cf , Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 0.31 cfs @ | 12.10 hrs , Volume= | 985 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 203.82' @ 12.10 hrs
Flood Elev=206.64'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 203.47' | 12.0" Round Culvert L= 20.9' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=203.47' / 203.33' S=0.0067 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.31 cfs @ 12.10 hrs HW=203.81' TW=203.66' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.31 cfs @ 1.90 fps )

## Summary for Pond CB17: CB \#17



## Summary for Pond CB18: CB \#18

| Inflow Area = | 24,853 | perviou | Inflow Depth > 1.87" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.28 cfs @ | 12.10 hrs , Volume= | 3,873 cf |
| Outflow | 1.28 cfs @ | 12.10 hrs , Volume= | $3,873 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.28 cfs @ | 12.10 hrs , Volume= | 3,873 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.37' @ 12.10 hrs
Flood Elev= 208.16'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $204.72^{\prime}$ | 15.0" Round Culvert $\mathrm{L}=25.1^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $204.72^{\prime} / 204.59^{\prime} \quad \mathrm{S}=0.0052$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=1.23 \mathrm{sf}$ |  |

Primary OutFlow Max=1.28 cfs @ 12.10 hrs HW=205.37' TW=205.02' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 1.28 cfs @ 2.90 fps )

## Summary for Pond CB20: CB \#20



Primary OutFlow Max=0.78 cfs @ 12.09 hrs HW=204.50' TW=203.95' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.78 cfs @ 2.66 fps )

## Summary for Pond CB21: CB \#21

| Inflow Area $=$ | $10,174 \mathrm{sf}, 87.04 \%$ Impervious, | Inflow Depth > $2.23 "$ | for 2 YR event |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.59 \mathrm{cfs} @$ | 12.09 hrs , Volume |
| Outflow | $=$ | $0.59 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |
| Primary | $=$ | $0.59 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |

Routed to Pond D12 : DMH \#12
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=204.78' @ 12.09 hrs
Flood Elev= 208.02'


Primary OutFlow Max=0.58 cfs @ 12.09 hrs HW=204.78' TW=203.95' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.58 cfs @ 2.43 fps )

## Summary for Pond CB22: CB \#22

| Inflow Area = | 12,001 sf, | 91.62\% Impervious, | Inflow Depth > 2.82" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.83 cfs @ | 12.09 hrs , Volume= | 2,817 cf |
| Outflow | 0.83 cfs @ | 12.09 hrs , Volume= | $2,817 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.83 cfs @ | 12.09 hrs , Volume= | 2,817 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 205.89' @ 12.09 hrs
Flood Elev=208.50'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 205.33' | 12.0" Round Culvert $\mathrm{L}=16.1^{\prime} \mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert=205.33' / 205.25' S=0.0050 '// Cc= 0.900 <br> $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 0.79 sf |

Primary OutFlow Max=0.80 cfs @ 12.09 hrs HW=205.88' TW=204.90' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.80 cfs @ 2.64 fps )

## Summary for Pond CB23: CB \#23



Routed to Pond D14 : DMH \#14
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.85' @ 12.09 hrs
Flood Elev= 208.57'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 205.41' | 12.0" Round Culvert L= 16.3' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=205.41' / 205.32 ' $\mathrm{S}=0.0055$ '/' Cc= 0.900 $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 0.79 sf |
| Primary OutFlow Max=0.53 cfs @ 12.09 hrs HW=205.84' TW=204.90' (Dynamic Tailwater) <br> L $_{1=\text { Culvert (Barrel Controls } 0.53 \text { cfs } @ 2.42 \mathrm{fps} \text { ) }}$ |  |  |  |

## Summary for Pond CB24: CB \#24

| Inflow Area = | 7,930 | 72.16\% Imperviou | Inflow Depth > 2.51" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.51 cfs @ | 12.09 hrs , Volume= | 1,660 cf |
| Outflow | 0.51 cfs @ | 12.09 hrs , Volume= | 1,660 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.51 cfs @ | 12.09 hrs , Volume= | 1,660 cf | Routed to Pond D16 : DMH \#16

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=205.64' @ 12.09 hrs
Flood Elev= 209.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $205.21^{\prime}$ | 12.0" Round Culvert $\mathrm{L}=12.1^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $205.21^{\prime} / 205.15^{\prime} \quad \mathrm{S}=0.0050$ |
|  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=0.49 cfs @ 12.09 hrs HW=205.63' TW=205.46' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.49 cfs @ 2.31 fps )

## Summary for Pond CB25: CB \#25

| Inflow Area = | 8,487 | mpervious, | Inflow Depth > 2.61" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.56 cfs @ | 12.09 hrs , Volume= | 1,846 cf |
| Outflow | 0.56 cfs @ | 12.09 hrs , Volume= | $1,846 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.56 cfs @ | 12.09 hrs , Volume= | 1,846 cf | Routed to Pond D16 : DMH \#16

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=205.64' @ 12.09 hrs
Flood Elev= 208.38'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $205.22^{\prime}$ | $15.0^{\prime \prime}$ Round Culvert $\mathrm{L}=11.4^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= $205.22^{\prime} / 205.16^{\prime} \quad \mathrm{S}=0.0053 \mathrm{I} / \mathrm{l} \quad \mathrm{Cc}=0.900$ |  |
| $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |  |  |  |

Primary OutFlow Max=0.54 cfs @ 12.09 hrs HW=205.63' TW=205.46' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.54 cfs @ 2.31 fps )

## Summary for Pond CB26: CB \#26



Routed to Pond D17 : DMH \#17
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=202.20' @ 12.09 hrs
Flood Elev=204.93'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $201.77^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=42.5^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= $201.77^{\prime} / 201.55^{\prime} \quad \mathrm{S}=0.0052$ '/' $\quad \mathrm{Cc}=0.900$ |  |
| $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |  |

Primary OutFlow Max=0.52 cfs @ 12.09 hrs HW=202.19' TW=201.05' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.52 cfs @ 2.42 fps )

## Summary for Pond CB27: CB \#27

| Inflow Area = | 6,111 | 91.90\% Impervious, | flow Depth > 2.82" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.42 cfs @ | 12.09 hrs , Volume= | 1,434 cf |
| Outflow | 0.42 cfs @ | 12.09 hrs , Volume= | $1,434 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.42 cfs @ | 12.09 hrs , Volume= | 1,434 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 201.38' @ 12.09 hrs
Flood Elev= 204.16'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $201.00 '$ | $\mathbf{1 2 . 0}$ " Round Culvert $\mathrm{L}=18.0^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 201.00 ' $200.90^{\prime} \quad \mathrm{S}=0.0056 \mathrm{I} / \mathrm{l} \quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=0.41 cfs @ 12.09 hrs HW=201.38' TW=201.04' (Dynamic Tailwater)
——1=Culvert (Barrel Controls 0.41 cfs @ 2.24 fps )

## Summary for Pond CB28: CB \#28

| Inflow Area = | 10,372 sf, | 33\% Impervious, | Inflow Depth > 2.06" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.56 cfs @ | 12.09 hrs , Volume= | 1,779 cf |
| Outflow | 0.56 cfs @ | 12.09 hrs , Volume= | $1,779 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | 0.56 cfs @ | 12.09 hrs , Volume= | 1,779 cf |

Routed to Pond D18: DMH \#18
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev= 198.22' @ 12.09 hrs
Flood Elev= 200.92'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 197.75' | 12.0" Round Culvert L= 13.7' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 197.75' / 197.69' S=0.0044 '/l' Cc=0.900 $n=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| $\underbrace{P r i m a r}_{1=C}$ | OutFIow lvert (O | 55 cfs trols 0.5 | 12.09 hrs HW=198.21' TW=198.05' (Dynamic Tailwater) cfs @ 2.25 fps ) |

## Summary for Pond CB29: CB \#29



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.89' @ 12.09 hrs
Flood Elev= 208.55'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 205.38' | 12.0" Round Culvert L= 13.5' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 205.38' / 205.31' S=0.0052 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |

Primary OutFlow Max=0.54 cfs @ 12.09 hrs HW=205.88' TW=205.75' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.54 cfs @ 2.02 fps )

## Summary for Pond CB30: CB \#30

| Inflow Area = | 8,933 sf, 8 | 82.40\% Impervious, | Inflow Depth > 2.61" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 0.59 cfs @ 12. | 12.09 hrs , Volume= | 1,943 cf |
| Outflow | 0.59 cfs @ 12 | 12.09 hrs , Volume= | $1,943 \mathrm{cf}$, Atten= $0 \%$, Lag= 0.0 min |
| Primary = Routed to Pond | $\begin{aligned} & 0.59 \mathrm{cfs} @ \\ & \text { D19 : DMH } \\ & \text { \# } \end{aligned}$ | 12.09 hrs , Volume= 19 | $1,943 \mathrm{cf}$ |
| Routing by Dyn-St <br> Peak Elev= 205.9 <br> Flood Elev= 208.5 | r-Ind method, @ 12.09 hrs | , Time Span= 0.00-2 | $.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 205.38' | 12.0" Round Cul Inlet / Outlet Inver $n=0.013$ Corruga | $\begin{aligned} & \text { Ivert } \mathrm{L}=17.5^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=205.38^{\prime} / 205.29 \text { ' } \mathrm{S}=0.0051 \text { '/' } \mathrm{Cc}=0.900 \\ & \text { ated PE, smooth interior, Flow Area= } 0.79 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=0.57 cfs @ 12.09 hrs HW=205.89' TW=205.75' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.57 cfs @ 2.05 fps )

## Summary for Pond CB31: CB \#31

| Inflow Area = | 16,365 | \% Impervious, | Inflow Depth > 2.23" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.95 cfs @ | 12.09 hrs , Volume= | 3,044 cf |
| Outflow | 0.95 cfs @ | 12.09 hrs , Volume= | $3,044 \mathrm{cf}$, Atten $=0 \%, L a g=0.0 \mathrm{~min}$ |
| Primary | 0.95 cfs @ | 12.09 hrs, Volume= | 3,044 cf |

Routed to Pond D21 : DMH \#21
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=204.80' @ 12.09 hrs
Flood Elev= 207.36'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $204.19^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=16.4^{\prime} \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= 204.19' $/ 204.11^{\prime} \mathrm{S}=0.0049$ '/' $\quad \mathrm{Cc}=0.900$ |  |
|  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |

Primary OutFlow Max=0.93 cfs @ 12.09 hrs HW=204.79' TW=204.06' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.93 cfs @ 2.69 fps )

## Summary for Pond CB32: CB \#32

| Inflow Area = | 12,710 | .47\% Imperviou | $w$ Depth > 2.32" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.76 cfs @ | 12.09 hrs , Volume= | 2,460 cf |
| Outflow | 0.76 cfs @ | 12.09 hrs , Volume= | $2,460 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.76 cfs @ | 12.09 hrs , Volume= | 2,460 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 204.73' @ 12.09 hrs
Flood Elev=207.35'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 204.19' | 12.0" Round Culvert L= 16.3' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 204.19' / 204.11' S=0.0049 '// Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.75 cfs @ 12.09 hrs HW=204.72' TW=204.06' (Dynamic Tailwater)
——=Culvert (Barrel Controls 0.75 cfs @ 2.54 fps )

## Summary for Pond CB33: CB \#33



## Summary for Pond CB34: CB \#34

| Inflow Area = | 8,622 | 80.51\% Imperviou | Inflow Depth > 2.51" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.55 cfs @ | 12.09 hrs , Volume= | 1,804 cf |
| Outflow | 0.55 cfs @ | 12.09 hrs , Volume= | $1,804 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.55 cfs @ | 12.09 hrs , Volume= | 1,804 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=205.69' @ 12.09 hrs
Flood Elev= 208.38'


Primary OutFlow Max=0.54 cfs @ 12.09 hrs HW=205.68' TW=205.53' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.54 cfs @ 2.15 fps )

## Summary for Pond CB35: CB \#35

| Inflow Area = | 4,149 | 98.10\% Imperviou | Inflow Depth > 3.04" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.30 cfs @ | 12.09 hrs , Volume= | 1,049 cf |
| Outflow | 0.30 cfs @ | 12.09 hrs , Volume= | $1,049 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.30 cfs @ | 12.09 hrs , Volume= | 1,049 cf | Routed to Pond D23: DMH \#23

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=207.36' @ 12.09 hrs
Flood Elev= 210.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $207.04 '$ | $\mathbf{1 2 . 0 "}$ Round Culvert $\mathrm{L}=15.2^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 207.04' $/ 206.96^{\prime} \quad \mathrm{S}=0.0053^{\prime} / l^{\prime} \quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=0.29 cfs @ 12.09 hrs HW=207.36' TW=207.11' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.29 cfs @ 2.01 fps )

## Summary for Pond CB36: CB \#36

| Inflow Area $=$ | $6,622 \mathrm{sf}, 100.00 \%$ Impervious, | Inflow Depth > 3.04" | for 2YR event |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | 0.47 cfs @ | 12.09 hrs , Volume |
| Outflow | $=$ | $0.47 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |
| Primary | $=$ | $0.47 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |

Routed to Pond D23 : DMH \#23
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.45' @ 12.09 hrs
Flood Elev=210.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $207.04 '$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=16.1^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 207.04' $/ 206.96^{\prime} \quad \mathrm{S}=0.0050$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=0.46 cfs @ 12.09 hrs HW=207.45' TW=207.11' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.46 cfs @ 2.25 fps )

## Summary for Pond CB38: CB \#38

| Inflow Area = | 7,637 sf,100.00\% Impervious, | low Depth > 3.04" for 2YR event |
| :---: | :---: | :---: |
| Inflow | 0.54 cfs @ 12.09 hrs, Volume= | 1,932 cf |
| Outflow | 0.54 cfs @ 12.09 hrs , Volume= | 1,932 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.54 cfs @ 12.09 hrs, Volume= | 1,932 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 210.28' @ 12.09 hrs
Flood Elev=212.86'


Primary OutFlow Max=0.53 cfs @ 12.09 hrs HW=210.26' TW=210.18' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.53 cfs @ 1.63 fps )

## Summary for Pond CB39: CB \#39



Routed to Pond D25: DMH \#25
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=210.28' @ 12.09 hrs
Flood Elev= 212.86'


## Summary for Pond CB40: CB \#40

| Inflow Area = | 4,211 sf,100.00\% Impervious, | Inflow Depth > 3.04" for 2YR event |
| :---: | :---: | :---: |
| Inflow | 0.30 cfs @ 12.09 hrs, Volume= | 1,065 cf |
| Outflow | 0.30 cfs @ 12.09 hrs, Volume= | $1,065 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | 0.30 cfs @ 12.09 hrs, Volume= | 1,065 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=214.02' @ 12.09 hrs
Flood Elev= 217.04'


Primary OutFlow Max=0.29 cfs @ 12.09 hrs HW=214.01' TW=213.87' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.29 cfs @ 1.89 fps )

## Summary for Pond CB41: CB \#41

| Inflow Area = | 5,586 sf, 100.00\% Impervious, | Inflow Depth > 3.04" for 2YR event |
| :---: | :---: | :---: |
| Inflow | 0.40 cfs @ 12.09 hrs, Volume= | 1,413 cf |
| Outflow | 0.40 cfs @ 12.09 hrs , Volume= | $1,413 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.40 cfs @ 12.09 hrs, Volume= | 1,413 cf | Routed to Pond D27 : DMH \#27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=214.27' @ 12.09 hrs
Flood Elev=217.06'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $213.89^{\prime}$ | $\mathbf{1 2 . 0}$ " Round Culvert $\mathrm{L}=18.4^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= $213.89^{\prime} / 213.80^{\prime} \quad \mathrm{S}=0.0049 \mathrm{I} / \mathrm{l} \quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=0.39 cfs @ 12.09 hrs HW=214.26' TW=213.87' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.39 cfs @ 2.16 fps )

## Summary for Pond CB43: CB \#43

| Inflow Area = | 3,109 sf, | .36\% Impervious | Depth > 2.14" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.17 cfs @ | 12.09 hrs , Volume= | 555 cf |
| Outflow | 0.17 cfs @ | 12.09 hrs , Volume= | 555 cf , Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 0.17 cfs @ | 12.09 hrs , Volume= | 555 cf | Routed to Pond D29: DMH \#29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=220.29' @ 12.09 hrs
Flood Elev=223.17'


Primary OutFlow Max=0.17 cfs @ 12.09 hrs HW=220.28' TW=220.20' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.17 cfs @ 1.41 fps )

## Summary for Pond CB44: CB \#44

| Inflow Area = | 1,978 | 84.43\% Impervious, | Depth > 2.42" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.12 cfs @ | 12.09 hrs , Volume= | 398 cf |
| Outflow | 0.12 cfs @ | 12.09 hrs , Volume= | 398 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.12 cfs @ | 12.09 hrs , Volume= | 398 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev=220.26' @ 12.09 hrs
Flood Elev=223.17'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $220.00 '$ | 12.0" Round Culvert $\mathrm{L}=14.9^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= $220.00^{\prime} / 219.93^{\prime} \quad \mathrm{S}=0.0047 \mathrm{I} / \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=0.12 cfs @ 12.09 hrs HW=220.26' TW=220.20' (Dynamic Tailwater)
——1=Culvert (Outlet Controls 0.12 cfs @ 1.14 fps )

## Summary for Pond CB45: CB \#45



## Summary for Pond CB46: CB \#46

 Routed to Pond D30 : DMH \#30

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=221.77' @ 12.10 hrs
Flood Elev= 224.69'
Device Routing Invert Outlet Devices
\#1 Primary 221.53' 12.0" Round Culvert L= 15.3' Ke=0.500
Inlet / Outlet Invert= 221.53' / 221.45' S=0.0052 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=0.17 cfs @ 12.10 hrs HW=221.77' TW=221.21' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.17 cfs @ 1.74 fps )

## Summary for Pond CB47: CB\#47

| Inflow Area = | 3,012 sf, 100.00\% Impervious, Inflow Depth > 3.04" for 2YR event |  |  |
| :---: | :---: | :---: | :---: |
| Inflow = | $0.21 \text { cfs @ } 1$ | .09 hrs , Volume= | 762 cf |
| Outflow = | 0.21 cfs @ 12 | 2.09 hrs, Volume= | 762 cf, Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pon | 0.21 cfs @ 12 D31 : DMH\#3 | .09 hrs , Volume= | 762 cf |
| Routing by Dyn-S <br> Peak Elev= 225.32 <br> Flood Elev= 230 | or-Ind method, ' @ 12.09 hrs $1^{\prime}$ | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 225.05' | 12.0" Round Cul Inlet / Outlet Inver $\mathrm{n}=0.012$ Corruga | $\begin{aligned} & \text { lvert } \mathrm{L}=20.9^{\prime} \mathrm{Ke}=0.500 \\ & \mathrm{t}=225.05^{\prime} / 224.95^{\prime} \mathrm{S}=0.0048 \mathrm{l} / \mathrm{Cc}=0.900 \end{aligned}$ $\text { ted PP, smooth interior, Flow Area= } 0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.21 cfs @ 12.09 hrs HW=225.31' TW=225.08' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.21 cfs @ 1.89 fps )

## Summary for Pond CB48: CB\#48

 Routed to Pond D31 : DMH\#31

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=225.40' @ 12.19 hrs
Flood Elev=230.25'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $224.82^{\prime}$ | $15.0^{\prime \prime}$ Round Culvert $\mathrm{L}=16.9^{\prime}$ <br>  |
|  |  | Inlet / Outlet Invert= $224.82^{\prime} / 224.74^{\prime} \mathrm{S}=0.500$ |  |
| $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |  |  |  |

Primary OutFlow Max=1.01 cfs @ 12.19 hrs HW=225.40' TW=225.14' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 1.01 cfs @ 2.70 fps )

## Summary for Pond CB49: CB \#49

| Inflow Area $=$ | $5,238 \mathrm{sf}, 84.59 \%$ | Impervious, | Inflow Depth $>$ |
| :--- | :--- | :--- | :--- |
| Inflow $2.61 "$ | for 2 YR event |  |  |
| O | $0.34 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $1,139 \mathrm{cf}$ |
| Outflow | $=$ | $0.34 \mathrm{cfs} @$ | 12.09 hrs , Volume= |
| Primary | $=$ | $0.34 \mathrm{cfs} @$ | 12.09 hrs , Volume= |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 203.25' @ 12.09 hrs
Flood Elev=205.93'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.76' | 12.0" Round Culvert L= 15.5' Ke= 0.500 <br> Inlet / Outlet Invert= 202.76' / 202.68' S=0.0052 '// Cc= 0.900 <br> $n=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79$ sf |

Primary OutFlow Max=0.34 cfs @ 12.09 hrs HW=203.24' TW=203.18' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.34 cfs @ 1.31 fps )

## Summary for Pond CB50: CB \#50



## Summary for Pond CB51: CB \#51

| Inflow Area = | 6,823 sf, 100.00\% Impervious, | Inflow Depth > 3.04" for 2YR event |
| :---: | :---: | :---: |
| Inflow | 0.49 cfs @ 12.09 hrs , Volume= | 1,726 cf |
| Outflow | 0.49 cfs @ 12.09 hrs , Volume= | $1,726 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.49 cfs @ 12.09 hrs , Volume= | 1,726 cf | Routed to Pond OCS7 : OCS \#7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=202.76' @ 12.09 hrs
Flood Elev=212.77'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.35' | 12.0" Round Culvert L=31.4' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 202.35' / 202.19' S=0.0051 '/' Cc= 0.900 $\mathrm{n}=0.013$, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.47 cfs @ 12.09 hrs HW=202.76' TW=202.32' (Dynamic Tailwater)
$\leftarrow_{1=C u l v e r t ~(B a r r e l ~ C o n t r o l s ~} 0.47$ cfs @ 2.33 fps )

## Summary for Pond CB52: CB \#52

 Routed to Pond OCS7 : OCS \#7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=203.15' @ 12.09 hrs
Flood Elev= 205.84'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.68 ' | 12.0" Round Culvert L= 25.5' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert=202.68' / 202.55' S=0.0051 '/l' Cc=0.900 <br> $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.59 cfs @ 12.09 hrs HW=203.14' TW=202.32' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.59 cfs @ 2.45 fps )

## Summary for Pond CB53: CB \#53

 Routed to Pond 3P : OCS \#8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=203.20' @ 12.09 hrs
Flood Elev=205.95'
Device Routing Invert Outlet Devices
\#1 Primary $\quad 202.78^{\prime} \quad 12.0 "$ Round Culvert L=32.0' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert=202.78' / 202.62' S=0.0050 '/' Cc=0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$
Primary OutFlow Max=0.49 cfs @ 12.09 hrs HW=203.20' TW=201.07' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.49 cfs @ 2.34 fps )

## Summary for Pond CB54: CB \#54



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 202.98' @ 12.09 hrs
Flood Elev= 205.82'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.66' | 12.0" Round Culvert L=36.7' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert= 202.66' / 202.48' S=0.0049 '/l' Cc= 0.900 <br> $n=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79$ sf |

Primary OutFlow Max=0.28 cfs @ 12.09 hrs HW=202.97' TW=201.07' (Dynamic Tailwater)
——=Culvert (Barrel Controls 0.28 cfs @ 2.03 fps )

## Summary for Pond CB7: CB\#5

 Routed to Pond D4 : DMH\#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=212.90' @ 12.09 hrs
Flood Elev=215.79'


## Summary for Pond CB8: CB\#8



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=214.11' @ 12.09 hrs
Flood Elev= 215.79'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | ---: | :--- | :--- |
| $\# 1$ | Primary | $213.79^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert L=15.1' $\mathrm{Ke}=0.500$ |  |

Inlet / Outlet Invert= 213.79' / 213.64' S=0.0099 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.35 cfs @ 12.09 hrs HW=214.11' TW=210.95' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.35 cfs @ 2.42 fps )

## Summary for Pond CB9: CB \#9

| Inflow Area = | 16,307 sf, | .95\% Impervious, | Inflow Depth > 2.92" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.14 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 3,974 cf |
| Outflow | 1.14 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | $3,974 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pon | $\begin{aligned} & 1.14 \mathrm{cfs} @ 12 \\ & 1 \text { D5 : DMH \#5 } \end{aligned}$ | .09 hrs , Volume= | 3,974 cf |
| Routing by Dyn-S <br> Peak Elev= 210.6 <br> Flood Elev= 213. | or-Ind method, @ 12.09 hrs | 「ime Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 210.10' | 12.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { lvert } \mathrm{L}=19.9 \text { ' } \mathrm{Ke}=0.500 \\ & \mathrm{t}=210.10 \text { ' } 209.71 \text { ' } \mathrm{S}=0.0196 \text { '/' } \mathrm{Cc}=0.900 \\ & \text { ted PE, smooth interior, Flow Area= } 0.79 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=1.11 cfs @ 12.09 hrs HW=210.65' TW=209.85' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.11 cfs @ 2.52 fps)

## Summary for Pond D10: DMH \#10

 Routed to Pond P207 : INFILTRATION POND \#2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=203.66' @ 12.10 hrs
Flood Elev= 206.49'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $203.33^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=15.6^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $203.33^{\prime} / 203.25^{\prime} \quad \mathrm{S}=0.0051 \mathrm{l}$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.900$ |  |

Primary OutFlow Max=0.31 cfs @ 12.10 hrs HW=203.66' TW=197.13' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.31 cfs @ 2.04 fps )

## Summary for Pond D11: DMH \#11

| Inflow Area = | 36,689 sf, 73.28\% Impervious, | Depth > 2.05" for 2YR event |
| :---: | :---: | :---: |
| Inflow | 2.02 cfs @ 12.10 hrs , Volume= | 6,256 cf |
| Outflow | 2.02 cfs @ 12.10 hrs , Volume= | $6,256 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 2.02 cfs @ 12.10 hrs , Volume= | 6,256 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 205.02' @ 12.10 hrs
Flood Elev= 208.33'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $204.25^{\prime}$ | $\mathbf{1 8 . 0 ^ { \prime \prime } \text { Round Culvert } \mathrm { L } = 4 4 . 6 ^ { \prime } \quad \mathrm { Ke } = 0 . 5 0 0}$ |
|  |  | Inlet / Outlet Invert= 204.25' $/ 204.03^{\prime} \quad \mathrm{S}=0.0049 \mathrm{I} / \mathrm{l} \quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.77 sf |  |

Primary OutFlow Max=1.99 cfs @ 12.10 hrs HW=205.01' TW=203.95' (Dynamic Tailwater)
——=Culvert (Barrel Controls 1.99 cfs @ 3.22 fps)

## Summary for Pond D12: DMH \#12

| Inflow Area = | 22,113 sf, 88.07\% Impervious, Inflow Depth > 2.49" for 2YR event |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 1.40 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 4,590 cf |  |
| Outflow | 1.40 cfs @ 12 | . 09 hrs , Volume= | $4,590 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |  |
| Primary $=\quad 1.40 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume $=$Routed to Pond D13: DMH \#13 |  |  |  |  |
|  |  |  |  |  |
| Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$ <br> Peak Elev= 203.96' @ 12.09 hrs <br> Flood Elev=207.78' |  |  |  |  |
|  |  |  |  |  |
|  |  |  |  |  |
| Device Routing | Invert Outlet Devices |  |  |  |
| Inlet / Outlet Invert=203.21' / 203.00' S=0.0050 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |  |  |  |  |
| Primary OutFlow Max=1.36 cfs @ 12.09 hrs HW=203.95' TW=202.96' (Dynamic Tailwater) —1 $_{1=\text { Culvert }}$ (Barrel Controls 1.36 cfs @ 3.06 fps ) |  |  |  |  |

## Summary for Pond D13: DMH \#13

| Inflow Area = | 81,632 | 72.61\% Impervious, | Inflow Depth > 2.22" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 4.21 cfs @ | 12.09 hrs , Volume= | 15,091 cf |
| Outflow | 4.21 cfs @ | 12.09 hrs , Volume= | 15,091 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 4.21 cfs @ | 12.09 hrs , Volume= | 15,091 cf |

Routed to Pond P207 : INFILTRATION POND \#2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=202.97' @ 12.09 hrs
Flood Elev= 208.12'
Device Routing Invert Outlet Devices
\#1 Primary
201.95' 24.0" Round Culvert L=60.1' $\mathrm{Ke}=0.500$

Inlet / Outlet Invert= 201.95' / 201.65' S=0.0050 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 3.14 sf
Primary OutFlow Max=4.15 cfs @ 12.09 hrs HW=202.96' TW=197.13' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 4.15 cfs @ 3.80 fps )

## Summary for Pond D14: DMH \#14



Primary OutFlow Max=2.37 cfs @ 12.09 hrs HW=204.90' TW=202.96' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 2.37 cfs @ 3.79 fps )

## Summary for Pond D16: DMH \#16



Routed to Pond D14 : DMH \#14
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.47' @ 12.09 hrs
Flood Elev= 208.59'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $204.90^{\prime}$ | $15.0^{\prime \prime}$ Round Culvert L=103.5' $\mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= 204.90' $/ 204.38^{\prime} \quad \mathrm{S}=0.0050$ '/' $\quad \mathrm{Cc}=0.900$ |  |
|  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |  |  |

Primary OutFlow Max=1.04 cfs @ 12.09 hrs HW=205.46' TW=204.90' (Dynamic Tailwater)
-1=Culvert (Outlet Controls 1.04 cfs @ 2.84 fps )

## Summary for Pond D17: DMH \#17

| Inflow Area = | 14,946 | 26\% Impervious, | 2.52" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.95 cfs @ | 12.09 hrs , Volume= | 3,144 cf |
| Outflow | 0.95 cfs @ | 12.09 hrs , Volume= | $3,144 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.95 cfs @ | 12.09 hrs , Volume= | 3,144 cf | Routed to Pond D18: DMH \#18

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 201.05' @ 12.09 hrs
Flood Elev= 204.84'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $200.55^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=91.6^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= $200.55^{\prime} / 197.69^{\prime} \quad \mathrm{S}=0.0312^{\prime} / /^{\prime} \quad \mathrm{Cc}=0.900$ |  |
| $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |  |

Primary OutFlow Max=0.93 cfs @ 12.09 hrs HW=201.04' TW=198.05' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 0.93 cfs @ 2.40 fps )

## Summary for Pond D18: DMH \#18



## Summary for Pond D19: DMH \#19

| Inflow Area = | 17,428 | 83.29\% Impervious, | nflow Depth > 2.61" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.15 cfs @ | 12.09 hrs , Volume= | 3,791 cf |
| Outflow | 1.15 cfs @ | 12.09 hrs , Volume= | $3,791 \mathrm{cf}$, Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 1.15 cfs @ | 12.09 hrs , Volume= | 3,791 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=205.76' @ 12.09 hrs
Flood Elev= 208.57'
Device Routing Invert Outlet Devices
\#1 Primary 205.19' 12.0" Round Culvert L= 82.5' Ke=0.500
Inlet / Outlet Invert= 205.19' / 204.43' S=0.0092 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=1.12 cfs @ 12.09 hrs HW=205.75' TW=204.78' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 1.12 cfs @ 3.57 fps )

## Summary for Pond D2: DMH\#2

| Inflow Area = | $73,240 \mathrm{sf}, 3$ | 37.72\% Impervious, | Inflow Depth > 1.22" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.72 cfs @ 12 | 12.13 hrs , Volume= | 7,465 cf |
| Outflow | 1.72 cfs @ 12 | 12.13 hrs , Volume= | 7,465 cf, Atten= 0\%, Lag= 0.0 min |
| Primary = Routed to Pon | $\begin{aligned} & 1.72 \mathrm{cfs} @ 12 \\ & \text { P205: INFILTH } \end{aligned}$ | 12.13 hrs , Volume= TRATION POND \#3 | 7,465 cf |
| Routing by Dyn-S <br> Peak Elev= 207.5 <br> Flood Elev= 212 | or-Ind method, @ 12.13 hrs | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 206.90' | 15.0" Round Cu Inlet / Outlet Inver $n=0.013$ Corruga | $\begin{aligned} & \text { Ivert } \mathrm{L}=38.2^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{rt}=206.90^{\prime} / 206.52 \text { ' } \quad \mathrm{S}=0.0099 \quad \mathrm{l} / \mathrm{Cc}=0.900 \\ & \text { ated PE, smooth interior, Flow Area= } 1.23 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=1.69 cfs @ 12.13 hrs HW=207.57' TW=205.31' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 1.69 cfs @ 3.69 fps )

## Summary for Pond D20: DMH \#20



Routed to Pond D21 : DMH \#21
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=204.78' @ 12.09 hrs
Flood Elev= 207.68'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $204.19^{\prime}$ | $15.0^{\prime \prime}$ Round Culvert L=63.5' $\quad$ Ke= $=0.500$ |
|  |  | Inlet / Outlet Invert= 204.19' $/ 203.87 \prime \quad \mathrm{~S}=0.0050$ '/' $\quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.23 sf |  |

Primary OutFlow Max=1.12 cfs @ 12.09 hrs HW=204.78' TW=204.06' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 1.12 cfs @ 2.90 fps )

## Summary for Pond D21: DMH \#21

| Inflow Area = | 71,317 | 79.77\% Impervious, | w Depth > 2.52" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 4.53 cfs @ | 12.09 hrs , Volume= | 15,002 cf |
| Outflow | 4.53 cfs @ | 12.09 hrs , Volume= | 15,002 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 4.53 cfs @ | 12.09 hrs , Volume= | 15,002 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 204.08' @ 12.09 hrs
Flood Elev=207.55'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $203.02^{\prime}$ | 24.0" Round Culvert $\mathrm{L}=72.4^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $203.02^{\prime} / 202.66^{\prime} \quad \mathrm{S}=0.0050$ |
|  |  | $\mathrm{n}=0.013 \quad \mathrm{Cc}=0.900$ |  |

Primary OutFlow Max=4.42 cfs @ 12.09 hrs HW=204.06' TW=200.94' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 4.42 cfs @ 3.89 fps )

## Summary for Pond D22: DMH \#22

| Inflow Area = | 24,8 | 39\% Impervious, | Inflow Depth > 2.76" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.67 cfs @ | 12.09 hrs , Volume= | 5,708 cf |
| Outflow | 1.67 cfs @ | 12.09 hrs , Volume= | 5,708 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.67 cfs @ | 12.09 hrs , Volume= | 5,708 cf |

Routed to Pond d21: DMH \#21
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=205.54' @ 12.09 hrs
Flood Elev=208.46'


## Summary for Pond D23: DMH \#23

| Inflow Area = | 10,771 | 99.27\% Impervious | Inflow Depth > 3.04" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.77 cfs @ | 12.09 hrs , Volume= | 2,724 cf |
| Outflow | 0.77 cfs @ | 12.09 hrs , Volume= | 2,724 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.77 cfs @ | 12.09 hrs , Volume= | 2,724 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.11' @ 12.09 hrs
Flood Elev= 210.30'
Device Routing Invert Outlet Devices
\#1 Primary 206.70' 15.0" Round Culvert L= 173.3' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert= 206.70' / 204.97' S=0.0100 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=0.75 cfs @ 12.09 hrs HW=207.11' TW=205.52' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.75 cfs @ 3.20 fps )

## Summary for Pond D25: DMH \#25

| Inflow Area = | 36,995 sf, | 87.96\% Impervious, | Inflow Depth > | 2.63" for 2YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 2.34 cfs @ | 12.09 hrs , Volume= | 8,121 cf |  |
| Outflow | 2.34 cfs @ | 12.09 hrs , Volume $=$ | 8,121 cf | , Atten= 0\%, Lag= 0.0 min |
| Primary | 2.34 cfs @ | 12.09 hrs , Volume= | 8,121 cf |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=210.19' @ 12.09 hrs
Flood Elev=213.11'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $209.36^{\prime}$ | $15.0^{\prime \prime}$ Round Culvert $\mathrm{L}=237.6^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= $209.36^{\prime} / 208.17^{\prime} \quad \mathrm{S}=0.0050 \quad \mathrm{I} / \mathrm{Cc}=0.900$ |  |
| $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |  |  |  |

Primary OutFlow Max=2.28 cfs @ 12.09 hrs HW=210.18' TW=202.37' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 2.28 cfs @ 3.78 fps )

## Summary for Pond D27: DMH \#27



Routed to Pond D35 : DMH \#35
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=213.88' @ 12.09 hrs
Flood Elev=217.00'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $213.34^{\prime}$ | $15.0^{\prime \prime}$ Round Culvert $\mathrm{L}=63.9^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $213.34^{\prime} / 212.38^{\prime} \quad \mathrm{S}=0.0150$ |
|  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= $=0.93 \mathrm{sf}$ |  |

Primary OutFlow Max=1.23 cfs @ 12.09 hrs HW=213.87' TW=212.81' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls 1.23 cfs @ 2.48 fps )

## Summary for Pond D28: DMH \#28



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 217.83' @ 12.09 hrs
Flood Elev= 220.17'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 217.46' | 12.0" Round Culvert L= 158.3' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert= 217.46' / $214.29^{\prime} \mathrm{S}=0.0200$ '/' Cc= 0.900 <br> $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.55 cfs @ 12.09 hrs HW=217.83' TW=213.87' (Dynamic Tailwater)
—1=Culvert (Inlet Controls $0.55 \mathrm{cfs} @ 2.07 \mathrm{fps}$ )

## Summary for Pond D29: DMH \#29



Routed to Pond D28: DMH \#28
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=220.20' @ 12.09 hrs
Flood Elev= 223.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $219.83^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=150.9^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= $219.83^{\prime} / 217.55^{\prime} \quad \mathrm{S}=0.0151^{\prime} / /^{\prime} \quad \mathrm{Cc}=0.900$ |  |
| $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |  |

Primary OutFlow Max=0.55 cfs @ 12.09 hrs HW=220.20' TW=217.83' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls $0.55 \mathrm{cfs} @ 2.07 \mathrm{fps}$ )

## Summary for Pond D30: DMH \#30

| Inflow Area = | 6,862 | 50.73\% Imperviou | Inflow Depth > 1.45" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.26 cfs @ | 12.10 hrs , Volume= | 832 cf |
| Outflow | 0.26 cfs @ | 12.10 hrs , Volume= | 832 cf , Atten= 0\%, Lag= 0.0 min |
| Primary | 0.26 cfs @ | 12.10 hrs , Volume= | 832 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=221.21' @ 12.10 hrs
Flood Elev= 224.95'
Device Routing Invert Outlet Devices
\#1 Primary 220.92' 12.0" Round Culvert L= 184.2' Ke= 0.500
Inlet / Outlet Invert= 220.92' / 220.00' S=0.0050 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=0.26 cfs @ 12.10 hrs HW=221.21' TW=220.20' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.26 cfs @ 2.04 fps )

## Summary for Pond D31: DMH\#31



Primary OutFlow Max=1.15 cfs @ 12.17 hrs HW=225.14' TW=210.96' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 1.15 cfs @ 2.44 fps )

## Summary for Pond D34: DMH \#34

| Inflow Area | 23,255 | .00\% Impervious, | Inflow De | 04" for 2YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 1.66 cfs @ | 12.09 hrs , Volume= | 5,882 cf |  |
| Outflow | 1.66 cfs @ | 12.09 hrs , Volume= | 5,882 cf, | Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 1.66 cfs @ | 12.09 hrs , Volume= | 5,882 cf |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 198.77' @ 12.09 hrs
Flood Elev= 202.00'


Primary OutFlow Max=1.61 cfs @ 12.09 hrs HW=198.75' TW=195.74' (Dynamic Tailwater)
_1=Culvert (Inlet Controls 1.61 cfs @ 2.82 fps )

## Summary for Pond D35: DMH \#35

| Inflow Area = | 21,746 | 79.51\% Impervious, | w Depth > 2.35" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.26 cfs @ | 12.09 hrs , Volume= | 4,263 cf |
| Outflow | 1.26 cfs @ | 12.09 hrs , Volume= | $4,263 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.26 cfs @ | 12.09 hrs , Volume= | 4,263 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 212.82' @ 12.09 hrs
Flood Elev= 215.70'


Primary OutFlow Max=1.23 cfs @ 12.09 hrs HW=212.81' TW=210.18' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.23 cfs @ 2.48 fps )

## Summary for Pond D4: DMH\#4

| Inflow Area = | 73,240 sf, | 37.72\% Impervious | Inflow Depth > 1.22" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.72 cfs @ | 12.13 hrs , Volume= | 7,465 cf |
| Outflow | 1.72 cfs @ | 12.13 hrs , Volume= | $7,465 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.72 cfs @ | 12.13 hrs , Volume= | 7,465 cf |

Routed to Pond D2: DMH\#2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=210.98' @ 12.13 hrs
Flood Elev=217.64'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 210.34' | 15.0" Round Culvert L= 222.3' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=210.34' / 207.01' S=0.0150 '/l' Cc=0.900 $n=0.012$ Corrugated PP, smooth interior, Flow Area $=1.23 \mathrm{sf}$ |

Primary OutFlow Max=1.69 cfs @ 12.13 hrs HW=210.97' TW=207.57' (Dynamic Tailwater)
_1=Culvert (Inlet Controls 1.69 cfs @ 2.71 fps )

## Summary for Pond D5: DMH \#5

| Inflow Area = | 30,441 | 76\% Impervious, | Inflow Depth > | 98" for 2YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 2.15 cfs @ | 12.09 hrs , Volume= | 7,549 cf |  |
| Outflow | 2.15 cfs @ | 12.09 hrs , Volume= | 7,549 cf, | , Atten= 0\%, Lag= 0.0 min |
| Primary | 2.15 cfs @ | 12.09 hrs , Volume= | 7,549 cf |  | Routed to Pond D6 : DMH \#6

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=209.86' @ 12.09 hrs
Flood Elev= 212.97'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 209.09' | 18.0" Round Culvert L= 183.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 209.09' / 208.17' S= 0.0050 '/l' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.77 sf |

Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=209.85' TW=208.81' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 2.09 cfs @ 3.39 fps )

## Summary for Pond D6: DMH \#6



Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=208.81' TW=207.13' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 2.09 cfs @ 3.51 fps )

## Summary for Pond D7: DMH \#7

| Inflow Area $=$ | $30,441 \mathrm{sf}$, 96.76\% Impervious, | Inflow Depth > 2.98" | for 2 YR event |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $2.15 \mathrm{cfs} @$ | 12.09 hrs , Volume |
| Outflow | $=$ | $2.15 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |
| Primary | $=$ | $2.15 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | Routed to Pond P212 : INFILTRATION POND \#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.14' @ 12.09 hrs
Flood Elev=213.17'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | 206.47 | $18.0^{\prime \prime}$ Round Culvert $\mathrm{L}=44.2^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 206.47' $/ 204.04 ' \quad \mathrm{~S}=0.0550$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.77 sf |  |

Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=207.13' TW=200.93' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 2.09 cfs @ 2.77 fps )

## Summary for Pond D8: DMH \#8



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 201.27' @ 12.09 hrs
Flood Elev= 204.72'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | :--- | :--- | :--- |
| $\# 1$ | Primary | $200.57^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=87.7^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |  |
|  |  | Inlet / Outlet Invert= 200.57' $/ 200.13^{\prime} \quad \mathrm{S}=0.0050$ |  |  |
| $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |  |  |  |

Primary OutFlow Max=1.17 cfs @ 12.09 hrs HW=201.26' TW=200.72' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 1.17 cfs @ 2.87 fps )

## Summary for Pond D9: DMH \#9



## Summary for Pond DE61: DRIP \#61



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 212.19 | 665 | 0.0 | 0 | 0 |
| 212.20 | 665 | 40.0 | 3 | 3 |
| 214.19 | 665 | 40.0 | 529 | 532 |
| 214.20 | 665 | 100.0 | 7 | 539 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 214.10' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.400 .60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 212.70' | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 212.70' / 212.65' S=0.0050 '/l' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.20 \mathrm{sf}$ |
| \#3 | Discarded | 212.19' | $\mathbf{0 . 1 7 0} \mathbf{i n} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01 ' |

Discarded OutFlow Max=0.00 cfs @ 7.35 hrs HW=212.21' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.24 cfs @ 12.15 hrs HW=213.08' TW=208.02' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.24 cfs @ 2.06 fps)

## Summary for Pond DE62: DRIP \#62

| Inflow Area = | 4,247 sf, | 92.68\% Impervious, | Inflow Depth > 2.82" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.29 cfs @ | 12.09 hrs , Volume= | 997 cf |
| Outflow | 0.24 cfs @ | 12.15 hrs , Volume= | 857 cf, Atten= 16\%, Lag= 3.5 min |
| Discarded $=$ | 0.00 cfs @ | 7.35 hrs , Volume= | 181 cf |
| Primary | 0.24 cfs @ | 12.15 hrs , Volume= | 676 cf | Routed to Reach 8R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=213.08' @ 12.15 hrs Surf.Area= 665 sf Storage= 238 cf
Plug-Flow detention time $=100.3$ min calculated for 857 cf ( $86 \%$ of inflow)
Center-of-Mass det. time= $39.1 \mathrm{~min}(812.1-773.0)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $212.19^{\prime}$ | 539 cf | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 212.19 | 665 | 0.0 | 0 | 0 |
| 212.20 | 665 | 40.0 | 3 | 3 |
| 214.19 | 665 | 40.0 | 529 | 532 |
| 214.20 | 665 | 100.0 | 7 | 539 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 214.10' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 212.70' | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 212.70' $/ 212.65$ ' S=0.0050 '/l' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.20 \mathrm{sf}$ |
| \#3 | Discarded | 212.19' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01 ' |

Discarded OutFlow Max=0.00 cfs @ 7.35 hrs HW=212.21' (Free Discharge)
$\complement_{3=\text { Exfiltration (Exfiltration Controls } 0.00 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=0.24 cfs @ 12.15 hrs HW=213.08' TW=208.02' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.24 cfs @ 2.06 fps )

## Summary for Pond DE63: DRIP \#63



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 206.99 | 404 | 0.0 | 0 | 0 |
| 207.00 | 404 | 40.0 | 2 | 2 |
| 208.99 | 404 | 40.0 | 322 | 323 |
| 209.00 | 404 | 100.0 | 4 | 327 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 208.90' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
| \#2 | Primary | 207.50' | Coef. (English) 2.802 .923 .083 .303 .32 |
|  |  |  | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 207.50' / 207.45' S=0.0050 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.20 sf |
| \#3 | Discarded | 206.99' | 0.170 in/hr Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.00 cfs @ 7.45 hrs HW=207.01' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.18 cfs @ 12.13 hrs HW=207.82' TW=202.04' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.18 cfs @ 1.90 fps)

## Summary for Pond DE64: DRIP \#64

| Inflow Area = | 3,470 | \% Impervious, | Inflow Depth > 2.82" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.24 cfs @ | 12.09 hrs , Volume= | 814 cf |
| Outflow | 0.21 cfs @ | 12.13 hrs , Volume= | 715 cf, Atten= 12\%, Lag= 2.8 min |
| Discarded = | 0.00 cfs @ | 7.05 hrs , Volume= | 129 cf |
| Primary | 0.21 cfs @ | 12.13 hrs , Volume= | 586 cf | Routed to Reach 12R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.85' @ 12.13 hrs Surf.Area= 470 sf Storage= 162 cf
Plug-Flow detention time $=90.8 \mathrm{~min}$ calculated for 715 cf ( $88 \%$ of inflow)
Center-of-Mass det. time= 35.0 min ( 808.0-773.0)


Discarded OutFlow Max=0.00 cfs @ 7.05 hrs HW=205.01' (Free Discharge)
$L^{-}=$Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.20 cfs @ 12.13 hrs HW=205.85' TW=202.04' (Dynamic Tailwater)

- $1=$ Broad-Crested Rectangular Weir ( Controls 0.00 cfs )

2=Culvert (Barrel Controls 0.20 cfs @ 1.97 fps )

## Summary for Pond DE65: DRIP \#65

| Inflow Area = | 3,016 | 88.69\% Impervious, | Inflow Depth > 2.71" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 0.20 cfs @ | 12.09 hrs , Volume= | 682 cf |
| Outflow | 0.18 cfs @ | 12.13 hrs , Volume= | 596 cf, Atten= 11\%, Lag= 2.6 min |
| Discarded = | 0.00 cfs @ | 7.45 hrs , Volume= | 107 cf |
| Primary | 0.18 cfs @ | 12.13 hrs , Volume= | 489 cf | Routed to Reach 12R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.82' @ 12.13 hrs Surf.Area= 404 sf Storage= 135 cf
Plug-Flow detention time $=89.4$ min calculated for 595 cf ( $87 \%$ of inflow)
Center-of-Mass det. time= 33.6 min ( 813.5-779.9)


## Summary for Pond DE66: DRIP \#66

| Inflow Area = | 3,407 sf | .46\% Impervious, | Inflow Depth > 2.82" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.23 cfs @ | 12.09 hrs , Volume= | 800 cf |
| Outflow | 0.21 cfs @ | 12.13 hrs , Volume= | 700 cf, Atten= 12\%, Lag= 2.8 min |
| Discarded = | 0.00 cfs @ | 6.25 hrs , Volume= | 129 cf |
| Primary | 0.20 cfs @ | 12.13 hrs , Volume= | 571 cf | Routed to Reach 12R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 208.65' @ 12.13 hrs Surf.Area= 470 sf Storage= 161 cf
Plug-Flow detention time= 91.7 min calculated for 700 cf ( $88 \%$ of inflow)
Center-of-Mass det. time= 35.3 min (808.3-773.0)

| Volume | Invert207.79 | t Avail.Storage |  | Storage Description |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| \#1 |  | ' 381 cf Custom Stage Data (Prismatic)Listed below (Recalc) |  |  |  |
| $\begin{array}{r} \text { Elevation } \\ \text { (feet) } \end{array}$ |  | Surf.Area (sq-ft) | Voids (\%) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
| 207.79 |  | 470 | 0.0 | 0 | 0 |
| 207.80 |  | 470 | 40.0 | 2 | 2 |
| 209.79 |  | 470 | 40.0 | 374 | 376 |
| 209.80 |  | 470 | 100.0 | 5 | 381 |
| Device R | Routing | Invert Outlet Devices |  |  |  |
| \#1 | Primary | 209.7 | $0^{\prime} \quad 180.0$ ' long x 0.5' breadth Broad-Crested Rectangular Weir <br> Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |  |  |
| \#2 | Primary | 208 | ( 6.0' Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |  |  |
|  |  |  | Inlet / Outlet Invert= 208.30' 208.25 ' S=0.0050 '/' Cc= 0.900 |  |  |
| \#3 | Discarded | d 207.79' 0.1 | $9^{\prime} \quad 0.170$ in/hr Exfiltration over Surface area Phase-In= 0.01 ' |  |  |

Discarded OutFlow Max=0.00 cfs @ 6.25 hrs HW=207.80' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.20 cfs @ 12.13 hrs HW=208.64' TW=202.04' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.20 cfs @ 1.96 fps)

## Summary for Pond DE67: DRIP \#67



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 207.99 | 470 | 0.0 | 0 | 0 |
| 208.00 | 470 | 40.0 | 2 | 2 |
| 209.99 | 470 | 40.0 | 374 | 376 |
| 210.00 | 470 | 100.0 | 5 | 381 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| \#1 | Primary | $209.90 '$ | 180.0' long $\times$ 0.5' breadth Broad-Crested Rectangular Weir |
|  |  |  | Head (feet) 0.20 0.40 $0.600 .80 \quad 1.00$ |
| Coef. (English) $2.80 \quad 2.923 .083 .303 .32$ |  |  |  |

Discarded OutFlow Max=0.00 cfs @ 7.00 hrs HW=208.01' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.21 cfs @ 12.13 hrs HW=208.85' TW=208.02' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
-2=Culvert (Barrel Controls 0.21 cfs @ 1.98 fps )

## Summary for Pond DE68: DRIP \#68

 Routed to Pond OCS4 : OCS\#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.82' @ 12.14 hrs Surf.Area= 665 sf Storage= 220 cf
Plug-Flow detention time= 97.1 min calculated for 848 cf ( $86 \%$ of inflow)
Center-of-Mass det. time $=37.0 \mathrm{~min}$ ( 810.0-773.0)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $206.99^{\prime}$ | 539 cf | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 206.99 | 665 | 0.0 | 0 | 0 |
| 207.00 | 665 | 40.0 | 3 | 3 |
| 208.99 | 665 | 40.0 | 529 | 532 |
| 209.00 | 665 | 100.0 | 7 | 539 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 208.90' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 207.50' | 6.0" Round Culvert L= 20.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 207.50' / 206.00' S=0.0750 '/' $\mathrm{Cc}=0.900$ |
| \#3 | Discarded | 206.99' | $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.20 \mathrm{~s}$ $\mathbf{0 . 1 7 0} \mathbf{i n} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01 ' |

Discarded OutFlow Max=0.00 cfs @ 7.35 hrs HW=207.01' (Free Discharge)
$\complement_{3=\text { Exfiltration (Exfiltration Controls } 0.00 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=0.25 cfs @ 12.14 hrs HW=207.81' TW=203.64' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs )
-2=Culvert (Inlet Controls 0.25 cfs @ 1.90 fps )

## Summary for Pond DE69: DRIP \#69



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 205.49 | 470 | 0.0 | 0 | 0 |
| 205.50 | 470 | 40.0 | 2 | 2 |
| 207.49 | 470 | 40.0 | 374 | 376 |
| 207.50 | 470 | 100.0 | 5 | 381 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 207.40' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 206.00' | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=206.00' 205.95 ' S=0.0050 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.20 sf |
| \#3 | Discarded | 205.49' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In=0.01' |

Discarded OutFlow Max=0.00 cfs @ 7.00 hrs HW=205.51' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.21 cfs @ 12.13 hrs HW=206.35' TW=201.08' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.21 cfs @ 1.98 fps )

## Summary for Pond DE70: DRIP \#70

| Inflow Area = | 3,476 sf, | .60\% Impervious, | pth > | 2.82" for 2YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 0.24 cfs @ | 12.09 hrs , Volume= | 816 cf |  |
| Outflow | 0.21 cfs @ | 12.13 hrs , Volume $=$ | 716 cf, | Atten= 12\%, Lag= 2.8 min |
| Discarded = | 0.00 cfs @ | 6.20 hrs , Volume= | 129 cf |  |
| Primary | 0.21 cfs @ | 12.13 hrs , Volume= | 587 cf |  | Routed to Pond P212 : INFILTRATION POND \#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.75' @ 12.13 hrs Surf.Area= 470 sf Storage= 162 cf
Plug-Flow detention time $=90.7$ min calculated for 716 cf ( $88 \%$ of inflow)
Center-of-Mass det. time= 35.0 min ( 808.0-773.0)


Discarded OutFlow Max=0.00 cfs @ 6.20 hrs HW=205.90' (Free Discharge)
$\complement_{3=\text { Exfiltration (Exfiltration Controls } 0.00 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=0.21 cfs @ 12.13 hrs HW=206.75' TW=201.08' (Dynamic Tailwater)

- 1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
$\mathbf{2 = C u l v e r t}$ (Barrel Controls 0.21 cfs @ 1.98 fps )


## Summary for Pond DE71: DRIP \#71

| Inflow Area = | 4,210 | 92.61\% Impervious | Inflow Depth > 2.82" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.29 cfs @ | 12.09 hrs , Volume= | 988 cf |
| Outflow | 0.24 cfs @ | 12.15 hrs , Volume= | 848 cf, Atten= 16\%, Lag= 3.5 min |
| Discarded | 0.00 cfs @ | 7.75 hrs , Volume= | 181 cf |
| Primary | 0.24 cfs @ | 12.15 hrs , Volume= | 667 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=207.38' @ 12.15 hrs Surf.Area= 665 sf Storage= 237 cf
Plug-Flow detention time $=100.7$ min calculated for 848 cf ( $86 \%$ of inflow)
Center-of-Mass det. time= $39.3 \mathrm{~min}(812.3-773.0)$


Discarded OutFlow Max=0.00 cfs @ 7.75 hrs HW=206.52' (Free Discharge)
L3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.24 cfs @ 12.15 hrs HW=207.38' TW=201.12' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.24 cfs @ 2.06 fps )

## Summary for Pond DECH: DRIP \#CH

| Inflow Area = | 6,262 sf, | 92.70\% Impervious, | Inflow Depth > 2.82" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.43 cfs @ | 12.09 hrs , Volume= | 1,470 cf |
| Outflow | 0.28 cfs @ | 12.19 hrs , Volume= | $1,469 \mathrm{cf}$, Atten $=35 \%$ Lag $=5.9 \mathrm{~min}$ |
| Discarded = | 0.04 cfs @ | 11.50 hrs , Volume= | 1,053 cf |
| Primary | 0.25 cfs @ | 12.19 hrs , Volume= | 416 cf | Routed to Pond CB18 : CB \#18

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 209.01' @ 12.19 hrs Surf.Area= 636 sf Storage= 260 cf
Plug-Flow detention time $=19.9 \mathrm{~min}$ calculated for $1,469 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time= $19.7 \min$ (792.7-773.0)

| Volume | Invert | t Avail.Storage |  | Storage Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| \#1 | 207.99 | ' 770 cf |  | Custom Stage Data (Prismatic)Listed below (Recalc) |  |  |
| Elevation (feet) |  | Surf.Area (sq-ft) | Voids (\%) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |  |
| 207.99 |  | 636 | 0.0 | 0 | 0 |  |
| 208.00 |  | 636 | 40.0 | 3 | 3 |  |
| 210.99 |  | 636 | 40.0 | 761 | 763 |  |
| 211.00 |  | 636 | 100.0 | 6 | 770 |  |
| Device | Routing | Invert Outlet Devices |  |  |  |  |
| \#1 | Primary | 210.90' 16 |  | 160.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.200 .400 .600 .80 \quad 1.00$ |  |  |
| \#2 | Primary | 208.50' 4. |  | 4.0" Round Culvert L=80.0' $\mathrm{Ke}=0.500$ |  |  |
|  |  |  | $\begin{aligned} & \text { Inlet } \\ & \mathrm{n}=0 \end{aligned}$ | nlet / Outlet Invert= 208.50' / 205.10' S=0.0425 '// Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.09 sf |  |  |
| \#3 | Discarded |  | .99' 2.410 | $0 \mathrm{in} / \mathrm{hr}$ Exfiltr | over Surfac | ce area Phase-In=0.01' |

Discarded OutFlow Max=0.04 cfs @ 11.50 hrs HW=208.02' (Free Discharge)
$L_{3}=$ Exfiltration (Exfiltration Controls 0.04 cfs )
Primary OutFlow Max=0.25 cfs @ 12.19 hrs HW=209.01' TW=205.27' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Inlet Controls 0.25 cfs @ 2.81 fps )

## Summary for Pond DMH32: DMH \#32



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev= 203.19' @ 12.09 hrs
Flood Elev=206.16'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.59' | 12.0" Round Culvert L= 19.2' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert=202.59' / 201.57 ' $\mathrm{S}=0.0531$ '// Cc= 0.900 <br> $n=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| Primary OutFlow Max=1.27 cfs @ 12.09 hrs HW=203.18' TW=200.94' (Dynamic Tailwater) L-1=Culvert (Inlet Controls 1.27 cfs @ 2.62 fps) |  |  |  |

## Summary for Pond OCS1: OCS\#1

| Inflow Area = | 48,573 s | 81.99\% Impervious, | Inflow Depth > 2.67" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 3.17 cfs @ | 12.09 hrs , Volume= | 10,805 cf |
| Outflow | 3.17 cfs @ | 12.09 hrs , Volume= | 10,805 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 3.17 cfs @ | 12.09 hrs , Volume= | 10,805 cf |

Routed to Pond P206 : STORMTECH INFILTRATION SYSTEM \#2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev= 195.75' @ 12.09 hrs
Flood Elev=201.48'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $195.00^{\prime}$ | $\mathbf{2 4 . 0}$ " Vert. Orifice/Grate |
| C $=0.600 \quad$ Limited to weir flow at low heads |  |  |  |

Primary OutFlow Max=3.09 cfs @ 12.09 hrs HW=195.74' TW=195.23' (Dynamic Tailwater)
—1=Orifice/Grate (Orifice Controls 3.09 cfs @ 2.93 fps )

## Summary for Pond OCS3: OCS\#3



Primary OutFlow Max=3.20 cfs @ 12.09 hrs HW=203.94' TW=203.47' (Dynamic Tailwater)
L1=Orifice/Grate (Orifice Controls 3.20 cfs @ 3.13 fps )

## Summary for Pond OCS4: OCS\#4

| Inflow Area = | 17,972 sf, 28.85\% Impervious, | Inflow Depth > 1.32" for 2YR event |
| :---: | :---: | :---: |
| Inflow | 0.64 cfs @ 12.11 hrs, Volume= | 1,977 cf |
| Outflow | 0.64 cfs @ 12.11 hrs, Volume= | 1,977 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.64 cfs @ 12.11 hrs , Volume= | 1,977 cf |

Routed to Pond P204 : STORMTECH INFILTRATION SYSTEM \#1
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=203.89' @ 12.49 hrs
Flood Elev= 208.00'
Device Routing Invert Outlet Devices
\#1 Primary $\quad 203.10^{\prime} \quad 18.0^{\prime \prime}$ Vert. Orifice/Grate $\quad \mathrm{C}=0.600$ Limited to weir flow at low heads

Primary OutFlow Max=0.64 cfs @ 12.11 hrs HW=203.59' TW=203.52' (Dynamic Tailwater)
$\left\llcorner_{1=O r i f i c e / G r a t e ~(O r i f i c e ~ C o n t r o l s ~}^{0.64}\right.$ cfs @ 1.29 fps )

## Summary for Pond OCS6: OCS \#6



Primary OutFlow Max=1.10 cfs @ 12.09 hrs HW=201.78' TW=201.55' (Dynamic Tailwater)
———Orifice/Grate (Orifice Controls 1.10 cfs @ 2.30 fps )

## Summary for Pond OCS7: OCS \#7



Primary OutFlow Max=1.07 cfs @ 12.09 hrs HW=202.32' TW=201.55' (Dynamic Tailwater)
—1 $^{1=O r i f i c e / G r a t e ~(O r i f i c e ~ C o n t r o l s ~} 1.07$ cfs @ 2.49 fps )

## Summary for Pond P204: STORMTECH INFILTRATION SYSTEM \#1

| Inflow Area = | 72,222 | 68.72\% Imperviou | Inflow Depth > 2.11" for 2YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 3.90 cfs @ | 12.09 hrs , Volume= | 12,675 cf |
| Outflow | 1.02 cfs @ | 12.49 hrs , Volume= | 10,996 cf, Atten $=74 \%$ Lag $=23.8 \mathrm{~min}$ |
| Discarded = | 0.09 cfs @ | 10.60 hrs , Volume= | 5,215 cf |
| Primary | 0.93 cfs @ | 12.49 hrs , Volume= | 5,781 cf |

Routed to Reach 20r : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 203.89' @ 12.49 hrs Surf.Area= 5,670 sf Storage= 5,017 cf
Flood Elev=208.75' Surf.Area=5,670 sf Storage= 13,379 cf
Plug-Flow detention time $=142.3$ min calculated for 10,973 of ( $87 \%$ of inflow)
Center-of-Mass det. time= 84.7 min ( 873.6-788.9)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1A | 202.50' | 5,923 cf | 77.50 'W x 67.70'L x 4.08'H STORMTECH SC-740 <br> 21,423 cf Overall $-6,615$ cf Embedded $=14,808$ cf $\times 40.0 \%$ Voids |
| \#2A | 203.08' | 6,615 cf | ADS_StormTech SC-740 +Capx 144 Inside \#1 Effective Size $=44.6^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12^{\prime} \mathrm{L}=45.9 \mathrm{cf}$ Overall Size=51.0"W x 30.0"H x 7.56'L with 0.44 ' Overlap 144 Chambers in 16 Rows |
| \#3B | 202.50' | 427 cf | 6.25 'W x 67.70'L x $3.50^{\prime}$ 'H ISOLATOR ROW <br> 1,481 cf Overall - 413 cf Embedded $=1,067$ cf $\times 40.0 \%$ Voids |
| \#4B | 203.00' | 413 cf | ADS_StormTech SC-740 +Capx 9 Inside \#3 <br> Effective Size $=44.6^{\prime \prime} \mathrm{W} \times 30.0$ " $\mathrm{H}=>6.45 \mathrm{sf} \times 7.12^{\prime} \mathrm{L}=45.9 \mathrm{cf}$ <br> Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with $0.44^{\prime}$ Overlap |

13,379 cf Total Available Storage
Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.75' | 15.0" Round Culvert L=35.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 202.75' / 201.00' S=0.0500 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |
| \#2 | Device 1 | 204.75' | 4.0' long x 0.5' breadth Broad-Crested Rectangular Weir |
|  |  |  | Head (feet) $0.20 \quad 0.400 .60 \quad 0.801 .00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#3 | Device 1 | 203.25' | 8.0" Vert. Orifice/Grate C=0.600 Limited to weir flow at low heads |
| \#4 | Discarded | 202.50' | $0.660 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= $0.01{ }^{\text {' }}$ |

Discarded OutFlow Max=0.09 cfs @ 10.60 hrs HW=202.57' (Free Discharge)
L4=Exfiltration (Exfiltration Controls 0.09 cfs )
Primary OutFlow Max=0.93 cfs @ 12.49 hrs HW=203.88' TW=200.04' (Dynamic Tailwater)
$L_{1}=$ Culvert (Passes 0.93 cfs of 4.24 cfs potential flow)
-2=Broad-Crested Rectangular Weir( Controls 0.00 cfs)
$\square_{3}=$ Orifice/Grate (Orifice Controls 0.93 cfs @ 2.71 fps )

## Summary for Pond P205: INFILTRATION POND \#3

| Inflow Area = | 88,676 s | 39.42\% Impervious, | pth > | 1.25" for 2YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 2.27 cfs @ | 12.12 hrs , Volume= | 9,251 cf |  |
| Outflow | 0.26 cfs @ | 13.49 hrs , Volume= | 9,244 cf, | Atten= 89\%, Lag= 82.2 min |
| Discarded $=$ | 0.26 cfs @ | 13.49 hrs , Volume= | 9,244 cf |  |
| Primary | 0.00 cfs @ | 0.00 hrs , Volume= | 0 cf |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 205.79' @ 13.49 hrs Surf.Area= 4,584 sf Storage= 3,362 cf
Plug-Flow detention time $=120.6$ min calculated for 9,244 cf ( $100 \%$ of inflow)
Center-of-Mass det. time= 120.1 min (954.0-833.9)


Discarded OutFlow Max=0.26 cfs @ 13.49 hrs HW=205.79' (Free Discharge)
—2=Exfiltration (Exfiltration Controls 0.26 cfs )
Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=205.00' TW=203.00' (Dynamic Tailwater)
廿-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs)

## Summary for Pond P206: STORMTECH INFILTRATION SYSTEM \#2



Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 195.67' @ 12.66 hrs Surf.Area= 6,072 sf Storage= 4,118 cf
Plug-Flow detention time= 54.5 min calculated for 13,142 cf ( $100 \%$ of inflow)
Center-of-Mass det. time $=54.4 \mathrm{~min}(832.0-777.6)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1A | 194.67' | 1,786 cf | 39.50 'W x 53.46'L x $3.33^{\prime}$ H FIELD A <br> 7,038 cf Overall $-2,573$ cf Embedded $=4,466$ cf $\times 40.0 \%$ Voids |
| \#2A | 195.00' | 2,573 cf | ADS_StormTech SC-740 +Capx 56 Inside \#1 Effective Size $=44.6^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12^{\prime} \mathrm{L}=45.9 \mathrm{cf}$ Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with 0.44 ' Overlap 56 Chambers in 8 Rows |
| \#3B | 194.67' | 3,296 cf | $58.50^{\prime} \mathrm{W}$ x $67 . \mathbf{7 0}^{\prime} \mathrm{L} \times 3.33^{\prime} \mathrm{H}$ FIELD B <br> 13,201 cf Overall - 4,962 cf Embedded $=8,239$ cf $\times 40.0 \%$ Voids |
| \#4B | 195.00' | 4,962 cf | ADS_StormTech SC-740 +Capx 108 Inside \#3 Effective Size $=44.6^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12^{\prime} \mathrm{L}=45.9 \mathrm{cf}$ Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with 0.44 ' Overlap 108 Chambers in 12 Rows |

Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 194.65' | 18.0" Round Culvert L=30.0' $\mathrm{Ke}=0.200$ |
|  |  |  | Inlet / Outlet Invert= 194.65' / 194.50' S=0.0050 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.77 sf |
| \#2 | Device 1 | 195.85' | 4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) |
| \#3 | Discarded | 194.67' | $3.500 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.49 cfs @ 11.75 hrs HW=194.73' (Free Discharge)
${ }^{-} 3=$ Exfiltration (Exfiltration Controls 0.49 cfs )
Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=194.67' TW=0.00' (Dynamic Tailwater)
$L_{1}=$ Culvert (Passes 0.00 cfs of 0.00 cfs potential flow)
$L_{2=S h a r p-C r e s t e d ~ R e c t a n g u l a r ~ W e i r(~ C o n t r o l s ~} 0.00 \mathrm{cfs}$ )

## Summary for Pond P207: INFILTRATION POND \#2



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: |
| 196.80 | 10,100 | 0 | 0 |
| 198.00 | 12,000 | 13,260 | 13,260 |
| 200.00 | 15,000 | 27,000 | 40,260 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 198.80' | 20.0' long x 21.0' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.400 .60 \quad 0.801 .001 .201 .401 .60$ |
| \#2 |  |  | Coef. (English) 2.682 .702 .702 .642 .632 .642 .642 .63 |
|  | Primary | 194.75' | 15.0" Round Culvert L= 40.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 194.75' / 194.55' S=0.0050 '/' Cc= 0.900 $n=0.012$ Corrugated PP, smooth interior, Flow Area $=1.23 \mathrm{sf}$ |
| \#3 | Device 2 | 198.80' | 6.0 x 6.0" Horiz. Orifice/Grate X 6.00 columns |
|  |  |  | X 6 rows $\mathrm{C}=0.600$ in 48.0" $\times 48.0^{\prime \prime}$ Grate ( $56 \%$ open area) |
|  |  |  | Limited to weir flow at low heads |
| \#4 | Device 2 | 197.40' | 8.0" Vert. Orifice/Grate $\mathrm{C}=0.600$ Limited to weir flow at low heads |
| \#5 | Discarded | 196.80' | 3.690 in/hr Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.96 cfs @ 12.71 hrs HW=197.50' (Free Discharge)
-5=Exfiltration (Exfiltration Controls 0.96 cfs)
Primary OutFlow Max=0.04 cfs @ 12.71 hrs HW=197.50' TW=192.01' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
-2=Culvert (Passes 0.04 cfs of 8.49 cfs potential flow)

- $3=$ Orifice/Grate ( Controls 0.00 cfs )
—4=Orifice/Grate (Orifice Controls 0.04 cfs @ 1.10 fps )


## Summary for Pond P210: POCKET WETLAND \#1



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: |
| 199.00 | 218 | 0 | 0 |
| 200.00 | 294 | 256 | 256 |
| 201.00 | 376 | 335 | 591 |
| 202.00 | 3,991 | 2,184 | 2,775 |
| 204.00 | 8,073 | 12,064 | 14,839 |
| 206.00 | 13,272 | 21,345 | 36,184 |
| 206.50 | 14,753 | 7,006 | 43,190 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 205.10' | 20.0' long x 15.0' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.400 .60 \quad 0.801 .001 .201 .401 .60$ |
|  |  |  | Coef. (English) 2.682 .702 .702 .642 .632 .642 .642 .63 |
| \#2 | Primary | 202.25' | 12.0" Round Culvert L=44.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 202.25' / 202.03' S=0.0050 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| \#3 | Device 2 | 202.30' | 2.5" Vert. Orifice/Grate $\mathrm{C}=0.600$ Limited to weir flow at low heads |
| \#4 | Device 2 | 204.50' | 6.0 " x 6.0" Horiz. Orifice/Grate X 6.00 columns |
|  |  |  | X 6 rows $\mathrm{C}=0.600$ in 48.0 " $\times 48.0$ " Grate ( $56 \%$ open area) |
|  |  |  | Limited to weir flow at low heads |

Primary OutFlow Max=0.12 cfs @ 15.47 hrs HW=202.97' TW=202.04' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs)
$-2=C u l v e r t$ (Passes 0.12 cfs of 1.31 cfs potential flow)

- 3=Orifice/Grate (Orifice Controls 0.12 cfs @ 3.62 fps)

4=Orifice/Grate (Controls 0.00 cfs )

## Summary for Pond P212: INFILTRATION POND \#1



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) | Wet.Area <br> (sq-ft) |
| ---: | ---: | ---: | ---: | ---: |
| 200.00 | 4,354 | 0 | 0 | 4,354 |
| 201.00 | 9,360 | 6,699 | 6,699 | 9,368 |
| 202.00 | 10,993 | 10,166 | 16,865 | 11,040 |
| 204.00 | 13,976 | 24,909 | 41,774 | 14,126 |

Device Routing Invert Outlet Devices
\#1 Primary 202.50' 25.0' long x 20.0' breadth Broad-Crested Rectangular Weir
Head (feet) 0.200 .400 .600 .801 .001 .201 .401 .60
Coef. (English) 2.682 .702 .702 .642 .632 .642 .642 .63
\#2 Discarded 200.00' $5.130 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01'
Discarded OutFlow Max=1.25 cfs @ 12.92 hrs HW=201.74' (Free Discharge)
L2=Exfiltration (Exfiltration Controls 1.25 cfs )
Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=200.00' TW=200.00' (Dynamic Tailwater)
L-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs)

## Summary for Pond P213: Stormtech Infiltration System \#3

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

| Inflow Area = | 31,986 sf | 93.23\% Impervious, | Depth > 2 | 2.87" for 2YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 2.22 cfs @ | 12.09 hrs , Volume= | 7,648 cf |  |
| Outflow | 0.39 cfs @ | 11.80 hrs , Volume= | $7,647 \mathrm{cf}$, | Atten= 82\%, Lag $=0.0 \mathrm{~min}$ |
| Discarded = | 0.39 cfs @ | 11.80 hrs , Volume= | 7,647 cf |  |
| Primary | 0.00 cfs @ | 0.00 hrs , Volume= | 0 cf |  |

Routed to Pond P212 : INFILTRATION POND \#1
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 201.93' @ 12.54 hrs Surf.Area= 3,317 sf Storage= 1,973 cf
Plug-Flow detention time=(not calculated: outflow precedes inflow)
Center-of-Mass det. time= 27.1 min (795.0-767.9)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1A | 200.95' | 2,990 cf | 49.00 'W x 67.70'L x 3.50'H Field A |
|  |  |  | 11,610 cf Overall - 4,135 cf Embedded $=7,475$ cf $\times 40.0 \%$ Voids |
| \#2A | 201.45' | $4,135 \mathrm{cf}$ | ADS_StormTech SC-740 +Capx 90 Inside \#1 |
|  |  |  | Effective Size $=44.6$ " $\mathrm{W} \times 30.0 \mathrm{H}$ H $=>6.45 \mathrm{sf} \times 7.12 \mathrm{~L}=45.9 \mathrm{cf}$ |
|  |  |  | Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with 0.44 ' Overlap |
|  |  |  | 90 Chambers in 10 Rows |
|  |  | 7,125 cf | Total Available Storage |

Storage Group A created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.30' | 12.0" Round Culvert $\mathrm{L}=60.0^{\prime} \mathrm{Ke}=0.500$ Inlet / Outlet Invert= 202.30' / 202.00' S=0.0050 '// Cc= $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area= 0.7 |
| \#2 | Discarded | 200.95' | $5.130 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= |
| \#3 | Device 1 | 204.25' | 4.0' long Sharp-Crested Rectangular Weir 2 En |
| \#4 | Device 1 | 203.35' | 6.0" W x 4.0" H Vert. Orifice/Grate $\mathrm{C}=0.600$ Limited to weir flow at low heads |
| Discarded OutFlow Max=0.39 cfs @ 11.80 hrs HW=201.02' (Free Discharge) $\mathrm{L}_{2}=$ Exfiltration (Exfiltration Controls 0.39 cfs ) |  |  |  |
| ```Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=200.95' TW=200.00' (Dynamic Tailwater) &-Culvert (Controls 0.00 cfs) -3=Sharp-Crested Rectangular Weir( Controls 0.00 cfs) 4=Orifice/Grate ( Controls 0.00 cfs)``` |  |  |  |

## Summary for Pond P214: STORMTECH INFILTRATION SYSTEM \#4

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=2)


Routed to Reach 9R : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=200.78' @ 12.43 hrs Surf.Area= 3,201 sf Storage= 1,357 cf
Plug-Flow detention time=(not calculated: outflow precedes inflow)
Center-of-Mass det. time= 10.0 min (778.8-768.8)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1A | 200.00' | 1,851 cf | $30.00^{\prime} \mathrm{W} \times 67.0^{\prime} \mathrm{L} \times 3.50^{\prime} \mathrm{H}$ Field A <br> 7,108 cf Overall - 2,481 cf Embedded $=4,627$ cf $\times 40.0 \%$ Voids |
| \#2A | 200.50' | 2,481 cf | ADS_StormTech SC-740 +Capx 54 Inside \#1 <br> Effective Size $=44.6$ "W $\times 30.0$ " $\mathrm{H}=>6.45 \mathrm{sf} \times 7.12 \mathrm{~L}=45.9 \mathrm{cf}$ Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with $0.44^{\prime}$ Overlap 54 Chambers in 6 Rows |
| \#3B | 200.00' | 1,087 cf | 25.25'W x 46.34'L x $3.50^{\prime}$ H Field B <br> 4,095 cf Overall - 1,378 cf Embedded $=2,717$ cf $\times 40.0 \%$ Voids |
| \#4B | 200.50' | 1,378 cf | ADS_StormTech SC-740 +Capx 30 Inside \#3 <br> Effective Size $=44.6$ " $\mathrm{W} \times 30.0^{\prime \prime} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12 \mathrm{~L}=45.9 \mathrm{cf}$ Overall Size= $51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with $0.44^{\prime}$ Overlap 30 Chambers in 5 Rows |
|  |  | 6,797 cf | Total Available Storage |

Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 201.00' | 12.0" Round Culvert L=25.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 201.00' / 200.88' S=0.0048 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| \#2 | Discarded | 200.00' | $8.280 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01 ' |
| \#3 | Device 1 | 203.40' | 4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) |
| \#4 | Device 1 | 202.40' | 8.0" Vert. Orifice/Grate $\mathrm{C}=0.600$ Limited to weir flow at low heads |

Discarded OutFlow Max=0.61 cfs @ 11.90 hrs HW=200.06' (Free Discharge)
—2=Exfiltration (Exfiltration Controls 0.61 cfs)
Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=200.00' TW=201.75' (Dynamic Tailwater)
$L_{1}=$ Culvert ( Controls 0.00 cfs)

- $3=$ Sharp-Crested Rectangular Weir ( Controls 0.00 cfs )
-4=Orifice/Grate ( Controls 0.00 cfs )


## Summary for Link AP1: ANALYSIS POINT 1

| Inflow Area $=$ | $9,943 \mathrm{sf}$, 92.79\% Impervious, | Inflow Depth > 2.71" | for 2 YR event |  |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.67 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $2,247 \mathrm{cf}$ |
| Primary | $=$ | $0.67 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $2,247 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Primary outflow $=$ Inflow, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$

## Summary for Link AP2: ANALYSIS POINT 2

| fow | 816,898 sf, 39.51\% Impervious | w Depth > 1.10" for 2YR event |
| :---: | :---: | :---: |
| Inflow | 7.31 cfs @ 12.40 hrs , Volume= | 75,144 cf |
| Primary | 7.31 cfs @ 12.40 hrs , Volume= | 75,144 cf, Atten= 0\%, Lag= 0.0 min |

Primary outflow $=$ Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP3: ANALYSIS POINT 3



Primary outflow $=$ Inflow, Time Span $=0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$

## Summary for Link AP4: ANALYSIS POINT \#4

| Inflow Area $=$ | $1,691,659 \mathrm{sf}, 25.34 \%$ Impervious, | Inflow Depth $>0.46 "$ | for 2 YR event |  |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $7.79 \mathrm{cfs} @$ | 12.60 hrs , Volume= | $65,174 \mathrm{cf}$ |
| Primary | $=$ | $7.79 \mathrm{cfs} @$ | 12.60 hrs , Volume= | $65,174 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Primary outflow $=$ Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Time span=0.00-24.00 hrs, $\mathrm{dt}=0.05 \mathrm{hrs}, 481$ points $\times 3$
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentB1: MULTIFAMILYBLDG

## SubcatchmentB2: MULTIFAMILYBLDG

SubcatchmentB3: MULTIFAMILY

SubcatchmentC10: CB \#10

SubcatchmentC11: CB \#11

SubcatchmentC12: CB \#12

SubcatchmentC13: CB \#13

SubcatchmentC14: CB \#14

SubcatchmentC15: CB \#15

SubcatchmentC16: CB \#16

SubcatchmentC17: CB \#17

SubcatchmentC18: CB \#18

SubcatchmentC20: CB \#20

SubcatchmentC21: CB \#21

SubcatchmentC22: CB \#22

SubcatchmentC23: CB \#23

Runoff Area=23,255 sf $100.00 \%$ Impervious Runoff Depth $>4.72^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=2.53 \mathrm{cfs} 9,148 \mathrm{cf}$

Runoff Area=17,561 sf $100.00 \%$ Impervious Runoff Depth $>4.72^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=1.91 cfs $6,908 \mathrm{cf}$

Runoff Area=19,981 sf $100.00 \%$ Impervious Runoff Depth>4.72" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff= $2.17 \mathrm{cfs} 7,860 \mathrm{cf}$

Runoff Area $=6,961$ sf $100.00 \%$ Impervious Runoff Depth $>4.72$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.76 \mathrm{cfs} 2,738 \mathrm{cf}$

Runoff Area $=7,173$ sf $100.00 \%$ Impervious Runoff Depth $>4.72$ " $\mathrm{T}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.78 \mathrm{cfs} 2,822 \mathrm{cf}$

Runoff Area $=5,238$ sf $100.00 \%$ Impervious Runoff Depth $>4.72$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.57 \mathrm{cfs} 2,060 \mathrm{cf}$

Runoff Area $=10,873$ sf $90.78 \%$ Impervious Runoff Depth $>4.49$ " Tc=6.0 min CN=96 Runoff=1.16 cfs $4,068 \mathrm{cf}$

Runoff Area=12,099 sf $86.22 \%$ Impervious Runoff Depth>3.94" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=91$ Runoff $=1.20 \mathrm{cfs} 3,973 \mathrm{cf}$

Runoff Area $=6,666$ sf $100.00 \%$ Impervious Runoff Depth $>4.72^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=98$ Runoff $=0.73 \mathrm{cfs} 2,622 \mathrm{cf}$

Runoff Area $=8,516$ sf $64.88 \%$ Impervious Runoff Depth $>2.76$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=79$ Runoff $=0.62 \mathrm{cfs} 1,962 \mathrm{cf}$

Runoff Area $=11,836$ sf $73.87 \%$ Impervious Runoff Depth $>4.05$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=92$ Runoff $=1.20 \mathrm{cfs} 3,992 \mathrm{cf}$

Runoff Area=18,591 sf $66.35 \%$ Impervious Runoff Depth>3.83" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=90$ Runoff=$=1.81 \mathrm{cfs} 5,941 \mathrm{cf}$

Runoff Area $=11,939$ sf $88.95 \%$ Impervious Runoff Depth $>4.38$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff $=1.26 \mathrm{cfs} 4,355 \mathrm{cf}$

Runoff Area=10,174 sf $87.04 \%$ Impervious Runoff Depth $>3.83$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=90$ Runoff $=0.99 \mathrm{cfs} 3,251 \mathrm{cf}$

Runoff Area=12,001 sf $91.62 \%$ Impervious Runoff Depth>4.49" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=1.28 \mathrm{cfs} 4,490 \mathrm{cf}$

Runoff Area=9,694 sf $61.00 \%$ Impervious Runoff Depth $>3.73$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=89$ Runoff $=0.93 \mathrm{cfs} 3,014 \mathrm{cf}$

SubcatchmentC24: CB \#24

## SubcatchmentC25: CB \#25

SubcatchmentC26: CB \#26

SubcatchmentC27: CB \#27

SubcatchmentC28: CB \#28

SubcatchmentC29: CB \#29

SubcatchmentC30: CB \#30

SubcatchmentC31: CB \#31

SubcatchmentC32: CB \#32

SubcatchmentC33: CB \#33

SubcatchmentC34: CB \#34

SubcatchmentC35: CB \#35

SubcatchmentC36: CB \#36

SubcatchmentC38: CB \#38

SubcatchmentC39: CB \#39

SubcatchmentC40: CB \#40

SubcatchmentC41: CB \#41

Runoff Area=7,930 sf $72.16 \%$ Impervious Runoff Depth>4.16" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff $=0.82 \mathrm{cfs} 2,746 \mathrm{cf}$

Runoff Area $=8,487$ sf $80.92 \%$ Impervious Runoff Depth $>4.27$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff $=0.89 \mathrm{cfs} 3,017 \mathrm{cf}$

Runoff Area=8,835 sf $63.75 \%$ Impervious Runoff Depth $>3.94$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=91$ Runoff $=0.88 \mathrm{cfs} 2,901 \mathrm{cf}$

Runoff Area=6,111 sf $91.90 \%$ Impervious Runoff Depth>4.49" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.65 \mathrm{cfs} 2,287 \mathrm{cf}$

Runoff Area=10,372 sf $51.33 \%$ Impervious Runoff Depth $>3.63$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=88$ Runoff $=0.97 \mathrm{cfs} 3,136 \mathrm{cf}$

Runoff Area=8,495 sf $84.21 \%$ Impervious Runoff Depth $>4.27$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff $=0.89 \mathrm{cfs} 3,020 \mathrm{cf}$

Runoff Area=8,933 sf $82.40 \%$ Impervious Runoff Depth $>4.27$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff $=0.93 \mathrm{cfs} 3,175 \mathrm{cf}$

Runoff Area $=16,365$ sf $68.64 \%$ Impervious Runoff Depth $>3.83$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=90$ Runoff $=1.59 \mathrm{cfs} 5,230 \mathrm{cf}$

Runoff Area $=12,710$ sf $70.47 \%$ Impervious Runoff Depth $>3.94$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=91$ Runoff=1.26 cfs $4,174 \mathrm{cf}$

Runoff Area=5,421 sf $83.90 \%$ Impervious Runoff Depth>4.27" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff $=0.57 \mathrm{cfs} 1,927 \mathrm{cf}$

Runoff Area=8,622 sf $80.51 \%$ Impervious Runoff Depth $>4.16$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff $=0.89 \mathrm{cfs} 2,986 \mathrm{cf}$

Runoff Area=4,149 sf $98.10 \%$ Impervious Runoff Depth $>4.72^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.45 \mathrm{cfs} 1,632 \mathrm{cf}$

Runoff Area=6,622 sf $100.00 \%$ Impervious Runoff Depth>4.72" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=$=0.72 \mathrm{cfs} 2,605 \mathrm{cf}$

Runoff Area=7,637 sf $100.00 \%$ Impervious Runoff Depth>4.72" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.83 \mathrm{cfs} 3,004 \mathrm{cf}$

Runoff Area $=7,612$ sf $100.00 \%$ Impervious Runoff Depth $>4.72$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.83 \mathrm{cfs} 2,994 \mathrm{cf}$

Runoff Area $=4,211$ sf $100.00 \%$ Impervious Runoff Depth $>4.72$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=98$ Runoff $=0.46 \mathrm{cfs} 1,656 \mathrm{cf}$

Runoff Area $=5,586$ sf $100.00 \%$ Impervious Runoff Depth $>4.72$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.61 \mathrm{cfs} 2,197 \mathrm{cf}$

SubcatchmentC43: CB \#43

## SubcatchmentC44: CB \#44

SubcatchmentC45: CB \#45

SubcatchmentC46: CB \#46

SubcatchmentC47: CB \#47

SubcatchmentC48: CB \#48

SubcatchmentC49: CB \#49

SubcatchmentC50: CB \#50

SubcatchmentC51: CB \#51

SubcatchmentC52: CB\#52

SubcatchmentC53: CB \#53

SubcatchmentC54: CB \#54

SubcatchmentC7: CB \#5

SubcatchmentC8: CB \#8

Subcatchment C9: CB \#9

SubcatchmentCH1: CLUBHOUSE

SubcatchmentMB1: MAIL KIOSK

Runoff Area=3,109 sf 75.36\% Impervious Runoff Depth>3.73" Tc=6.0 min CN=89 Runoff=0.30 cfs 967 cf

Runoff Area=1,978 sf $84.43 \%$ Impervious Runoff Depth $>4.05$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=92$ Runoff $=0.20 \mathrm{cfs} 667 \mathrm{cf}$

Runoff Area=2,465 sf $50.30 \%$ Impervious Runoff Depth $>2.86$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=80$ Runoff=0.19 cfs 587 cf

Runoff Area=4,397 sf $50.97 \%$ Impervious Runoff Depth>2.86" Tc=6.0 min CN=80 Runoff=0.33 cfs $1,046 \mathrm{cf}$

Runoff Area=3,012 sf 100.00\% Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.33 cfs $1,185 \mathrm{cf}$

Runoff Area=60,128 sf 25.16\% Impervious Runoff Depth>2.00" Flow Length=400' Tc=11.8 min CN=70 Runoff=2.61 cfs 10,030 cf

Runoff Area=5,238 sf $84.59 \%$ Impervious Runoff Depth $>4.27$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=94$ Runoff=0.55 cfs 1,862 cf

Runoff Area=15,040 sf $77.20 \%$ Impervious Runoff Depth $>4.16$ " Tc=6.0 min CN=93 Runoff=1.55 cfs 5,209 cf

Runoff Area=6,823 sf $100.00 \%$ Impervious Runoff Depth>4.72" Tc=6.0 min CN=98 Runoff=0.74 cfs 2,684 cf

Runoff Area=9,052 sf $87.14 \%$ Impervious Runoff Depth>4.38" Tc=6.0 min CN=95 Runoff=0.96 cfs 3,302 cf

Runoff Area=7,863 sf $86.52 \%$ Impervious Runoff Depth $>4.16$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff=0.81 cfs 2,723 cf

Runoff Area=4,821 sf 86.85\% Impervious Runoff Depth>3.94" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=91$ Runoff=0.48 cfs $1,583 \mathrm{cf}$

Runoff Area=4,650 sf 100.00\% Impervious Runoff Depth>4.72" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=98$ Runoff=0.51 cfs $1,829 \mathrm{cf}$

Runoff Area=5,450 sf $88.75 \%$ Impervious Runoff Depth>4.27" Tc=6.0 min CN=94 Runoff=0.57 cfs $1,937 \mathrm{cf}$

Runoff Area=16,307 sf 93.95\% Impervious Runoff Depth>4.60" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=97$ Runoff=1.76 cfs $6,257 \mathrm{cf}$

Runoff Area=6,262 sf 92.70\% Impervious Runoff Depth>4.49" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=0.67 cfs 2,343 cf

Runoff Area=938 sf 100.00\% Impervious Runoff Depth>4.72" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=0.10 cfs 369 cf

SubcatchmentS201: SUMMER STREET Runoff Area=9,943 sf 92.79\% Impervious Runoff Depth>4.38" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff $=1.05 \mathrm{cfs} 3,627 \mathrm{cf}$

SubcatchmentS202: EXISTING WETLANDRunoff Area=432,269 sf $42.08 \%$ Impervious Runoff Depth $>2.49$ " Flow Length=856' $\quad \mathrm{Cc}=23.2 \mathrm{~min} \quad \mathrm{CN}=76$ Runoff=18.32 cfs $89,722 \mathrm{cf}$

SubcatchmentS203: POCKET WETLAND\#1 Runoff Area=25,587 sf $0.00 \%$ Impervious Runoff Depth $>2.16$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=72$ Runoff=$=1.45 \mathrm{cfs} 4,616 \mathrm{cf}$

## SubcatchmentS204: EXISTING

Runoff Area=308,203 sf $31.07 \%$ Impervious Runoff Depth $>2.41$ " Flow Length=632' Tc=22.6 min CN=75 Runoff=12.75 cfs 61,782 cf

SubcatchmentS205: ISOLATEDWETLANDRunoff Area=55,420 sf 16.57\% Impervious Runoff Depth>2.00" Tc=6.0 min CN=70 Runoff=2.89 cfs $9,258 \mathrm{cf}$

SubcatchmentS206: OVERLANDFLOW Runoff Area=891,295 sf 2.91\% Impervious Runoff Depth>1.61" Flow Length=1,467' Tc=34.5 min CN=65 Runoff=19.55 cfs 119,746 cf

SubcatchmentS207:INFILTRATIONPOND Runoff Area=20,803 sf $0.00 \%$ Impervious Runoff Depth $>3.63$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=88$ Runoff= $1.94 \mathrm{cfs} 6,290 \mathrm{cf}$

## SubcatchmentS208: GRASS AREA

Runoff Area=13,760 sf $9.33 \%$ Impervious Runoff Depth>2.41" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=75$ Runoff=0.88 cfs $2,769 \mathrm{cf}$

SubcatchmentS209: WETLANDC Runoff Area=107,073 sf $0.38 \%$ Impervious Runoff Depth $>2.23$ " Flow Length=607' Slope=0.0150 '/l' Tc=28.9 min CN=73 Runoff=3.68 cfs 19,940 cf

SubcatchmentS210: INFILTRATIONPOND Runoff Area=75,890 sf 0.00\% Impervious Runoff Depth>2.67" Flow Length=580' Slope=0.0150 '/l' Tc=16.5 min CN=78 Runoff=3.96 cfs $16,882 \mathrm{cf}$

## SubcatchmentS211: S211

SubcatchmentS212: SWALE

## SubcatchmentS213: COURTYARD

## SubcatchmentT1: Trench Drain 1

## SubcatchmentT2: Drive Under B2

SubcatchmentTH1: TOWN HOUSE\#1

SubcatchmentTH10: TOWN HOUSE \#10

Runoff Area=15,436 sf $47.47 \%$ Impervious Runoff Depth $>2.76$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=79$ Runoff $=1.13 \mathrm{cfs} 3,556 \mathrm{cf}$

Runoff Area=52,768 sf $0.60 \%$ Impervious Runoff Depth>1.62" Flow Length=418' Tc=23.1 min CN=65 Runoff=1.38 cfs $7,112 \mathrm{cf}$

Runoff Area $=21,407$ sf $48.10 \%$ Impervious Runoff Depth $>2.76$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=79$ Runoff=1.56 cfs $4,932 \mathrm{cf}$

Runoff Area=11,173 sf $75.10 \%$ Impervious Runoff Depth $>4.16$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff=1.15 cfs $3,869 \mathrm{cf}$

Runoff Area=4,445 sf $64.30 \%$ Impervious Runoff Depth $>2.68$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=78$ Runoff $=0.31 \mathrm{cfs} 991 \mathrm{cf}$

Runoff Area=4,247 sf $92.68 \%$ Impervious Runoff Depth $>4.49$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.45 \mathrm{cfs} 1,589 \mathrm{cf}$

Runoff Area=3,476 sf $91.60 \%$ Impervious Runoff Depth>4.49" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.37 \mathrm{cfs} 1,301 \mathrm{cf}$

Subcatchment TH11: TOWN HOUSE \#11

## Subcatchment TH2: TOWN HOUSE \#2

Subcatchment TH3: TOWN HOUSE \#3

Subcatchment TH4: TOWN HOUSE\#4

Subcatchment TH5: TOWN HOUSE \#5

SubcatchmentTH6: TOWN HOUSE \#6

SubcatchmentTH7: TOWN HOUSE\#7

SubcatchmentTH8: TOWN HOUSE \#8

SubcatchmentTH9: TOWN HOUSE \#9

Runoff Area=4,210 sf 92.61\% Impervious Runoff Depth>4.49" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=96$ Runoff= $0.45 \mathrm{cfs} 1,575 \mathrm{cf}$

Runoff Area=4,247 sf 92.68\% Impervious Runoff Depth>4.49" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=96$ Runoff= $0.45 \mathrm{cfs} 1,589 \mathrm{cf}$

Runoff Area=3,013 sf $88.68 \%$ Impervious Runoff Depth $>4.38$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff=0.32 cfs $1,099 \mathrm{cf}$

Runoff Area=3,470 sf $91.59 \%$ Impervious Runoff Depth>4.49" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=96$ Runoff=0.37 cfs $1,298 \mathrm{cf}$

Runoff Area=3,016 sf 88.69\% Impervious Runoff Depth>4.38" Tc=6.0 min CN=95 Runoff=0.32 cfs $1,100 \mathrm{cf}$

Runoff Area=3,407 sf $91.46 \%$ Impervious Runoff Depth $>4.49$ " Tc=6.0 min $\mathrm{CN}=96$ Runoff=0.36 cfs $1,275 \mathrm{cf}$

Runoff Area=3,481 sf $91.61 \%$ Impervious Runoff Depth>4.49" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=0.37 cfs 1,302 cf

Runoff Area=4,212 sf 92.62\% Impervious Runoff Depth>4.49" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=0.45 cfs $1,576 \mathrm{cf}$

Runoff Area=3,480 sf $91.61 \%$ Impervious Runoff Depth>4.49" Tc=6.0 min CN=96 Runoff=0.37 cfs $1,302 \mathrm{cf}$

Reach 8R: OVERLANDFLOW Avg. Flow Depth=0.06' Max Vel=0.08 fps Inflow=1.10 cfs 3,558 cf $\mathrm{n}=0.400 \mathrm{~L}=563.0^{\prime} \mathrm{S}=0.0213 \mathrm{l} / \mathrm{l}$ Capacity=28.09 cfs Outflow=0.26 cfs 3,276 cf

Reach 9R: OVERLANDFLOW
Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0 cf $\mathrm{n}=0.400 \mathrm{~L}=211.0^{\prime} \mathrm{S}=0.0652 \mathrm{I} / \mathrm{Capacity=} 23.45 \mathrm{cfs}$ Outflow=0.00 cfs 0 cf

Reach 10R: OVERLANDFLOW Avg. Flow Depth=0.19' Max Vel=0.22 fps Inflow=0.96 cfs 5,876 cf


Reach 11R: 4x4 Open Bottom Culvert Avg. Flow Depth=0.63' Max Vel=1.62 fps Inflow=4.11 cfs 40,067 cf $48.0 " x 48.0 "$ Box Pipe $n=0.069 \quad L=30.0 ' S=0.0150$ '/' Capacity=42.20 cfs Outflow=4.11 cfs 40,059 cf

Reach 12R: OVERLANDFLOW Avg. Flow Depth=0.10' Max Vel=0.12 fps Inflow=1.23 cfs 3,881 cf $\mathrm{n}=0.400 \mathrm{~L}=250.0^{\prime} \quad \mathrm{S}=0.0240 \mathrm{I} / \mathrm{l}$ Capacity=29.80 cfs Outflow=0.61 cfs $3,787 \mathrm{cf}$

Reach 14R: OVERLANDFLOW Avg. Flow Depth=0.07' Max Vel=0.09 fps Inflow=1.38 cfs 7,111 cf $\mathrm{n}=0.400 \mathrm{~L}=852.0^{\prime} \mathrm{S}=0.0246 \mathrm{I} / \mathrm{l}$ Capacity=31.55 cfs Outflow=0.31 cfs 6,155 cf

Reach 15R: OVERLANDFLOW Avg. Flow Depth=0.05' Max Vel=0.07 fps Inflow=0.19 cfs 7,883 cf $\mathrm{n}=0.400 \mathrm{~L}=300.0^{\prime} \mathrm{S}=0.0200 \mathrm{I} / \mathrm{l}$ Capacity=27.21 cfs Outflow=0.19 cfs $7,158 \mathrm{cf}$

Reach 18R: OVERLANDFLOW Avg. Flow Depth=0.03' Max Vel=0.06 fps Inflow=0.40 cfs 1,449 cf $\mathrm{n}=0.400 \mathrm{~L}=609.0^{\prime} \mathrm{S}=0.0279$ '/' Capacity=38.42 cfs Outflow=0.09 cfs $1,312 \mathrm{cf}$

Reach 20R: OVERLANDFLOW Avg. Flow Depth=0.18' Max Vel=0.11 fps Inflow=1.79 cfs 13,822 cf


Reach 23R: OVERLANDFLOW Avg. Flow Depth=0.29' Max Vel=0.22 fps Inflow=4.11 cfs $40,059 \mathrm{cf}$ $\mathrm{n}=0.400 \mathrm{~L}=237 . \mathbf{O}^{\prime} \quad \mathrm{S}=0.0211^{\mathrm{I}} / \mathrm{I} \quad$ Capacity=$=31.93 \mathrm{cfs} \quad$ Outflow=3.63 cfs $39,543 \mathrm{cf}$

Reach R202: OVERLANDFLOW Avg. Flow Depth=0.39' Max Vel=0.19 fps Inflow=18.32 cfs $89,705 \mathrm{cf}$ $\mathrm{n}=0.400 \mathrm{~L}=700.0^{\prime} \mathrm{S}=0.0107 \mathrm{l} / \mathrm{Capacity=} \mathbf{4 2 . 5 6 \mathrm { cfs } \text { Outflow=8.31 cfs 84,798 cf }}$

Reach R211: OVERLANDFLOW Avg. Flow Depth=0.19' Max Vel=0.11 fps Inflow=3.94 cfs $6,192 \mathrm{cf}$ $\mathrm{n}=0.400 \mathrm{~L}=600.0$ ' $\mathrm{S}=0.0087$ '/' Capacity=14.51 cfs Outflow=0.78 cfs $5,906 \mathrm{cf}$

Reach SC1: Stream Crossing\#1 Avg. Flow Depth=0.34' Max Vel=3.33 fps Inflow=18.32 cfs 89,722 cf 192.0" $\times 60.0$ ", $\mathrm{R}=207.0^{\prime \prime}$ Arch Pipe $\mathrm{n}=0.030 \mathrm{~L}=43.1$ ' $\mathrm{S}=0.0200$ '/' Capacity=722.91 cfs Outflow=18.32 cfs $89,705 \mathrm{cf}$

Reach SC2: Stream Crossing\#2 Avg. Flow Depth=0.07' Max Vel=1.32 fps Inflow=1.38 cfs 7,112 cf $192.0^{\prime \prime} \times 60.0$ ", $R=180.0$ " Arch Pipe $n=0.030 \quad L=36.5^{\prime} \quad S=0.0241$ '/' Capacity=768.96 cfs Outflow=1.38 cfs $7,111 \mathrm{cf}$

Pond 1P: DMH \#33

Pond 3P: OCS \#8

## Pond 5R: TRENCH DRAIN

Pond 11P: YARD DRAIN

Pond CB10: CB \#10

Pond CB11: CB \#11

Pond CB12: CB \#12

Pond CB13: CB \#13

Pond CB14: CB \#14

Pond CB15: CB \#15

Pond CB16: CB \#16

Peak Elev=206.22' Inflow=1.73 cfs 6,129 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=46.7^{\prime} \mathrm{S}=0.0251$ '//' Outflow=1.73 cfs $6,129 \mathrm{cf}$

Peak Elev=201.47' Inflow=1.29 cfs 4,306 cf Outflow=1.29 cfs 4,306 cf

Peak Elev=198.02' Inflow=1.15 cfs 3,869 cf 8.0" Round Culvert n=0.012 L=36.0' S=0.0200 '/' Outflow=1.15 cfs 3,869 cf

Peak Elev=207.37' Storage=671 cf Inflow=1.56 cfs 4,932 cf Outflow=1.14 cfs 4,891 cf

Peak Elev=210.33' Inflow=0.76 cfs 2,738 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=33.8$ ' $\mathrm{S}=0.0050$ '/' Outflow=0.76 cfs $2,738 \mathrm{cf}$

Peak Elev=210.43' Inflow=0.78 cfs 2,822 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=26.3^{\prime} \mathrm{S}=0.0103$ '/' Outflow=0.78 cfs $2,822 \mathrm{cf}$

Peak Elev=207.06' Inflow=0.57 cfs 2,060 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=41.3^{\prime} \mathrm{S}=0.0249$ '/' Outflow=0.57 cfs $2,060 \mathrm{cf}$

Peak Elev=207.26' Inflow=1.16 cfs 4,068 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=43.7^{\prime} \mathrm{S}=0.0249$ '/' Outflow=1.16 cfs $4,068 \mathrm{cf}$

Peak Elev=201.69' Inflow=1.20 cfs 3,973 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=23.2^{\prime} \mathrm{S}=0.0052$ '//' Outflow=1.20 cfs $3,973 \mathrm{cf}$

Peak Elev=201.60' Inflow=0.73 cfs 2,622 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=15.6^{\prime} \mathrm{S}=0.0051$ '/' Outflow=0.73 cfs $2,622 \mathrm{cf}$

Peak Elev=203.99' Inflow=0.62 cfs 1,962 cf 12.0" Round Culvert n=0.013 L=20.9' S=0.0067 '/' Outflow=0.62 cfs 1,962 cf

| 19097 Post-Development |  | Type III 24-hr 10YR Rainfall=4.96" |
| :---: | :---: | :---: |
| Prepared by Howard Stein Hudson Associates |  | Printed 6/19/2023 |
| HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Page 120 |  |  |
| Pond CB17: CB \#17 |  | Peak Elev=205.64' Inflow=1.20 cfs 3,992 cf |
|  | 12.0" Round Culvert n=0.013 | $\mathrm{L}=13.8$ ' S=0.0094 '/' Outflow=1.20 cfs 3,992 cf |
| Pond CB18: CB \#18 |  | Peak Elev=205.61' Inflow=2.14 cfs 6,857 cf |
|  | 15.0" Round Culvert n=0.013 | $\mathrm{L}=25.1$ ' S=0.0052 '/' Outflow=2.14 cfs 6,857 cf |
| Pond CB20: CB \#20 |  | Peak Elev=204.67' Inflow=1.26 cfs 4,355 cf |
|  | 12.0" Round Culvert n=0.013 | $\mathrm{L}=30.3$ ' S=0.0053 '/' Outflow=1.26 cfs 4,355 cf |
| Pond CB21: CB \#21 |  | Peak Elev=204.94' Inflow=0.99 cfs 3,251 cf |
|  | 12.0" Round Culvert n=0.013 | $\mathrm{L}=26.0$ ' S=0.0050 '/' Outflow=0.99 cfs 3,251 cf |
| Pond CB22: CB \#22 |  | Peak Elev=206.05' Inflow=1.28 cfs 4,490 cf |
|  | 12.0" Round Culvert n=0.012 | $\mathrm{L}=16.1$ ' S=0.0050 '/' Outflow=1.28 cfs 4,490 cf |
| Pond CB23: CB \#23 |  | Peak Elev=206.00' Inflow=0.93 cfs 3,014 cf |
|  | 12.0" Round Culvert n=0.012 | L=16.3' S=0.0055 '/' Outflow=0.93 cfs 3,014 cf |
| Pond CB24: CB \#24 |  | Peak Elev=205.83' Inflow=0.82 cfs 2,746 cf |
|  | 12.0" Round Culvert n=0.012 | $\mathrm{L}=12.1$ ' S=0.0050 '/' Outflow=0.82 cfs 2,746 cf |
| Pond CB25: CB \#25 |  | Peak Elev=205.82' Inflow=0.89 cfs 3,017 cf |
|  | 15.0" Round Culvert n=0.012 | L=11.4' S=0.0053 '/' Outflow=0.89 cfs 3,017 cf |
| Pond CB26: CB \#26 |  | Peak Elev=202.34' Inflow=0.88 cfs 2,901 cf |
|  | 12.0" Round Culvert n=0.013 | L=42.5' S=0.0052 '/' Outflow=0.88 cfs 2,901 cf |
| Pond CB27: CB \#27 |  | Peak Elev=201.49' Inflow=0.65 cfs 2,287 cf |
|  | 12.0" Round Culvert n=0.013 | L=18.0' S=0.0056 '/' Outflow=0.65 cfs 2,287 cf |
| Pond CB28: CB \#28 |  | Peak Elev=198.44' Inflow=0.97 cfs 3,136 cf |
|  | 12.0" Round Culvert n=0.013 | $\mathrm{L}=13.7$ ' S=0.0044 '/' Outflow=0.97 cfs 3,136 cf |
| Pond CB29: CB \#29 |  | Peak Elev=206.08' Inflow=0.89 cfs 3,020 cf |
|  | 12.0" Round Culvert n=0.013 | L=13.5' S=0.0052 '/' Outflow=0.89 cfs 3,020 cf |
| Pond CB30: CB \#30 |  | Peak Elev=206.09' Inflow=0.93 cfs 3,175 cf |
|  | 12.0" Round Culvert n=0.013 | $\mathrm{L}=17.5$ ' S=0.0051 '/' Outflow=0.93 cfs 3,175 cf |
| Pond CB31: CB \#31 |  | Peak Elev=205.02' Inflow=1.59 cfs 5,230 cf |
|  | 12.0" Round Culvert n=0.013 | L=16.4' S=0.0049 '/' Outflow=1.59 cfs 5,230 cf |
| Pond CB32: CB \#32 |  | Peak Elev=204.91' Inflow=1.26 cfs 4,174 cf |
|  | 12.0" Round Culvert n=0.013 | L=16.3' S=0.0049 '/' Outflow=1.26 cfs 4,174 cf |
| Pond CB33: CB \#33 |  | Peak Elev=205.84' Inflow=0.57 cfs 1,927 cf |
|  | 12.0" Round Culvert n=0.013 | L=11.7' S=0.0051 '/' Outflow=0.57 cfs 1,927 cf |
| Pond CB34: CB \#34 |  | Peak Elev=205.89' Inflow=0.89 cfs 2,986 cf |
|  | 12.0" Round Culvert n=0.013 | L=16.5' S=0.0048 '/' Outflow=0.89 cfs 2,986 cf |


| 19097 Post-Development Type III 24-hr 10YR Rainfall=4.96" |  |
| :---: | :---: |
| Prepared by Howard Stein Hudson Associates Printed 6/19/2023 |  |
| HydroCAD® 10.20-3c | © 2023 HydroCAD Software Solutions LLC Page 121 |
| Pond CB35: CB \#35 | Peak Elev=207.44' Inflow=0.45 cfs 1,632 cf |
|  | 12.0" Round Culvert n=0.013 L=15.2' S=0.0053 '/' Outflow=0.45 cfs 1,632 cf |
| Pond CB36: CB \#36 | Peak Elev=207.56' Inflow=0.72 cfs 2,605 cf |
|  | 12.0" Round Culvert n=0.013 L=16.1' S=0.0050 '/' Outfow=0.72 cfs $2,605 \mathrm{cf}$ |
| Pond CB38: CB \#38 | Peak Elev=210.56' Inflow=0.83 cfs 3,004 cf |
|  | 12.0" Round Culvert n=0.012 L=16.7' S=0.0048 '/' Outfow=0.83 cfs 3,004 cf |
| Pond CB39: CB \#39 | Peak Elev=210.56' Inflow=0.83 cfs 2,994 cf |
|  | 12.0" Round Culvert n=0.013 L=16.4' S=0.0049 '/' Outfow=0.83 cfs $2,994 \mathrm{cf}$ |
| Pond CB40: CB \#40 | Peak Elev=214.16' Inflow=0.46 cfs 1,656 cf |
|  | 12.0" Round Culvert n=0.013 L=17.8' S=0.0073 '/' Outfow=0.46 cfs 1,656 cf |
| Pond CB41: CB \#41 | Peak Elev=214.37' Inflow=0.61 cfs 2,197 cf |
|  | 12.0" Round Culvert n=0.013 L=18.4' S=0.0049 '/' Outfow=0.61 cfs 2,197 cf |
| Pond CB43: CB \#43 | Peak Elev=220.42' Inflow=0.30 cfs 967 cf |
|  | 12.0" Round Culvert n=0.013 L=14.9' S=0.0047 '/' Outflow=0.30 cfs 967 cf |
| Pond CB44: CB \#44 | Peak Elev=220.39' Inflow=0.20 cfs 667 cf |
|  | 12.0" Round Culvert n=0.013 L=14.9' S=0.0047 '/' Outflow=0.20 cfs 667 cf |
| Pond CB45: CB \#45 | Peak Elev=221.54' Inflow=0.19 cfs 587 cf |
|  | 12.0" Round Culvert n=0.013 L=18.2' S=0.0049 '/' Outflow=0.19 cfs 587 cf |
| Pond CB46: CB \#46 | Peak Elev=221.87' Inflow=0.33 cfs 1,046 cf |
|  | 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=15.3$ ' S=0.0052 '/' Oufflow=0.33 cfs 1,046 cf |
| Pond CB47: CB\#47 | Peak Elev=225.52' Inflow=0.33 cfs 1,185 cf |
|  | 12.0" Round Culvert n=0.012 L=20.9' S=0.0048 '/' Outfow=0.33 cfs 1,185 cf |
| Pond CB48: CB\#48 | Peak Elev=225.81' Inflow=2.61 cfs 10,030 cf |
|  | 15.0" Round Culvert n=0.012 L=16.9' S=0.0047 '/' Oufflow=2.61 cfs 10,030 cf |
| Pond CB49: CB \#49 | Peak Elev=203.45' Inflow=0.55 cfs 1,862 cf |
|  | 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=15.5$ ' S=0.0052 '/' Outfow=0.55 cfs $1,862 \mathrm{cf}$ |
| Pond CB50: CB \#50 | Peak Elev=203.64' Inflow=1.55 cfs 5,209 cf |
|  | 12.0" Round Culvert n=0.013 L=15.3' S=0.0052 '/' Outfow=1.55 cfs 5,209 cf |
| Pond CB51: CB \#51 | Peak Elev=202.87' Inflow=0.74 cfs 2,684 cf |
|  | 12.0" Round Culvert n=0.013 L=31.4' S=0.0051 '/' Ouffow=0.74 cfs 2,684 cf |
| Pond CB52: CB \#52 | Peak Elev=203.29' Inflow=0.96 cfs 3,302 cf |
|  | 12.0" Round Culvert n=0.013 L=25.5' S=0.0051 '/' Outflow=0.96 cfs 3,302 cf |
| Pond CB53: CB \#53 | Peak Elev=203.33' Inflow=0.81 cfs 2,723 cf |
|  | 12.0" Round Culvert n=0.013 L=32.0' S=0.0050 '/' Outflow=0.81 cfs $2,723 \mathrm{cf}$ |


| 19097 Post-Development |  | Type III 24-hr 10YR Rainfall=4.96" |
| :---: | :---: | :---: |
| Prepared by Howard Stein Hudson Associates |  | Printed 6/19/2023 |
| HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Page 122 |  |  |
| Pond CB54: CB \#54 |  | Peak Elev=203.07' Inflow=0.48 cfs 1,583 cf |
|  | 12.0" Round Culvert n=0.013 L | L=36.7' S=0.0049 '/' Outflow=0.48 cfs 1,583 cf |
| Pond CB7: CB\#5 |  | Peak Elev=212.99' Inflow=0.51 cfs 1,829 cf |
|  | 12.0" Round Culvert n=0.012 | L=15.1' S=0.0099 '/' Outflow=0.51 cfs 1,829 cf |
| Pond CB8: CB\#8 |  | Peak Elev=214.21' Inflow=0.57 cfs 1,937 cf |
|  | 12.0" Round Culvert n=0.013 | L=15.1' S=0.0099 '/' Outflow=0.57 cfs 1,937 cf |
| Pond CB9: CB \#9 |  | Peak Elev=210.82' Inflow=1.76 cfs 6,257 cf |
|  | 12.0" Round Culvert n=0.013 | L=19.9' S=0.0196 '/' Outfow=1.76 cfs 6,257 cf |
| Pond D10: DMH \#10 |  | Peak Elev=203.81' Inflow=0.62 cfs 1,962 cf |
|  | 12.0" Round Culvert n=0.013 L | L=15.6' S=0.0051 '/' Outflow=0.62 cfs 1,962 cf |
| Pond D11: DMH \#11 |  | Peak Elev=205.28' Inflow=3.34 cfs 10,849 cf |
|  | 18.0" Round Culvert n=0.013 L= | L=44.6' S=0.0049 '/' Outflow=3.34 cfs 10,849 cf |
| Pond D12: DMH \#12 |  | Peak Elev=204.24' Inflow=2.25 cfs 7,606 cf |
|  | 12.0" Round Culvert n=0.013 | L=41.9' S=0.0050 '/' Outflow=2.25 cfs 7,606 cf |
| Pond D13: DMH \#13 |  | Peak Elev=203.33' Inflow=7.09 cfs 25,765 cf |
|  | 24.0" Round Culvert n=0.013 L= | L=60.1' S=0.0050 '/' Outflow=7.09 cfs 25,765 cf |
| Pond D14: DMH \#14 |  | Peak Elev=205.16' Inflow=3.91 cfs 13,267 cf |
|  | 18.0" Round Culvert n=0.012 L= | =256.3' S=0.0050 '/' Outflow=3.91 cfs 13,267 cf |
| Pond D16: DMH \#16 |  | Peak Elev=205.68' Inflow=1.70 cfs 5,763 cf |
|  | 15.0" Round Culvert n=0.012 L= | L=103.5' S=0.0050 '/' Outflow=1.70 cfs 5,763 cf |
| Pond D17: DMH \#17 |  | Peak Elev=201.21' Inflow=1.53 cfs 5,188 cf |
|  | 12.0" Round Culvert n=0.013 | L=91.6' S=0.0312 '/' Outfow=1.53 cfs 5,188 cf |
| Pond D18: DMH \#18 |  | Peak Elev=198.27' Inflow=2.50 cfs 8,324 cf |
|  | 15.0" Round Culvert n=0.013 L | L=46.3' S=0.0099 '/' Outflow=2.50 cfs 8,324 cf |
| Pond D19: DMH \#19 |  | Peak Elev=205.94' Inflow=1.82 cfs 6,195 cf |
|  | 12.0" Round Culvert n=0.013 L | L=82.5' S=0.0092 '/' Outflow=1.82 cfs 6,195 cf |
| Pond D2: DMH\#2 |  | Peak Elev=207.99' Inflow=3.67 cfs 14,981 cf |
|  | 15.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=$ | =38.2' S=0.0099 '/' Outflow=3.67 cfs 14,981 cf |
| Pond D20: DMH \#20 |  | Peak Elev=204.96' Inflow=1.82 cfs 6,195 cf |
|  | 15.0" Round Culvert n=0.013 | L=63.5' S=0.0050'/' Outflow=1.82 cfs 6,195 cf |
| Pond D21: DMH \#21 |  | Peak Elev=204.42' Inflow=7.30 cfs 24,749 cf |
|  | 24.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=$ | L=72.4' S=0.0050 $/ \mathrm{l}$ ' Outflow=7.30 cfs 24,749 cf |
| Pond D22: DMH \#22 |  | Peak Elev=205.74' Inflow=2.62 cfs 9,150 cf |
|  | 15.0" Round Culvert n=0.013 L= | L=134.2' S=0.0071 '/' Outflow=2.62 cfs 9,150 cf |


| 19097 Post-Development | ent Type III 24-hr 10YR Rainfall=4.96" |
| :---: | :---: |
| Prepared by Howard Stein Hudson Associates Printed 6/19/2023 | Hudson Associates Printed 6/19/2023 |
| HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Page 123 |  |
| Pond D23: DMH \#23 | Peak Elev=207.23' Inflow=1.17 cfs 4,237 cf |
|  | 15.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=173.3$ ' S=0.0100 '/' Outflow=1.17 cfs $4,237 \mathrm{cf}$ |
| Pond D25: DMH \#25 | Peak Elev=210.49' Inflow=3.74 cfs 13,119 cf |
|  | 15.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=237.6$ ' S=0.0050 '/' Outflow=3.74 cfs $13,119 \mathrm{cf}$ |
| Pond D27: DMH \#27 | Peak Elev=214.05' Inflow=2.08 cfs 7,120 cf |
|  | 15.0" Round Culvert n=0.012 L=63.9' S=0.0150 '/' Outflow=2.08 cfs 7,120 cf |
| Pond D28: DMH \#28 | Peak Elev=217.98' Inflow=1.01 cfs 3,267 cf |
|  | 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=158.3$ ' S=0.0200 '/' Outflow=1.01 cfs 3,267 cf |
| Pond D29: DMH \#29 | Peak Elev=220.35' Inflow=1.01 cfs 3,267 cf |
|  | 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=150.9$ ' $\mathrm{S}=0.0151$ '/' Outflow=1.01 cfs $3,267 \mathrm{cf}$ |
| Pond D30: DMH \#30 | Peak Elev=221.34' Inflow=0.52 cfs 1,633 cf |
|  | 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=184.2$ ' S=0.0050 '/' Outflow=0.52 cfs 1,633 cf |
| Pond D31: DMH\#31 | Peak Elev=225.49' Inflow=2.84 cfs 11,215 cf |
|  | 15.0" Round Culvert n=0.012 L=288.5' S=0.0400 '/' Outflow=2.84 cfs 11,215 cf |
| Pond D34: DMH \#34 | Peak Elev=199.01' Inflow=2.53 cfs 9,148 cf |
|  | 12.0" Round Culvert n=0.012 L=52.0' S=0.0200 '/' Outflow=2.53 cfs 9,148 cf |
| Pond D35: DMH \#35 | Peak Elev=212.99' Inflow=2.08 cfs 7,120 cf |
|  | 15.0" Round Culvert $\mathrm{n}=0.012 \mathrm{L=171.5}$ ' S=0.0150 '/' Outflow=2.08 cfs 7,120 cf |
| Pond D4: DMH\#4 | Peak Elev=211.36' Inflow=3.67 cfs 14,981 cf |
|  | 15.0" Round Culvert n=0.012 L=222.3' S=0.0150 '/' Outflow=3.67 cfs 14,981 cf |
| Pond D5: DMH \#5 | Peak Elev=210.09' Inflow=3.30 cfs 11,817 cf |
|  | 18.0" Round Culvert n=0.013 L=183.0' S=0.0050 '/' Outflow=3.30 cfs 11,817 cf |
| Pond D6: DMH \#6 | Peak Elev=209.03' Inflow=3.30 cfs 11,817 cf |
|  | 18.0" Round Culvert n=0.013 L=299.7' S=0.0050 '/' Outflow=3.30 cfs 11,817 cf |
| Pond D7: DMH \#7 | Peak Elev=207.33' Inflow=3.30 cfs 11,817 cf |
|  | 18.0" Round Culvert n=0.013 L=44.2' S=0.0550 '/' Outflow=3.30 cfs 11,817 cf |
| Pond D8: DMH \#8 | Peak Elev=201.54' Inflow=1.93 cfs 6,595 cf |
|  | 12.0" Round Culvert n=0.013 L=87.7' S=0.0050 '/' Outflow=1.93 cfs 6,595 cf |
| Pond D9: DMH \#9 | Peak Elev=200.97' Inflow=1.93 cfs 6,595 cf |
|  | 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=11.9$ ' S=0.0050 '/' Outflow=1.93 cfs 6,595 cf |
| Pond DE61: DRIP \#61 | Peak Elev=213.22' Storage=273 cf Inflow=0.45 cfs 1,589 cf |
|  | Discarded=0.00 cfs 196 cf Primary=0.38 cfs 1,249 cf Outflow=0.39 cfs 1,445 cf |
| Pond DE62: DRIP \#62 | Peak Elev=213.22' Storage=273 cf Inflow=0.45 cfs 1,589 cf |
|  | Discarded=0.00 cfs 196 cf Primary=0.38 cfs 1,249 cf Outflow=0.39 cfs 1,445 cf |



Pond P204: STORMTECHINFILTRATION Peak Elev=204.72' Storage=8,487 cf Inflow=6.49 cfs 21,764 cf Discarded=0.09 cfs 5,743 cf Primary=1.79 cfs 13,822 cf Outflow=1.88 cfs 19,565 cf
Pond P205: INFILTRATIONPOND \#3 Peak Elev=206.70' Storage=7,871 cf Inflow=4.73 cfs $18,538 \mathrm{cf}$

Discarded=0.30 cfs 14,188 cf Primary= 0.40 cfs 1,449 cf Outflow=0.70 cfs 15,637 cf
Pond P206: STORMTECHINFILTRATION Peak Elev=196.10' Storage=6,143 cf Inflow=6.18 cfs $21,341 \mathrm{cf}$ Discarded=0.49 cfs 18,384 cf Primary= 1.63 cfs 2,954 cf Outflow=2.12 cfs $21,338 \mathrm{cf}$

Pond P207: INFILTRATIONPOND \#2 Peak Elev=198.06' Storage=13,926 cf Inflow=11.58 cfs 40,612 cf Discarded=1.03 cfs 34,722 cf Primary $=0.96$ cfs 5,876 cf Outflow=1.99 cfs 40,597 cf

Pond P210: POCKET WETLAND\#1 Peak Elev=203.77' Storage=13,036 cf Inflow=5.18 cfs $17,735 \mathrm{cf}$ Outflow=0.19 cfs 7,883 cf

Pond P212: INFILTRATIONPOND \#1 Peak Elev=202.65' Storage=24,325 cf Inflow=16.29 cfs 63,872 cf Discarded=1.42 cfs 57,664 cf Primary=3.94 cfs 6,192 cf Outflow=5.35 cfs 63,856 cf

Pond P213: Stormtech Infiltration System Peak Elev=202.65' Storage=3,828 cf Inflow=3.43 cfs $12,114 \mathrm{cf}$ Discarded= 0.39 cfs 12,113 cf Primary=0.00 cfs 0 cf Outflow=0.39 cfs 12,113 cf

Pond P214: STORMTECHINFILTRATION Peak Elev=201.46' Storage=3,080 cf Inflow=3.46 cfs $12,166 \mathrm{cf}$ Discarded=0.61 cfs 12,168 cf Primary $=0.00 \mathrm{cfs} 0 \mathrm{cf}$ Outflow=0.61 cfs $12,168 \mathrm{cf}$

## Link AP1: ANALYSISPOINT 1

## Link AP2: ANALYSISPOINT 2

Inflow=18.24 cfs 157,893 cf Primary=18.24cfs 157,893 cf

## Link AP3: ANALYSISPOINT 3

## Link AP4: ANALYSISPOINT \#4

Inflow=2.89 cfs 9,258 cf Primary $=2.89$ cfs 9,258 cf

Inflow=25.19 cfs 178,860 cf Primary=25.19 cfs 178,860 cf

> Total Runoff Area $=2,573,920$ sf Runoff Volume $=532,571$ cf Average Runoff Depth $=2.48$ " 70.09\% Pervious $=1,803,997$ sf $29.91 \%$ Impervious $=769,923$ sf

## Summary for Subcatchment B1: MULTIFAMILY BLDG \#1

Runoff =
$=\quad 2.53 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume=
9,148 cf, Depth> 4.72"
Routed to Pond D34 : DMH \#34
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 20,156 | 98 R | Roofs, HSG CRoofs, HSG D |  |  |
|  | 3,099 | 98 R |  |  |  |
|  | 23,255 | 98 V | Weighted Average |  |  |
|  | 23,255 |  | 100.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{r} \text { c } \begin{array}{r} \text { Length } \\ \text { (feet) } \end{array} \\ \hline \end{array}$ | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |

## Summary for Subcatchment B2: MULTIFAMILY BLDG \#2

```
Runoff = 1.91 cfs @ 12.09 hrs, Volume=
6,908 cf, Depth> 4.72"
```

Routed to Pond OCS3 : OCS\#3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | ea (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 7,873 | 98 | Roofs, HSG A |  |  |
|  | 9,688 |  | Roofs, HSG C |  |  |
|  | 17,561 | 98 | Weighted Average 100.00\% Impervious Area |  |  |
|  | 17,561 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment B3: MULTIFAMILY BUILDING \#3

Runoff $=\quad 2.17$ cfs @ 12.09 hrs, Volume= 7,860 cf, Depth> 4.72"
Routed to Pond P214 : STORMTECH INFILTRATION SYSTEM \#4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 608 | 98 | Roofs, HSG A |
| 19,373 | 98 | Roofs, HSG C |
| 19,981 | 98 | Weighted Average |
| 19,981 |  | $100.00 \%$ Impervious Area |



Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 339 | 98 | Paved roads w/curbs \& sewers, HSG B |
| 5,703 | 98 | Paved roads w/curbs \& sewers, HSG C |
| 919 | 98 | Paved roads w/curbs \& sewers, HSG D |

## Summary for Subcatchment C11: CB \#11

Runoff $=\quad 0.78$ cfs @ 12.09 hrs, Volume $=\quad 2,822 \mathrm{cf}$, Depth> 4.72"
Routed to Pond CB11 : CB \#11
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"


## Summary for Subcatchment C12: CB \#12

Runoff $=\quad 0.57$ cfs @ 12.09 hrs, Volume= 2,060 cf, Depth> 4.72"
Routed to Pond CB12 : CB \#12
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


## Summary for Subcatchment C13: CB \#13

Runoff $=\quad 1.16$ cfs @ 12.09 hrs, Volume= $\quad 4,068 \mathrm{cf}$, Depth> 4.49"
Routed to Pond CB13 : CB \#13
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,003 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 7,547 | 98 | Paved parking, HSG C |
| 2,323 | 98 | Roofs, HSG C |

Summary for Subcatchment C14: CB \#14
Runoff = 1.20 cfs @ 12.09 hrs, Volume= $3,973 \mathrm{cf}$, Depth> 3.94"
Routed to Pond CB14 : CB \#14
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,195 | 39 | >75\% Grass cover, Good, HSG A |  |  |
|  | 7,649 | 98 P | Paved parking, HSG A |  |  |
|  | 472 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 2,783 | 98 P | Paved parking, HSG C |  |  |
|  | 12,099 | 91 | Weighted Average |  |  |
|  | 1,667 |  | 13.78\% Pervious Area |  |  |
|  | 10,432 |  | 86.22\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) |  | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C15: CB \#15

Runoff $=0.73$ cfs @ 12.09 hrs, Volume=
2,622 cf, Depth> 4.72"
Routed to Pond CB15 : CB \#15
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 5,000 |  |  |  |  |
|  | 1,666 |  | Paved parking, HSG A Paved parking, HSG C |  |  |
|  | $\begin{aligned} & \hline 6,666 \\ & 6,666 \end{aligned}$ | 98 | Weighted Average 100.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{rr} c & \text { Length } \\ \text { ) } & \text { (feet) } \\ \hline \end{array}$ | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |

## Summary for Subcatchment C16: CB \#16

```
Runoff = 0.62 cfs @ 12.09 hrs, Volume= 1,962 cf, Depth> 2.76"
```

Routed to Pond CB16 : CB \#16
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 2,467 | 39 | $>75 \%$ Grass cover, Good, HSG A |
| 4,380 | 98 | Paved parking, HSG A |
| 524 | 74 | >75\% Grass cover, Good, HSG C |
| 1,145 | 98 | Paved parking, HSG C |
| 8,516 | 79 | Weighted Average |
| 2,991 |  | $35.12 \%$ Pervious Area |
| 5,525 |  | $64.88 \%$ Impervious Area |
| Tc | Length | Slope <br> (ft/ft) |
| Velocity  <br> (ft/sec) Capacity <br> (min) (cfs) |  |  |

Direct Entry,
Summary for Subcatchment C17: CB \#17
Runoff = 1.20 cfs @ 12.09 hrs, Volume= 3,992 cf, Depth> 4.05"
Routed to Pond CB17 : CB \#17
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,093 |  |  |  |  |
|  | 8,743 |  | >75\% Grass cover, Good, HSG C Paved parking, HSG C |  |  |
|  | 11,836 | 92 V | Weighted Average |  |  |
|  | 3,093 |  | 26.13\% Pervious Area |  |  |
|  | 8,743 |  | 73.87\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | $\begin{gathered} \text { Velocity } \\ (\mathrm{ft} / \mathrm{sec}) \end{gathered}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entr |
|  |  |  | Summary for Subcatchment C18: CB \#18 |  |  |

Runoff $=\quad 1.81$ cfs @ 12.09 hrs, Volume= 5,941 cf, Depth> 3.83"

Routed to Pond CB18 : CB \#18
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 6,255 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 12,336 | 98 | Paved parking, HSG C |
| 18,591 | 90 | Weighted Average |
| 6,255 |  | $33.65 \%$ Pervious Area |
| 12,336 |  | $66.35 \%$ Impervious Area |
| Tc | Length | Slope <br> (ft/ft) |
| Velocity  <br> (ft/sec) Capacity <br> (min) (cfs) |  |  |

6.0

Direct Entry,
Summary for Subcatchment C20: CB \#20
Runoff $=\quad 1.26$ cfs @ 12.09 hrs, Volume= 4,355 cf, Depth> 4.38"
Routed to Pond CB20 : CB \#20
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,319 | 98 | Paved parking, HSG A |
| 1,319 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 7,301 | 98 | Paved parking, HSG C |
| 11,939 | 95 | Weighted Average |
| 1,319 |  | $11.05 \%$ Pervious Area <br> 10,620 |
|  | $88.95 \%$ Impervious Area |  |


| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C21: CB \#21

Runoff $=\quad 0.99$ cfs @ 12.09 hrs, Volume= 3,251 cf, Depth> 3.83"
Routed to Pond CB21 : CB \#21
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,319 | 39 > | >75\% Grass cover, Good, HSG A Paved parking, HSG A Paved parking, HSG C |  |  |
|  | 7,301 | 98 P |  |  |  |
|  | 1,554 | 98 P |  |  |  |
|  | 10,174 | 90 | Weighted Average <br> 12.96\% Pervious Area <br> 87.04\% Impervious Area |  |  |
|  | 1,319 |  |  |  |  |
|  | 8,855 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |

## Summary for Subcatchment C22: CB \#22

Runoff $=\quad 1.28$ cfs @ 12.09 hrs, Volume $=\quad 4,490$ cf, Depth> 4.49"
Routed to Pond CB22 : CB \#22
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| :---: | :---: | :---: |
| 2,946 | 98 | Paved parking, HSG A |
| 177 | 74 | >75\% Grass cover, Good, HSG C |
| 2,641 | 98 | Paved parking, HSG C |
| 829 | 80 | >75\% Grass cover, Good, HSG D |
| 5,408 | 98 | Paved parking, HSG D |
| 12,001 | 96 | Weighted Average |
| 1,006 |  | 8.38\% Pervious Area |
| 10,995 |  | 91.62\% Impervious Area |
| Tc Length (min) (feet) | Slope (ft/ft) | Velocity Capacity Description (ft/sec) (cfs) |

## Summary for Subcatchment C23: CB \#23

Runoff $=0.93$ cfs @ 12.09 hrs, Volume=
3,014 cf, Depth> 3.73"
Routed to Pond CB23 : CB \#23
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG A |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 242 | 39 > |  |  |  |
|  | 3,016 | 98 P | Paved parking, HSG A |  |  |
|  | 1,267 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 218 | 98 P | Paved parking, HSG C |  |  |
|  | 2,272 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 2,679 | 98 P | Paved parking, HSG D |  |  |
|  | 9,694 | 89 | Weighted Average |  |  |
|  | 3,781 |  | 39.00\% Pervious Area |  |  |
|  | 5,913 |  | 61.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{r} \text { Length } \\ \text { (feet) } \end{array}$ | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C24: CB \#24

Runoff $=0.82$ cfs @ 12.09 hrs , Volume=
2,746 cf, Depth> 4.16"
Routed to Pond CB24 : CB \#24
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


## Summary for Subcatchment C25: CB \#25

Runoff $=\quad 0.89$ cfs @ 12.09 hrs, Volume= 3,017 cf, Depth> 4.27"
Routed to Pond CB25 : CB \#25
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


## Summary for Subcatchment C26: CB \#26

Runoff = 0.88 cfs @ 12.09 hrs, Volume= 2,901 cf, Depth> 3.94"

Routed to Pond CB26 : CB \#26
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,203 | 80 | >75\% Grass cover, Good, HSG D |
| 5,632 | 98 | Paved parking, HSG D |

## Summary for Subcatchment C27: CB \#27

Runoff $=\quad 0.65$ cfs @ 12.09 hrs, Volume= 2,287 cf, Depth> 4.49"
Routed to Pond CB27 : CB \#27
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 98 | 39 | $>75 \%$ Grass cover, Good, HSG A |
| 131 | 98 | Paved parking, HSG A |
| 397 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 5,485 | 98 | Paved parking, HSG D |
| 6,111 | 96 | Weighted Average |
| 495 |  | 8.10\% Pervious Area |
| 5,616 |  | $91.90 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ |
| ---: | ---: | ---: | ---: | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description | Direct Entry, |
| :--- |

## Summary for Subcatchment C28: CB \#28

Runoff $=\quad 0.97$ cfs @ 12.09 hrs, Volume= 3,136 cf, Depth> 3.63"
Routed to Pond CB28 : CB \#28
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,751 | $74>$ |  |  |  |
|  | 2,841 | 98 P | Paved parking, HSG C |  |  |
|  | 2,297 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 2,483 | 98 P | Paved parking, HSG D |  |  |
|  | 10,372 | 88 | Weighted Average |  |  |
|  | 5,048 |  | 48.67\% Pervious Area |  |  |
|  | 5,324 |  | 51.33\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C29: CB \#29

Runoff $=\quad 0.89$ cfs @ 12.09 hrs, Volume= $\quad 3,020 \mathrm{cf}$, Depth> 4.27"
Routed to Pond CB29: CB \#29
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"

| $\begin{array}{r} \text { Area (sf) } \\ \hline 1,341 \end{array}$ |  | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 74 |  |  |  |
|  | 5,330 | 98 P | >75\% Grass cover, Good, HSG C Paved parking, HSG C |  |  |
|  | 1,824 | 98 R | Roofs, HSG |  |  |
|  | 8,495 | 94 | Weighted | verage |  |
|  | 1,341 |  | 15.79\% Pe | vious Area |  |
|  | 7,154 |  | 84.21\% Im | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |

## Summary for Subcatchment C30: CB \#30

Runoff $=\quad 0.93$ cfs @ 12.09 hrs, Volume= 3,175 cf, Depth> 4.27"

Routed to Pond CB30 : CB \#30
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,572 | 74 |  |  |  |
|  | 6,310 | 98 P | Paved parking, HSG C |  |  |
|  | 1,051 | 98 | Roofs, HSG C |  |  |
|  | 8,933 | 94 | Weighted Average |  |  |
|  | 1,572 |  | 17.60\% Pervious Area |  |  |
|  | 7,361 |  | 82.40\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C31: CB \#31

Runoff $=\quad 1.59$ cfs @ 12.09 hrs, Volume= 5,230 cf, Depth> 3.83"
Routed to Pond CB31 : CB \#31
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 5,132 | 74 | >75\% Grass cover, Good, HSG C |  |  |
|  | 9,132 | 98 P | Paved parking, HSG C |  |  |
|  | 2,101 | 98 R | Roofs, HSG C |  |  |
|  | 16,365 | 90 | Weighted Average |  |  |
|  | 5,132 |  | 31.36\% Pervious Area |  |  |
|  | 11,233 |  | 68.64\% Imp | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ftft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment C32: CB \#32

Runoff $=\quad 1.26$ cfs @ 12.09 hrs, Volume $=\quad 4,174$ cf, Depth> 3.94"
Routed to Pond CB32 : CB \#32
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) |  | CN D | $>75 \%$ Grass cover, Good, HSG CPaved parking, HSG CRoofs, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\begin{aligned} & \hline 74 \\ & 98 \\ & 98 \\ & \hline \end{aligned}$ |  |  |  |
| 3,753 |  |  |  |  |  |
| 1,889 |  |  |  |  |  |
|  | 12,710 | 91 | Weighted Average |  |  |
|  | 3,753 |  | 29.53\% Pervious Area |  |  |
|  | 8,957 |  | 70.47\% Imp | ervious Area |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C33: CB \#33

Runoff $=\quad 0.57$ cfs @ 12.09 hrs, Volume= 1,927 cf, Depth> 4.27"
Routed to Pond CB33 : CB \#33
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 873 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 3,693 | 98 | Paved parking, HSG C |
| 855 | 98 | Roofs, HSG C |

6.0

Direct Entry,

## Summary for Subcatchment C34: CB \#34

Runoff $=\quad 0.89$ cfs @ 12.09 hrs, Volume= 2,986 cf, Depth> 4.16"
Routed to Pond CB34 : CB \#34
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,680 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 5,115 | 98 | Paved parking, HSG C |
| 1,827 | 98 | Roofs, HSG C |
| 8,622 | 93 | Weighted Average |
| 1,680 |  | $19.49 \%$ Pervious Area |
| 6,942 |  | $80.51 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- |

## Summary for Subcatchment C35: CB \#35

Runoff $=\quad 0.45 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume= $\quad 1,632 \mathrm{cf}$, Depth> 4.72"
Routed to Pond CB35 : CB \#35
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,578 | 98 P | Paved parking, HSG C |  |  |
|  | 79 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 492 | 98 P | Paved parking, HSG D |  |  |
|  | 4,149 | $98 \quad 1$ | Weighted Average |  |  |
|  | 79 |  | 1.90\% Pervious Area |  |  |
|  | 4,070 |  | 98.10\% Im | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C36: CB \#36

Runoff =
0.72 cfs @ 12.09 hrs , Volume=
2,605 cf, Depth> 4.72"

Routed to Pond CB36 : CB \#36
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | ea (sf) | CN Description |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 6,622 | 98 | aved park | ng, HSG C |  |
| 6,622 |  | 100.00\% Impervious Area |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity <br> (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

Summary for Subcatchment C38: CB \#38
Runoff $=\quad 0.83$ cfs @ 12.09 hrs, Volume $=3,004 \mathrm{cf}$, Depth> 4.72" Routed to Pond CB38 : CB \#38

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


Direct Entry,

## Summary for Subcatchment C40: CB \#40

Runoff $=\quad 0.46$ cfs @ 12.09 hrs, Volume= 1,656 cf, Depth> 4.72"
Routed to Pond CB40 : CB \#40
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


## Summary for Subcatchment C41: CB \#41

Runoff $=\quad 0.61$ cfs @ 12.09 hrs, Volume= 2,197 cf, Depth> 4.72"

Routed to Pond CB41 : CB \#41
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


## Summary for Subcatchment C43: CB \#43

Runoff $=\quad 0.30$ cfs @ 12.09 hrs, Volume= 967 cf, Depth> 3.73"
Routed to Pond CB43 : CB \#43
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,343 | 98 P | Paved parking, HSG B $>75 \%$ Grass cover, Good, HSG B |  |  |
|  | 766 | $61>$ |  |  |  |
|  | 3,109 | 89 W | Weighted Average |  |  |
|  | 766 |  | 24.64\% Pervious Area |  |  |
|  | 2,343 |  | 75.36\% Impervious Area |  |  |
| Tc <br> (min) | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

Summary for Subcatchment C44: CB \#44
Runoff = 0.20 cfs @ 12.09 hrs, Volume= 667 cf, Depth> 4.05"
Routed to Pond CB44 : CB \#44
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,670 | 98 | Paved parking, HSG B |
| 308 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 1,978 | 92 | Weighted Average |
| 308 |  | $15.57 \%$ Pervious Area |
| 1,670 |  | $84.43 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ |
| ---: | ---: | ---: | ---: |
| 6.0 |  | Capacity <br> $(\mathrm{cfs})$ | Description |
|  |  | Direct Entry, |  |
|  |  | Summary for Subcatchment C45: CB \#45 |  |

Runoff $=\quad 0.19$ cfs @ 12.09 hrs, Volume= 587 cf, Depth> 2.86"
Routed to Pond CB45 : CB \#45
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,240 | 98 | Paved parking, HSG B |
| 1,225 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 2,465 | 80 | Weighted Average |
| 1,225 |  | $49.70 \%$ Pervious Area |
| 1,240 | $50.30 \%$ Impervious Area |  |
| Tc | Length | Slope  <br> (fint/ft Velocity <br> (ft/sec) Capacity <br> (min) (ffs) |

Direct Entry,
Summary for Subcatchment C46: CB \#46
Runoff $=\quad 0.33$ cfs @ 12.09 hrs, Volume= 1,046 cf, Depth> 2.86"
Routed to Pond CB46 : CB \#46
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,241 | 98 P | Paved parking, HSG B |  |  |
|  | 2,156 | $61>$ | >75\% Grass cover, Good, HSG B |  |  |
|  | 4,397 | 80 | Weighted Average |  |  |
|  | 2,156 |  | 49.03\% Pervious Area |  |  |
|  | 2,241 |  | 50.97\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

Summary for Subcatchment C47: CB \#47
Runoff $=\quad 0.33$ cfs @ 12.09 hrs, Volume= $\quad 1,185 \mathrm{cf}$, Depth> 4.72"
Routed to Pond CB47 : CB\#47
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) CN Description |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\frac{3,012}{3,012}$ |  | 98 Paved roads w/curbs \& sewers, HSG B |  |  |  |
|  |  |  | 00.00\% Im | pervious A |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{aligned} & \text { Length } \\ & \text { (feet) } \end{aligned}$ | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |

## Summary for Subcatchment C48: CB \#48

Runoff $=\quad 2.61$ cfs @ 12.17 hrs, Volume= $\quad 10,030$ cf, Depth> 2.00"
Routed to Pond CB48 : CB\#48
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) |  | CN | Description |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{array}{r} 3,877 \\ 56,251 \\ \hline \end{array}$ |  | Paved roads w/curbs \& sewers, HSG B 1 acre lots, $20 \% \mathrm{imp}$, HSG B |  |  |  |
|  | $\begin{aligned} & 60,128 \\ & 45,001 \\ & 15,127 \end{aligned}$ | 70 | Weighted Average <br> 74.84\% Pervious Area <br> 25.16\% Impervious Area |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | Capacity (cfs) | Description |  |
| 7.0 | 50 | 0.0800 | 0.12 |  | Sheet Flow, <br> Woods: Light underbrush n=0.400 | P2=3.27" |
| 4.8 | 350 | 0.0600 | 1.22 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |  |

11.8400 Total

Summary for Subcatchment C49: CB \#49
Runoff $=\quad 0.55$ cfs @ 12.09 hrs, Volume= 1,862 cf, Depth> 4.27"
Routed to Pond CB49 : CB \#49
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 4,431 | 98 | Paved roads w/curbs \& sewers, HSG C |
| 807 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 5,238 | 94 | Weighted Average |
| 807 |  | 15.41\% Pervious Area |
| 4,431 |  | $84.59 \%$ Impervious Area |



Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| $\begin{array}{r} \text { Area }(\mathrm{sf}) \\ \hline 3,429 \end{array}$ |  | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 74 > | $>75 \%$ Grass cover, Good, HSG C Paved roads w/curbs \& sewers, HSG C |  |  |
|  | 11,611 | 98 P |  |  |  |
|  | 15,040 | 93 V | Weighted A | verage |  |
|  | 3,429 |  | 22.80\% Per | vious Area |  |
|  | 11,611 |  | 77.20\% Imp | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{gathered} \text { Capacity } \\ \text { (cfs) } \end{gathered}$ | Description |

Direct Entry,
Summary for Subcatchment C51: CB \#51
Runoff $=\quad 0.74$ cfs @ 12.09 hrs, Volume= $\quad 2,684$ cf, Depth> 4.72"
Routed to Pond CB51 : CB \#51
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,147 | 98 R | Roofs, HSG C |  |  |
|  | 3,676 | 98 P | Paved parking, HSG C |  |  |
|  | 6,823 | 98 | Weighted Average |  |  |
|  | 6,823 |  | 100.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

Summary for Subcatchment C52: CB\#52
Runoff $=\quad 0.96$ cfs @ 12.09 hrs, Volume $=3,302 \mathrm{cf}$, Depth> 4.38"
Routed to Pond CB52 : CB \#52
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"
$\left.\begin{array}{rrl}\text { Area (sf) } & \text { CN } & \text { Description } \\ \begin{array}{rl}1,164 & 74\end{array} & \begin{array}{l}>75 \% \text { Grass cover, Good, HSG C } \\ 7,888\end{array} & 98 \\ \text { Paved parking, HSG C }\end{array}\right]$

## Summary for Subcatchment C53: CB \#53

Runoff $=\quad 0.81$ cfs @ 12.09 hrs, Volume= 2,723 cf, Depth> 4.16"

Routed to Pond CB53 : CB \#53
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 287 | 39 > | >75\% Grass cover, Good, HSG A |  |  |
|  | 3,287 | 98 P | Paved parking, HSG A |  |  |
|  | 773 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 3,516 | 98 P | Paved parking, HSG C |  |  |
|  | 7,863 | 93 V | Weighted Average |  |  |
|  | 1,060 |  | 13.48\% Pervious Area |  |  |
|  | 6,803 |  | 86.52\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ftft) | Velocity (ft/sec) | Capacity (cfs) | Description |

6.0

Direct Entry,

## Summary for Subcatchment C54: CB \#54

Runoff $=\quad 0.48$ cfs @ 12.09 hrs, Volume= $1,583 \mathrm{cf}$, Depth> 3.94"
Routed to Pond CB54 : CB \#54
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 550 | 39 | $>75 \%$ Grass cover, Good, HSG A |
| 4,176 | 98 | Paved parking, HSG A |
| 84 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 11 | 98 | Paved parking, HSG C |
| 4,821 | 91 | Weighted Average |
| 634 |  | $13.15 \%$ Pervious Area |
| 4,187 |  | $86.85 \%$ Impervious Area |



## Summary for Subcatchment C8: CB \#8

Runoff $=\quad 0.57$ cfs @ 12.09 hrs, Volume $=\quad 1,937$ cf, Depth> 4.27"
Routed to Pond CB8 : CB\#8
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


## Summary for Subcatchment C9: CB \#9

Runoff $=\quad 1.76$ cfs @ 12.09 hrs, Volume= $\quad 6,257$ cf, Depth> 4.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


## Summary for Subcatchment CH1: CLUBHOUSE

Runoff $=0.67$ cfs @ 12.09 hrs , Volume=
2,343 cf, Depth> 4.49"
Routed to Pond DECH : DRIP \#CH
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ |
| ---: | ---: | | Slope |
| ---: |
| $(\mathrm{ft} / \mathrm{ft})$ | | Velocity |
| ---: |
| $(\mathrm{ft} / \mathrm{sec})$ | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description | Direct Entry, |
| :--- |

## Summary for Subcatchment S201: SUMMER STREET ACCESS APRON

Runoff $=\quad 1.05$ cfs @ 12.09 hrs, Volume $=\quad 3,627 \mathrm{cf}$, Depth> 4.38"
Routed to Link AP1 : ANALYSIS POINT 1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 717 | 61 | >75\% Grass cover, Good, HSG B |
| 9,226 | 98 | Paved parking, HSG B |

6.0

Direct Entry,

## Summary for Subcatchment S202: EXISTING WETLAND

Runoff $=\quad 18.32$ cfs @ 12.33 hrs, Volume $=89,722$ cf, Depth> 2.49"
Routed to Reach SC1 : Stream Crossing \#1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 136,496 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 83,935 | 55 | Woods, Good, HSG B |
| 29 | 98 | Paved parking, HSG B |
| 13,946 | 98 | Roofs, HSG B |
| 9,038 | 48 | Brush, Good, HSG B |
| 2,573 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 17,121 | 70 | Woods, Good, HSG C |
| 98 | 98 | Paved parking, HSG C |
| 1,097 | 65 | Brush, Good, HSG C |
| 126 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 132 | 98 | Paved parking, HSG D |
| 167,678 | 98 | Water Surface, HSG D |
| 432,269 | 76 | Weighted Average |
| 250,386 |  | 57.92\% Pervious Area |
| 181,883 |  | $42.08 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ | Description |
| ---: | ---: | ---: | ---: | :--- | :--- |
| 5.6 | 50 | 0.0200 | 0.15 | Sheet Flow, <br> Grass: Short $\mathrm{n}=0.150 \quad \mathrm{P} 2=3.27 "$ <br> 1.4 | 118 |
| 0.0400 | 1.40 | Shallow Concentrated Flow, <br> Short Grass Pasture Kv=7.0 fps <br> Shallow Concentrated Flow, <br> Woodland Kv=5.0 fps |  |  |  |
| 16.2 | 688 | 0.0200 | 0.71 |  |  |

## Summary for Subcatchment S203: POCKET WETLAND \#1

Runoff $=\quad 1.45$ cfs @ 12.10 hrs, Volume= 4,616 cf, Depth> 2.16"

Routed to Pond p210: POCKET WETLAND \#1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 12,682 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 1,060 | 98 | Water Surface, 0\% imp, HSG B |
| 7,785 | 74 | >75\% Grass cover, Good, HSG C |
| 4,060 | 98 | Water Surface, 0\% imp, HSG C |

## Summary for Subcatchment S204: EXISTING WETLANDS

Runoff $=12.75$ cfs @ 12.32 hrs, Volume $=61,782$ cf, Depth $>2.41^{\prime \prime}$
Routed to Link ap2 : ANALYSIS POINT 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 53,739 | 61 | >75\% Grass cover, Good, HSG B |
| 17,975 | 55 | Woods, Good, HSG B |
| 20,940 | 48 | Brush, Good, HSG B |
| 41,421 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 68,342 | 70 | Woods, Good, HSG C |
| 116 | 98 | Paved parking, HSG C |
| 1,904 | 65 | Brush, Good, HSG C |
| 1,528 | 65 | Brush, Good, HSG C |
| 2,508 | 77 | Woods, Good, HSG D |
| 161 | 98 | Paved parking, HSG D |
| 4,073 | 73 | Brush, Good, HSG D |
| 95,496 | 98 | Water Surface, HSG D |

22.6632 Total

## Summary for Subcatchment S205: ISOLATED WETLAND

Runoff $=\quad 2.89$ cfs @ 12.10 hrs , Volume=
9,258 cf, Depth> 2.00"
Routed to Link AP3 : ANALYSIS POINT 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 10,910 | 30 | Woods, Good, HSG A |
| 3,684 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 2,275 | 70 | Woods, Good, HSG C |
| 171 | 98 | Paved parking, HSG C |
| 1,706 | 65 | Brush, Good, HSG C |
| 1,940 | 80 | >75\% Grass cover, Good, HSG D |
| 23,513 | 77 | Woods, Good, HSG D |
| 393 | 98 | Paved parking, HSG D |
| 2,208 | 73 | Brush, Good, HSG D |
| 8,620 | 98 | Water Surface, HSG D |
| 55,420 | 70 | Weighted Average |
| 46,236 |  | $83.43 \%$ Pervious Area |
| 9,184 |  | $16.57 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- | Description | Direct Entry, |
| :--- |
| 6.0 |
|  |

Runoff $=19.55$ cfs @ 12.52 hrs, Volume= 119,746 cf, Depth> 1.61" Routed to Link AP4 : ANALYSIS POINT \#4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


## Summary for Subcatchment S207: INFILTRATION POND \#2

Runoff $=\quad 1.94$ cfs @ 12.09 hrs, Volume= 6,290 cf, Depth> 3.63"

Routed to Pond P207 : INFILTRATION POND \#2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | Water Surface, 0\% imp, HSG A |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 839 | 98 |  |  |  |
|  | 8,802 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 11,162 | 98 | Water Surf | ce, 0\% imp | , HSG C |
|  | $\begin{aligned} & 20,803 \\ & 20,803 \end{aligned}$ | 88 | Weighted Average 100.00\% Pervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment S208: GRASS AREA

Runoff $=\quad 0.88$ cfs @ 12.09 hrs, Volume $=\quad 2,769 \mathrm{cf}$, Depth> 2.41"
Routed to Pond OCS4 : OCS\#4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN D | Description |
| :---: | :---: | :---: |
| 476 | $39>$ | >75\% Grass cover, Good, HSG A |
| 12,000 | $74>$ | >75\% Grass cover, Good, HSG C |
| 168 | 98 P | Paved parking, HSG A |
| 1,116 | 98 P | Paved parking, HSG C |
| 13,760 | 75 | Weighted Average |
| 12,476 |  | 90.67\% Pervious Area |
| 1,284 |  | 9.33\% Impervious Area |
| Tc Length (min) (feet) | Slope (ft/ft) | Velocity Capacity Description (ft/sec) (cfs) |

6.0

## Direct Entry,

## Summary for Subcatchment S209: WETLAND C

Runoff $=\quad 3.68$ cfs @ 12.42 hrs, Volume= 19,940 cf, Depth> 2.23"
Routed to Reach 11R : 4x4 Open Bottom Culvert
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


## Summary for Subcatchment S210: INFILTRATION POND \#1

Runoff $=\quad 3.96$ cfs @ 12.23 hrs, Volume $=\quad 16,882 \mathrm{cf}$, Depth> 2.67"
Routed to Pond P212 : INFILTRATION POND \#1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"
$\left.\begin{array}{rrll}\text { Area (sf) } & \text { CN } & \text { Description } \\ 13,844 & 98 & \text { Water Surface, 0\% imp, HSG C } \\ 59,814 & 74 & \begin{array}{l}>75 \% \\ 2,232\end{array} & 65 \\ \text { Brush, Good, HSG C C }\end{array}\right]$

## Summary for Subcatchment S211: S211

Runoff $=1.13$ cfs @ 12.09 hrs, Volume= 3,556 cf, Depth> 2.76"

Routed to Pond P205 : INFILTRATION POND \#3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


### 23.1418 Total

## Summary for Subcatchment S213: COURTYARD

Runoff $=\quad 1.56 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume $=$
Routed to Pond 11P :YARD DRAIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 5,047 | 39 | $>75 \%$ Grass cover, Good, HSG A |
| 1,678 | 98 | Paved parking, HSG A |
| 168 | 98 | Roofs, HSG A |
| 532 | 98 | Water Surface, 0\% imp, HSG A |
| 4,518 | 74 | >75\% Grass cover, Good, HSG C |
| 7,080 | 98 | Paved parking, HSG C |
| 878 | 98 | Roofs, HSG C |
| 718 | 98 | Water Surface, 0\% imp, HSG C |
| 296 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 492 | 98 | Paved parking, HSG D |
| 21,407 | 79 | Weighted Average |
| 11,111 |  | $51.90 \%$ Pervious Area |
| 10,296 |  | $48.10 \%$ Impervious Area |
| Tc | Length | Slope |
| (feet) | Velocity |  |
| (ft/ft) | Cft/sec) | Capacity |
| (cfs) |  |  |

## Summary for Subcatchment T1: Trench Drain 1

Runoff $=\quad 1.15 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume $=\quad 3,869 \mathrm{cf}$, Depth> 4.16"
Routed to Pond 5R : TRENCH DRAIN
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN | Description $>75 \%$ Grass cover, Good, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,443 | 74 |  |  |  |
|  | 4,228 | 98 | Paved parking, HSG C |  |  |
|  | 1,339 | 80 | >75\% Grass cover, Good, HSG D |  |  |
|  | 4,163 | 98 | Paved parking, HSG D |  |  |
|  | 11,173 | 93 | Weighted Average |  |  |
|  | 2,782 |  | 24.90\% Pervious Area |  |  |
|  | 8,391 |  | 75.10\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | $\begin{array}{rr} c & \text { Length } \\ \text { 1) } & \text { (feet) } \\ \hline \end{array}$ | Slope $(\mathrm{ft} \mid \mathrm{ft})$ | $\begin{array}{rr} \text { e } \begin{array}{r} \text { Velocity } \\ \text { t) } \\ (\mathrm{ft} / \mathrm{sec}) \end{array} \\ \hline \end{array}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment T2: Drive Under B2

Runoff $=\quad 0.31$ cfs @ 12.09 hrs, Volume= 991 cf, Depth> 2.68"
Routed to Reach 11R : 4x4 Open Bottom Culvert
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,936 | 98 | Roofs, HSG C$>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 311 | 74 |  |  |  |
|  | 4,247 | 96 | Weighted Average |  |  |
|  | 311 |  | 7.32\% Pervious Area |  |  |
|  | 3,936 |  | 92.68\% Imp | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{r} \text { Length } \\ \text { (feet) } \end{array}$ | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment TH10: TOWN HOUSE \#10

Runoff $=\quad 0.37$ cfs @ 12.09 hrs, Volume= $\quad 1,301$ cf, Depth> 4.49"
Routed to Pond DE70 : DRIP \#70
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN | Description | Roofs, HSG C |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,184 | 98 | Roofs, HSG C <br> $>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 292 | 74 |  |  |  |  |
|  | 3,476 | 96 | Weighted Average |  |  |
|  | 292 |  | 8.40\% Pervious Area |  |  |
|  | 3,184 |  | 91.60\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |

6.0 Direct Entry,

## Summary for Subcatchment TH11: TOWN HOUSE \#11

Runoff $=\quad 0.45$ cfs @ 12.09 hrs, Volume= $\quad 1,575 \mathrm{cf}$, Depth> 4.49"
Routed to Pond DE71 : DRIP \#71
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,899 | 98 R | Roofs, HSG C$>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 311 | $74>$ |  |  |  |
|  | 4,210 | 96 | Weighted Average |  |  |
|  | 311 |  | 7.39\% Pervious Area |  |  |
|  | 3,899 |  | 92.61\% Imp | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ (\mathrm{cfs}) \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment TH2: TOWN HOUSE \#2

Runoff $=\quad 0.45$ cfs @ 12.09 hrs, Volume= 1,589 cf, Depth> 4.49"
Routed to Pond DE62 : DRIP \#62
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,936 | 98 R | Roofs, HSG C |  |  |
|  | 311 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 4,247 | 96 V | Weighted Average |  |  |
|  | 311 |  | 7.32\% Pervious Area |  |  |
|  | 3,936 |  | 92.68\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH3: TOWN HOUSE \#3

Runoff = 0.32 cfs @ 12.09 hrs, Volume= 1,099 cf, Depth> 4.38"
Routed to Pond DE63 : DRIP \#63
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN | Rescript, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,672 | 98 |  |  |  |
|  | 341 | 74 | >75\% Gras | s cover, God | od, HSG C |
|  | 3,013 | $95 \quad 1$ | Weighted Average |  |  |
|  | 341 |  | 11.32\% Pervious Area |  |  |
|  | 2,672 |  | 88.68\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity <br> (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH4: TOWN HOUSE \#4

Runoff $=\quad 0.37$ cfs @ 12.09 hrs, Volume= $\quad 1,298$ cf, Depth> 4.49"
Routed to Pond DE64 : DRIP \#64
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,178 | 98 | Roofs, HSG C |
| 292 | 74 | >75\% Grass cover, Good, HSG C |
| 3,470 | 96 | Weighted Average |
| 292 |  | $8.41 \%$ Pervious Area |
| 3,178 |  | $91.59 \%$ Impervious Area |
| Tc | Length | Slope  <br> (ft/ft) Velocity <br> (ft/sec) Capacity <br> (min) (cfs) |

6.0

Direct Entry,

## Summary for Subcatchment TH5: TOWN HOUSE \#5

Runoff $=\quad 0.32$ cfs @ 12.09 hrs, Volume $=\quad 1,100 \mathrm{cf}$, Depth> 4.38" Routed to Pond DE65 : DRIP \#65

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"


## Summary for Subcatchment TH6: TOWN HOUSE \#6

Runoff $=\quad 0.36$ cfs @ 12.09 hrs, Volume= $\quad 1,275$ cf, Depth> 4.49"
Routed to Pond DE66 : DRIP \#66
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,116 | 98 R | Roofs, HSG C$>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 291 | $74>$ |  |  |  |
|  | 3,407 | 96 | Weighted Average |  |  |
|  | 291 |  | 8.54\% Pervious Area |  |  |
|  | 3,116 |  | 91.46\% Imp | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ (\mathrm{cfs}) \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment TH7: TOWN HOUSE \#7

Runoff $=\quad 0.37$ cfs @ 12.09 hrs, Volume= 1,302 cf, Depth> 4.49"
Routed to Pond DE67 : DRIP \#67
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,189 | 98 R | Roofs, HSG C |  |  |
|  | 292 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 3,481 | 96 | Weighted Average |  |  |
|  | 292 |  | 8.39\% Pervious Area |  |  |
|  | 3,189 |  | 91.61\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH8: TOWN HOUSE \#8

Runoff $=\quad 0.45$ cfs @ 12.09 hrs, Volume $=\quad 1,576$ cf, Depth> 4.49"

Routed to Pond DE68 : DRIP \#68
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN | Rescript, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,901 | $\begin{aligned} & 98 \\ & 74 \\ & \hline \end{aligned}$ |  |  |  |
|  | 311 |  | Roofs, HSG C$>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 4,212 | 96 | Weighted Average |  |  |
|  | 311 |  | 7.38\% Pervious Area |  |  |
|  | 3,901 |  | 92.62\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity <br> (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH9: TOWN HOUSE \#9

Runoff $=\quad 0.37$ cfs @ 12.09 hrs, Volume= $\quad 1,302$ cf, Depth> 4.49"
Routed to Pond DE69 : DRIP \#69
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 10YR Rainfall=4.96"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,188 | 98 R | Roofs, HSG C |  |  |
|  | 292 | $74>$ | >75\% Gras | s cover, G | od, HSG C |
|  | 3,480 | 96 | Weighted Average |  |  |
|  | 292 |  | 8.39\% Pervious Area |  |  |
|  | 3,188 |  | 91.61\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |

## Summary for Reach 8R: OVERLAND FLOW

Inflow Area = $\quad 11,975$ sf, $92.37 \%$ Impervious, Inflow Depth > 3.57" for $10 Y R$ event Inflow $=1.10$ cfs @ 12.14 hrs, Volume= $3,558 \mathrm{cf}$ Outflow $=0.26 \mathrm{cfs}$ @ 12.59 hrs , Volume $=\quad 3,276 \mathrm{cf}$, Atten= $76 \%$, Lag= 27.1 min Routed to Link AP4 : ANALYSIS POINT \#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.08 \mathrm{fps}$, Min. Travel Time $=110.7 \mathrm{~min}$
Avg. Velocity $=0.04 \mathrm{fps}$, Avg. Travel Time $=214.4 \mathrm{~min}$

Peak Storage $=1,755$ cf @ 12.59 hrs
Average Depth at Peak Storage= 0.06 ' , Surface Width= 50.62'
Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 28.09 cfs
$50.00^{\prime} \times 1.00^{\prime}$ deep channel, $n=0.400$
Side Slope Z-value= 5.0 '/' Top Width=60.00'
Length $=563.0^{\prime}$ Slope $=0.0213$ '/'
Inlet Invert= 208.00', Outlet Invert= 196.00'


## Summary for Reach 9R: OVERLAND FLOW

Inflow Area $=\quad 32,665$ sf, $94.81 \%$ Impervious, Inflow Depth $=0.00$ " for 10 YR event
Inflow = 0.00 cfs @ 0.00 hrs, Volume=

0 cf
Outflow = $0.00 \mathrm{cfs} @ 0.00 \mathrm{hrs}$, Volume= 0 cf , Atten= 0\%, Lag= 0.0 min
Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.00 \mathrm{fps}, \mathrm{Min}$. Travel Time $=0.0 \mathrm{~min}$
Avg. Velocity $=0.00 \mathrm{fps}$, Avg. Travel Time $=0.0 \mathrm{~min}$
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage=0.00'
Bank-Full Depth $=1.00$ Flow Area $=30.0$ sf, Capacity $=23.45$ cfs
20.00' x 1.00' deep channel, $n=0.400$

Side Slope Z-value= 10.0 '/' Top Width= 40.00'
Length= 211.0' Slope= 0.0652 '/'
Inlet Invert= 201.75', Outlet Invert= 188.00'


## Summary for Reach 10R: OVERLAND FLOW

Inflow Area = 129,716 sf, 63.13\% Impervious, Inflow Depth $=0.54$ " for $10 Y R$ event Inflow $=0.96$ cfs @ 12.60 hrs , Volume $=\quad 5,876 \mathrm{cf}$ Outflow $=\quad 0.92$ cfs @ 12.86 hrs, Volume $=5,876 \mathrm{cf}$, Atten= $4 \%$, Lag= 15.2 min

Routed to Link AP4 : ANALYSIS POINT \#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.22 \mathrm{fps}, \mathrm{Min}$. Travel Time $=12.3 \mathrm{~min}$
Avg. Velocity $=0.07 \mathrm{fps}$, Avg. Travel Time $=38.0 \mathrm{~min}$
Peak Storage= 680 cf @ 12.86 hrs
Average Depth at Peak Storage=0.19' , Surface Width= 23.79'
Bank-Full Depth=1.00' Flow Area= 30.0 sf, Capacity= 17.57 cfs
20.00' x 1.00' deep channel, $n=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 40.00'
Length= 164.0' Slope= 0.0366 '/'
Inlet Invert= 192.00', Outlet Invert= 186.00'


## Summary for Reach 11R: 4x4 Open Bottom Culvert

[52] Hint: Inlet/Outlet conditions not evaluated
Inflow Area = 424,818 sf, $45.99 \%$ Impervious, Inflow Depth > 1.13" for 10YR event
Inflow $=\quad 4.11 \mathrm{cfs}$ @ 12.47 hrs , Volume $=\quad 40,067 \mathrm{cf}$
Outflow $=\quad 4.11 \mathrm{cfs} @ 12.47 \mathrm{hrs}$, Volume $=\quad 40,059 \mathrm{cf}$, Atten= $0 \%$, Lag= 0.2 min
Routed to Reach 23R : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity= 1.62 fps , Min. Travel Time $=0.3 \mathrm{~min}$
Avg. Velocity $=0.72 \mathrm{fps}$, Avg. Travel Time $=0.7 \mathrm{~min}$
Peak Storage= 76 cf @ 12.47 hrs
Average Depth at Peak Storage= 0.63' , Surface Width= 4.00'
Bank-Full Depth= 4.00' Flow Area= 16.0 sf, Capacity= 42.20 cfs
48.0" W x 48.0" H Box Pipe
$\mathrm{n}=0.069$ Riprap, 6 -inch
Length=30.0' Slope= 0.0150 '/'
Inlet Invert= 194.00', Outlet Invert= 193.55'


## Summary for Reach 12R: OVERLAND FLOW

| Inflow Area = | 12,906 | 90.20\% Impervious | Inflow Depth > 3.61" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.23 cfs @ | 12.13 hrs , Volume= | 3,881 cf |
| Outflow | 0.61 cfs @ | 12.33 hrs , Volume= | 3,787 cf, Atten= 51\%, Lag= 12.2 min |

Routed to Link AP2 : ANALYSIS POINT 2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity= $0.12 \mathrm{fps}, \mathrm{Min}$. Travel Time $=34.1 \mathrm{~min}$
Avg. Velocity $=0.04 \mathrm{fps}$, Avg. Travel Time $=93.2 \mathrm{~min}$
Peak Storage $=1,246$ cf @ 12.33 hrs
Average Depth at Peak Storage=0.10' , Surface Width= 50.99'
Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 29.80 cfs
$50.00^{\prime} \times 1.00^{\prime}$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length $=250.0^{\prime}$ Slope $=0.0240$ '/'
Inlet Invert= 202.00', Outlet Invert= 196.00'


## Summary for Reach 14R: OVERLAND FLOW

| Inflow A | 52,768 | 0.60\% Impervious, | 1.62" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.38 cfs @ | 12.35 hrs , Volume= | 7,111 cf |
| Outflow | 0.31 cfs @ | 13.19 hrs , Volume= | $6,155 \mathrm{cf}$, Atten= 77\%, Lag= 49.9 min |

Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.09 \mathrm{fps}$, Min. Travel Time $=150.5 \mathrm{~min}$
Avg. Velocity $=0.06 \mathrm{fps}$, Avg. Travel Time $=226.9 \mathrm{~min}$
Peak Storage= 2,832 cf @ 13.19 hrs
Average Depth at Peak Storage=0.07' , Surface Width= 51.31'
Bank-Full Depth= 1.00 Flow Area $=60.0$ sf, Capacity $=31.55$ cfs
50.00' x 1.00' deep channel, n= 0.400 Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 70.00'
Length=852.0' Slope= 0.0246 '/'
Inlet Invert= 207.00', Outlet Invert= 186.00'


## Summary for Reach 15R: OVERLAND FLOW



Routed to Link AP2 : ANALYSIS POINT 2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.07 \mathrm{fps}, \mathrm{Min}$. Travel Time $=68.4 \mathrm{~min}$
Avg. Velocity $=0.07 \mathrm{fps}$, Avg. Travel Time $=74.8 \mathrm{~min}$
Peak Storage= 783 cf @ 16.81 hrs
Average Depth at Peak Storage $=0.05^{\prime}$, Surface Width= 50.52'
Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 27.21 cfs
50.00 x 1.00' deep channel, $\mathrm{n}=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length=300.0' Slope= 0.0200 '/'
Inlet Invert= 202.00', Outlet Invert= 196.00'


Summary for Reach 18R: OVERLAND FLOW
Inflow Area $=\quad 88,676 \mathrm{sf}, 39.42 \%$ Impervious, Inflow Depth $=0.20$ for 10 YR event Inflow $=0.40 \mathrm{cfs}$ @ 12.90 hrs , Volume= $\quad 1,449 \mathrm{cf}$
Outflow $=\quad 0.09 \mathrm{cfs} @ 14.19 \mathrm{hrs}$, Volume $=\quad 1,312 \mathrm{cf}$, Atten= $=76 \%$, Lag= 77.7 min
Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.06 \mathrm{fps}$, Min. Travel Time $=167.4 \mathrm{~min}$
Avg. Velocity $=0.04 \mathrm{fps}$, Avg. Travel Time $=265.5 \mathrm{~min}$
Peak Storage= 953 cf @ 14.19 hrs
Average Depth at Peak Storage=0.03' , Surface Width= 51.54'
Bank-Full Depth= 1.00' Flow Area= 75.0 sf, Capacity= 38.42 cfs
$50.00^{\prime} \times 1.00$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 25.0 ' $/$ ' Top Width= 100.00'
Length=609.0' Slope= 0.0279 '/'
Inlet Invert= 203.00', Outlet Invert= 186.00'


## Summary for Reach 20R: OVERLAND FLOW

Inflow Area =
72,222 sf, 68.72\% Impervious, Inflow Depth > 2.30" for 10YR event
Inflow =
1.79 cfs @ 12.47 hrs, Volume=

13,822 cf
Outflow $=1.03$ cfs @ 13.55 hrs, Volume=
$13,230 \mathrm{cf}$, Atten $=43 \%$, Lag $=65.2 \mathrm{~min}$
Routed to Reach 11R : 4x4 Open Bottom Culvert
Routing by Dyn-Stor-Ind method, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.11 \mathrm{fps}$, Min. Travel Time $=83.0 \mathrm{~min}$
Avg. Velocity $=0.06 \mathrm{fps}$, Avg. Travel Time $=155.3 \mathrm{~min}$
Peak Storage= 5,110 cf @ 13.55 hrs
Average Depth at Peak Storage= 0.18', Surface Width= 51.79'
Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 18.54 cfs
$50.00^{\prime} \times 1.00^{\prime}$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length=560.0' Slope= 0.0093 '/'
Inlet Invert= 200.00', Outlet Invert= 194.80'


## Summary for Reach 23R: OVERLAND FLOW

| Inflow Area = | 424,818 sf, | 45.99\% Impervious, | Inflow Depth > 1.13" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 4.11 cfs @ | 12.47 hrs , Volume= | 40,059 cf |
| Outflow | 3.63 cfs @ | 12.73 hrs , Volume= | 39,543 cf, Atten= 12\%, Lag= 15.7 min |

Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.22 \mathrm{fps}, \mathrm{Min}$. Travel Time $=17.8 \mathrm{~min}$
Avg. Velocity $=0.10 \mathrm{fps}$, Avg. Travel Time $=40.2 \mathrm{~min}$

Peak Storage= 3,866 cf @ 12.73 hrs
Average Depth at Peak Storage= 0.29' , Surface Width= 61.69'
Bank-Full Depth= 1.00' Flow Area= 70.0 sf, Capacity $=31.93$ cfs
$50.00^{\prime} \times 1.00$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 20.0 '/' Top Width= 90.00'
Length $=237.0^{\prime}$ Slope $=0.0211$ '/'
Inlet Invert= 193.00', Outlet Invert= 188.00'


## Summary for Reach R202: OVERLAND FLOW

[62] Hint: Exceeded Reach SC1 OUTLET depth by 0.21 ' @ 13.05 hrs

| Inflow Area | 432,269 sf, 42.08\% Impervious, | 2.49" for 10YR event |
| :---: | :---: | :---: |
| Inflow | 18.32 cfs @ 12.33 hrs, Volume= | 89,705 cf |
| Outflow | 8.31 cfs @ 12.74 hrs , Volume= | $84,798 \mathrm{cf}, \mathrm{Atten}=55 \%, \mathrm{Lag}=24.8 \mathrm{~min}$ |

Routed to Link AP2 : ANALYSIS POINT 2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.19 \mathrm{fps}, \mathrm{Min}$. Travel Time $=60.1 \mathrm{~min}$
Avg. Velocity $=0.09 \mathrm{fps}$, Avg. Travel Time $=135.3 \mathrm{~min}$
Peak Storage= 29,976 cf @ 12.74 hrs
Average Depth at Peak Storage= $0.39^{\prime}$, Surface Width= $119.51^{\prime}$
Bank-Full Depth= 1.00' Flow Area= 125.0 sf, Capacity $=42.56$ cfs
100.00 x 1.00' deep channel, $\mathrm{n}=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 150.00'
Length=700.0' Slope= 0.0107 '/'
Inlet Invert= 205.50', Outlet Invert= 198.00'


## Summary for Reach R211: OVERLAND FLOW

| Inflow Area $=$ | $241,078 \mathrm{sf}, 59.10 \%$ Impervious, | Inflow Depth $=0.31 "$ | for 10 YR event |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $3.94 \mathrm{cfs} @$ | 12.51 hrs , Volume $=$ |
| Outflow | $=$ | $0.78 \mathrm{cfs} @$ | 13.04 hrs , Volume $=$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.11 \mathrm{fps}, \mathrm{Min}$. Travel Time $=92.1 \mathrm{~min}$
Avg. Velocity $=0.05 \mathrm{fps}$, Avg. Travel Time $=217.1 \mathrm{~min}$
Peak Storage $=4,295$ cf @ 13.04 hrs
Average Depth at Peak Storage $=0.19^{\prime}$, Surface Width= 40.68'
Bank-Full Depth=1.00' Flow Area= 50.0 sf, Capacity= 14.51 cfs
$35.00^{\prime} \times 1.00$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 15.0 '/' Top Width= 65.00'
Length=600.0' Slope= 0.0087 '/'
Inlet Invert= 200.00', Outlet Invert= 194.80'


## Summary for Reach SC1: Stream Crossing \#1

[52] Hint: Inlet/Outlet conditions not evaluated

| Inflow Area = | 432,269 sf, 42.08\% Impervious, | W Depth > 2.49" for 10YR event |
| :---: | :---: | :---: |
| Inflow | 18.32 cfs @ 12.33 hrs, Volume= | 89,722 cf |
| Outflow | 18.32 cfs @ 12.33 hrs , Volume= | $89,705 \mathrm{cf}, \mathrm{Atten}=0 \%$ Lag $=0.2 \mathrm{~min}$ |

Routed to Reach R202 : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=3.33 \mathrm{fps}$, Min. Travel Time $=0.2 \mathrm{~min}$
Avg. Velocity $=1.19 \mathrm{fps}$, Avg. Travel Time $=0.6 \mathrm{~min}$
Peak Storage= 237 cf @ 12.33 hrs
Average Depth at Peak Storage= 0.34' , Surface Width= 16.00'
Bank-Full Depth= 5.00' Flow Area= 69.8 sf, Capacity= 722.91 cfs
192.0" W x 60.0" H, R=207.0" Arch Pipe
$\mathrm{n}=0.030$ Stream, clean \& straight
Length= 43.1' Slope= 0.0200 '/'
Inlet Invert= 206.37', Outlet Invert= 205.51'


## Summary for Reach SC2: Stream Crossing \#2

[52] Hint: Inlet/Outlet conditions not evaluated


Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity= 1.32 fps , Min. Travel Time $=0.5 \mathrm{~min}$
Avg. Velocity $=1.05 \mathrm{fps}$, Avg. Travel Time $=0.6 \mathrm{~min}$
Peak Storage= 38 cf @ 12.35 hrs
Average Depth at Peak Storage $=0.07$ ' , Surface Width $=16.00^{\prime}$
Bank-Full Depth $=5.00^{\prime}$ Flow Area= 68.1 sf, Capacity= 768.96 cfs
192.0" W x 60.0" H, R=180.0" Arch Pipe
$\mathrm{n}=0.030$ Stream, clean \& straight
Length= 36.5' Slope= 0.0241 '/'
Inlet Invert= 208.52', Outlet Invert= 207.64'


## Summary for Pond 1P: DMH \#33

| Inflow Area | 16,111 sf, | erv | 4.56" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.73 cfs @ | 12.09 hrs , Volume= | 6,129 cf |
| Outflow | 1.73 cfs @ | 12.09 hrs , Volume= | $6,129 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.73 cfs @ | 12.09 hrs , Volume= | 6,129 cf |

Routed to Pond OCS6 : OCS \#6
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.22' @ 12.09 hrs
Flood Elev= 209.64'
Device Routing Invert Outlet Devices
\#1 Primary $\quad 205.50^{\prime} \quad 12 . \mathbf{0}^{\prime \prime}$ Round Culvert L=46.7' Ke= 0.500
Inlet / Outlet Invert=205.50' / 204.33' S=0.0251 '/l Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$
Primary OutFlow Max=1.69 cfs @ 12.09 hrs HW=206.20' TW=202.11' (Dynamic Tailwater)
L1=Culvert (Inlet Controls 1.69 cfs @ 2.86 fps )

## Summary for Pond 3P: OCS \#8

| Inflow Area = | 12,684 | 86.64\% Impervious | Inflow Depth > 4.07" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.29 cfs @ | 12.09 hrs , Volume= | 4,306 cf |
| Outflow | 1.29 cfs @ | 12.09 hrs , Volume= | $4,306 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.29 cfs @ | 12.09 hrs , Volume= | 4,306 cf |

Routed to Pond P214 : STORMTECH INFILTRATION SYSTEM \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=201.47' @ 12.53 hrs
Flood Elev= 206.36'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | :---: | :---: | :---: | :---: |
| $\# 1$ | Primary | $200.62^{\prime}$ | $12.0^{\prime \prime}$ Vert. Orifice/Grate $\quad$ C= $0.600 \quad$ Limited to weir flow at low heads |  |

Primary OutFlow Max=1.25 cfs @ 12.09 hrs HW=201.21' TW=200.83' (Dynamic Tailwater)
—1=Orifice/Grate (Orifice Controls 1.25 cfs @ 2.61 fps)

## Summary for Pond 5R: TRENCH DRAIN



Primary OutFlow Max=1.12 cfs @ 12.09 hrs HW=198.00' TW=195.61' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.12 cfs @ 3.21 fps )

## Summary for Pond 11P: YARD DRAIN

| Inflow Area $=$ | $21,407 \mathrm{sf}, 48.10 \%$ Impervious, | Inflow Depth $>$ | $2.76 "$ |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $1.56 \mathrm{cfs} @$ | 12.09 hrs, Volume $=$ |
| Outflow | $=$ | $1.14 \mathrm{cfs} @$ | 12.17 hrs , Volume $=$ |
| Primary | $=$ | $1.14 \mathrm{cfs} @$ | 12.17 hrs , Volume $=$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 207.37' @ 12.17 hrs Surf.Area= 5,792 sf Storage= 671 cf
Plug-Flow detention time= 18.2 min calculated for 4,881 cf ( $99 \%$ of inflow)
Center-of-Mass det. time= $13.3 \min (837.5-824.3$ )


Primary OutFlow Max=1.12 cfs @ 12.17 hrs HW=207.37' TW=203.12' (Dynamic Tailwater)
$\left\llcorner_{1=C u l v e r t ~(P a s s e s ~}^{1.12 \text { cfs of } 6.60 \text { cfs potential flow) }}\right.$
—2=Orifice/Grate (Weir Controls 1.12 cfs @ 1.14 fps )

## Summary for Pond CB10: CB \#10

| Inflow Area = | 6,961 | 0.00\% Imperviou | ow Depth > 4.72" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.76 cfs @ | 12.09 hrs , Volume= | 2,738 cf |
| Outflow | 0.76 cfs @ | 12.09 hrs , Volume= | 2,738 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.76 cfs @ | 12.09 hrs , Volume= | 2,738 cf | Routed to Pond D5 : DMH \#5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=210.33' @ 12.09 hrs
Flood Elev= 212.93'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | ---: | :--- | :--- |
| $\# 1$ | Primary | $209.76^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=33.8^{\prime} \quad \mathrm{Ke}=0.500$ |  |
|  |  |  | Inlet / Outlet Invert= 209.76' $/ 209.59^{\prime} \quad \mathrm{S}=0.0050$ | $\prime / \prime \quad \mathrm{Cc}=0.900$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |

Primary OutFlow Max=0.74 cfs @ 12.09 hrs HW=210.31' TW=210.07' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.74 cfs @ 2.38 fps )

## Summary for Pond CB11: CB \#11



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 210.43' @ 12.09 hrs
Flood Elev= 213.13'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $209.94 '$ | $\mathbf{1 2 . 0}$ " Round Culvert $\mathrm{L}=26.3^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 209.94' $/ 209.67 \prime$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=0.76 cfs @ 12.09 hrs HW=210.42' TW=210.07' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.76 cfs @ 2.95 fps )

## Summary for Pond CB12: CB \#12



Routed to Pond 1P : DMH \#33
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=207.06' @ 12.09 hrs
Flood Elev= 209.84'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 206.68' | 12.0" Round Culvert L= 41.3' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 206.68' / 205.65' S=0.0249 '/l' Cc=0.900 $n=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| $\underbrace{P r i m a r}_{1=C}$ | OutFIow Ivert (Inl | .55 cfs ols 0.5 | 12.09 hrs HW=207.05' TW=206.20' (Dynamic Tailwater) © 2.08 fps ) |

## Summary for Pond CB13: CB \#13



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.26' @ 12.09 hrs
Flood Elev= 209.86'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | ---: | :--- | :--- |
| $\# 1$ | Primary | $206.70^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert L=43.7' $\mathrm{Ke}=0.500$ |  |

Inlet / Outlet Invert= 206.70' / 205.61' S=0.0249 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=1.13 cfs @ 12.09 hrs HW=207.25' TW=206.20' (Dynamic Tailwater)
—1 $^{1=C u l v e r t ~(I n l e t ~ C o n t r o l s ~} 1.13$ cfs @ 2.53 fps )

## Summary for Pond CB14: CB \#14

| Inflow Area = | 12,099 sf, 8 | 86.22\% Impervious, | Inflow Depth > 3.94" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.20 cfs @ 12 | 12.09 hrs , Volume= | 3,973 cf |
| Outflow | 1.20 cfs @ 12. | 12.09 hrs , Volume= | $3,973 \mathrm{cf}$, Atten= $0 \%$, Lag= 0.0 min |
| Primary = Routed to Pond | $\begin{aligned} & 1.20 \mathrm{cfs} @ 12 \\ & \mathrm{D} 8 \text { : DMH \#8 } \end{aligned}$ | 12.09 hrs , Volume= | 3,973 cf |
| Routing by Dyn-S <br> Peak Elev= 201.6 <br> Flood Elev= 203 | or-Ind method, ' @ 12.09 hrs | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | $t$ Outlet Devices |  |
| \#1 Primary | $200.79^{\prime}$ | 12.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { vert } \mathrm{L}=23.2^{\prime} \mathrm{Ke}=0.500 \\ & \mathrm{t}=200.79^{\prime} / 200.67^{\prime} \quad \mathrm{S}=0.0052^{\prime} / l^{\prime} \quad \mathrm{Cc}=0.900 \end{aligned}$ $\text { ted PE, smooth interior, Flow Area= } 0.79 \text { sf }$ |

Primary OutFlow Max=1.17 cfs @ 12.09 hrs HW=201.67' TW=201.52' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 1.17 cfs @ 2.14 fps )

## Summary for Pond CB15: CB \#15

| Inflow Area = | 6,66 | pervious, | 2" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.73 cfs @ | 12.09 hrs , Volume= | 2,622 cf |
| Outflow | 0.73 cfs @ | 12.09 hrs , Volume= | 2,622 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.73 cfs @ | 12.09 hrs , Volume= | 2,622 cf | Routed to Pond D8 : DMH \#8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=201.60' @ 12.09 hrs
Flood Elev= 203.95'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $200.79^{\prime}$ | $\mathbf{1 2 . 0}$ " Round Culvert $\mathrm{L}=15.6^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $200.79^{\prime} / 200.71^{\prime} \quad \mathrm{S}=0.0051$ '/' $\quad \mathrm{Cc}=0.900$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=0.71 cfs @ 12.09 hrs HW=201.58' TW=201.52' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.71 cfs @ 1.45 fps )

## Summary for Pond CB16: CB \#16

| Inflow Area = | 8,516 | 64.88\% Impervious, | w Depth > 2.76" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.62 cfs @ | 12.09 hrs , Volume= | 1,962 cf |
| Outflow | 0.62 cfs @ | 12.09 hrs , Volume= | 1,962 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.62 cfs @ | 12.09 hrs , Volume= | 1,962 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 203.99' @ 12.09 hrs
Flood Elev= 206.64'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 203.47' | 12.0" Round Culvert L= 20.9' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=203.47' / 203.33' S=0.0067 '// Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.61 cfs @ 12.09 hrs HW=203.98' TW=203.81' (Dynamic Tailwater)
——1=Culvert (Outlet Controls 0.61 cfs @ 2.20 fps )

## Summary for Pond CB17: CB \#17



## Summary for Pond CB18: CB \#18

| Inflow Area $=$ | $24,853 \mathrm{sf}, 72.99 \%$ | Impervious, | Inflow Depth > 3.31" | for 10 YR event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $2.14 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $6,857 \mathrm{cf}$ |
| Outflow | $=$ | $2.14 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $6,857 \mathrm{cf}$, Atten= $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | $=$ | $2.14 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $6,857 \mathrm{cf}$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.61' @ 12.09 hrs
Flood Elev= 208.16'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $204.72^{\prime}$ | 15.0" Round Culvert $\mathrm{L}=25.1^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $204.72^{\prime} / 204.59 ' \quad \mathrm{~S}=0.0052$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.23 \mathrm{sf}$ |  |

Primary OutFlow Max=2.11 cfs @ 12.09 hrs HW=205.60' TW=205.27' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 2.11 cfs @ 3.22 fps )

## Summary for Pond CB20: CB \#20

| Inflow Area = | 11,939 sf, | .95\% Impervious, | Inflow Depth > 4.38" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.26 cfs @ 12 | 09 hrs , Volume= | 4,355 cf |
| Outflow | 1.26 cfs @ 12 | 09 hrs , Volume= | $4,355 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary = Routed to Pon | $1.26 \mathrm{cfs} @ 12$ | .09 hrs , Volume= | 4,355 cf |
| Routing by Dyn-S <br> Peak Elev= 204.6 <br> Flood Elev= 207 | r-Ind method, @ 12.09 hrs | ime Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 203.97' | 12.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \hline \text { vert } \mathrm{L}=30.3^{\prime} \mathrm{Ke}=0.500 \\ & \mathrm{t}=203.97^{\prime} / 203.81^{\prime} \quad \mathrm{S}=0.0053^{\prime} /{ }^{\prime} \quad \mathrm{Cc}=0.900 \end{aligned}$ $\text { ted PE, smooth interior, Flow Area= } 0.79 \mathrm{sf}$ |

Primary OutFlow Max=1.23 cfs @ 12.09 hrs HW=204.66' TW=204.22' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 1.23 cfs @ 2.98 fps )

## Summary for Pond CB21: CB \#21

| Inflow Area = | 10,17 | pervious, | 3.83" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.99 cfs @ | 12.09 hrs , Volume= | 3,251 cf |
| Outflow | 0.99 cfs @ | 12.09 hrs , Volume= | $3,251 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.99 cfs @ | 12.09 hrs , Volume= | 3,251 cf |

Routed to Pond D12 : DMH \#12
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=204.94' @ 12.09 hrs
Flood Elev= 208.02'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $204.32^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert L=26.0' $\mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= 204.32' $/ 204.19^{\prime} \mathrm{S}=0.0050$ '/' $\mathrm{Cc}=0.900$ |  |
|  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |

Primary OutFlow Max=0.97 cfs @ 12.09 hrs HW=204.93' TW=204.22' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.97 cfs @ 2.76 fps )

## Summary for Pond CB22: CB \#22



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 206.05' @ 12.09 hrs
Flood Elev=208.50'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $205.33^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=16.1^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= $205.33^{\prime} / 205.25^{\prime} \quad \mathrm{S}=0.0050 \quad$ '/' $\quad \mathrm{Cc}=0.900$ |  |
| $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 0.79 sf |  |  |  |

Primary OutFlow Max=1.25 cfs @ 12.09 hrs HW=206.04' TW=205.14' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 1.25 cfs @ 2.95 fps )

## Summary for Pond CB23: CB \#23



## Summary for Pond CB24: CB \#24

| Inflow Area = | 7,930 | 72.16\% Impervious | Inflow Depth > 4.16" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.82 cfs @ | 12.09 hrs , Volume= | 2,746 cf |
| Outflow | 0.82 cfs @ | 12.09 hrs , Volume= | $2,746 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.82 cfs @ | 12.09 hrs , Volume= | 2,746 cf | Routed to Pond D16 : DMH \#16

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=205.83' @ 12.09 hrs
Flood Elev= 209.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $205.21^{\prime}$ | 12.0" Round Culvert $\mathrm{L}=12.1^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $205.21^{\prime} / 205.15^{\prime} \quad \mathrm{S}=0.0050$ |
|  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=0.79 cfs @ 12.09 hrs HW=205.82' TW=205.67' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.79 cfs @ 2.29 fps )

## Summary for Pond CB25: CB \#25

| Inflow Area = | 8,487 sf, | 92\% Impervious, | Inflow Depth > 4.27" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 0.89 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 3,017 cf |
| Outflow = | 0.89 cfs @ 12 | . 09 hrs , Volume $=$ | $3,017 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pon | $\begin{aligned} & 0.89 \mathrm{cfs} @ 12 \\ & \mathrm{D} 16 \text { : DMH \#1 } \end{aligned}$ | .09 hrs , Volume= | 3,017 cf |
| Routing by Dyn-S <br> Peak Elev= 205.8 <br> Flood Elev= 208 | or-Ind method, ' @ 12.09 hrs | ime Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 205.22' | 15.0" Round Culv Inlet / Outlet Inve $\mathrm{n}=0.012$ Corruga | $\begin{aligned} & \text { Ivert } \mathrm{L}=11.4^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{rt}=205.22^{\prime} / 205.16 ' \quad \mathrm{~S}=0.0053 \mathrm{I} / \mathrm{Cc}=0.900 \\ & \text { ated PP, smooth interior, Flow Area= } 1.23 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=0.86 cfs @ 12.09 hrs HW=205.81' TW=205.67' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.86 cfs @ 2.24 fps )

## Summary for Pond CB26: CB \#26

| Inflow Area = | 8,835 | \% Impervious, | 3.94" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.88 cfs @ | 12.09 hrs , Volume= | 2,901 cf |
| Outflow | 0.88 cfs @ | 12.09 hrs , Volume= | 2,901 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.88 cfs @ | 12.09 hrs, Volume= | 2,901 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=202.34' @ 12.09 hrs
Flood Elev= 204.93'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 201.77' | 12.0" Round Culvert L=42.5' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 201.77' / 201.55 ' S = 0.0052 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |

Primary OutFlow Max=0.86 cfs @ 12.09 hrs HW=202.33' TW=201.20' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.86 cfs @ 2.75 fps )

## Summary for Pond CB27: CB \#27



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 201.49' @ 12.09 hrs
Flood Elev=204.16'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $201.00 '$ | $\mathbf{1 2 . 0}$ " Round Culvert $\mathrm{L}=18.0^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= $201.00^{\prime} / 200.90^{\prime} \quad \mathrm{S}=0.0056 \mathrm{I} / \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=0.64 cfs @ 12.09 hrs HW=201.48' TW=201.20' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.64 cfs @ 2.50 fps )

## Summary for Pond CB28: CB \#28

| Inflow Area = | 10,372 sf, | pervious, | Inflow Depth > 3.63" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.97 cfs @ | 12.09 hrs , Volume= | 3,136 cf |
| Outflow | 0.97 cfs @ | 12.09 hrs , Volume= | $3,136 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.97 cfs @ | 12.09 hrs , Volume= | 3,136 cf |

Routed to Pond D18 : DMH \#18
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev= 198.44' @ 12.09 hrs
Flood Elev= 200.92'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 197.75' | 12.0" Round Culvert L= 13.7' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 197.75' / 197.69' S=0.0044 '/l' Cc=0.900 $n=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| $\underbrace{P r i m a r}_{1=C}$ | OutFIow lvert (O | .95 cfs trols 0. | 12.09 hrs HW=198.43' TW=198.26' (Dynamic Tailwater) cfs @ 2.36 fps ) |

## Summary for Pond CB29: CB \#29

| Inflow Area = | 8,495 | 84.21\% Impervious, | Inflow Depth > 4.27" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.89 cfs @ | 12.09 hrs , Volume= | 3,020 cf |
| Outflow | 0.89 cfs @ | 12.09 hrs , Volume= | $3,020 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.89 cfs @ | 12.09 hrs , Volume= | $3,020 \mathrm{cf}$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=206.08' @ 12.09 hrs
Flood Elev= 208.55'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $205.38^{\prime}$ | 12.0" Round Culvert $\mathrm{L}=13.5^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $205.38^{\prime} / 205.31^{\prime} \quad \mathrm{S}=0.0052$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=0.86 cfs @ 12.09 hrs HW=206.06' TW=205.93' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.86 cfs @ 2.13 fps )

## Summary for Pond CB30: CB \#30

| Inflow Area = | 8,933 sf, 82.40\% Impervious, Inflow Depth > 4.27" for 10YR event |  |  |
| :---: | :---: | :---: | :---: |
| Inflow = | 0.93 cfs @ 12. | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 3,175 cf |
| Outflow | 0.93 cfs @ 12. | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | $3,175 \mathrm{cf}$, Atten= $0 \%$, Lag 0.0 min |
| Primary = Routed to Pon | $\begin{aligned} & 0.93 \mathrm{cfs} @ 12 \\ & \text { D19: DMH \#1 } \end{aligned}$ | .09 hrs , Volume= | 3,175 cf |
| Routing by Dyn-S <br> Peak Elev= 206.0 <br> Flood Elev= 208 | or-Ind method, @ 12.09 hrs $4^{\prime}$ | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 205.38' | 12.0" Round Culv Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { Ivert } \mathrm{L}=17.5^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=205.38^{\prime} / 205.29 \text { ' } \mathrm{S}=0.0051 \mathrm{l} / \mathrm{Cc}=0.900 \\ & \text { ated PE, smooth interior, Flow Area= } 0.79 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=0.91 cfs @ 12.09 hrs HW=206.08' TW=205.93' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.91 cfs @ 2.18 fps )

## Summary for Pond CB31: CB \#31

| Inflow Area $=$ | $16,365 \mathrm{sf}$, | $68.64 \%$ Impervious, | Inflow Depth > 3.83" | for 10 YR event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $1.59 \mathrm{cfs} @$ | 12.09 hrs , Volume | $5,230 \mathrm{cf}$ |
| Outflow | $=$ | $1.59 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $5,230 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | $=$ | $1.59 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $5,230 \mathrm{cf}$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.02' @ 12.09 hrs
Flood Elev=207.36'


Primary OutFlow Max=1.56 cfs @ 12.09 hrs HW=205.01' TW=204.39' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 1.56 cfs @ 3.07 fps )

## Summary for Pond CB32: CB \#32

| Inflow Area = | 12,710 | pervious | 3.94" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.26 cfs @ | 12.09 hrs , Volume= | 4,174 cf |
| Outflow | 1.26 cfs @ | 12.09 hrs , Volume= | $4,174 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.26 cfs @ | 12.09 hrs , Volume= | 4,174 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 204.91' @ 12.09 hrs
Flood Elev= 207.35'


Primary OutFlow Max=1.23 cfs @ 12.09 hrs HW=204.90' TW=204.39' (Dynamic Tailwater)
——1=Culvert (Barrel Controls 1.23 cfs @ 2.89 fps )

## Summary for Pond CB33: CB \#33

| Inflow Area = |  | 5,421 sf, 83.90\% Impervious, Inflow Depth > 4.27" for 10YR event |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | = | 0.57 cfs @ 1 | 2.09 hrs , Volume $=1,927 \mathrm{cf}$ |  |
| Outflow | = | 0.57 cfs @ | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 1,927 cf, Atten= 0\%, Lag= 0.0 min |
| Primary Route |  | $0.57 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume=D22 : DMH \#22 |  | 1,927 cf |
| Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$ <br> Peak Elev=205.84' @ 12.09 hrs <br> Flood Elev=208.45' |  |  |  |  |
| Device | Routing | Invert | Outlet Devices |  |
| \#1 | Primary | 205.28' | 12.0" Round Cul Inlet / Outlet Inve $n=0.013$ Corrug | $\begin{aligned} & \text { Ivert } \mathrm{L}=11.7^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{rt}=205.28^{\prime} / 205.22 \mathrm{~S}=0.0051 \mathrm{I} /{ }^{\prime} \quad \mathrm{Cc}=0.900 \\ & \text { ated PE, smooth interior, Flow Area= } 0.79 \mathrm{sf} \end{aligned}$ |
| Primary OutFlow Max=0.55 cfs @ 12.09 hrs HW=205.82' TW=205.72' (Dynamic Tailwater) <br> $\left\llcorner_{1=C u l v e r t ~(O u t l e t ~ C o n t r o l s ~} 0.55\right.$ cfs @ 1.83 fps ) |  |  |  |  |

## Summary for Pond CB34: CB \#34

| Inflow Area = | 8,622 | 80.51\% Impervious, | Inflow Depth > 4.16" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.89 cfs @ | 12.09 hrs , Volume= | 2,986 cf |
| Outflow | 0.89 cfs @ | 12.09 hrs , Volume= | 2,986 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.89 cfs @ | 12.09 hrs , Volume= | 2,986 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.89' @ 12.09 hrs
Flood Elev= 208.38'


Primary OutFlow Max=0.86 cfs @ 12.09 hrs HW=205.88' TW=205.72' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.86 cfs @ 2.21 fps )

## Summary for Pond CB35: CB \#35

 Routed to Pond D23: DMH \#23

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=207.44' @ 12.09 hrs
Flood Elev= 210.21'


Primary OutFlow Max=0.44 cfs @ 12.09 hrs HW=207.44' TW=207.22' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.44 cfs @ 2.24 fps )

## Summary for Pond CB36: CB \#36

| Inflow Area | 6,622 | 00.00\% Impervious, | Inflow Dep | 72" for 10YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 0.72 cfs @ | 12.09 hrs , Volume= | 2,605 cf |  |
| Outflow | 0.72 cfs @ | 12.09 hrs , Volume= | 2,605 cf, | Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 0.72 cfs @ | 12.09 hrs , Volume= | 2,605 cf |  |

Routed to Pond D23 : DMH \#23
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.56' @ 12.09 hrs
Flood Elev=210.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $207.04 '$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=16.1^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 207.04' $/ 206.96^{\prime} \quad \mathrm{S}=0.0050$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=0.70 cfs @ 12.09 hrs HW=207.55' TW=207.22' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.70 cfs @ 2.50 fps )

## Summary for Pond CB38: CB \#38



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 210.56' @ 12.09 hrs
Flood Elev=212.86'


Primary OutFlow Max=0.81 cfs @ 12.09 hrs HW=210.54' TW=210.47' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.81 cfs @ 1.54 fps )

## Summary for Pond CB39: CB \#39



Routed to Pond D25: DMH \#25
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=210.56' @ 12.09 hrs
Flood Elev=212.86'


## Summary for Pond CB40: CB \#40

| Inflow Area = | 4,211 sf,100.00\% Impervious, | flow Depth > 4.72" for 10YR event |
| :---: | :---: | :---: |
| Inflow | 0.46 cfs @ 12.09 hrs , Volume= | 1,656 cf |
| Outflow | 0.46 cfs @ 12.09 hrs , Volume= | $1,656 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.46 cfs @ 12.09 hrs , Volume= | 1,656 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=214.16' @ 12.09 hrs
Flood Elev= 217.04'
Device Routing Invert Outlet Devices
\#1 Primary 213.68' 12.0" Round Culvert L= 17.8' Ke=0.500
Inlet / Outlet Invert= 213.68' / 213.55' S=0.0073 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=0.45 cfs @ 12.09 hrs HW=214.15' TW=214.04' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.45 cfs @ 1.78 fps )

## Summary for Pond CB41: CB \#41

| Inflow Area = | 5, | perviou | Inflow Depth > 4.72" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.61 cfs @ | 12.09 hrs , Volume= | 2,197 cf |
| Outflow | 0.61 cfs @ | 12.09 hrs , Volume= | 2,197 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.61 cfs @ | 12.09 hrs , Volume= | 2,197 cf | Routed to Pond D27 : DMH \#27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=214.37' @ 12.09 hrs
Flood Elev=217.06'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $213.89^{\prime}$ | $\mathbf{1 2 . 0}$ " Round Culvert $\mathrm{L}=18.4^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= $213.89^{\prime} / 213.80^{\prime} \quad \mathrm{S}=0.0049 \mathrm{I} / \mathrm{l} \quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=0.59 cfs @ 12.09 hrs HW=214.36' TW=214.04' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.59 cfs @ 2.40 fps )

## Summary for Pond CB43: CB \#43



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=220.42' @ 12.09 hrs
Flood Elev=223.17'


Primary OutFlow Max=0.29 cfs @ 12.09 hrs HW=220.41' TW=220.34' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.29 cfs @ 1.40 fps )

## Summary for Pond CB44: CB \#44



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 220.39' @ 12.09 hrs
Flood Elev= 223.17'


Primary OutFlow Max=0.20 cfs @ 12.09 hrs HW=220.38' TW=220.34' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.20 cfs @ 1.05 fps )

## Summary for Pond CB45: CB \#45

| Inflow Area = | 2,465 sf, | 50.30\% Impervious, | Inflow Depth > 2.86" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.19 cfs @ | 12.09 hrs , Volume= | 587 cf |
| Outflow | 0.19 cfs @ | 12.09 hrs , Volume= | 587 cf , Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 0.19 cfs @ | 12.09 hrs , Volume= | 587 cf |

Routed to Pond D30 : DMH \#30
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=221.54' @ 12.09 hrs
Flood Elev=224.46'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 221.29' | 12.0" Round Culvert L= 18.2' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=221.29' / 221.20' S=0.0049 '/l' Cc=0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| $\underbrace{P r i m a r}_{1=C}$ | OutFIow lvert (Ba | 18 cfs trols 0. | 12.09 hrs HW=221.54' TW=221.33' (Dynamic Tailwater) cfs @ 1.78 fps ) |

## Summary for Pond CB46: CB \#46

| Inflow Area $=$ | $4,397 \mathrm{sf}, 50.97 \%$ | Impervious, Inflow Depth > $2.86 "$ | for 10 YR event |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.33 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |
| Outflow | $=$ | $0.33 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |
| Primary | $=$ | $0.33 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=221.87' @ 12.09 hrs
Flood Elev= 224.69'
Device Routing Invert Outlet Devices
\#1 Primary 221.53' 12.0" Round Culvert L= 15.3' Ke=0.500
Inlet / Outlet Invert= 221.53' / 221.45' S=0.0052 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=0.33 cfs @ 12.09 hrs HW=221.87' TW=221.33' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.33 cfs @ 2.07 fps )

## Summary for Pond CB47: CB\#47

| Inflow Area = | 3,012 sf, 100 | .00\% Impervious, | Inflow Depth > 4.72" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 0.33 cfs @ 12. | . 09 hrs , Volume= | 1,185 cf |
| Outflow = | 0.33 cfs @ 12. | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | $1,185 \mathrm{cf}$, Atten= $0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pon | $\begin{aligned} & 0.33 \mathrm{cfs} @ 12 \\ & \text { D31 : DMH\#31 } \end{aligned}$ | .09 hrs , Volume= | 1,185 cf |
| Routing by Dyn-S <br> Peak Elev= 225.5 <br> Flood Elev= 230 | or-Ind method, ' @ 12.15 hrs $1^{\prime}$ | me Span= 0.00- | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 225.05' | 12.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.012$ Corrug | $\begin{aligned} & \text { Ivert } \mathrm{L}=20.9^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=225.05^{\prime} / 224.95^{\prime} \quad \mathrm{S}=0.0048 \text { '/' } \quad \mathrm{Cc}=0.900 \\ & \text { ated PP, smooth interior, Flow Area= } 0.79 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=0.33 cfs @ 12.09 hrs HW=225.47' TW=225.38' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.33 cfs @ 1.53 fps )

## Summary for Pond CB48: CB\#48

 Routed to Pond D31 : DMH\#31

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=225.81' @ 12.17 hrs
Flood Elev=230.25'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $224.82^{\prime}$ | $15.0^{\prime \prime}$ Round Culvert L=16.9' Ke= <br>  <br>  <br>  |
|  |  | Inlet / Outlet Invert= 224.82' $/ 224.74$ ' $\mathrm{S}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |  |

Primary OutFlow Max=2.56 cfs @ 12.17 hrs HW=225.80' TW=225.48' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 2.56 cfs @ 3.40 fps )

## Summary for Pond CB49: CB \#49

| Inflow Area = | 5,238 s | 84.59\% Impervious, | Inflow Depth > 4.27" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.55 cfs @ | 12.09 hrs , Volume= | 1,862 cf |
| Outflow | 0.55 cfs @ | 12.09 hrs , Volume= | 1,862 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.55 cfs @ | 12.09 hrs , Volume= | 1,862 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 203.45' @ 12.09 hrs
Flood Elev=205.93'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.76' | 12.0" Round Culvert L= 15.5' Ke= 0.500 <br> Inlet / Outlet Invert= 202.76' / 202.68' S=0.0052 '/l' Cc=0.900 <br> $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.53 cfs @ 12.09 hrs HW=203.44' TW=203.39' (Dynamic Tailwater)
——1=Culvert (Outlet Controls 0.53 cfs @ 1.32 fps )

## Summary for Pond CB50: CB \#50



## Summary for Pond CB51: CB \#51

 Routed to Pond OCS7 : OCS \#7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=202.87' @ 12.09 hrs
Flood Elev= 212.77'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.35' | 12.0" Round Culvert L=31.4' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 202.35' / 202.19' S=0.0051 '/' Cc= 0.900 $\mathrm{n}=0.013$, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.72 cfs @ 12.09 hrs HW=202.86' TW=202.47' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.72 cfs @ 2.59 fps )

## Summary for Pond CB52: CB \#52

| Inflow Area = | 9,052 sf, | 87.14\% Impervious, | Inflow Depth > 4.38" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.96 cfs @ | 12.09 hrs , Volume= | 3,302 cf |
| Outflow | 0.96 cfs @ | 12.09 hrs , Volume= | $3,302 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.96 cfs @ | 12.09 hrs , Volume= | 3,302 cf | Routed to Pond OCS7 : OCS \#7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=203.29' @ 12.09 hrs
Flood Elev= 205.84'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.68 ' | 12.0" Round Culvert L= 25.5' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert=202.68' / 202.55' S=0.0051 '/l' Cc=0.900 <br> $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.93 cfs @ 12.09 hrs HW=203.28' TW=202.48' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.93 cfs @ 2.74 fps )

## Summary for Pond CB53: CB \#53

| Inflow Area | 7,863 sf, | 52\% Impervious, | Inflow Depth > 4.16" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.81 cfs @ | 12.09 hrs , Volume= | 2,723 cf |
| Outflow | 0.81 cfs @ | 12.09 hrs , Volume= | $2,723 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.81 cfs @ | 12.09 hrs , Volume= | 2,723 cf | Routed to Pond 3P : OCS \#8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=203.33' @ 12.09 hrs
Flood Elev=205.95'


Primary OutFlow Max=0.79 cfs @ 12.09 hrs HW=203.32' TW=201.21' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.79 cfs @ 2.64 fps )

## Summary for Pond CB54: CB \#54

| Inflow Area = | 4,821 | 86.85\% Impervious | w Depth > 3.94" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.48 cfs @ | 12.09 hrs , Volume= | 1,583 cf |
| Outflow | 0.48 cfs @ | 12.09 hrs , Volume= | $1,583 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.48 cfs @ | 12.09 hrs , Volume= | 1,583 cf | Routed to Pond 3P : OCS \#8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 203.07' @ 12.09 hrs
Flood Elev= 205.82'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.66' | 12.0" Round Culvert L=36.7' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=202.66' / 202.48' S=0.0049 '// Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.47 cfs @ 12.09 hrs HW=203.07' TW=201.21' (Dynamic Tailwater)
——1=Culvert (Barrel Controls 0.47 cfs @ 2.31 fps )

## Summary for Pond CB7: CB\#5

 Routed to Pond D4 : DMH\#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=212.99' @ 12.09 hrs
Flood Elev=215.79'


## Summary for Pond CB8: CB\#8



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=214.21' @ 12.09 hrs
Flood Elev= 215.79'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | ---: | :--- | :--- |
| $\# 1$ | Primary | $213.79^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert L= 15.1' $\mathrm{Ke}=0.500$ |  |

Inlet / Outlet Invert= 213.79' / 213.64' S=0.0099 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.55 cfs @ 12.09 hrs HW=214.20' TW=211.30' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.55 cfs @ 2.68 fps )

## Summary for Pond CB9: CB \#9

| Inflow Area = | 16,307 sf, 93. | 93.95\% Impervious, | Inflow Depth > 4.60" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.76 cfs @ 12 | 12.09 hrs , Volume= | 6,257 cf |
| Outflow | 1.76 cfs @ 12 | 12.09 hrs , Volume= | 6,257 cf, Atten= 0\%, Lag= 0.0 min |
| Primary = Routed to Pond | $1.76 \mathrm{cfs} @ 12$ | 12.09 hrs , Volume= | 6,257 cf |
| Routing by Dyn-S <br> Peak Elev= 210.82 <br> Flood Elev= 213 | r-Ind method, @ 12.09 hrs | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | $t$ Outlet Devices |  |
| \#1 Primary | $210.10^{\prime}$ | 12.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { vert } \mathrm{L}=19.9^{\prime} \mathrm{Ke}=0.500 \\ & \mathrm{t}=210.10^{\prime} / 209.71^{\prime} \mathrm{S}=0.0196 \text { '/' } \mathrm{Cc}=0.900 \end{aligned}$ $\text { ted PE, smooth interior, Flow Area }=0.79 \mathrm{sf}$ |

Primary OutFlow Max=1.71 cfs @ 12.09 hrs HW=210.81' TW=210.07' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 1.71 cfs @ 2.87 fps )

## Summary for Pond D10: DMH \#10

| Inflow Area = | 8,516 | , | w Depth > 2.76" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.62 cfs @ | 12.09 hrs , Volume= | 1,962 cf |
| Outflow | 0.62 cfs @ | 12.09 hrs , Volume= | $1,962 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.62 cfs @ | 12.09 hrs , Volume= | 1,962 cf | Routed to Pond P207 : INFILTRATION POND \#2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=203.81' @ 12.09 hrs
Flood Elev= 206.49'


Primary OutFlow Max=0.61 cfs @ 12.09 hrs HW=203.81' TW=197.48' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.61 cfs @ 2.43 fps )

## Summary for Pond D11: DMH \#11

| Inflow Area = | 36,689 | 73.28\% Impervious, | ow Depth > 3.55" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 3.34 cfs @ | 12.09 hrs , Volume= | 10,849 cf |
| Outflow | 3.34 cfs @ | 12.09 hrs , Volume= | 10,849 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 3.34 cfs @ | 12.09 hrs , Volume= | 10,849 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 205.28' @ 12.09 hrs
Flood Elev= 208.33'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $204.25^{\prime}$ | $\mathbf{1 8 . 0 ^ { \prime \prime } \text { Round Culvert } \mathrm { L } = 4 4 . 6 ^ { \prime } \quad \mathrm { Ke } = 0 . 5 0 0}$ |
|  |  | Inlet / Outlet Invert= 204.25' $/ 204.03^{\prime} \quad \mathrm{S}=0.0049 \mathrm{I} / \mathrm{l} \quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.77 sf |  |

Primary OutFlow Max=3.28 cfs @ 12.09 hrs HW=205.27' TW=204.50' (Dynamic Tailwater)
——=Culvert (Barrel Controls 3.28 cfs @ 3.64 fps )

## Summary for Pond D12: DMH \#12



Primary OutFlow Max=2.20 cfs @ 12.09 hrs HW=204.22' TW=203.31' (Dynamic Tailwater)
——1=Culvert (Barrel Controls 2.20 cfs @ 3.44 fps )

## Summary for Pond D13: DMH \#13

| Inflow Area $=$ | $81,632 \mathrm{sf}, 72.61 \%$ | Impervious, | Inflow Depth > 3.79" |
| :--- | :--- | :--- | :--- |

Routed to Pond P207 : INFILTRATION POND \#2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev= 203.33' @ 12.09 hrs
Flood Elev= 208.12'
Device Routing Invert Outlet Devices
\#1 Primary
201.95' 24.0" Round Culvert L=60.1' $\mathrm{Ke}=0.500$

Inlet / Outlet Invert= 201.95' / 201.65' S=0.0050 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 3.14 sf
Primary OutFlow Max=6.99 cfs @ 12.09 hrs HW=203.32' TW=197.48' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 6.99 cfs @ 4.30 fps )

## Summary for Pond D14: DMH \#14



Primary OutFlow Max=3.81 cfs @ 12.09 hrs HW=205.14' TW=203.31' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 3.81 cfs @ 4.25 fps )

## Summary for Pond D16: DMH \#16



Routed to Pond D14 : DMH \#14
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.68' @ 12.09 hrs
Flood Elev= 208.59'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $204.90^{\prime}$ | $15.0^{\prime \prime}$ Round Culvert L=103.5' $\mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= 204.90' $/ 204.38^{\prime} \quad \mathrm{S}=0.0050$ '/' $\quad \mathrm{Cc}=0.900$ |  |
|  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |  |  |

Primary OutFlow Max=1.66 cfs @ 12.09 hrs HW=205.67' TW=205.14' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 1.66 cfs @ 3.01 fps )

## Summary for Pond D17: DMH \#17

| Inflow Area = | 14,946 s | 75.26\% Impervious | Depth > 4.17" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.53 cfs @ | 12.09 hrs , Volume= | 5,188 cf |
| Outflow | 1.53 cfs @ | 12.09 hrs , Volume= | $5,188 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.53 cfs @ | 12.09 hrs , Volume= | 5,188 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 201.21' @ 12.09 hrs
Flood Elev= 204.84'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 200.55' | 12.0" Round Culvert L=91.6' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=200.55' / 197.69' S=0.0312 '// Cc=0.900 $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=1.49 cfs @ 12.09 hrs HW=201.20' TW=198.26' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.49 cfs @ 2.75 fps )

## Summary for Pond D18: DMH \#18

| Inflow Area = | 25,318 sf | 65.46\% Impervious, | Depth > 3 | .95" for 10YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 2.50 cfs @ | 12.09 hrs , Volume= | 8,324 cf |  |
| Outflow | 2.50 cfs @ | 12.09 hrs , Volume= | 8,324 cf, | Atten= 0\%, Lag= 0.0 min |
| Primary | 2.50 cfs @ | 12.09 hrs , Volume= | 8,324 cf |  |

Routed to Pond OCS1 : OCS\#1
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 198.27' @ 12.09 hrs
Flood Elev= 201.13'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 197.44' | 15.0" Round Culvert L= 46.3' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 197.44' / 196.98' S=0.0099 '// Cc= 0.900 $n=0.013$ Corrugated PE, smooth interior, Flow Area= 1.23 sf |

Primary OutFlow Max=2.44 cfs @ 12.09 hrs HW=198.26' TW=196.02' (Dynamic Tailwater)
①=Culvert (Barrel Controls 2.44 cfs @ 4.06 fps )

## Summary for Pond D19: DMH \#19

| Inflow Area = | 17,428 | 83.29\% Impervious, | Inflow Depth > 4.27" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.82 cfs @ | 12.09 hrs , Volume= | 6,195 cf |
| Outflow | 1.82 cfs @ | 12.09 hrs , Volume= | $6,195 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.82 cfs @ | 12.09 hrs, Volume= | 6,195 cf |

Routed to Pond d20 : DMH \#20
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=205.94' @ 12.09 hrs
Flood Elev= 208.57'
Device Routing Invert Outlet Devices
\#1 Primary 205.19' 12.0" Round Culvert L= 82.5' Ke=0.500
Inlet / Outlet Invert= 205.19' / 204.43' S=0.0092 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=1.77 cfs @ 12.09 hrs HW=205.93' TW=204.95' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 1.77 cfs @ 3.96 fps )

## Summary for Pond D2: DMH\#2

| Inflow Area = | 73,240 sf, | 7.72\% Impervious, | Inflow Depth > 2.45" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 3.67 cfs @ 12 | . 14 hrs, Volume= | 14,981 cf |
| Outflow | 3.67 cfs @ 12 | . 14 hrs , Volume= | $14,981 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary = Routed to Pond | 3.67 cfs @ 12 P205 : INFILTR | .14 hrs , Volume= RATION POND \#3 | 14,981 cf |
| Routing by Dyn-S <br> Peak Elev= 207.9 <br> Flood Elev= 212.0 | or-Ind method, @ 12.14 hrs | ime Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 206.90' | 15.0" Round Culv Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { Ivert } \mathrm{L}=38.2^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=206.90^{\prime} / 206.52 \text { ' } \mathrm{S}=0.0099 \quad \mathrm{l} / \mathrm{Cc}=0.900 \\ & \text { ated PE, smooth interior, Flow Area= } 1.23 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=3.63 cfs @ 12.14 hrs HW=207.98' TW=205.83' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 3.63 cfs @ 4.32 fps)

## Summary for Pond D20: DMH \#20

| Inflow Area | 17,428 sf, | 83.29\% Impervious, | Infl | 27" for 10YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 1.82 cfs @ | 12.09 hrs , Volume= | 6,195 cf |  |
| Outflow | 1.82 cfs @ | 12.09 hrs , Volume= | 6,195 cf, | Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 1.82 cfs @ | 12.09 hrs , Volume= | 6,195 cf |  |

Routed to Pond D21 : DMH \#21
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=204.96' @ 12.09 hrs
Flood Elev=207.68'


Primary OutFlow Max=1.77 cfs @ 12.09 hrs HW=204.95' TW=204.39' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 1.77 cfs @ 3.25 fps )

## Summary for Pond D21: DMH \#21

| Inflow Area $=$ | $71,317 \mathrm{sf}, 79.77 \%$ Impervious, | Inflow Depth $>$ | $4.16 "$ | for 10 YR event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $7.30 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $24,749 \mathrm{cf}$ |
| Outflow | $=$ | $7.30 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $24,749 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | $=$ | $7.30 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $24,749 \mathrm{cf}$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 204.42' @ 12.09 hrs
Flood Elev= 207.55'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $203.02^{\prime}$ | 24.0" Round Culvert $\mathrm{L}=72.4^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 203.02' $/ 202.66^{\prime} \quad \mathrm{S}=0.0050 \quad$ '/' $\quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 3.14 sf |  |

Primary OutFlow Max=7.11 cfs @ 12.09 hrs HW=204.39' TW=201.62' (Dynamic Tailwater)
——=Culvert (Barrel Controls 7.11 cfs @ 4.36 fps )

## Summary for Pond D22: DMH \#22



Routed to Pond d21: DMH \#21
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=205.74' @ 12.09 hrs
Flood Elev=208.46'


Primary OutFlow Max=2.55 cfs @ 12.09 hrs HW=205.72' TW=204.39' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 2.55 cfs @ 4.05 fps )

## Summary for Pond D23: DMH \#23



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=207.23' @ 12.09 hrs
Flood Elev= 210.30'
Device Routing Invert Outlet Devices
\#1 Primary 206.70' 15.0" Round Culvert L= 173.3' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert= 206.70' / 204.97' S=0.0100 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.23 sf
Primary OutFlow Max=1.14 cfs @ 12.09 hrs HW=207.22' TW=205.72' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 1.14 cfs @ 3.49 fps )

## Summary for Pond D25: DMH \#25

| Inflow Area = | 36,995 sf, 87.96\% Impervious, | Inflow Depth > 4.26" for 10YR event |
| :---: | :---: | :---: |
| Inflow | 3.74 cfs @ 12.09 hrs , Volume= | 13,119 cf |
| Outflow | 3.74 cfs @ 12.09 hrs , Volume= | $13,119 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 3.74 cfs @ 12.09 hrs , Volume= | 13,119 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=210.49' @ 12.09 hrs
Flood Elev=213.11'


Primary OutFlow Max=3.64 cfs @ 12.09 hrs HW=210.47' TW=202.94' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 3.64 cfs @ 4.20 fps )

## Summary for Pond D27: DMH \#27



Routed to Pond D35 : DMH \#35
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=214.05' @ 12.09 hrs
Flood Elev=217.00'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $213.34^{\prime}$ | $15.0^{\prime \prime}$ Round Culvert $\mathrm{L}=63.9^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $213.34^{\prime} / 212.38^{\prime} \quad \mathrm{S}=0.0150$ |
|  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= $=0.93 \mathrm{sf}$ |  |

Primary OutFlow Max=2.03 cfs @ 12.09 hrs HW=214.04' TW=212.98' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 2.03 cfs @ 2.85 fps )

## Summary for Pond D28: DMH \#28

| Inflow Area = | 11 | pervious, | 3.28 " for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.01 cfs @ | 12.09 hrs , Volume= | 3,267 cf |
| Outflow | 1.01 cfs @ | 12.09 hrs , Volume= | 3,267 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.01 cfs @ | 12.09 hrs , Volume= | 3,267 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 217.98' @ 12.09 hrs
Flood Elev= 220.17'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | :--- | :--- | :--- |
| $\# 1$ | Primary | $217.46^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=158.3^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |  |
|  |  | Inlet / Outlet Invert= $217.46^{\prime} / 214.29^{\prime} \quad \mathrm{S}=0.0200$ | $\mathrm{I} / /^{\prime} \quad \mathrm{Cc}=0.900$ |  |
| $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |  |  |

Primary OutFlow Max=0.99 cfs @ 12.09 hrs HW=217.97' TW=214.04' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 0.99 cfs @ 2.44 fps )

## Summary for Pond D29: DMH \#29



Routed to Pond D28: DMH \#28
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=220.35' @ 12.09 hrs
Flood Elev= 223.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $219.83^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=150.9^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= $219.83^{\prime} / 217.55^{\prime} \quad \mathrm{S}=0.0151$ <br> $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=0.99 cfs @ 12.09 hrs HW=220.34' TW=217.97' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls $0.99 \mathrm{cfs} @ 2.44 \mathrm{fps}$ )

## Summary for Pond D30: DMH \#30



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=221.34' @ 12.09 hrs
Flood Elev= 224.95'
Device Routing Invert Outlet Devices
\#1 Primary 220.92' 12.0" Round Culvert L= 184.2' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert= 220.92' / 220.00' S=0.0050 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=0.51 cfs @ 12.09 hrs HW=221.33' TW=220.34' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.51 cfs @ 2.44 fps )

## Summary for Pond D31: DMH\#31

| Inflow Area = | 63,140 sf, | 28.73\% Impervious, | Inflow Depth > 2.13" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 2.84 cfs @ 12 | 12.16 hrs , Volume= | 11,215 cf |
| Outflow | 2.84 cfs @ 12 | 12.16 hrs , Volume= | $11,215 \mathrm{cf}$, Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pon | $2.84 \mathrm{cfs} @ 12$ | 12.16 hrs , Volume= | 11,215 cf |
| Routing by Dyn-S <br> Peak Elev= 225.4 <br> Flood Elev= 229 | r-Ind method, @ 12.16 hrs | , Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 224.63' | 15.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.012$ Corrug | $\begin{aligned} & \text { lvert } \mathrm{L}=288.5^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=224.63^{\prime} / 213.09^{\prime} \quad \mathrm{S}=0.0400 \mathrm{l} \\ & \hline \end{aligned}$ $\text { ted PP, smooth interior, Flow Area= } 1.23 \text { sf }$ |

Primary OutFlow Max=2.79 cfs @ 12.16 hrs HW=225.48' TW=211.34' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 2.79 cfs @ 3.14 fps )

## Summary for Pond D34: DMH \#34

| Inflow Area | 23,255 | 0.00\% Impervious, | Inflo | 72" for 10YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 2.53 cfs @ | 12.09 hrs , Volume= | 9,148 cf |  |
| Outflow | 2.53 cfs @ | 12.09 hrs , Volume= | 9,148 cf, | Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 2.53 cfs @ | 12.09 hrs , Volume= | 9,148 cf |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 199.01' @ 12.09 hrs
Flood Elev= 202.00'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 198.07' | 12.0" Round Culvert L=52.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 198.07' / 197.03' S=0.0200 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 0.79 sf |

Primary OutFlow Max=2.46 cfs @ 12.09 hrs HW=198.99' TW=196.02' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 2.46 cfs @ 3.26 fps )

## Summary for Pond D35: DMH \#35

| Inflow Area $=$ | $21,746 \mathrm{sf}, 79.51 \%$ Impervious, | Inflow Depth > 3.93" | for 10 YR event |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $2.08 \mathrm{cfs} @$ | 12.09 hrs , Volume= |
| Outflow | $=$ | $2.08 \mathrm{cfs} @$ | 12.09 hrs , Volume= |
| Primary | $=$ | $2.08 \mathrm{cfs} @$ | 12.09 hrs , Volume= |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 212.99' @ 12.09 hrs
Flood Elev=215.70'


Primary OutFlow Max=2.03 cfs @ 12.09 hrs HW=212.98' TW=210.47' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 2.03 cfs @ 2.85 fps )

## Summary for Pond D4: DMH\#4

| Inflow Area = | 73,240 | \% Impervious, | Inflow Depth > 2.45" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 3.67 cfs @ | 12.14 hrs , Volume= | 14,981 cf |
| Outflow | 3.67 cfs @ | 12.14 hrs , Volume= | 14,981 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 3.67 cfs @ | 12.14 hrs , Volume= | 14,981 cf |

Routed to Pond D2 : DMH\#2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=211.36' @ 12.14 hrs
Flood Elev=217.64'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 210.34' | 15.0" Round Culvert L= 222.3' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=210.34' / 207.01' S=0.0150 '// Cc= 0.900 $n=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |

Primary OutFlow Max=3.63 cfs @ 12.14 hrs HW=211.35' TW=207.98' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls 3.63 cfs @ 3.42 fps )

## Summary for Pond D5: DMH \#5



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=210.09' @ 12.09 hrs
Flood Elev= 212.97'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $209.09 '$ | 18.0" Round Culvert L= 183.0' $\quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 209.09' $/ 208.17 \prime \mathrm{~S}=0.0050 \mathrm{I} / \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.77 sf |  |

Primary OutFlow Max=3.21 cfs @ 12.09 hrs HW=210.07' TW=209.02' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 3.21 cfs @ 3.72 fps )

## Summary for Pond D6: DMH \#6

| Inflow Area = | 30,441 sf, 9 | 96.76\% Impervious, | Inflow Depth > 4.66" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 3.30 cfs @ 12 | 12.09 hrs , Volume= | 11,817 cf |
| Outflow = | 3.30 cfs @ 12. | 12.09 hrs , Volume= | $11,817 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary = Routed to Pond | $\begin{aligned} & 3.30 \mathrm{cfs} @ 12 \\ & \text { D7 : DMH \#7 } \end{aligned}$ | 12.09 hrs , Volume= | 11,817 cf |
| Routing by Dyn-S <br> Peak Elev= 209.03 <br> Flood Elev= 214 | or-Ind method, ' @ 12.09 hrs ' | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | $t$ Outlet Devices |  |
| \#1 Primary | $208.07 \text { ' }$ | 18.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{array}{ll} \hline \text { Ivert } \mathrm{L}=299.7^{\prime} \quad \mathrm{Ke}=0.500 \\ t=208.077^{\prime} / 206.57^{\prime} \quad \mathrm{S}=0.0050 \end{array}$ $\text { ted PE, smooth interior, Flow Area= } 1.77 \mathrm{sf}$ |

Primary OutFlow Max=3.21 cfs @ 12.09 hrs HW=209.02' TW=207.31' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 3.21 cfs @ 3.89 fps )

## Summary for Pond D7: DMH \#7

 Routed to Pond P212 : INFILTRATION POND \#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.33' @ 12.09 hrs
Flood Elev=213.17'


Primary OutFlow Max=3.21 cfs @ 12.09 hrs HW=207.31' TW=201.62' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 3.21 cfs @ 3.13 fps)

## Summary for Pond D8: DMH \#8



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 201.54' @ 12.09 hrs
Flood Elev= 204.72'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 200.57' | 12.0" Round Culvert L=87.7' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=200.57' / 200.13' S=0.0050 '//' Cc= 0.900 $n=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |

Primary OutFlow Max=1.88 cfs @ 12.09 hrs HW=201.52' TW=200.95' (Dynamic Tailwater)
——1=Culvert (Outlet Controls 1.88 cfs @ 3.14 fps )

## Summary for Pond D9: DMH \#9



## Summary for Pond DE61: DRIP \#61

| Inflow Area = | 4,247 | 92.68\% Impervious, | Inflow Depth > 4.49" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.45 cfs @ | 12.09 hrs , Volume= | 1,589 cf |
| Outflow | 0.39 cfs @ | 12.14 hrs , Volume= | $1,445 \mathrm{cf}$, Atten $=15 \%, L a g=3.3 \mathrm{~min}$ |
| Discarded = | 0.00 cfs @ | 5.25 hrs , Volume= | 196 cf |
| Primary | 0.38 cfs @ | 12.14 hrs , Volume= | 1,249 cf | Routed to Reach 8R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=213.22' @ 12.14 hrs Surf.Area= 665 sf Storage= 273 cf
Plug-Flow detention time= 80.1 min calculated for 1,445 cf ( $91 \%$ of inflow)
Center-of-Mass det. time= 34.3 min (796.4-762.1)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | :--- |
| $\# 1$ | $212.19^{\prime}$ | 539 cf | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 212.19 | 665 | 0.0 | 0 | 0 |
| 212.20 | 665 | 40.0 | 3 | 3 |
| 214.19 | 665 | 40.0 | 529 | 532 |
| 214.20 | 665 | 100.0 | 7 | 539 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 214.10' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
| \#2 | Primary | 212.70' | Coef. (English) 2.802 .923 .083 .303 .32 |
|  |  |  | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 212.70' / 212.65' S=0.0050 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.20 sf |
| \#3 | Discarded | 212.19' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.00 cfs @ 5.25 hrs HW=212.21' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.38 cfs @ 12.14 hrs HW=213.21' TW=208.04' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.38 cfs @ 2.33 fps )

## Summary for Pond DE62: DRIP \#62

| Inflow Area = | 4,247 sf, | 92.68\% Impervious, | Inflow Depth > 4.49" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.45 cfs @ | 12.09 hrs , Volume= | 1,589 cf |
| Outflow | 0.39 cfs @ | 12.14 hrs , Volume= | 1,445 cf, Atten= 15\%, Lag= 3.3 min |
| Discarded $=$ | 0.00 cfs @ | 5.25 hrs , Volume= | 196 cf |
| Primary | 0.38 cfs @ | 12.14 hrs , Volume= | 1,249 cf | Routed to Reach 8R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=213.22' @ 12.14 hrs Surf.Area= 665 sf Storage= 273 cf
Plug-Flow detention time $=80.1$ min calculated for 1,445 cf ( $91 \%$ of inflow)
Center-of-Mass det. time= $34.3 \mathrm{~min}(796.4-762.1)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $212.19^{\prime}$ | 539 cf | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 212.19 | 665 | 0.0 | 0 | 0 |
| 212.20 | 665 | 40.0 | 3 | 3 |
| 214.19 | 665 | 40.0 | 529 | 532 |
| 214.20 | 665 | 100.0 | 7 | 539 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 214.10' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 212.70' | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 212.70' $/ 212.65$ ' S=0.0050 '/l' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.20 \mathrm{sf}$ |
| \#3 | Discarded | 212.19' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01 ' |

Discarded OutFlow Max=0.00 cfs @ 5.25 hrs HW=212.21' (Free Discharge)
$\complement_{3=\text { Exfiltration (Exfiltration Controls } 0.00 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=0.38 cfs @ 12.14 hrs HW=213.21' TW=208.04' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs )
-2=Culvert (Barrel Controls 0.38 cfs @ 2.33 fps )

## Summary for Pond DE63: DRIP \#63



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 206.99 | 404 | 0.0 | 0 | 0 |
| 207.00 | 404 | 40.0 | 2 | 2 |
| 208.99 | 404 | 40.0 | 322 | 323 |
| 209.00 | 404 | 100.0 | 4 | 327 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 208.90' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
| \#2 | Primary | 207.50' | Coef. (English) 2.802 .923 .083 .303 .32 |
|  |  |  | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 207.50' / 207.45' S=0.0050 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.20 sf |
| \#3 | Discarded | 206.99' | 0.170 in/hr Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.00 cfs @ 5.45 hrs HW=207.01' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.28 cfs @ 12.12 hrs HW=207.92' TW=202.08' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
-2=Culvert (Barrel Controls 0.28 cfs @ 2.15 fps )

## Summary for Pond DE64: DRIP \#64

| Inflow Area = | 3,470 sf, | .59\% Impervious, | Depth > | 49" for 10YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 0.37 cfs @ | 12.09 hrs , Volume= | 1,298 cf |  |
| Outflow | 0.33 cfs @ | 12.13 hrs , Volume $=$ | 1,197 cf, | Atten= 11\%, Lag= 2.5 min |
| Discarded | 0.00 cfs @ | 4.85 hrs , Volume= | 140 cf |  |
| Primary | 0.33 cfs @ | 12.13 hrs , Volume= | 1,057 cf |  |

Routed to Reach 12R : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.97' @ 12.13 hrs Surf.Area= 470 sf Storage= 184 cf
Plug-Flow detention time= 71.7 min calculated for 1,197 cf ( $92 \%$ of inflow)
Center-of-Mass det. time= $30.6 \mathrm{~min}(792.8-762.1)$


Discarded OutFlow Max=0.00 cfs @ 4.85 hrs HW=205.01' (Free Discharge)
$L^{-}=$Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.32 cfs @ 12.13 hrs HW=205.96' TW=202.08' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
$\mathbf{2 = C u l v e r t}$ (Barrel Controls 0.32 cfs @ 2.23 fps )

## Summary for Pond DE65: DRIP \#65

| Inflow Area = | 3,016 | 88.69\% Impervious, | Inflow Depth > 4.38" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.32 cfs @ | 12.09 hrs , Volume= | 1,100 cf |
| Outflow | 0.29 cfs @ | 12.12 hrs , Volume= | $1,013 \mathrm{cf}$, Atten= 9\%, Lag= 2.2 min |
| Discarded | 0.00 cfs @ | 5.45 hrs , Volume= | 117 cf |
| Primary | 0.29 cfs @ | 12.12 hrs , Volume= | 896 cf | Routed to Reach 12R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.93' @ 12.12 hrs Surf.Area= 404 sf Storage= 152 cf
Plug-Flow detention time $=69.6 \mathrm{~min}$ calculated for $1,011 \mathrm{cf}$ ( $92 \%$ of inflow)
Center-of-Mass det. time $=29.0 \mathrm{~min}(797.0-768.0)$


Discarded OutFlow Max=0.00 cfs @ 5.45 hrs HW=206.01' (Free Discharge)
L3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.28 cfs @ 12.12 hrs HW=206.92' TW=202.08' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs )
-2=Culvert (Barrel Controls 0.28 cfs @ 2.15 fps )

## Summary for Pond DE66: DRIP \#66

| Inflow Area = | 3,407 sf, | 46\% Impervious, | Inflow Depth > 4.49" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.36 cfs @ | 12.09 hrs , Volume= | 1,275 cf |
| Outflow | 0.33 cfs @ | 12.13 hrs , Volume= | 1,173 cf, Atten= 11\%, Lag= 2.5 min |
| Discarded = | 0.00 cfs @ | 4.10 hrs , Volume= | 140 cf |
| Primary | 0.32 cfs @ | 12.13 hrs , Volume= | 1,033 cf | Routed to Reach 12R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 208.76' @ 12.13 hrs Surf.Area= 470 sf Storage= 183 cf
Plug-Flow detention time= 71.9 min calculated for 1,171 cf ( $92 \%$ of inflow)
Center-of-Mass det. time= 30.9 min (793.0-762.1)

| Volume | Invert207.79 | t Avail.Storage |  | Storage Description |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| \#1 |  | ' 381 cf Custom Stage Data (Prismatic)Listed below (Recalc) |  |  |  |
| $\begin{array}{r} \text { Elevation } \\ \text { (feet) } \end{array}$ |  | Surf.Area (sq-ft) | Voids (\%) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
| 207.79 |  | 470 | 0.0 | 0 | 0 |
| 207.80 |  | 470 | 40.0 | 2 | 2 |
| 209.79 |  | 470 | 40.0 | 374 | 376 |
| 209.80 |  | 470 | 100.0 | 5 | 381 |
| Device R | Routing | Invert Outlet Devices |  |  |  |
| \#1 | Primary | 209.7 | $0^{\prime} \quad 180.0$ ' long x 0.5' breadth Broad-Crested Rectangular Weir <br> Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |  |  |
| \#2 | Primary | 208 | ( 6.0' Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |  |  |
|  |  |  | Inlet / Outlet Invert= 208.30' 208.25 ' S=0.0050 '/' Cc= 0.900 |  |  |
| \#3 | Discarded | d 207.79' 0.1 | $9^{\prime} \quad 0.170$ in/hr Exfiltration over Surface area Phase-In= 0.01 ' |  |  |

Discarded OutFlow Max=0.00 cfs @ 4.10 hrs HW=207.80' (Free Discharge)
$L^{-} 3=$ Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.32 cfs @ 12.13 hrs HW=208.76' TW=202.08' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.32 cfs @ 2.22 fps )

## Summary for Pond DE67: DRIP \#67



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 207.99 | 470 | 0.0 | 0 | 0 |
| 208.00 | 470 | 40.0 | 2 | 2 |
| 209.99 | 470 | 40.0 | 374 | 376 |
| 210.00 | 470 | 100.0 | 5 | 381 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| \#1 | Primary | $209.90 '$ | 180.0' long $\times$ 0.5' breadth Broad-Crested Rectangular Weir |
|  |  |  | Head (feet) 0.20 $0.400 .600 .80 \quad 1.00$ |
| Coef. (English) $2.80 \quad 2.923 .083 .303 .32$ |  |  |  |

Discarded OutFlow Max=0.00 cfs @ 4.85 hrs HW=208.01' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.32 cfs @ 12.13 hrs HW=208.96' TW=208.04' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.32 cfs @ 2.23 fps )

## Summary for Pond DE68: DRIP \#68

| Inflow Area = | 4,212 sf, | \%\% Impervious, | Inflow Depth > 4.49" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.45 cfs @ | 12.09 hrs , Volume= | 1,576 cf |
| Outflow | 0.40 cfs @ | 12.13 hrs , Volume= | 1,434 cf, Atten= 12\%, Lag $=2.8 \mathrm{~min}$ |
| Discarded = | 0.00 cfs @ | 5.25 hrs , Volume= | 196 cf |
| Primary | 0.39 cfs @ | 12.13 hrs , Volume= | 1,238 cf | Routed to Pond OCS4 : OCS\#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.92' @ 12.13 hrs Surf.Area= 665 sf Storage= 248 cf
Plug-Flow detention time $=77.4 \mathrm{~min}$ calculated for $1,431 \mathrm{cf}$ ( $91 \%$ of inflow)
Center-of-Mass det. time= 32.5 min (794.6-762.1)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | :--- |
| $\# 1$ | $206.99^{\prime}$ | 539 cf | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 206.99 | 665 | 0.0 | 0 | 0 |
| 207.00 | 665 | 40.0 | 3 | 3 |
| 208.99 | 665 | 40.0 | 529 | 532 |
| 209.00 | 665 | 100.0 | 7 | 539 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 208.90' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 207.50' | 6.0" Round Culvert L=20.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 207.50' / 206.00' S=0.0750 '/l' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.20 \mathrm{sf}$ |
| \#3 | Discarded | 206.99' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01 ' |

Discarded OutFlow Max=0.00 cfs @ 5.25 hrs HW=207.01' (Free Discharge)
$L_{3=\text { Exfiltration (Exfiltration Controls } 0.00 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=0.39 cfs @ 12.13 hrs HW=207.92' TW=204.29' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Inlet Controls 0.39 cfs @ 2.20 fps )

## Summary for Pond DE69: DRIP \#69



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 205.49 | 470 | 0.0 | 0 | 0 |
| 205.50 | 470 | 40.0 | 2 | 2 |
| 207.49 | 470 | 40.0 | 374 | 376 |
| 207.50 | 470 | 100.0 | 5 | 381 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 207.40' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 206.00' | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=206.00' 205.95 ' S=0.0050 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.20 sf |
| \#3 | Discarded | 205.49' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In=0.01' |

Discarded OutFlow Max=0.00 cfs @ 4.85 hrs HW=205.51' (Free Discharge)
$L_{3}=$ Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.32 cfs @ 12.13 hrs HW=206.46' TW=201.82' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.32 cfs @ 2.23 fps )

## Summary for Pond DE70: DRIP \#70

| Inflow Area = | 3,476 sf, | pervious, | Depth > | 49" for 10YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 0.37 cfs @ | 12.09 hrs , Volume= | 1,301 cf |  |
| Outflow | 0.33 cfs @ | 12.13 hrs , Volume= | 1,199 cf, | Atten= 11\%, Lag= 2.5 min |
| Discarded | 0.00 cfs @ | 4.05 hrs , Volume= | 140 cf |  |
| Primary | 0.33 cfs @ | 12.13 hrs , Volume= | 1,059 cf |  | Routed to Pond P212 : INFILTRATION POND \#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.87' @ 12.13 hrs Surf.Area= 470 sf Storage= 184 cf
Plug-Flow detention time $=71.7 \mathrm{~min}$ calculated for 1,199 cf ( $92 \%$ of inflow)
Center-of-Mass det. time= 30.6 min (792.7-762.1)


Discarded OutFlow Max=0.00 cfs @ 4.05 hrs HW=205.90' (Free Discharge)
$L^{-}=$Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.32 cfs @ 12.13 hrs HW=206.86' TW=201.82' (Dynamic Tailwater)

- 1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
$\mathbf{2 = C u l v e r t}$ (Barrel Controls 0.32 cfs @ 2.23 fps )


## Summary for Pond DE71: DRIP \#71

| Inflow Area = | 4,210 | 92.61\% Impervious, | Inflow Depth > 4.49" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.45 cfs @ | 12.09 hrs , Volume= | 1,575 cf |
| Outflow | 0.38 cfs @ | 12.14 hrs , Volume= | 1,431 cf, Atten= 15\%, Lag $=3.3 \mathrm{~min}$ |
| Discarded | 0.00 cfs @ | 5.75 hrs , Volume= | 196 cf |
| Primary | 0.38 cfs @ | 12.14 hrs , Volume= | 1,235 cf | Routed to Pond P212 : INFILTRATION POND \#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.51' @ 12.14 hrs Surf.Area= 665 sf Storage= 273 cf
Plug-Flow detention time $=79.8 \mathrm{~min}$ calculated for 1,428 cf ( $91 \%$ of inflow)
Center-of-Mass det. time= 34.4 min (796.5-762.1)


Discarded OutFlow Max=0.00 cfs @ 5.75 hrs HW=206.52' (Free Discharge)
L3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.38 cfs @ 12.14 hrs HW=207.51' TW=201.88' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
-2=Culvert (Barrel Controls 0.38 cfs @ 2.33 fps )

## Summary for Pond DECH: DRIP \#CH

| Inflow Area | 6,262 s | 92.70\% Impervious, | Depth > | 49" for 10YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 0.67 cfs @ | 12.09 hrs , Volume= | 2,343 cf |  |
| Outflow | 0.42 cfs @ | 12.19 hrs , Volume= | 2,343 cf, | Atten $=38 \%$, Lag $=6.5 \mathrm{~min}$ |
| Discarded = | 0.04 cfs @ | 10.75 hrs , Volume= | 1,427 cf |  |
| Primary | 0.38 cfs @ | 12.19 hrs , Volume= | 916 cf |  | Routed to Pond CB18 : CB \#18

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 209.49' @ 12.19 hrs Surf.Area= 636 sf Storage= 382 cf
Plug-Flow detention time= 20.3 min calculated for 2,343 cf ( $100 \%$ of inflow)
Center-of-Mass det. time= 20.2 min (782.3-762.1)

| Volume | Invert | t Avail.Storage |  | Storage Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| \#1 | 207.99 | ' 770 cf |  | Custom Stage Data (Prismatic)Listed below (Recalc) |  |  |
| Elevation (feet) |  | Surf.Area (sq-ft) | Voids (\%) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |  |
| 207.99 |  | 636 | 0.0 | 0 | 0 |  |
| 208.00 |  | 636 | 40.0 | 3 | 3 |  |
| 210.99 |  | 636 | 40.0 | 761 | 763 |  |
| 211.00 |  | 636 | 100.0 | 6 | 770 |  |
| Device | Routing | Invert Outlet Devices |  |  |  |  |
| \#1 | Primary | 210.90' 16 |  | 160.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.200 .400 .600 .80 \quad 1.00$ |  |  |
| \#2 | Primary | 208.50' 4. |  | 4.0" Round Culvert L=80.0' $\mathrm{Ke}=0.500$ |  |  |
|  |  |  | $\begin{aligned} & \text { Inlet } \\ & \mathrm{n}=0 \end{aligned}$ | nlet / Outlet Invert= 208.50' / 205.10' S=0.0425 '// Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.09 sf |  |  |
| \#3 | Discarded |  | .99' 2.410 | $0 \mathrm{in} / \mathrm{hr}$ Exfiltr | over Surfac | ce area Phase-In=0.01' |

Discarded OutFlow Max=0.04 cfs @ 10.75 hrs HW=208.02' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.04 cfs)
Primary OutFlow Max=0.38 cfs @ 12.19 hrs HW=209.49' TW=205.42' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Inlet Controls 0.38 cfs @ 4.37 fps )

## Summary for Pond DMH32: DMH \#32



## Summary for Pond OCS1: OCS\#1

| Inflow | 48,573 | 81.99\% Impervious, | Inflow Depth > 4.32" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 5.03 cfs @ | 12.09 hrs , Volume= | 17,471 cf |
| Outflow | 5.03 cfs @ | 12.09 hrs , Volume= | $17,471 \mathrm{cf}$, Atten $=0 \%$ Lag $=0.0 \mathrm{~min}$ |
| Primary | 5.03 cfs @ | 12.09 hrs , Volume= | 17,471 cf |

Routed to Pond P206 : STORMTECH INFILTRATION SYSTEM \#2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev= 196.14' @ 12.32 hrs
Flood Elev= 201.48'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $195.00^{\prime}$ | $\mathbf{2 4 . 0} \mathbf{0}^{\prime \prime}$ Vert. Orifice/Grate $\quad$ C= $0.600 \quad$ Limited to weir flow at low heads |

Primary OutFlow Max=4.92 cfs @ 12.09 hrs HW=196.02' TW=195.61' (Dynamic Tailwater)
—1=Orifice/Grate (Orifice Controls 4.92 cfs @ 3.06 fps )

## Summary for Pond OCS3: OCS\#3



Primary OutFlow Max=5.16 cfs @ 12.09 hrs HW=204.49' TW=204.10' (Dynamic Tailwater)
L-1=Orifice/Grate (Orifice Controls 5.16 cfs @ 3.02 fps)

## Summary for Pond OCS4: OCS\#4

| Inflow Area = | 17,972 sf, 28.85\% Impervious, | Inflow Depth > 2.68" for 10YR event |
| :---: | :---: | :---: |
| Inflow | 1.25 cfs @ 12.10 hrs , Volume= | 4,007 cf |
| Outflow | 1.25 cfs @ 12.10 hrs , Volume= | $4,007 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.25 cfs @ 12.10 hrs , Volume= | 4,007 cf |

Routed to Pond P204 : STORMTECH INFILTRATION SYSTEM \#1
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=204.72' @ 12.46 hrs
Flood Elev= 208.00'
Device Routing Invert Outlet Devices
\#1 Primary $\quad 203.10^{\prime} \quad 18.0^{\prime \prime}$ Vert. Orifice/Grate $\quad \mathrm{C}=0.600$ Limited to weir flow at low heads

Primary OutFlow Max=1.25 cfs @ 12.10 hrs HW=204.19' TW=204.15' (Dynamic Tailwater)
_1 $^{1}$ Orifice/Grate (Orifice Controls 1.25 cfs @ 0.91 fps )

## Summary for Pond OCS6: OCS \#6



Primary OutFlow Max=1.71 cfs @ 12.09 hrs HW=202.11' TW=201.89' (Dynamic Tailwater)
——1=Orifice/Grate (Orifice Controls 1.71 cfs @ 2.27 fps)

## Summary for Pond OCS7: OCS \#7



Primary OutFlow Max=1.65 cfs @ 12.09 hrs HW=202.48' TW=201.89' (Dynamic Tailwater)
\&1=Orifice/Grate (Orifice Controls 1.65 cfs @ 2.84 fps)

## Summary for Pond P204: STORMTECH INFILTRATION SYSTEM \#1

| Inflow Area = | 72,222 | 68.72\% Imperviou | Inflow Depth > 3.62" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 6.49 cfs @ | 12.09 hrs , Volume= | 21,764 cf |
| Outflow | 1.88 cfs @ | 12.47 hrs , Volume= | 19,565 cf, Atten $=71 \%$, Lag $=22.4 \mathrm{~min}$ |
| Discarded | 0.09 cfs @ | 9.15 hrs , Volume= | 5,743 cf |
| Primary | 1.79 cfs @ | 12.47 hrs , Volume= | 13,822 cf |

Routed to Reach 20r : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 204.72' @ 12.47 hrs Surf.Area= 5,670 sf Storage= 8,487 cf Flood Elev=208.75' Surf.Area=5,670 sf Storage= 13,379 cf

Plug-Flow detention time= 110.9 min calculated for $19,565 \mathrm{cf}$ ( $90 \%$ of inflow)
Center-of-Mass det. time $=62.5 \min (841.6-779.1)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1A | 202.50' | 5,923 cf | $77.50^{\prime} \mathrm{W} \times 67.70^{\prime} \mathrm{L} \times 4.08$ 'H STORMTECH SC-740 <br> 21,423 cf Overall $-6,615$ cf Embedded $=14,808$ cf $\times 40.0 \%$ Voids |
| \#2A | 203.08' | 6,615 cf | ADS_StormTech SC-740 +Capx 144 Inside \#1 <br> Effective Size= 44.6 "W x $30.0^{\prime \prime} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12 \mathrm{~L}=45.9 \mathrm{cf}$ Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with $0.44^{\prime}$ Overlap 144 Chambers in 16 Rows |
| \#3B | 202.50' | 427 cf | $6.25^{\prime} \mathrm{W} \times 67.70^{\prime} \mathrm{L} \times 3.50^{\prime} \mathrm{H}$ ISOLATOR ROW <br> 1,481 cf Overall -413 cf Embedded $=1,067$ cf $\times 40.0 \%$ Voids |
| \#4B | 203.00' | 413 cf | ADS_StormTech SC-740 +Capx 9 Inside \#3 <br> Effective Size $=44.6 \mathrm{~W} \mathrm{~W} \times 30.0^{\prime \prime} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12 \mathrm{~L}=45.9 \mathrm{cf}$ Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with $0.44^{\prime}$ Overlap |

13,379 cf Total Available Storage
Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.75' | 15.0" Round Culvert L=35.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 202.75' / 201.00' S=0.0500 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |
| \#2 | Device 1 | 204.75' | 4.0' long x 0.5 ' breadth Broad-Crested Rectangular Weir |
|  |  |  | Head (feet) 0.200 .400 .600 .801 .00 |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#3 | Device 1 | 203.25' | 8.0" Vert. Orifice/Grate C=0.600 Limited to weir flow at low heads |
| \#4 | Discarded | 202.50' | 0.660 in/hr Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.09 cfs @ 9.15 hrs HW=202.56' (Free Discharge)
L4=Exfiltration (Exfiltration Controls 0.09 cfs )
Primary OutFlow Max=1.79 cfs @ 12.47 hrs HW=204.71' TW=200.09' (Dynamic Tailwater)
4-1=Culvert (Passes 1.79 cfs of 6.84 cfs potential flow)
-2=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
$\square_{3}=$ Orifice/Grate (Orifice Controls 1.79 cfs @ 5.12 fps )

## Summary for Pond P205: INFILTRATION POND \#3

 Routed to Reach 18R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 206.70' @ 12.90 hrs Surf.Area= 5,363 sf Storage= 7,871 cf
Plug-Flow detention time= 232.2 min calculated for 15,605 cf ( $84 \%$ of inflow)
Center-of-Mass det. time= 164.8 min ( 986.3-821.5)


Discarded OutFlow Max=0.30 cfs @ 12.90 hrs HW=206.70' (Free Discharge)
—2=Exfiltration (Exfiltration Controls 0.30 cfs )
Primary OutFlow Max=0.40 cfs @ 12.90 hrs HW=206.70' TW=203.01' (Dynamic Tailwater)


## Summary for Pond P206: STORMTECH INFILTRATION SYSTEM \#2

| Inflow Area = | 59,746 sf, | 80.70\% Impervious, | Inflow Depth > 4.29" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 6.18 cfs @ | 12.09 hrs , Volume= | 21,341 cf |
| Outflow | 2.12 cfs @ | 12.37 hrs , Volume= | 21,338 cf, Atten $=66 \%$ Lag $=16.9 \mathrm{~min}$ |
| Discarded = | 0.49 cfs @ | 11.45 hrs , Volume= | 18,384 cf |
| Primary | 1.63 cfs @ | 12.37 hrs , Volume= | 2,954 cf | Routed to Link AP4 : ANALYSIS POINT \#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 196.10' @ 12.37 hrs Surf.Area= 6,072 sf Storage= 6,143 cf
Plug-Flow detention time $=65.3 \mathrm{~min}$ calculated for $21,338 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time=65.2 min ( 832.6-767.4)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1A | 194.67' | 1,786 cf | 39.50 'W x 53.46'L x $3.33^{\prime}$ H FIELD A <br> 7,038 cf Overall $-2,573$ cf Embedded $=4,466$ cf $\times 40.0 \%$ Voids |
| \#2A | 195.00' | 2,573 cf | ADS_StormTech SC-740 +Capx 56 Inside \#1 Effective Size $=44.6^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12^{\prime} \mathrm{L}=45.9 \mathrm{cf}$ Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with 0.44 ' Overlap 56 Chambers in 8 Rows |
| \#3B | 194.67' | 3,296 cf | $58.50^{\prime} \mathrm{W}$ x $67 . \mathbf{7 0}^{\prime} \mathrm{L} \times 3.33^{\prime} \mathrm{H}$ FIELD B <br> 13,201 cf Overall - 4,962 cf Embedded $=8,239$ cf $\times 40.0 \%$ Voids |
| \#4B | 195.00' | 4,962 cf | ADS_StormTech SC-740 +Capx 108 Inside \#3 Effective Size $=44.6^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12^{\prime} \mathrm{L}=45.9 \mathrm{cf}$ Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with 0.44 ' Overlap 108 Chambers in 12 Rows |

Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 194.65' | 18.0" Round Culvert L=30.0' $\mathrm{Ke}=0.200$ |
|  |  |  | Inlet / Outlet Invert= 194.65' / 194.50' S=0.0050 '/l Cc= 0.900 $n=0.013$ Corrugated PE, smooth interior, Flow Area= 1.77 sf |
| \#2 | Device 1 | 195.85' | 4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) |
| \#3 | Discarded | 194.67' | $3.500 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01 ' |

Discarded OutFlow Max=0.49 cfs @ 11.45 hrs HW=194.71' (Free Discharge)
${ }^{-} 3=$ Exfiltration (Exfiltration Controls 0.49 cfs )
Primary OutFlow Max=1.62 cfs @ 12.37 hrs HW=196.10' TW=0.00' (Dynamic Tailwater)
$L_{1}=$ Culvert (Passes 1.62 cfs of 6.17 cfs potential flow)


## Summary for Pond P207: INFILTRATION POND \#2



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: |
| 196.80 | 10,100 | 0 | 0 |
| 198.00 | 12,000 | 13,260 | 13,260 |
| 200.00 | 15,000 | 27,000 | 40,260 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 198.80' | 20.0' long x 21.0' breadth Broad-Crested Rectangular Weir Head (feet) $0.200 .40 \quad 0.60 \quad 0.801 .001 .201 .401 .60$ |
|  |  |  | Coef. (English) 2.682 .702 .702 .642 .632 .642 .642 .63 |
| \#2 | Primary | 194.75' | 15.0" Round Culvert L= 40.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 194.75' / 194.55' S=0.0050 '//' Cc=0.900 $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |
| \#3 | Device 2 | 198.80' | 6.0 x 6.0" Horiz. Orifice/Grate X 6.00 columns |
|  |  |  | X 6 rows $\mathrm{C}=0.600$ in 48.0" $\times 48.0$ " Grate ( $56 \%$ open area) |
|  |  |  | Limited to weir flow at low heads |
| \#4 | Device 2 | 197.40' | 8.0" Vert. Orifice/Grate $\mathrm{C}=0.600$ Limited to weir flow at low heads |
| \#5 | Discarded | 196.80' | 3.690 in/hr Exfiltration over Surface area Phase-In= $0.01{ }^{\prime}$ |

Discarded OutFlow Max=1.03 cfs @ 12.60 hrs HW=198.06' (Free Discharge) -5=Exfiltration (Exfiltration Controls 1.03 cfs)

Primary OutFlow Max=0.96 cfs @ 12.60 hrs HW=198.06' TW=192.18' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs)
-2=Culvert (Passes 0.96 cfs of 9.67 cfs potential flow)

- $3=$ Orifice/Grate ( Controls 0.00 cfs )

4=Orifice/Grate (Orifice Controls 0.96 cfs @ 2.76 fps)

## Summary for Pond P210: POCKET WETLAND \#1



| Elevation <br> (feet) | Surf.Area <br> $(\mathrm{sq}$-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: |
| 199.00 | 218 | 0 | 0 |
| 200.00 | 294 | 256 | 256 |
| 201.00 | 376 | 335 | 591 |
| 202.00 | 3,991 | 2,184 | 2,775 |
| 204.00 | 8,073 | 12,064 | 14,839 |
| 206.00 | 13,272 | 21,345 | 36,184 |
| 206.50 | 14,753 | 7,006 | 43,190 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 205.10' | 20.0' long x 15.0' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.400 .60 \quad 0.801 .001 .201 .401 .60$ |
|  |  |  | Coef. (English) 2.682 .702 .702 .642 .632 .642 .642 .63 |
| \#2 | Primary | 202.25' | 12.0" Round Culvert L=44.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 202.25' / 202.03' S=0.0050 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| \#3 | Device 2 | 202.30' | 2.5" Vert. Orifice/Grate $\mathrm{C}=0.600$ Limited to weir flow at low heads |
| \#4 | Device 2 | 204.50' | 6.0 " x 6.0" Horiz. Orifice/Grate X 6.00 columns |
|  |  |  | X 6 rows $\mathrm{C}=0.600$ in 48.0 " $\times 48.0$ " Grate ( $56 \%$ open area) |
|  |  |  | Limited to weir flow at low heads |

Primary OutFlow Max=0.19 cfs @ 15.68 hrs HW=203.77' TW=202.05' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs)
$-2=C u l v e r t$ (Passes 0.19 cfs of 3.19 cfs potential flow)

- $3=$ Orifice/Grate (Orifice Controls 0.19 cfs @ 5.63 fps )

4=Orifice/Grate ( Controls 0.00 cfs )

## Summary for Pond P212: INFILTRATION POND \#1



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) | Wet.Area <br> (sq-ft) |
| ---: | ---: | ---: | ---: | ---: |
| 200.00 | 4,354 | 0 | 0 | 4,354 |
| 201.00 | 9,360 | 6,699 | 6,699 | 9,368 |
| 202.00 | 10,993 | 10,166 | 16,865 | 11,040 |
| 204.00 | 13,976 | 24,909 | 41,774 | 14,126 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.50' | 25.0' long x 20.0' breadth Broad-Crested Rectangular |
|  |  |  | Head (feet) 0.200 .400 .600 .801 .001 .201 .401 .60 |
|  |  |  | Coef. (English) 2.682 .702 .702 .642 .632 .642 .642 .63 |
| \#2 | Discarded | 200.00' | $5.130 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= $0.01^{\prime}$ |

Discarded OutFlow Max=1.42 cfs @ 12.51 hrs HW=202.65' (Free Discharge)

Primary OutFlow Max=3.91 cfs @ 12.51 hrs HW=202.65' TW=200.07' (Dynamic Tailwater)
L-1=Broad-Crested Rectangular Weir(Weir Controls 3.91 cfs @ 1.04 fps)

## Summary for Pond P213: Stormtech Infiltration System \#3



Storage Group A created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.30' | 12.0" Round Culvert L=60.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 202.30' / 202.00' S=0.0050 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area= 0.79 sf |
| \#2 | Discarded | 200.95' | $5.130 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In=0.01' |
| \#3 | Device 1 | 204.25' | 4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) |

\#4 Device $1 \quad$ 203.35' 6.0" W x 4.0" H Vert. Orifice/Grate C= 0.600
Limited to weir flow at low heads

```
Discarded OutFlow Max=0.39 cfs @ 11.65 hrs HW=200.99' (Free Discharge)
L2=Exfiltration (Exfiltration Controls 0.39 cfs )
Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=200.95' TW=200.00' (Dynamic Tailwater)
-1 \(=\) Culvert ( Controls 0.00 cfs)
    - \(3=\) Sharp-Crested Rectangular Weir ( Controls 0.00 cfs)
    4=Orifice/Grate (Controls 0.00 cfs )
```


## Summary for Pond P214: STORMTECH INFILTRATION SYSTEM \#4

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=3)


6,797 cf Total Available Storage
Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard
Device Routing Invert Outlet Devices
\#1 Primary 201.00' 12.0" Round Culvert L=25.0' Ke= 0.500
Inlet / Outlet Invert= 201.00' / 200.88' S=0.0048 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
\#2 Discarded 200.00' $8.280 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01'
\#3 Device $1 \quad 203.40$ 4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
\#4 Device 1 202.40' 8.0" Vert. Orifice/Grate $\mathrm{C}=0.600$ Limited to weir flow at low heads
Discarded OutFlow Max=0.61 cfs @ 11.75 hrs HW=200.05' (Free Discharge)
L2=Exfiltration (Exfiltration Controls 0.61 cfs)


Summary for Link AP1: ANALYSIS POINT 1

| Inflow Area $=$ | $9,943 \mathrm{sf}$, 92.79\% Impervious, | Inflow Depth > 4.38" | for 10 YR event |  |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | 1.05 cfs @ | 12.09 hrs , Volume $=$ | $3,627 \mathrm{cf}$ |
| Primary | $=$ | 1.05 cfs @ | 12.09 hrs , Volume $=$ | $3,627 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Primary outflow $=$ Inflow, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$

## Summary for Link AP2: ANALYSIS POINT 2

| Inflow Area $=$ | $816,898 \mathrm{sf}$, | $39.51 \%$ Impervious, | Inflow Depth > 2.32" | for 10 YR event |
| :--- | ---: | ---: | ---: | ---: |
| Inflow | $=$ | $18.24 \mathrm{cfs} @$ | 12.40 hrs , Volume | $157,893 \mathrm{cf}$ |
| Primary | $=$ | $18.24 \mathrm{cfs} @$ | 12.40 hrs , Volume $=$ | $157,893 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP3: ANALYSIS POINT 3



Primary outflow $=$ Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP4: ANALYSIS POINT \#4

| ow Area = | 1,691,659 | 25.34\% Impervious | $w$ Depth > 1.27" for 10YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 25.19 cfs @ | 12.53 hrs , Volume= | 178,860 cf |
| Primary | 25.19 cfs @ | 12.53 hrs , Volume= | 178,860 cf, Atten= 0\%, Lag= 0.0 m |

Primary outflow $=$ Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Time span=0.00-24.00 hrs, $\mathrm{dt}=0.05 \mathrm{hrs}, 481$ points $\times 3$
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentB1: MULTIFAMILYBLDG

## SubcatchmentB2: MULTIFAMILYBLDG

SubcatchmentB3: MULTIFAMILY

SubcatchmentC10: CB \#10

SubcatchmentC11: CB \#11

SubcatchmentC12: CB \#12

SubcatchmentC13: CB \#13

SubcatchmentC14: CB \#14

SubcatchmentC15: CB \#15

SubcatchmentC16: CB \#16

SubcatchmentC17: CB \#17

SubcatchmentC18: CB \#18

SubcatchmentC20: CB \#20

SubcatchmentC21: CB \#21

SubcatchmentC22: CB \#22

SubcatchmentC23: CB \#23

Runoff Area=23,255 sf $100.00 \%$ Impervious Runoff Depth>6.05" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=3.21 \mathrm{cfs} 11,720 \mathrm{cf}$

Runoff Area=17,561 sf $100.00 \%$ Impervious Runoff Depth $>6.05^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=2.43 cfs $8,851 \mathrm{cf}$

Runoff Area=19,981 sf $100.00 \%$ Impervious Runoff Depth>6.05" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=2.76 \mathrm{cfs} 10,070 \mathrm{cf}$

Runoff Area=6,961 sf $100.00 \%$ Impervious Runoff Depth>6.05" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.96 \mathrm{cfs} 3,508 \mathrm{cf}$

Runoff Area $=7,173$ sf $100.00 \%$ Impervious Runoff Depth $>6.05$ " $\mathrm{T}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.99 \mathrm{cfs} 3,615 \mathrm{cf}$

Runoff Area $=5,238$ sf $100.00 \%$ Impervious Runoff Depth $>6.05$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=0.72 cfs $2,640 \mathrm{cf}$

Runoff Area $=10,873$ sf $90.78 \%$ Impervious Runoff Depth $>5.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=1.49 cfs $5,267 \mathrm{cf}$

Runoff Area=12,099 sf $86.22 \%$ Impervious Runoff Depth $>5.24$ " Tc=6.0 min CN=91 Runoff=1.57 cfs $5,281 \mathrm{cf}$

Runoff Area $=6,666$ sf $100.00 \%$ Impervious Runoff Depth $>6.05$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=0.92 cfs $3,360 \mathrm{cf}$

Runoff Area=8,516 sf $64.88 \%$ Impervious Runoff Depth $>3.94$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=79$ Runoff $=0.88 \mathrm{cfs} 2,794 \mathrm{cf}$

Runoff Area $=11,836$ sf $73.87 \%$ Impervious Runoff Depth $>5.35$ " Tc=6.0 $\mathrm{min} \mathrm{CN}=92$ Runoff=1.56 cfs $5,278 \mathrm{cf}$

Runoff Area=18,591 sf $66.35 \%$ Impervious Runoff Depth $>5.13^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=90$ Runoff $=2.38 \mathrm{cfs} 7,940 \mathrm{cf}$

Runoff Area $=11,939$ sf $88.95 \%$ Impervious Runoff Depth $>5.70$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff=1.62 cfs $5,667 \mathrm{cf}$

Runoff Area $=10,174$ sf $87.04 \%$ Impervious Runoff Depth $>5.13^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=90$ Runoff $=1.30 \mathrm{cfs} 4,345 \mathrm{cf}$

Runoff Area=12,001 sf $91.62 \%$ Impervious Runoff Depth $>5.81$ " Tc=6.0 min CN=96 Runoff=1.64 cfs 5,813 cf

Runoff Area=9,694 sf 61.00\% Impervious Runoff Depth $>5.01$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=89$ Runoff $=1.22 \mathrm{cfs} 4,050 \mathrm{cf}$

SubcatchmentC24: CB \#24

## SubcatchmentC25: CB \#25

SubcatchmentC26: CB \#26

SubcatchmentC27: CB \#27

SubcatchmentC28: CB \#28

SubcatchmentC29: CB \#29

SubcatchmentC30: CB \#30

SubcatchmentC31: CB \#31

SubcatchmentC32: CB \#32

SubcatchmentC33: CB \#33

SubcatchmentC34: CB \#34

SubcatchmentC35: CB \#35

SubcatchmentC36: CB \#36

SubcatchmentC38: CB \#38

SubcatchmentC39: CB \#39

SubcatchmentC40: CB \#40

SubcatchmentC41: CB \#41

Runoff Area=7,930 sf 72.16\% Impervious Runoff Depth>5.47" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff=1.06 cfs $3,612 \mathrm{cf}$

Runoff Area=8,487 sf $80.92 \%$ Impervious Runoff Depth $>5.58$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff=1.14 cfs $3,947 \mathrm{cf}$

Runoff Area $=8,835$ sf $63.75 \%$ Impervious Runoff Depth $>5.24$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=91$ Runoff $=1.15 \mathrm{cfs} 3,856 \mathrm{cf}$

Runoff Area=6,111 sf $91.90 \%$ Impervious Runoff Depth>5.81" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=$=0.84 \mathrm{cfs} 2,960 \mathrm{cf}$

Runoff Area $=10,372$ sf $51.33 \%$ Impervious Runoff Depth $>4.90^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=88$ Runoff=1.29 cfs $4,237 \mathrm{cf}$

Runoff Area=8,495 sf $84.21 \%$ Impervious Runoff Depth $>5.58$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff $=1.14 \mathrm{cfs} 3,950 \mathrm{cf}$

Runoff Area=8,933 sf $82.40 \%$ Impervious Runoff Depth $>5.58$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff=1.20 cfs $4,154 \mathrm{cf}$

Runoff Area=16,365 sf $68.64 \%$ Impervious Runoff Depth $>5.13$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=90$ Runoff=2.10 cfs $6,989 \mathrm{cf}$

Runoff Area=12,710 sf $70.47 \%$ Impervious Runoff Depth $>5.24$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=91$ Runoff $=1.65 \mathrm{cfs} 5,548 \mathrm{cf}$

Runoff Area=5,421 sf 83.90\% Impervious Runoff Depth>5.58" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff $=0.73 \mathrm{cfs} 2,521 \mathrm{cf}$

Runoff Area $=8,622$ sf $80.51 \%$ Impervious Runoff Depth $>5.47$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff=1.15 cfs $3,927 \mathrm{cf}$

Runoff Area=4,149 sf $98.10 \%$ Impervious Runoff Depth>6.05" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.57 \mathrm{cfs} 2,091 \mathrm{cf}$

Runoff Area=6,622 sf $100.00 \%$ Impervious Runoff Depth>6.05" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=0.92 cfs $3,337 \mathrm{cf}$

Runoff Area=7,637 sf $100.00 \%$ Impervious Runoff Depth>6.05" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=1.06 cfs $3,849 \mathrm{cf}$

Runoff Area $=7,612$ sf $100.00 \%$ Impervious Runoff Depth $>6.05{ }^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=1.05 \mathrm{cfs} 3,836 \mathrm{cf}$

Runoff Area $=4,211$ sf $100.00 \%$ Impervious Runoff Depth $>6.05$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=0.58 cfs 2,122 cf

Runoff Area $=5,586$ sf $100.00 \%$ Impervious Runoff Depth>6.05" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff= $0.77 \mathrm{cfs} 2,815 \mathrm{cf}$

SubcatchmentC43: CB \#43

## SubcatchmentC44: CB \#44

SubcatchmentC45: CB \#45

SubcatchmentC46: CB \#46

SubcatchmentC47: CB \#47

SubcatchmentC48: CB \#48

SubcatchmentC49: CB \#49

SubcatchmentC50: CB \#50

SubcatchmentC51: CB \#51

SubcatchmentC52: CB\#52

SubcatchmentC53: CB \#53

SubcatchmentC54: CB \#54

SubcatchmentC7: CB \#5

SubcatchmentC8: CB \#8

SubcatchmentC9: CB \#9

SubcatchmentCH1: CLUBHOUSE

SubcatchmentMB1: MAIL KIOSK

Runoff Area=3,109 sf 75.36\% Impervious Runoff Depth>5.01" Tc=6.0 min CN=89 Runoff=0.39 cfs 1,299 cf

Runoff Area=1,978 sf $84.43 \%$ Impervious Runoff Depth $>5.35$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=92$ Runoff=0.26 cfs 882 cf

Runoff Area=2,465 sf $50.30 \%$ Impervious Runoff Depth $>4.04$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=80$ Runoff=0.26 cfs 830 cf

Runoff Area=4,397 sf $50.97 \%$ Impervious Runoff Depth>4.04" Tc=6.0 min CN=80 Runoff=0.47 cfs 1,481 cf

Runoff Area=3,012 sf 100.00\% Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.42 cfs 1,518 cf

Runoff Area=60,128 sf 25.16\% Impervious Runoff Depth>3.03" Flow Length=400' Tc=11.8 min CN=70 Runoff=4.02 cfs $15,183 \mathrm{cf}$

Runoff Area $=5,238$ sf $84.59 \%$ Impervious Runoff Depth $>5.58$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=94$ Runoff= $0.70 \mathrm{cfs} 2,436 \mathrm{cf}$

Runoff Area $=15,040$ sf $77.20 \%$ Impervious Runoff Depth $>5.47$ " Tc=6.0 min CN=93 Runoff=2.00 cfs 6,850 cf

Runoff Area=6,823 sf $100.00 \%$ Impervious Runoff Depth>6.05" Tc=6.0 min CN=98 Runoff=0.94 cfs $3,439 \mathrm{cf}$

Runoff Area=9,052 sf $87.14 \%$ Impervious Runoff Depth>5.70" Tc=6.0 min CN=95 Runoff=1.23 cfs $4,297 \mathrm{cf}$

Runoff Area $=7,863$ sf $86.52 \%$ Impervious Runoff Depth $>5.47$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff=1.05 cfs $3,581 \mathrm{cf}$

Runoff Area=4,821 sf 86.85\% Impervious Runoff Depth>5.24" Tc=6.0 min CN=91 Runoff=0.63 cfs 2,104 cf

Runoff Area=4,650 sf 100.00\% Impervious Runoff Depth>6.05" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=98$ Runoff=0.64 cfs 2,344 cf

Runoff Area $=5,450$ sf $88.75 \%$ Impervious Runoff Depth $>5.58$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff=0.73 cfs $2,534 \mathrm{cf}$

Runoff Area=16,307 sf $93.95 \%$ Impervious Runoff Depth>5.93" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=97$ Runoff=2.24 cfs $8,058 \mathrm{cf}$

Runoff Area=6,262 sf $92.70 \%$ Impervious Runoff Depth $>5.81^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=0.86 cfs $3,033 \mathrm{cf}$

Runoff Area=938 sf 100.00\% Impervious Runoff Depth>6.05" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=0.13 cfs 473 cf

SubcatchmentS201: SUMMER STREET Runoff Area=9,943 sf 92.79\% Impervious Runoff Depth>5.70" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff $=1.35 \mathrm{cfs} 4,720 \mathrm{cf}$

SubcatchmentS202: EXISTING WETLANDRunoff Area=432,269 sf $42.08 \%$ Impervious Runoff Depth $>3.62$ " Flow Length=856' Tc=23.2 $\mathrm{min} \quad \mathrm{CN}=76$ Runoff=26.72 cfs $130,245 \mathrm{cf}$

SubcatchmentS203: POCKET WETLAND\#1 Runoff Area=25,587 sf $0.00 \%$ Impervious Runoff Depth $>3.23$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=72$ Runoff=2.18 cfs $6,885 \mathrm{cf}$

## SubcatchmentS204: EXISTING

Runoff Area=308,203 sf $31.07 \%$ Impervious Runoff Depth $>3.52^{\prime \prime}$ Flow Length=632' Tc=22.6 min CN=75 Runoff=18.73 cfs $90,283 \mathrm{cf}$

SubcatchmentS205: ISOLATEDWETLANDRunoff Area=55,420 sf $16.57 \%$ Impervious Runoff Depth>3.03" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=70$ Runoff=4.43 cfs $14,013 \mathrm{cf}$

SubcatchmentS206: OVERLANDFLOW Runoff Area=891,295 sf $2.91 \%$ Impervious Runoff Depth>2.54" Flow Length=1,467' Tc=34.5 min CN=65 Runoff=31.85 cfs 188,907 cf

SubcatchmentS207:INFILTRATIONPOND Runoff Area=20,803 sf $0.00 \%$ Impervious Runoff Depth $>4.90^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=88$ Runoff= $2.59 \mathrm{cfs} 8,498 \mathrm{cf}$

## SubcatchmentS208: GRASS AREA

Runoff Area=13,760 sf $9.33 \%$ Impervious Runoff Depth $>3.53$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=75$ Runoff=1.28 cfs $4,045 \mathrm{cf}$

## SubcatchmentS209: WETLANDC Runoff Area=107,073 sf $0.38 \%$ Impervious Runoff Depth $>3.31$ "

 Flow Length=607' Slope=0.0150 '/' Tc=28.9 min CN=73 Runoff=5.50 cfs 29,544 cfSubcatchmentS210: INFILTRATIONPOND Runoff Area=75,890 sf $0.00 \%$ Impervious Runoff Depth $>3.83$ " Flow Length=580' Slope=0.0150 '/l' Tc=16.5 min CN=78 Runoff=5.67 cfs 24,190 cf

## SubcatchmentS211: S211

SubcatchmentS212: SWALE

## SubcatchmentS213: COURTYARD

## SubcatchmentT1: Trench Drain 1

## SubcatchmentT2: Drive Under B2

SubcatchmentTH1: TOWN HOUSE\#1

SubcatchmentTH10: TOWN HOUSE \#10

Runoff Area $=15,436$ sf $47.47 \%$ Impervious Runoff Depth $>3.94$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=79$ Runoff $=1.60 \mathrm{cfs} 5,064 \mathrm{cf}$

Runoff Area=52,768 sf $0.60 \%$ Impervious Runoff Depth $>2.55$ " Flow Length=418' $\mathrm{Tc}=23.1 \mathrm{~min} \quad \mathrm{CN}=65$ Runoff $=2.25 \mathrm{cfs} 11,217 \mathrm{cf}$

Runoff Area=21,407 sf $48.10 \%$ Impervious Runoff Depth $>3.94$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=79$ Runoff=2.21 cfs 7,022 cf

Runoff Area=11,173 sf $75.10 \%$ Impervious Runoff Depth $>5.47$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff=1.49 cfs $5,089 \mathrm{cf}$

Runoff Area=4,445 sf $64.30 \%$ Impervious Runoff Depth $>3.83$ " $\mathrm{T}=6.0 \mathrm{~min} \mathrm{CN}=78$ Runoff $=0.45 \mathrm{cfs} 1,420 \mathrm{cf}$

Runoff Area $=4,247$ sf $92.68 \%$ Impervious Runoff Depth $>5.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.58 \mathrm{cfs} 2,057 \mathrm{cf}$

Runoff Area=3,476 sf $91.60 \%$ Impervious Runoff Depth $>5.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=0.48 cfs $1,684 \mathrm{cf}$

## Subcatchment TH2: TOWN HOUSE \#2

SubcatchmentTH3: TOWN HOUSE \#3

SubcatchmentTH4: TOWN HOUSE\#4

Subcatchment TH5: TOWN HOUSE \#5

SubcatchmentTH6: TOWN HOUSE \#6

SubcatchmentTH7: TOWN HOUSE\#7

SubcatchmentTH8: TOWN HOUSE \#8

SubcatchmentTH9: TOWN HOUSE \#9

Runoff Area $=4,247$ sf $92.68 \%$ Impervious Runoff Depth $>5.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.58 \mathrm{cfs} 2,057 \mathrm{cf}$

Runoff Area=3,013 sf $88.68 \%$ Impervious Runoff Depth $>5.70$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff $=0.41 \mathrm{cfs} 1,430 \mathrm{cf}$

Runoff Area=3,470 sf $91.59 \%$ Impervious Runoff Depth $>5.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=$=0.47 \mathrm{cfs} 1,681 \mathrm{cf}$

Runoff Area=3,016 sf $88.69 \%$ Impervious Runoff Depth $>5.70$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff=$=0.41 \mathrm{cfs} 1,432 \mathrm{cf}$

Runoff Area $=3,407$ sf $91.46 \%$ Impervious Runoff Depth $>5.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.47 \mathrm{cfs} 1,650 \mathrm{cf}$

Runoff Area=3,481 sf $91.61 \%$ Impervious Runoff Depth $>5.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=0.48 cfs $1,686 \mathrm{cf}$

Runoff Area=4,212 sf $92.62 \%$ Impervious Runoff Depth $>5.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.58 \mathrm{cfs} 2,040 \mathrm{cf}$

Runoff Area=3,480 sf $91.61 \%$ Impervious Runoff Depth $>5.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.48 \mathrm{cfs} 1,686 \mathrm{cf}$

Reach 8R: OVERLANDFLOW Avg. Flow Depth=0.08' Max Vel=0.10 fps Inflow=1.38 cfs 4,854 cf $\mathrm{n}=0.400 \mathrm{~L}=563.0$ ' $\mathrm{S}=0.0213$ '/' Capacity=28.09 cfs Outflow=0.40 cfs $4,526 \mathrm{cf}$

## Reach 9R: OVERLANDFLOW

Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0 cf $\mathrm{n}=0.400$ L=211.0' $\mathrm{S}=0.0652$ '/' Capacity= 23.45 cfs Outflow=0.00 cfs 0 cf

Reach 10R: OVERLANDFLOW Avg. Flow Depth=0.25' Max Vel=0.26 fps Inflow=1.47 cfs 12,790 cf $\mathrm{n}=0.400 \mathrm{~L}=164.0$ ' $\mathrm{S}=0.0366$ '/' Capacity=17.57 cfs Outflow=1.45 cfs $12,790 \mathrm{cf}$

Reach 11R: 4x4 Open Bottom Culvert Avg. Flow Depth=1.12' Max Vel=2.11 fps Inflow=9.42 cfs 70,760 cf $48.0 " x 48.0 "$ Box Pipe $n=0.069 \quad L=30.0^{\prime} \quad S=0.0150$ '/' Capacity=42.20 cfs Outflow=9.42 cfs 70,749 cf

Reach 12R: OVERLANDFLOW Avg. Flow Depth=0.12' Max Vel=0.14 fps Inflow=1.58 cfs 5,279 cf $\mathrm{n}=0.400 \mathrm{~L}=250.0^{\prime} \mathrm{S}=0.0240 \mathrm{l} / \mathrm{C}$ Capacity=29.80 cfs Outflow=0.85 cfs 5,158 cf

Reach 14R: OVERLANDFLOW Avg. Flow Depth=0.10' Max Vel=0.13 fps Inflow=2.25 cfs 11,215 cf $\mathrm{n}=0.400 \mathrm{~L}=852.0$ ' $\mathrm{S}=0.0246$ '/' Capacity=31.55 cfs Outflow=0.65 cfs 10,062 cf

Reach 15R: OVERLANDFLOW Avg. Flow Depth=0.06' Max Vel=0.08 fps Inflow=0.23 cfs 9,701 cf $\mathrm{n}=0.400 \mathrm{~L}=300.0^{\prime} \mathrm{S}=0.0200 \mathrm{I} / \mathrm{l}$ Capacity=27.21 cfs Outflow=0.23 cfs 8,871 cf

Reach 18R: OVERLANDFLOW Avg. Flow Depth=0.11' Max Vel=0.14 fps Inflow=3.39 cfs 7,468 cf $\mathrm{n}=0.400 \mathrm{~L}=609.0^{\prime} \mathrm{S}=0.0279$ '/' Capacity=38.42 cfs Outflow=0.80 cfs 7,269 cf

Reach 23R: OVERLANDFLOW Avg. Flow Depth $=0.48^{\prime} \quad$ Max Vel $=0.30 \mathrm{fps}$ Inflow=9.42 cfs $70,749 \mathrm{cf}$ $\mathrm{n}=0.400 \mathrm{~L}=237.0^{\prime} \quad \mathrm{S}=0.0211$ '// Capacity=31.93 cfs Outflow=8.56 cfs 70,122 cf

Reach R202: OVERLANDFLOW Avg. Flow Depth=0.52' Max Vel=0.23 fps Inflow=26.73 cfs $130,223 \mathrm{cf}$ n=0.400 L=700.0' S=0.0107 '/' Capacity=42.56 cfs Outflow=13.59 cfs 124,425 cf

Reach R211: OVERLANDFLOW Avg. Flow Depth=0.45 Max Vel=0.18 fps Inflow=11.59 cfs 20,184 cf n=0.400 L=600.0' S=0.0087 '/' Capacity=14.51 cfs Outflow=3.47 cfs 19,854 cf

Reach SC1: Stream Crossing\#1 Avg. Flow Depth=0.43' Max Vel=3.86 fps Inflow=26.72 cfs 130,245 cf $192.0^{\prime \prime} \times 60.0^{\prime \prime}, \mathrm{R}=207.0$ " Arch Pipe $\mathrm{n}=0.030 \mathrm{~L}=43.1$ ' $\mathrm{S}=0.0200$ '/' Capacity=722.91 cfs Outflow=26.73 cfs $130,223 \mathrm{cf}$

Reach SC2: Stream Crossing\#2 Avg. Flow Depth=0.09' Max Vel=1.57 fps Inflow=2.25 cfs $11,217 \mathrm{cf}$ $192.0^{\prime \prime} \times 60.0^{\prime \prime}, \mathrm{R}=180.0^{\prime \prime}$ Arch Pipe $\mathrm{n}=0.030 \mathrm{~L}=36.5^{\prime} \mathrm{S}=0.0241$ '/' Capacity=768.96 cfs Outflow=2.25 cfs $11,215 \mathrm{cf}$

## Pond 1P: DMH \#33

Pond 3P: OCS \#8

## Pond 5R: TRENCH DRAIN

Pond 11P: YARD DRAIN

Pond CB10: CB \#10

Pond CB11: CB \#11

Pond CB12: CB \#12

Pond CB13: CB \#13

Pond CB14: CB \#14

Pond CB15: CB \#15

Pond CB16: CB \#16

Peak Elev=206.34' Inflow=2.21 cfs 7,907 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=46.7^{\prime} \mathrm{S}=0.0251$ '//' Outflow=2.21 cfs $7,907 \mathrm{cf}$

Peak Elev=202.09' Inflow=1.67 cfs 5,685 cf Outflow=1.67 cfs 5,685 cf

Peak Elev=198.33' Inflow=1.49 cfs 5,089 cf 8.0" Round Culvert n=0.012 L=36.0' S=0.0200 '/' Outflow=1.49 cfs $5,089 \mathrm{cf}$

Peak Elev=207.41' Storage=883 cf Inflow=2.21 cfs 7,022 cf Outflow=1.67 cfs 6,973 cf

Peak Elev=210.46' Inflow=0.96 cfs 3,508 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=33.8$ ' $\mathrm{S}=0.0050$ '/' Outflow=0.96 cfs $3,508 \mathrm{cf}$

Peak Elev=210.55' Inflow=0.99 cfs 3,615 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=26.3^{\prime} \mathrm{S}=0.0103$ '/' Outflow=0.99 cfs $3,615 \mathrm{cf}$

Peak Elev=207.11' Inflow=0.72 cfs 2,640 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=41.3^{\prime} \mathrm{S}=0.0249$ '/' Outflow=0.72 cfs $2,640 \mathrm{cf}$

Peak Elev=207.35' Inflow=1.49 cfs 5,267 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=43.7^{\prime} \mathrm{S}=0.0249$ '/' Outflow=1.49 cfs $5,267 \mathrm{cf}$

Peak Elev=201.95' Inflow=1.57 cfs 5,281 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=23.2$ ' $\mathrm{S}=0.0052$ '/' Outflow=1.57 cfs $5,281 \mathrm{cf}$

Peak Elev=201.84' Inflow=0.92 cfs 3,360 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=15.6^{\prime} \mathrm{S}=0.0051$ '/' Outflow=0.92 cfs $3,360 \mathrm{cf}$

Peak Elev=204.11' Inflow=0.88 cfs 2,794 cf 12.0" Round Culvert n=0.013 L=20.9' S=0.0067 '/' Outflow=0.88 cfs 2,794 cf

Pond CB17: CB \#17

## Pond CB18: CB \#18

Pond CB20: CB \#20

Pond CB21: CB \#21

Pond CB22: CB \#22

Pond CB23: CB \#23

Pond CB24: CB \#24

Pond CB25: CB \#25

Pond CB26: CB \#26

Pond CB27: CB \#27

Pond CB28: CB \#28

Pond CB29: CB \#29

Pond CB30: CB \#30

Pond CB31: CB \#31

Pond CB32: CB \#32

Pond CB33: CB \#33

Pond CB34: CB \#34

Peak Elev=205.78' Inflow=1.56 cfs 5,278 cf
12.0" Round Culvert $n=0.013$ L=13.8' S=0.0094 '/' Outflow=1.56 cfs 5,278 cf

Peak Elev=205.80' Inflow=2.77 cfs 9,314 cf 15.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=25.1^{\prime} \mathrm{S}=0.0052$ '/' Outflow=2.77 cfs 9,314 cf

Peak Elev=204.88' Inflow=1.62 cfs 5,667 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=30.3^{\prime} \mathrm{S}=0.0053$ '/' Outflow=1.62 cfs 5,667 cf

Peak Elev=205.05' Inflow=1.30 cfs 4,345 cf 12.0" Round Culvert n=0.013 L=26.0' S=0.0050 '/' Outflow=1.30 cfs 4,345 cf

Peak Elev=206.17' Inflow=1.64 cfs 5,813 cf 12.0" Round Culvert n=0.012 L=16.1' S=0.0050 '/' Outflow=1.64 cfs 5,813 cf

Peak Elev=206.10' Inflow=1.22 cfs 4,050 cf 12.0" Round Culvert $n=0.012$ L=16.3' $S=0.0055$ '/' Outflow=1.22 cfs $4,050 \mathrm{cf}$

Peak Elev=205.98' Inflow=1.06 cfs 3,612 cf 12.0" Round Culvert n=0.012 L=12.1' S=0.0050 '/' Outflow=1.06 cfs 3,612 cf

Peak Elev=205.96' Inflow=1.14 cfs 3,947 cf 15.0" Round Culvert n=0.012 L=11.4' S=0.0053 '/' Outflow=1.14 cfs 3,947 cf

Peak Elev=202.43' Inflow=1.15 cfs 3,856 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=42.5$ ' $\mathrm{S}=0.0052$ '/' Outflow=1.15 cfs $3,856 \mathrm{cf}$

Peak Elev=201.56' Inflow=0.84 cfs 2,960 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=18.0^{\prime} \mathrm{S}=0.0056$ '/' Outflow=0.84 cfs 2,960 cf

Peak Elev=198.60' Inflow=1.29 cfs 4,237 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=13.7$ ' $\mathrm{S}=0.0044$ '/' Outflow=1.29 cfs $4,237 \mathrm{cf}$

Peak Elev=206.22' Inflow=1.14 cfs 3,950 cf 12.0" Round Culvert $n=0.013$ L=13.5' S=0.0052 '/' Outflow=1.14 cfs 3,950 cf

Peak Elev=206.24' Inflow=1.20 cfs 4,154 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=17.5^{\prime} \mathrm{S}=0.0051$ '/' Outflow=1.20 cfs $4,154 \mathrm{cf}$

Peak Elev=205.18' Inflow=2.10 cfs 6,989 cf 12.0" Round Culvert $n=0.013$ L=16.4' $S=0.0049$ '/' Outflow=2.10 cfs 6,989 cf

Peak Elev=205.04' Inflow=1.65 cfs 5,548 cf 12.0" Round Culvert $n=0.013$ L=16.3' S=0.0049 '/' Outflow=1.65 cfs 5,548 cf

Peak Elev=205.97' Inflow=0.73 cfs 2,521 cf 12.0" Round Culvert $n=0.013$ L=11.7' $S=0.0051$ '/' Outflow=0.73 cfs 2,521 cf

Peak Elev=206.04' Inflow=1.15 cfs 3,927 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=16.5^{\prime} \mathrm{S}=0.0048$ '/' Outflow=1.15 cfs 3,927 cf

Pond CB35: CB \#35

Pond CB36: CB \#36

Pond CB38: CB \#38

Pond CB39: CB \#39

Pond CB40: CB \#40

Pond CB41: CB \#41

Pond CB43: CB \#43

Pond CB44: CB \#44

Pond CB45: CB \#45

Pond CB46: CB \#46

## Pond CB47: CB\#47

Pond CB48: CB\#48

Pond CB49: CB \#49

Pond CB50: CB \#50

Pond CB51: CB \#51

Pond CB52: CB \#52

Pond CB53: CB \#53

Peak Elev=207.50' Inflow=0.57 cfs 2,091 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=15.2$ ' $\mathrm{S}=0.0053$ '/' Outflow=0.57 cfs $2,091 \mathrm{cf}$

Peak Elev=207.64' Inflow=0.92 cfs 3,337 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=16.1^{\prime} \mathrm{S}=0.0050$ '/' Outflow=0.92 cfs $3,337 \mathrm{cf}$

Peak Elev=210.83' Inflow=1.06 cfs 3,849 cf 12.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=16.7^{\prime} \mathrm{S}=0.0048$ '/' Outflow=1.06 cfs $3,849 \mathrm{cf}$

Peak Elev=210.83' Inflow=1.05 cfs 3,836 cf 12.0" Round Culvert n=0.013 L=16.4' S=0.0049'/' Outflow=1.05 cfs 3,836 cf

Peak Elev=214.28' Inflow=0.58 cfs 2,122 cf 12.0" Round Culvert n=0.013 L=17.8' S=0.0073 '/' Outflow=0.58 cfs 2,122 cf

Peak Elev=214.43' Inflow=0.77 cfs 2,815 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=18.4$ ' $\mathrm{S}=0.0049$ '/' Outflow=0.77 cfs $2,815 \mathrm{cf}$

Peak Elev=220.52' Inflow=0.39 cfs 1,299 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=14.9$ ' $\mathrm{S}=0.0047$ '//' Outflow=0.39 cfs $1,299 \mathrm{cf}$

Peak Elev=220.49' Inflow=0.26 cfs 882 cf 12.0" Round Culvert n=0.013 L=14.9' S=0.0047 '/' Outflow=0.26 cfs 882 cf

Peak Elev=221.59' Inflow=0.26 cfs 830 cf 12.0" Round Culvert n=0.013 L=18.2' S=0.0049 '/' Outflow=0.26 cfs 830 cf

Peak Elev=221.94' Inflow=0.47 cfs $1,481 \mathrm{cf}$ 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=15.3^{\prime} \mathrm{S}=0.0052$ '/' Outflow=0.47 cfs $1,481 \mathrm{cf}$

Peak Elev=225.79' Inflow=0.42 cfs $1,518 \mathrm{cf}$ 12.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=20.9$ ' $\mathrm{S}=0.0048$ '//' Outflow=0.42 cfs $1,518 \mathrm{cf}$

Peak Elev=226.24' Inflow=4.02 cfs 15,183 cf 15.0" Round Culvert n=0.012 L=16.9' S=0.0047 '// Outflow=4.02 cfs 15,183 cf

Peak Elev=203.65' Inflow=0.70 cfs 2,436 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=15.5^{\prime} \mathrm{S}=0.0052$ '/' Outflow=0.70 cfs $2,436 \mathrm{cf}$

Peak Elev=203.87' Inflow=2.00 cfs 6,850 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=15.3^{\prime} \mathrm{S}=0.0052$ '/' Outflow=2.00 cfs $6,850 \mathrm{cf}$

Peak Elev=203.36' Inflow=0.94 cfs 3,439 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=31.4$ ' $\mathrm{S}=0.0051$ '//' Outflow=0.94 cfs $3,439 \mathrm{cf}$

Peak Elev=203.38' Inflow=1.23 cfs 4,297 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=25.5^{\prime} \mathrm{S}=0.0051$ '/' Outflow=1.23 cfs $4,297 \mathrm{cf}$

Peak Elev=203.42' Inflow=1.05 cfs 3,581 cf 12.0" Round Culvert n=0.013 L=32.0' S=0.0050 '/' Outflow=1.05 cfs 3,581 cf

Pond CB54: CB \#54

## Pond CB7: CB\#5

Pond CB8: CB\#8

Pond CB9: CB \#9

Pond D10: DMH \#10

Pond D11: DMH \#11

Pond D12: DMH \#12

Pond D13: DMH \#13

Pond D14: DMH \#14

Pond D16: DMH \#16

Pond D17: DMH \#17

Pond D18: DMH \#18

Pond D19: DMH \#19

Pond D2: DMH\#2

Pond D20: DMH \#20

Pond D21: DMH \#21

Pond D22: DMH \#22

Peak Elev=203.14' Inflow=0.63 cfs 2,104 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=36.7^{\prime} \mathrm{S}=0.0049$ '//' Outflow=0.63 cfs 2,104 cf

Peak Elev=213.04' Inflow=0.64 cfs 2,344 cf 12.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=15.1^{\prime} \mathrm{S}=0.0099$ '//' Outflow=0.64 cfs 2,344 cf

Peak Elev=214.27' Inflow=0.73 cfs 2,534 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=15.1^{\prime} \mathrm{S}=0.0099$ '//' Outflow=0.73 cfs $2,534 \mathrm{cf}$

Peak Elev=210.95' Inflow=2.24 cfs 8,058 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=19.9$ ' $\mathrm{S}=0.0196$ '/' Outflow=2.24 cfs $8,058 \mathrm{cf}$

Peak Elev=203.91' Inflow=0.88 cfs 2,794 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=15.6^{\prime} \mathrm{S}=0.0051$ '/' Outflow=0.88 cfs 2,794 cf

Peak Elev=205.64' Inflow=4.32 cfs 14,592 cf 18.0" Round Culvert $n=0.013 \mathrm{~L}=44.6^{\prime} \mathrm{S}=0.0049$ '//' Outflow=4.32 cfs 14,592 cf

Peak Elev=204.60' Inflow=2.92 cfs 10,012 cf 12.0" Round Culvert n=0.013 L=41.9' S=0.0050 '/' Outflow=2.92 cfs 10,012 cf

Peak Elev=203.59' Inflow=9.39 cfs 34,406 cf 24.0" Round Culvert n=0.013 L=60.1' S=0.0050 '/' Outflow=9.39 cfs 34,406 cf

Peak Elev=205.34' Inflow=5.06 cfs 17,421 cf 18.0" Round Culvert n=0.012 L=256.3' S=0.0050 '/' Outflow=5.06 cfs 17,421 cf

Peak Elev=205.84' Inflow=2.20 cfs 7,558 cf 15.0" Round Culvert n=0.012 L=103.5' $\mathrm{S}=0.0050$ '/' Outflow=2.20 cfs $7,558 \mathrm{cf}$

Peak Elev=201.33' Inflow=1.98 cfs 6,816 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=91.6^{\prime} \mathrm{S}=0.0312$ '//' Outflow=1.98 cfs $6,816 \mathrm{cf}$

Peak Elev=198.43' Inflow=3.27 cfs 11,053 cf 15.0" Round Culvert n=0.013 L=46.3' S=0.0099 '/' Outflow=3.27 cfs 11,053 cf

Peak Elev=206.09' Inflow=2.34 cfs 8,105 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=82.5^{\prime} \mathrm{S}=0.0092$ '/' Outflow=2.34 cfs $8,105 \mathrm{cf}$

Peak Elev=208.37' Inflow=5.38 cfs 21,579 cf 15.0" Round Culvert n=0.013 L=38.2' S=0.0099 '// Outflow=5.38 cfs 21,579 cf

Peak Elev=205.13' Inflow=2.34 cfs 8,105 cf 15.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=63.5$ ' $\mathrm{S}=0.0050$ '//' Outflow=2.34 cfs $8,105 \mathrm{cf}$

Peak Elev=204.66' Inflow=9.46 cfs $32,518 \mathrm{cf}$ 24.0" Round Culvert n=0.013 L=72.4' S=0.0050 '/' Outflow=9.46 cfs 32,518 cf

Peak Elev=205.89' Inflow=3.37 cfs 11,876 cf 15.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=134.2^{\prime} \mathrm{S}=0.0071$ '/' Outflow=3.37 cfs $11,876 \mathrm{cf}$

Prepared by Howard
HydroCAD® 10.20-3c s
Pond D23: DMH \#23

Pond D25: DMH \#25

Pond D27: DMH \#27

Pond D28: DMH \#28

Pond D29: DMH \#29

Pond D30: DMH \#30

Pond D31: DMH\#31

Pond D34: DMH \#34

Pond D35: DMH \#35

Pond D4: DMH\#4

Pond D5: DMH \#5

Pond D6: DMH \#6

Pond D7: DMH \#7

Pond D8: DMH \#8

Pond D9: DMH \#9

Pond DE61: DRIP \#61

Pond DE62: DRIP \#62

Peak Elev=207.31' Inflow=1.49 cfs 5,429 cf 15.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=173.3$ ' $\mathrm{S}=0.0100$ '/' Outflow=1.49 cfs $5,429 \mathrm{cf}$

Peak Elev=210.76' Inflow=4.84 cfs 17,114 cf 15.0" Round Culvert n=0.012 L=237.6' $\mathrm{S}=0.0050$ '//' Outflow=4.84 cfs $17,114 \mathrm{cf}$

Peak Elev=214.18' Inflow=2.73 cfs 9,429 cf 15.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=63.9$ ' $\mathrm{S}=0.0150$ '/' Outflow=2.73 cfs $9,429 \mathrm{cf}$

Peak Elev=218.08' Inflow=1.38 cfs 4,491 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=158.3^{\prime} \mathrm{S}=0.0200$ '/' Outflow=1.38 cfs $4,491 \mathrm{cf}$

Peak Elev=220.45' Inflow=1.38 cfs 4,491 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=150.9$ ' $\mathrm{S}=0.0151$ '/' Outflow=1.38 cfs $4,491 \mathrm{cf}$

Peak Elev=221.43' Inflow=0.73 cfs 2,311 cf 12.0" Round Culvert n=0.013 L=184.2' $\mathrm{S}=0.0050$ '/' Outflow=0.73 cfs 2,311 cf

Peak Elev=225.78' Inflow=4.31 cfs 16,701 cf 15.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=288.5^{\prime} \mathrm{S}=0.0400$ '/' Outflow=4.31 cfs $16,701 \mathrm{cf}$

Peak Elev=199.29' Inflow=3.21 cfs 11,720 cf 12.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=52.0$ ' $\mathrm{S}=0.0200$ '/' Outflow=3.21 cfs $11,720 \mathrm{cf}$

Peak Elev=213.12' Inflow=2.73 cfs 9,429 cf 15.0" Round Culvert n=0.012 L=171.5' S=0.0150 '/' Outflow=2.73 cfs 9,429 cf

Peak Elev=211.79' Inflow=5.38 cfs 21,579 cf 15.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=222.3^{\prime} \mathrm{S}=0.0150$ '/' Outflow=5.38 cfs $21,579 \mathrm{cf}$

Peak Elev=210.26' Inflow=4.20 cfs 15,182 cf 18.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=183.0$ ' $\mathrm{S}=0.0050$ '/' Outflow=4.20 cfs $15,182 \mathrm{cf}$

Peak Elev=209.19' Inflow=4.20 cfs 15,182 cf 18.0" Round Culvert n=0.013 L=299.7' S=0.0050 '/' Outflow=4.20 cfs 15,182 cf

Peak Elev=207.46' Inflow=4.20 cfs 15,182 cf 18.0" Round Culvert n=0.013 L=44.2' S=0.0550 '/' Outflow=4.20 cfs 15,182 cf

Peak Elev=201.79' Inflow=2.49 cfs 8,641 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=87.7^{\prime} \mathrm{S}=0.0050$ '/' Outflow=2.49 cfs $8,641 \mathrm{cf}$

Peak Elev=201.16' Inflow=2.49 cfs 8,641 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=11.9$ ' $\mathrm{S}=0.0050$ '//' Outflow=2.49 cfs $8,641 \mathrm{cf}$

Peak Elev=213.33' Storage=303 cf Inflow=0.58 cfs 2,057 cf Discarded $=0.00 \mathrm{cfs} 203 \mathrm{cf}$ Primary $=0.48 \mathrm{cfs} 1,708 \mathrm{cf}$ Outflow=0.48 cfs $1,911 \mathrm{cf}$

Peak Elev=213.33' Storage=303 cf Inflow=0.58 cfs 2,057 cf Discarded=0.00 cfs 203 cf Primary $=0.48$ cfs 1,708 cf Outflow=0.48 cfs 1,911 cf

Peak Elev=206.06' Storage=201 cf Inflow=0.47 cfs $1,681 \mathrm{cf}$ Discarded=0.00 cfs 144 cf Primary $=0.42$ cfs 1,433 cf Outflow=0.42 cfs $1,578 \mathrm{cf}$

Peak Elev=207.01' Storage=165 cf Inflow=0.41 cfs 1,432 cf Discarded=0.00 cfs 121 cf Primary $=0.37$ cfs 1,222 cf Outflow $=0.37$ cfs 1,343 cf

Peak Elev=208.85' Storage=199 cf Inflow=0.47 cfs 1,650 cf Discarded=0.00 cfs 144 cf Primary=0.41 cfs 1,403 cf Outflow=0.42 cfs 1,547 cf

Peak Elev=209.06' Storage=201 cf Inflow=0.48 cfs 1,686 cf Discarded=0.00 cfs 144 cf Primary $=0.42$ cfs 1,439 cf Outflow=0.42 cfs 1,583 cf

Peak Elev=208.02' Storage=274 cf Inflow=0.58 cfs 2,040 cf Discarded=0.00 cfs 203 cf Primary=0.49 cfs 1,694 cf Outflow=0.49 cfs $1,897 \mathrm{cf}$

Peak Elev=206.56' Storage=201 cf Inflow=0.48 cfs 1,686 cf Discarded=0.00 cfs 144 cf Primary $=0.42$ cfs 1,438 cf Outflow= 0.42 cfs $1,582 \mathrm{cf}$

Peak Elev=206.96' Storage=201 cf Inflow=0.48 cfs 1,684 cf Discarded=0.00 cfs 144 cf Primary $=0.42$ cfs 1,436 cf Outflow=0.42 cfs $1,581 \mathrm{cf}$

Peak Elev=207.62' Storage=302 cf Inflow=0.58 cfs 2,039 cf Discarded=0.00 cfs 203 cf Primary $=0.48$ cfs 1,690 cf Outflow=0.48 cfs 1,893 cf

Peak Elev=209.99' Storage=508 cf Inflow=0.86 cfs 3,033 cf Discarded $=0.04$ cfs 1,659 cf Primary $=0.42$ cfs 1,373 cf Outflow $=0.45 \mathrm{cfs} 3,033 \mathrm{cf}$

Pond DMH32: DMH \#32

Pond OCS1: OCS\#1

Pond OCS3: OCS\#3

Pond OCS4: OCS\#4

Pond OCS6: OCS \#6

Pond OCS7: OCS \#7

Peak Elev=203.60' Inflow=2.71 cfs 9,286 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=19.2^{\prime} \mathrm{S}=0.0531$ '//' Outflow=2.71 cfs $9,286 \mathrm{cf}$

Peak Elev=196.41' Inflow=6.49 cfs 22,774 cf Outflow=6.49 cfs 22,774 cf

Peak Elev=205.31' Inflow=6.75 cfs 23,442 cf Outflow=6.75 cfs 23,442 cf

Peak Elev=205.10' Inflow=1.75 cfs 5,738 cf Outflow=1.75 cfs $5,738 \mathrm{cf}$

Peak Elev=203.36' Inflow=2.21 cfs 7,907 cf Outflow=2.21 cfs 7,907 cf

Peak Elev=203.36' Inflow=2.17 cfs 7,735 cf Outflow=2.17 cfs $7,735 \mathrm{cf}$

Pond P204: STORMTECHINFILTRATION Peak Elev=205.09' Storage=9,827 cf Inflow=8.49 cfs $29,181 \mathrm{cf}$ Discarded=0.09 cfs 6,064 cf Primary= 4.31 cfs 20,716 cf Outflow=4.39 cfs 26,780 cf
Prepared by Howard Stein Hudson Associates Printed 6/19/2023 HydroCAD® 10.20-3c s/n 02930 © 2023 HydroCAD Software Solutions LLC Page 229

Pond P205: INFILTRATIONPOND \#3 Peak Elev=206.84' Storage=8,662 cf Inflow=6.85 cfs 26,643 cf
Discarded=0.31 cfs 15,146 cf Primary=3.39 cfs 7,468 cf Outflow=3.69 cfs 22,614 cf
Pond P206: STORMTECHINFILTRATION Peak Elev=196.29' Storage=6,969 cf Inflow=7.97 cfs $27,862 \mathrm{cf}$ Discarded=0.49 cfs $21,285 \mathrm{cf}$ Primary=3.67 cfs 6,574 cf Outflow=4.16 cfs 27,859 cf

Pond P207: INFILTRATIONPOND \#2 Peak Elev=198.50' Storage=19,452 cf Inflow=15.34 cfs $54,338 \mathrm{cf}$ Discarded=1.09 cfs 41,530 cf Primary=1.47 cfs 12,790 cf Outflow=2.56 cfs $54,320 \mathrm{cf}$

Pond P210: POCKET WETLAND\#1 Peak Elev=204.35' Storage=17,796 cf Inflow=7.02 cfs 23,999 cf Outflow=0.23 cfs 9,701 cf

Pond P212: INFILTRATIONPOND \#1 Peak Elev=202.81' Storage=26,231 cf Inflow=21.45 cfs 85,741 cf Discarded=1.44 cfs 64,639 cf Primary=11.59 cfs 20,184 cf Outflow=13.03 cfs 84,823 cf

Pond P213: Stormtech Infiltration System Peak Elev=203.36' Storage=5,458 cf Inflow=4.38 cfs 15,642 cf Discarded=0.39 cfs 15,640 cf Primary $=0.00$ cfs 1 cf Outflow=0.40 cfs 15,641 cf

Pond P214: STORMTECHINFILTRATION Peak Elev=202.09' Storage=4,534 cf Inflow=4.44 cfs $15,756 \mathrm{cf}$ Discarded=0.61 cfs 15,774 cf Primary $=0.00 \mathrm{cfs} 0 \mathrm{cf}$ Outflow=0.61 cfs $15,774 \mathrm{cf}$

## Link AP1: ANALYSISPOINT 1

## Link AP2: ANALYSISPOINT 2

## Link AP3: ANALYSISPOINT 3

## Link AP4: ANALYSISPOINT \#4

Inflow=1.35 cfs 4,720 cf Primary $=1.35$ cfs 4,720 cf

Inflow=28.46 cfs 229,211 cf Primary $=28.46$ cfs 229,211 cf

Inflow=4.43 cfs $14,013 \mathrm{cf}$ Primary $=4.43$ cfs $14,013 \mathrm{cf}$

Inflow=43.69 cfs $300,250 \mathrm{cf}$ Primary=43.69 cfs 300,250 cf

$$
\begin{aligned}
& \text { Total Runoff Area }=2,573,920 \mathrm{sf} \text { Runoff Volume }=764,876 \mathrm{cf} \\
& 70.09 \% \text { Pervious }=1,803,997 \mathrm{sf} \quad 29.91 \% \text { Impervious }=769,923 \mathrm{sf}
\end{aligned}
$$

## Summary for Subcatchment B1: MULTIFAMILY BLDG \#1

Runoff $=\quad 3.21$ cfs @ 12.09 hrs, Volume= $11,720 \mathrm{cf}$, Depth> 6.05"
Routed to Pond D34 : DMH \#34
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 20,156 | 98 R | Roofs, HSG CRoofs, HSG D |  |  |
|  | 3,099 | 98 R |  |  |  |
|  | 23,255 | 98 V | Weighted Average |  |  |
|  | 23,255 |  | 100.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{r} \text { c } \begin{array}{r} \text { Length } \\ \text { (feet) } \end{array} \\ \hline \end{array}$ | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |

## Summary for Subcatchment B2: MULTIFAMILY BLDG \#2

Runoff $=\quad 2.43$ cfs @ 12.09 hrs, Volume $=\quad 8,851$ cf, Depth> 6.05"
Routed to Pond OCS3 : OCS\#3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | rea (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 7,873 |  | Roofs, HSG A |  |  |
|  | 9,688 | 98 | Roofs, HSG C |  |  |
|  | $\begin{aligned} & \hline 17,561 \\ & 17,561 \end{aligned}$ | 98 | Weighted Average 100.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{r} \text { Length } \\ \text { (feet) } \\ \hline \end{array}$ | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment B3: MULTIFAMILY BUILDING \#3

Runoff $=\quad 2.76$ cfs @ 12.09 hrs, Volume $=10,070$ cf, Depth> 6.05"
Routed to Pond P214 : STORMTECH INFILTRATION SYSTEM \#4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 608 | 98 | Roofs, HSG A |
| 19,373 | 98 | Roofs, HSG C |
| 19,981 | 98 | Weighted Average |
| 19,981 |  | $100.00 \%$ Impervious Area |



Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3399 | 98 | Paved roads w/curbs \& sewers, HSG B |
| 5,703 | 98 | Paved roads w/curbs \& sewers, HSG C |
| 919 | 98 | Paved roads w/curbs \& sewers, HSG D |

## Summary for Subcatchment C11: CB \#11

Runoff $=\quad 0.99$ cfs @ 12.09 hrs, Volume= 3,615 cf, Depth> 6.05"
Routed to Pond CB11 : CB \#11
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"


## Summary for Subcatchment C12: CB \#12

Runoff $=\quad 0.72$ cfs @ 12.09 hrs, Volume $=2,640 \mathrm{cf}$, Depth> 6.05"

Routed to Pond CB12 : CB \#12
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) |  | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 5,238 | 98 P | aved road | w/curbs \& | sewers, HSG C |
|  | 5,238 |  | 00.00\% Im | pervious A |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{gathered} \begin{array}{c} \text { Length } \\ \text { (feet) } \end{array} \\ \hline \end{gathered}$ | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |

## Summary for Subcatchment C13: CB \#13

Runoff =
$=\quad 1.49$ cfs @ 12.09 hrs, Volume=
5,267 cf, Depth> 5.81"
Routed to Pond CB13 : CB \#13
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,003 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 7,547 | 98 | Paved parking, HSG C |
| 2,323 | 98 | Roofs, HSG C |

Summary for Subcatchment C14: CB \#14
Runoff $=\quad 1.57$ cfs @ 12.09 hrs, Volume= 5,281 cf, Depth> 5.24"
Routed to Pond CB14 : CB \#14
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,195 | 39 | >75\% Grass cover, Good, HSG A |  |  |
|  | 7,649 | 98 P | Paved parking, HSG A |  |  |
|  | 472 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 2,783 | 98 P | Paved parking, HSG C |  |  |
|  | 12,099 | 91 | Weighted Average |  |  |
|  | 1,667 |  | 13.78\% Pervious Area |  |  |
|  | 10,432 |  | 86.22\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) |  | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C15: CB \#15

Runoff $=\quad 0.92$ cfs @ 12.09 hrs, Volume= 3,360 cf, Depth> 6.05"

Routed to Pond CB15 : CB \#15
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & 5,000 \\ & 1,666 \end{aligned}$ | $\begin{array}{ll} \hline 98 & F \\ 98 & F \end{array}$ | Paved parking, HSG A Paved parking, HSG C |  |  |
|  | $\begin{aligned} & \hline 6,666 \\ & 6,666 \end{aligned}$ | 98 | Weighted Average 100.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |

## Summary for Subcatchment C16: CB \#16

```
Runoff = 0.88 cfs @ 12.09 hrs, Volume= 2,794 cf, Depth> 3.94"
```

Routed to Pond CB16 : CB \#16
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"


Direct Entry,
Summary for Subcatchment C17: CB \#17
Runoff $=\quad 1.56$ cfs @ 12.09 hrs, Volume $=\quad 5,278 \mathrm{cf}$, Depth> 5.35"
Routed to Pond CB17 : CB \#17
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"


Runoff $=\quad 2.38$ cfs @ 12.09 hrs, Volume= $\quad 7,940$ cf, Depth> 5.13"
Routed to Pond CB18 : CB \#18
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 6,255 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 12,336 | 98 | Paved parking, HSG C |
| 18,591 | 90 | Weighted Average |
| 6,255 |  | $33.65 \%$ Pervious Area |
| 12,336 |  | $66.35 \%$ Impervious Area |
| Tc | Length | Slope <br> (ft/ft) |
| Velocity  <br> (ft/sec) Capacity <br> (min) (cfs) |  |  |

6.0

Direct Entry,
Summary for Subcatchment C20: CB \#20
Runoff = 1.62 cfs @ 12.09 hrs, Volume=
5,667 cf, Depth> 5.70"
Routed to Pond CB20 : CB \#20
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,319 | 98 | Paved parking, HSG A |
| 1,319 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 7,301 | 98 | Paved parking, HSG C |
| 11,939 | 95 | Weighted Average |
| 1,319 |  | 11.05\% Pervious Area |
| 10,620 |  | $88.95 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ |
| ---: | ---: | ---: | ---: | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description | Direct Entry, |
| :--- |

## Summary for Subcatchment C21: CB \#21

Runoff = 1.30 cfs @ 12.09 hrs, Volume= 4,345 cf, Depth> 5.13"
Routed to Pond CB21 : CB \#21
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,319 |  | >75\% Grass cover, Good, HSG A |  |  |
|  | 7,301 | 98 P | Paved parking, HSG A |  |  |
|  | 1,554 | 98 P | Paved parking, HSG C |  |  |
|  | 10,174 | $90 \quad 1$ | Weighted Average |  |  |
|  | 1,319 |  | 12.96\% Pervious Area |  |  |
|  | 8,855 |  | 87.04\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C22: CB \#22

Runoff $=\quad 1.64$ cfs @ 12.09 hrs, Volume $=\quad 5,813 \mathrm{cf}$, Depth> 5.81"
Routed to Pond CB22 : CB \#22
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| :---: | :---: | :---: |
| 2,946 | 98 | Paved parking, HSG A |
| 177 | 74 | >75\% Grass cover, Good, HSG C |
| 2,641 | 98 | Paved parking, HSG C |
| 829 | 80 | >75\% Grass cover, Good, HSG D |
| 5,408 | 98 | Paved parking, HSG D |
| 12,001 | 96 | Weighted Average |
| 1,006 |  | 8.38\% Pervious Area |
| 10,995 |  | 91.62\% Impervious Area |
| Tc Length (min) (feet) | Slope (ft/ft) | Velocity Capacity Description (ft/sec) (cfs) |

## Summary for Subcatchment C23: CB \#23

Runoff $=\quad 1.22$ cfs @ 12.09 hrs, Volume= 4,050 cf, Depth> 5.01"

Routed to Pond CB23 : CB \#23
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG A |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 242 | 39 > |  |  |  |
|  | 3,016 | 98 P | Paved parking, HSG A |  |  |
|  | 1,267 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 218 | 98 P | Paved parking, HSG C |  |  |
|  | 2,272 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 2,679 | 98 P | Paved parking, HSG D |  |  |
|  | 9,694 | 89 | Weighted Average |  |  |
|  | 3,781 |  | 39.00\% Pervious Area |  |  |
|  | 5,913 |  | 61.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{rr} \text { c } \begin{array}{r} \text { Length } \\ \text { (feet) } \end{array} \\ \hline \end{array}$ | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C24: CB \#24

Runoff $=1.06$ cfs @ 12.09 hrs, Volume=

3,612 cf, Depth> 5.47"
Routed to Pond CB24 : CB \#24
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 5,722 | 98 P | Paved parking, HSG D |  |  |
|  | 2,208 | $80>$ | >75\% Grass cover, Good, HSG D |  |  |
|  | 7,930 | 93 V | Weighted Average |  |  |
|  | 2,208 |  | 27.84\% Pervious Area |  |  |
|  | 5,722 |  | 72.16\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C25: CB \#25

Runoff $=\quad 1.14$ cfs @ 12.09 hrs, Volume= $\quad 3,947$ cf, Depth> 5.58"
Routed to Pond CB25 : CB \#25
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG A |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 211 | 39 > |  |  |  |
|  | 519 | 98 P | Paved parking, HSG A |  |  |
|  | 15 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 300 | 98 P | Paved parking, HSG C |  |  |
|  | 1,393 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 6,049 | 98 P | Paved parking, HSG D |  |  |
|  | 8,487 | 94 | Weighted Average |  |  |
|  | 1,619 |  | 19.08\% Pervious Area |  |  |
|  | 6,868 |  | 80.92\% Im | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C26: CB \#26

Runoff $=\quad 1.15$ cfs @ 12.09 hrs, Volume= $\quad 3,856$ cf, Depth> 5.24"

Routed to Pond CB26 : CB \#26
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,203 | 80 | >75\% Grass cover, Good, HSG D |
| 5,632 | 98 | Paved parking, HSG D |

## Summary for Subcatchment C27: CB \#27

Runoff $=\quad 0.84 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume= $\quad 2,960 \mathrm{cf}$, Depth> 5.81"
Routed to Pond CB27 : CB \#27
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 98 | 39 | $>75 \%$ Grass cover, Good, HSG A |
| 131 | 98 | Paved parking, HSG A |
| 397 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 5,485 | 98 | Paved parking, HSG D |
| 6,111 | 96 | Weighted Average |
| 495 |  | 8.10\% Pervious Area |
| 5,616 |  | $91.90 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ |
| ---: | ---: | ---: | ---: | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description |  |
| :--- |
| 6.0 |

## Summary for Subcatchment C28: CB \#28

Runoff $=\quad 1.29$ cfs @ 12.09 hrs, Volume $=4,237$ cf, Depth> 4.90"
Routed to Pond CB28 : CB \#28
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN | >75\% Grass cover, Good, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,751 | 74 |  |  |  |
|  | 2,841 | 98 | Paved parking, HSG C |  |  |
|  | 2,297 | 80 | >75\% Grass cover, Good, HSG D |  |  |
|  | 2,483 | 98 | Paved parking, HSG D |  |  |
|  | 10,372 | 88 | Weighted Average |  |  |
|  | 5,048 |  | 48.67\% Pervious Area |  |  |
|  | 5,324 |  | 51.33\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C29: CB \#29

Runoff $=\quad 1.14$ cfs @ 12.09 hrs, Volume= $\quad 3,950 \mathrm{cf}$, Depth> 5.58"
Routed to Pond CB29: CB \#29
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| $\begin{array}{r} \text { Area (sf) } \\ \hline 1,341 \end{array}$ |  | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 74 |  |  |  |
|  | 5,330 | 98 P | >75\% Grass cover, Good, HSG C Paved parking, HSG C |  |  |
|  | 1,824 | 98 R | Roofs, HSG |  |  |
|  | 8,495 | 94 | Weighted | verage |  |
|  | 1,341 |  | 15.79\% Pe | vious Area |  |
|  | 7,154 |  | 84.21\% Im | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |

## Summary for Subcatchment C30: CB \#30

Runoff $=\quad 1.20$ cfs @ 12.09 hrs, Volume= 4,154 cf, Depth> 5.58"

Routed to Pond CB30 : CB \#30
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,572 | 74 |  |  |  |
|  | 6,310 | 98 P | Paved parking, HSG C |  |  |
|  | 1,051 | 98 | Roofs, HSG C |  |  |
|  | 8,933 | 94 | Weighted Average |  |  |
|  | 1,572 |  | 17.60\% Pervious Area |  |  |
|  | 7,361 |  | 82.40\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C31: CB \#31

Runoff $=\quad 2.10 \mathrm{cfs}$ @ 12.09 hrs, Volume= $\quad 6,989 \mathrm{cf}$, Depth> 5.13"
Routed to Pond CB31 : CB \#31
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 5,132 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 9,132 | 98 P | Paved parking, HSG C |  |  |
|  | 2,101 | 98 R | Roofs, HSG C |  |  |
|  | 16,365 | 90 | Weighted Average |  |  |
|  | 5,132 |  | 31.36\% Pervious Area |  |  |
|  | 11,233 |  | 68.64\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ (\mathrm{cfs}) \\ \hline \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment C32: CB \#32

Runoff $=\quad 1.65$ cfs @ 12.09 hrs, Volume $=\quad 5,548 \mathrm{cf}$, Depth> 5.24"
Routed to Pond CB32 : CB \#32
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) |  | CN D | $>75 \%$ Grass cover, Good, HSG CPaved parking, HSG CRoofs, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,753 | $\begin{aligned} & \hline 74 \\ & 98 \\ & 98 \\ & \hline \end{aligned}$ |  |  |  |
|  | 7,068 |  |  |  |  |
|  | 1,889 |  |  |  |  |
|  | 12,710 | 91 V | Weighted A | verage |  |
|  | 3,753 |  | 29.53\% Per | vious Area |  |
|  | 8,957 |  | 70.47\% Imp | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C33: CB \#33

Runoff $=\quad 0.73$ cfs @ 12.09 hrs, Volume= $\quad 2,521 \mathrm{cf}$, Depth> 5.58"
Routed to Pond CB33 : CB \#33
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

| $\begin{array}{r} \text { Area (sf) } \\ \hline 873 \end{array}$ |  | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 74 |  |  |  |
|  | 3,693 | 98 P | >75\% Grass cover, Good, HSG CPaved parking, HSG C |  |  |
|  | 855 | 98 R | Roofs, HSG |  |  |
|  | $\begin{array}{r} 5,421 \\ 873 \\ 4,548 \end{array}$ | $94 \begin{array}{r}1 \\ \\ \\ 8\end{array}$ | Weighted <br> 16.10\% Pe <br> 83.90\% Im | verage vious Area ervious A |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ |  | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment C34: CB \#34

Runoff $=\quad 1.15$ cfs @ 12.09 hrs, Volume $=\quad 3,927$ cf, Depth> 5.47"
Routed to Pond CB34 : CB \#34
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,680 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 5,115 | 98 | Paved parking, HSG C |
| 1,827 | 98 | Roofs, HSG C |
| 8,622 | 93 | Weighted Average |
| 1,680 |  | $19.49 \%$ Pervious Area |
| 6,942 |  | $80.51 \%$ Impervious Area |



Routed to Pond CB35 : CB \#35
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,578 | 98 P |  |  |  |
|  | 79 | 80 > | Paved parking, HSG C <br> >75\% Grass cover, Good, HSG D |  |  |
|  | 492 | 98 P |  |  |  |
|  | 4,149 | 98 V | Weighted Average |  |  |
|  | 79 |  | 1.90\% Pervious Area |  |  |
|  | 4,070 |  | 98.10\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C36: CB \#36

Runoff $=\quad 0.92$ cfs @ 12.09 hrs, Volume $=\quad 3,337$ cf, Depth> 6.05"
Routed to Pond CB36 : CB \#36
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | ea (sf) | CN Description |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 6,622 | 98 | aved park | ng, HSG C |  |
| 6,622 |  | 100.00\% Impervious Area |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity <br> (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

Summary for Subcatchment C38: CB \#38
Runoff = 1.06 cfs @ 12.09 hrs, Volume= 3,849 cf, Depth> 6.05"
Routed to Pond CB38 : CB \#38
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

6.0

Direct Entry,

## Summary for Subcatchment C40: CB \#40

Runoff $=\quad 0.58$ cfs @ 12.09 hrs, Volume $=2,122 \mathrm{cf}$, Depth> 6.05"
Routed to Pond CB40 : CB \#40
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | rea (sf) | CN Description |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 4,211 |  | 98 Paved parking, HSG B |  |  |  |
|  |  | 100.00\% Impervious Area |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C41: CB \#41

Runoff =
$=\quad 0.77$ cfs @ 12.09 hrs , Volume=
2,815 cf, Depth> 6.05"
Routed to Pond CB41 : CB \#41
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"


## Summary for Subcatchment C43: CB \#43

Runoff $=\quad 0.39$ cfs @ 12.09 hrs, Volume= $1,299 \mathrm{cf}$, Depth> 5.01"
Routed to Pond CB43 : CB \#43
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,343 | 98 P | Paved parking, HSG B $>75 \%$ Grass cover, Good, HSG B |  |  |
|  | 766 | $61>$ |  |  |  |
|  | 3,109 | 89 V | Weighted Average |  |  |
|  | 766 |  | 24.64\% Pervious Area |  |  |
|  | 2,343 |  | 75.36\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

Summary for Subcatchment C44: CB \#44
Runoff $=\quad 0.26$ cfs @ 12.09 hrs, Volume= 882 cf, Depth> 5.35"
Routed to Pond CB44 : CB \#44
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,670 | 98 | Paved parking, HSG B |
| 308 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 1,978 | 92 | Weighted Average |
| 308 |  | $15.57 \%$ Pervious Area |
| 1,670 |  | $84.43 \%$ Impervious Area |



Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & 1,240 \\ & 1,225 \end{aligned}$ | $\begin{array}{ll} 98 \\ 61 \end{array}$ | Paved parking, HSG B $>75 \%$ Grass cover, Good, HSG B |  |  |
|  | $\begin{aligned} & 2,465 \\ & 1,225 \\ & 1,240 \end{aligned}$ | 80 | Weighted Average 49.70\% Pervious Area 50.30\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |

## Summary for Subcatchment C46: CB \#46

Runoff $=\quad 0.47$ cfs @ 12.09 hrs, Volume $=1,481 \mathrm{cf}$, Depth> 4.04"

Routed to Pond CB46 : CB \#46
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,241 | 98 P | Paved parking, HSG B |  |  |
|  | 2,156 | $61>$ | >75\% Grass cover, Good, HSG B |  |  |
|  | 4,397 | 80 | Weighted Average |  |  |
|  | 2,156 |  | 49.03\% Pervious Area |  |  |
|  | 2,241 |  | 50.97\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

Summary for Subcatchment C47: CB \#47
Runoff $=\quad 0.42$ cfs @ 12.09 hrs, Volume= $\quad 1,518 \mathrm{cf}$, Depth> 6.05"
Routed to Pond CB47 : CB\#47
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) CN Description |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\frac{3,012}{3,012}$ |  | 98 | Paved roads w/curbs \& sewers, HSG B |  |  |
|  |  |  | 00.00\% Im | pervious A |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |

## Summary for Subcatchment C48: CB \#48

Runoff $=\quad 4.02$ cfs @ 12.17 hrs, Volume= $15,183 \mathrm{cf}$, Depth> 3.03"
Routed to Pond CB48 : CB\#48
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"
$\left.\begin{array}{rrrll}\text { Area (sf) } & \text { CN } & \text { Description } \\ 3,877 \\ 56,251 & 98 & \text { Paved roads w/curbs \& sewers, HSG B } \\ 68 & \text { 1 acre lots, 20\% imp, HSG B }\end{array}\right]$
11.8400 Total

Summary for Subcatchment C49: CB \#49
Runoff $=\quad 0.70$ cfs @ 12.09 hrs, Volume= 2,436 cf, Depth> 5.58"
Routed to Pond CB49 : CB \#49
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 4,431 | 98 | Paved roads w/curbs \& sewers, HSG C |
| 807 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 5,238 | 94 | Weighted Average |
| 807 |  | 15.41\% Pervious Area |
| 4,431 |  | $84.59 \%$ Impervious Area |



Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| $\begin{array}{r} \text { Area }(\mathrm{sf}) \\ \hline 3,429 \end{array}$ |  | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 74 > | $>75 \%$ Grass cover, Good, HSG C Paved roads w/curbs \& sewers, HSG C |  |  |
|  | 11,611 | 98 P |  |  |  |
|  | 15,040 | 93 V | Weighted A | verage |  |
|  | 3,429 |  | 22.80\% Per | vious Area |  |
|  | 11,611 |  | 77.20\% Imp | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{gathered} \text { Capacity } \\ \text { (cfs) } \end{gathered}$ | Description |

Direct Entry,

## Summary for Subcatchment C51: CB \#51

Runoff $=\quad 0.94$ cfs @ 12.09 hrs, Volume= $\quad 3,439$ cf, Depth> 6.05"
Routed to Pond CB51 : CB \#51
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,147 | 98 R | Roofs, HSG C Paved parking, HSG C |  |  |
|  | 3,676 | 98 P |  |  |  |
|  | 6,823 | 98 | Weighted Average 100.00\% Impervious Area |  |  |
|  | 6,823 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry, |

Summary for Subcatchment C52: CB\#52
Runoff = 1.23 cfs @ 12.09 hrs, Volume= 4,297 cf, Depth> 5.70"
Routed to Pond CB52 : CB \#52
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,164 | 74 | >75\% Grass cover, Good, HSG C |
| 7,888 | 98 | Paved parking, HSG C |

## Summary for Subcatchment C53: CB \#53

Runoff $=\quad 1.05$ cfs @ 12.09 hrs, Volume= 3,581 cf, Depth> 5.47"
Routed to Pond CB53 : CB \#53
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 287 | 39 > | >75\% Grass cover, Good, HSG A |  |  |
|  | 3,287 | 98 P | Paved parking, HSG A |  |  |
|  | 773 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 3,516 | 98 P | Paved parking, HSG C |  |  |
|  | 7,863 | 93 V | Weighted Average |  |  |
|  | 1,060 |  | 13.48\% Pervious Area |  |  |
|  | 6,803 |  | 86.52\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ftft) | Velocity (ft/sec) | Capacity (cfs) | Description |

6.0

Direct Entry,

## Summary for Subcatchment C54: CB \#54

Runoff $=\quad 0.63$ cfs @ 12.09 hrs, Volume $=\quad 2,104$ cf, Depth> 5.24"
Routed to Pond CB54 : CB \#54
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 550 | 39 | $>75 \%$ Grass cover, Good, HSG A |
| 4,176 | 98 | Paved parking, HSG A |
| 84 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 11 | 98 | Paved parking, HSG C |
| 4,821 | 91 | Weighted Average |
| 634 |  | 13.15\% Pervious Area |
| 4,187 |  | $86.85 \%$ Impervious Area |



Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | ea (sf) | CN Description |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4,650 | 98 P | aved road | w/curbs | sewers, HSG B |
| 4,650 |  | 100.00\% Impervious Area |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry, |

## Summary for Subcatchment C8: CB \#8

Runoff $=\quad 0.73$ cfs @ 12.09 hrs, Volume= 2,534 cf, Depth> 5.58"
Routed to Pond CB8 : CB\#8
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Description <br> Paved roads w/curbs \& sewers, HSG B |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4,837 |  |  |  |  |
|  | 613 |  | Paved roads w/curbs \& sewers, HSG B $>75 \%$ Grass cover, Good, HSG B |  |  |
|  | 5,450 | $94 \begin{array}{rr} \\ \\ & 11 \\ & 8\end{array}$ | Weighted Average <br> 11.25\% Pervious Area <br> 88.75\% Impervious Area |  |  |
|  | 613 |  |  |  |  |
|  | 4,837 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | $\begin{gathered} \text { Slope } \\ (\mathrm{ft} / \mathrm{ft}) \end{gathered}$ | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  | - |  |  | Direct Entry, |

## Summary for Subcatchment C9: CB \#9

Runoff $=\quad 2.24$ cfs @ 12.09 hrs, Volume= 8,058 cf, Depth> 5.93"
Routed to Pond CB9 : CB \#9
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"


## Summary for Subcatchment CH1: CLUBHOUSE

Runoff $=\quad 0.86$ cfs @ 12.09 hrs , Volume= $3,033 \mathrm{cf}$, Depth> 5.81"
Routed to Pond DECH : DRIP \#CH
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ |
| ---: | ---: | ---: | ---: | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description | Direct Entry, |
| :--- |

## Summary for Subcatchment S201: SUMMER STREET ACCESS APRON

Runoff $=\quad 1.35$ cfs @ 12.09 hrs, Volume= $4,720 \mathrm{cf}$, Depth> 5.70"
Routed to Link AP1 : ANALYSIS POINT 1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 717 | 61 | >75\% Grass cover, Good, HSG B |
| 9,226 | 98 | Paved parking, HSG B |

6.0

Direct Entry,

## Summary for Subcatchment S202: EXISTING WETLAND

Runoff $=\quad 26.72$ cfs @ 12.32 hrs, Volume $=130,245$ cf, Depth> 3.62"
Routed to Reach SC1 : Stream Crossing \#1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 136,496 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 83,935 | 55 | Woods, Good, HSG B |
| 29 | 98 | Paved parking, HSG B |
| 13,946 | 98 | Roofs, HSG B |
| 9,038 | 48 | Brush, Good, HSG B |
| 2,573 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 17,121 | 70 | Woods, Good, HSG C |
| 98 | 98 | Paved parking, HSG C |
| 1,097 | 65 | Brush, Good, HSG C |
| 126 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 132 | 98 | Paved parking, HSG D |
| 167,678 | 98 | Water Surface, HSG D |
| 432,269 | 76 | Weighted Average |
| 250,386 |  | 57.92\% Pervious Area |
| 181,883 |  | $42.08 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ | Description |
| ---: | ---: | ---: | ---: | :--- | :--- |
| 5.6 | 50 | 0.0200 | 0.15 | Sheet Flow, <br> Grass: Short $\mathrm{n}=0.150 \quad \mathrm{P} 2=3.27 "$ <br> 1.4 | 118 |
| 0.0400 | 1.40 | Shallow Concentrated Flow, <br> Short Grass Pasture Kv=7.0 fps <br> Shallow Concentrated Flow, <br> Woodland Kv=5.0 fps |  |  |  |
| 16.2 | 688 | 0.0200 | 0.71 |  |  |

## Summary for Subcatchment S203: POCKET WETLAND \#1

Runoff $=\quad 2.18$ cfs @ 12.09 hrs, Volume $=\quad 6,885$ cf, Depth> 3.23"

Routed to Pond p210 : POCKET WETLAND \#1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"


## Summary for Subcatchment S204: EXISTING WETLANDS

Runoff $=\quad 18.73$ cfs @ 12.31 hrs, Volume $=90,283 \mathrm{cf}$, Depth> 3.52"
Routed to Link ap2 : ANALYSIS POINT 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"


## Summary for Subcatchment S205: ISOLATED WETLAND

Runoff $=\quad 4.43$ cfs @ 12.09 hrs, Volume= 14,013 cf, Depth> 3.03"
Routed to Link AP3 : ANALYSIS POINT 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 10,910 | 30 | Woods, Good, HSG A |
| 3,684 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 2,275 | 70 | Woods, Good, HSG C |
| 171 | 98 | Paved parking, HSG C |
| 1,706 | 65 | Brush, Good, HSG C |
| 1,940 | 80 | >75\% Grass cover, Good, HSG D |
| 23,513 | 77 | Woods, Good, HSG D |
| 393 | 98 | Paved parking, HSG D |
| 2,208 | 73 | Brush, Good, HSG D |
| 8,620 | 98 | Water Surface, HSG D |
| 55,420 | 70 | Weighted Average |
| 46,236 |  | 83.43\% Pervious Area |
| 9,184 |  | $16.57 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- | Description | Direct Entry, |
| :--- |
| 6.0 |
|  |

Runoff $=31.85$ cfs @ 12.50 hrs, Volume $=188,907$ cf, Depth> 2.54" Routed to Link AP4 : ANALYSIS POINT \#4

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 16,514 | 39 | >75\% Grass cover, Good, HSG A |
| 118,226 | 30 | Woods, Good, HSG A |
| 713 | 98 | Paved parking, HSG A |
| 41,148 | 30 | Brush, Good, HSG A |
| 17,568 | 51 | 1 acre lots, 20\% imp, HSG A |
| 37,410 | 61 | >75\% Grass cover, Good, HSG B |
| 13,900 | 55 | Woods, Good, HSG B |
| 54,538 | 48 | Brush, Good, HSG B |
| 91,202 | 68 | 1 acre lots, 20\% imp, HSG B |
| 77,444 | 74 | >75\% Grass cover, Good, HSG C |
| 114,763 | 70 | Woods, Good, HSG C |
| 3,493 | 98 | Paved parking, HSG C |
| 57,740 | 65 | Brush, Good, HSG C |
| 5,763 | 80 | >75\% Grass cover, Good, HSG D |
| 126,141 | 77 | Woods, Good, HSG D |
| 114,732 | 98 | Water Surface, 0\% imp, HSG D |

## Summary for Subcatchment S207: INFILTRATION POND \#2

Runoff $=\quad 2.59 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume= $\quad 8,498 \mathrm{cf}$, Depth> 4.90"

Routed to Pond P207 : INFILTRATION POND \#2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Water Surface, 0\% imp, HSG A |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 839 | 98 |  |  |  |
|  | 8,802 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 11,162 | 98 | Water Surf | ce, 0\% imp | , HSG C |
|  | $\begin{aligned} & 20,803 \\ & 20,803 \end{aligned}$ | 88 | Weighted Average 100.00\% Pervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment S208: GRASS AREA

Runoff $=\quad 1.28$ cfs @ 12.09 hrs, Volume $=\quad 4,045$ cf, Depth> 3.53"
Routed to Pond OCS4 : OCS\#4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN D | Description |
| :---: | :---: | :---: |
| 476 | $39>$ | >75\% Grass cover, Good, HSG A |
| 12,000 | $74>$ | >75\% Grass cover, Good, HSG C |
| 168 | 98 P | Paved parking, HSG A |
| 1,116 | 98 P | Paved parking, HSG C |
| 13,760 | 75 | Weighted Average |
| 12,476 |  | 90.67\% Pervious Area |
| 1,284 |  | 9.33\% Impervious Area |
| Tc Length ( min ) (feet) | Slope (ft/ft) | Velocity Capacity Description (ft/sec) (cfs) |

6.0

## Direct Entry,

## Summary for Subcatchment S209: WETLAND C

Runoff $=\quad 5.50$ cfs @ 12.41 hrs, Volume= 29,544 cf, Depth> 3.31"
Routed to Reach 11R : 4x4 Open Bottom Culvert
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"


## Summary for Subcatchment S210: INFILTRATION POND \#1

Runoff $=$| $5.67 \mathrm{cfs} @$ |
| ---: |
| Routed to Pond |
| P212 | INFILTRATION VOND \#1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"


## Summary for Subcatchment S211: S211

Runoff $=1.60$ cfs @ 12.09 hrs, Volume= 5,064 cf, Depth> 3.94"

Routed to Pond P205 : INFILTRATION POND \#3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"


Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"


### 23.1418 Total

## Summary for Subcatchment S213: COURTYARD

Runoff $=\quad 2.21 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume= $\quad 7,022 \mathrm{cf}$, Depth> 3.94"
Routed to Pond $11 \mathrm{P}:$ YARD DRAIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG A |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 5,047 | 39 > |  |  |  |
|  | 1,678 | 98 P | Paved parking, HSG A |  |  |
|  | 168 | 98 R | Roofs, HSG A |  |  |
|  | 532 | 98 V | Water Surface, 0\% imp, HSG A |  |  |
|  | 4,518 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 7,080 | 98 P | Paved parking, HSG C |  |  |
|  | 878 | 98 R | Roofs, HSG C |  |  |
|  | 718 | 98 | Water Surface, 0\% imp, HSG C |  |  |
|  | 296 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 492 | 98 P | Paved parking, HSG D |  |  |
|  | 21,407 | 79 | Weighted Average |  |  |
|  | 11,111 |  | 51.90\% Pervious Area |  |  |
|  | 10,296 |  | 48.10\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment T1: Trench Drain 1

Runoff $=\quad 1.49 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume= $\quad 5,089 \mathrm{cf}$, Depth> 5.47"
Routed to Pond 5R : TRENCH DRAIN
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN | Description $>75 \%$ Grass cover, Good, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,443 | 74 |  |  |  |
|  | 4,228 | 98 | Paved parking, HSG C |  |  |
|  | 1,339 | 80 | >75\% Grass cover, Good, HSG D |  |  |
|  | 4,163 | 98 | Paved parking, HSG D |  |  |
|  | 11,173 | 93 | Weighted Average |  |  |
|  | 2,782 |  | 24.90\% Pervious Area |  |  |
|  | 8,391 |  | 75.10\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | $\begin{array}{rr} c & \text { Length } \\ \text { 1) } & \text { (feet) } \\ \hline \end{array}$ | Slope $(\mathrm{ft} \mid \mathrm{ft})$ | $\begin{array}{rr} \text { e } \begin{array}{r} \text { Velocity } \\ \text { t) } \\ (\mathrm{ft} / \mathrm{sec}) \end{array} \\ \hline \end{array}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment T2: Drive Under B2

Runoff $=\quad 0.45$ cfs @ 12.09 hrs, Volume= 1,420 cf, Depth> 3.83"

Routed to Reach 11R : 4x4 Open Bottom Culvert
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"


Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"


## Summary for Subcatchment TH10: TOWN HOUSE \#10

Runoff $=\quad 0.48$ cfs @ 12.09 hrs, Volume $=\quad 1,684$ cf, Depth> 5.81"
Routed to Pond DE70 : DRIP \#70
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

| Area (sf)3,184 |  | $\begin{array}{r} \mathrm{CN} \\ \hline 98 \\ 74 \\ \hline \end{array}$ | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Roofs, HSG C <br> $>75 \%$ Grass cover, Good, HSG C |
|  | $\begin{array}{r} 3,476 \\ 292 \\ 3,184 \end{array}$ |  | $96 \quad 1$ | Weighted A <br> 8.40\% Perv <br> 91.60\% Im | verage ous Area ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} \mid \mathrm{ft})$ | Velocity (ft/sec) | Capacity (cfs) | Description |

6.0 Direct Entry,

## Summary for Subcatchment TH11: TOWN HOUSE \#11

Runoff $=\quad 0.58$ cfs @ 12.09 hrs, Volume= $\quad 2,039 \mathrm{cf}$, Depth> 5.81" Routed to Pond DE71 : DRIP \#71

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,899 | 98 R | $\begin{aligned} & \text { Roofs, HSG C } \\ & >75 \% \text { Grass cover, Good, HSG C } \end{aligned}$ |  |  |
|  | 311 | $74>$ |  |  |  |
|  | 4,210 | $96 \quad 7$ | Weighted Average |  |  |
|  | 311 |  | 7.39\% Pervious Area |  |  |
|  | 3,899 |  | 92.61\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{rr} c & \text { Length } \\ \text { ) } & \text { (feet) } \\ \hline \end{array}$ | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment TH2: TOWN HOUSE \#2

Runoff $=\quad 0.58$ cfs @ 12.09 hrs, Volume $=\quad 2,057 \mathrm{cf}$, Depth $>5.81^{\prime \prime}$
Routed to Pond DE62 : DRIP \#62
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,936 | 98 R | Roofs, HSG C |  |  |
|  | 311 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 4,247 | 96 | Weighted Average |  |  |
|  | 311 |  | 7.32\% Pervious Area |  |  |
|  | 3,936 |  | 92.68\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) |  | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH3: TOWN HOUSE \#3

Runoff $=\quad 0.41$ cfs @ 12.09 hrs, Volume= 1,430 cf, Depth> 5.70"

Routed to Pond DE63 : DRIP \#63
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN | Rescript, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,672 | 98 |  |  |  |
|  | 341 | 74 | >75\% Gras | s cover, God | od, HSG C |
|  | 3,013 | $95 \quad 1$ | Weighted Average |  |  |
|  | 341 |  | 11.32\% Pervious Area |  |  |
|  | 2,672 |  | 88.68\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity <br> (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH4: TOWN HOUSE \#4

Runoff $=\quad 0.47$ cfs @ 12.09 hrs, Volume $=\quad 1,681 \mathrm{cf}$, Depth> 5.81"
Routed to Pond DE64 : DRIP \#64
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,178 | 98 | Roofs, HSG C |
| 292 | 74 | >75\% Grass cover, Good, HSG C |
| 3,470 | 96 | Weighted Average |
| 292 |  | $8.41 \%$ Pervious Area |
| 3,178 |  | $91.59 \%$ Impervious Area |
| Tc | Length | Slope  <br> (ft/ft) Velocity <br> (ft/sec) Capacity <br> (min) (cfs) |

6.0

Direct Entry,

## Summary for Subcatchment TH5: TOWN HOUSE \#5

Runoff $=\quad 0.41$ cfs @ 12.09 hrs, Volume $=\quad 1,432$ cf, Depth> 5.70" Routed to Pond DE65 : DRIP \#65

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN | Rooscrs, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,675 | 9874 |  |  |  |
|  | 341 |  | Roofs, HSG C$>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 3,016 | 95 | Weighted Average 11.31\% Pervious Area 88.69\% Impervious Area |  |  |
|  | 341 |  |  |  |  |
|  | 2,675 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH6: TOWN HOUSE \#6

Runoff $=\quad 0.47$ cfs @ 12.09 hrs, Volume= $\quad 1,650 \mathrm{cf}$, Depth> 5.81"
Routed to Pond DE66 : DRIP \#66
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,116 | 98 R | Roofs, HSG C$>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 291 | $74>$ |  |  |  |
|  | 3,407 | 96 | Weighted Average |  |  |
|  | 291 |  | 8.54\% Pervious Area |  |  |
|  | 3,116 |  | 91.46\% Imp | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ (\mathrm{cfs}) \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment TH7: TOWN HOUSE \#7

Runoff $=\quad 0.48$ cfs @ 12.09 hrs, Volume= 1,686 cf, Depth> 5.81"
Routed to Pond DE67 : DRIP \#67
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,189 | 98 R | Roofs, HSG C |  |  |
|  | 292 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 3,481 | 96 | Weighted Average |  |  |
|  | 292 |  | 8.39\% Pervious Area |  |  |
|  | 3,189 |  | 91.61\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH8: TOWN HOUSE \#8

Runoff $=\quad 0.58$ cfs @ 12.09 hrs, Volume $=\quad 2,040 \mathrm{cf}$, Depth> 5.81"

Routed to Pond DE68 : DRIP \#68
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN | Rescript, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,901 | $\begin{aligned} & 98 \\ & 74 \\ & \hline \end{aligned}$ |  |  |  |
|  | 311 |  | Roofs, HSG C$>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 4,212 | 96 | Weighted Average |  |  |
|  | 311 |  | 7.38\% Pervious Area |  |  |
|  | 3,901 |  | 92.62\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity <br> (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH9: TOWN HOUSE \#9

Runoff $=\quad 0.48 \mathrm{cfs} @ 12.09$ hrs, Volume $=\quad 1,686 \mathrm{cf}$, Depth> 5.81"
Routed to Pond DE69 : DRIP \#69
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 25YR Rainfall=6.29"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,188 | 98 R | Roofs, HSG C |  |  |
|  | 292 | $74>$ | >75\% Gras | s cover, G | od, HSG C |
|  | 3,480 | 96 | Weighted Average |  |  |
|  | 292 |  | 8.39\% Pervious Area |  |  |
|  | 3,188 |  | 91.61\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |

## Summary for Reach 8R: OVERLAND FLOW

Inflow Area = $\quad 11,975$ sf, $92.37 \%$ Impervious, Inflow Depth > 4.86" for 25YR event Inflow $=1.38$ cfs @ 12.14 hrs, Volume= 4,854 cf Outflow $=0.40 \mathrm{cfs}$ @ 12.54 hrs , Volume $=\quad 4,526 \mathrm{cf}$, Atten= $71 \%$, Lag= 23.9 min Routed to Link AP4 : ANALYSIS POINT \#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.10 \mathrm{fps}, \mathrm{Min}$. Travel Time $=94.0 \mathrm{~min}$
Avg. Velocity $=0.05 \mathrm{fps}$, Avg. Travel Time $=197.1 \mathrm{~min}$

Peak Storage= 2,258 cf @ 12.54 hrs
Average Depth at Peak Storage= 0.08' , Surface Width= 50.80'
Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 28.09 cfs
$50.00^{\prime} \times 1.00^{\prime}$ deep channel, $n=0.400$
Side Slope Z-value= 5.0 '/' Top Width=60.00'
Length $=563.0^{\prime}$ Slope $=0.0213$ '/'
Inlet Invert= 208.00', Outlet Invert= 196.00'


## Summary for Reach 9R: OVERLAND FLOW

Inflow Area = 32,665 sf, $94.81 \%$ Impervious, Inflow Depth $=0.00$ " for 25 YR event Inflow $=0.00 \mathrm{cfs}$ @ 0.00 hrs , Volume= 0 cf Outflow = $0.00 \mathrm{cfs} @ 0.00 \mathrm{hrs}$, Volume= 0 cf , Atten= $0 \%$, Lag= 0.0 min

Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.00 \mathrm{fps}$, Min. Travel Time $=0.0 \mathrm{~min}$
Avg. Velocity $=0.00 \mathrm{fps}$, Avg. Travel Time $=0.0 \mathrm{~min}$
Peak Storage= 0 cf @ 0.00 hrs
Average Depth at Peak Storage=0.00'
Bank-Full Depth $=1.00$ Flow Area= 30.0 sf, Capacity $=23.45$ cfs
20.00' x 1.00' deep channel, $n=0.400$

Side Slope Z-value= 10.0 '/' Top Width= 40.00'
Length $=211.0^{\prime}$ Slope $=0.0652{ }^{\prime} / /$
Inlet Invert= 201.75', Outlet Invert= 188.00'


## Summary for Reach 10R: OVERLAND FLOW

Inflow Area = $\quad 129,716$ sf, $63.13 \%$ Impervious, Inflow Depth $=1.18$ " for 25 YR event Inflow = 1.47 cfs @ 12.61 hrs, Volume= 12,790 cf Outflow =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.26 \mathrm{fps}, \mathrm{Min}$. Travel Time $=10.5 \mathrm{~min}$
Avg. Velocity $=0.10 \mathrm{fps}$, Avg. Travel Time $=28.4$ min
Peak Storage= 910 cf @ 12.81 hrs
Average Depth at Peak Storage $=0.25^{\prime}$, Surface Width $=24.94^{\prime}$
Bank-Full Depth=1.00' Flow Area= 30.0 sf, Capacity= 17.57 cfs
20.00' x 1.00' deep channel, $n=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 40.00'
Length= 164.0' Slope= 0.0366 '/'
Inlet Invert= 192.00', Outlet Invert= 186.00'


## Summary for Reach 11R: 4x4 Open Bottom Culvert

[52] Hint: Inlet/Outlet conditions not evaluated
[62] Hint: Exceeded Reach 20R OUTLET depth by 0.11' @ 12.50 hrs
[61] Hint: Exceeded Reach R211 outlet invert by 0.31' @ 12.55 hrs
Inflow Area = 424,818 sf, 45.99\% Impervious, Inflow Depth > 2.00" for 25YR event
Inflow $=\quad 9.42$ cfs @ 12.53 hrs , Volume= $\quad 70,760 \mathrm{cf}$
Outflow $=\quad 9.42$ cfs @ 12.53 hrs , Volume $=\quad 70,749 \mathrm{cf}$, Atten $=0 \%$, Lag= 0.2 min
Routed to Reach 23R : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=2.11 \mathrm{fps}$, Min. Travel Time $=0.2 \mathrm{~min}$
Avg. Velocity $=0.82 \mathrm{fps}$, Avg. Travel Time $=0.6 \mathrm{~min}$
Peak Storage= 134 cf @ 12.53 hrs
Average Depth at Peak Storage= 1.12' , Surface Width= 4.00'
Bank-Full Depth= 4.00' Flow Area= 16.0 sf, Capacity= 42.20 cfs
48.0" W x 48.0" H Box Pipe
$\mathrm{n}=0.069$ Riprap, 6 -inch
Length= 30.0' Slope $=0.0150$ '/'
Inlet Invert= 194.00', Outlet Invert= 193.55'


## Summary for Reach 12R: OVERLAND FLOW

| Inflow Area $=$ | 12,906 sf, $90.20 \%$ Impervious, | Inflow Depth $>4.91 "$ | for 25 YR event |  |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $1.58 \mathrm{cfs} @$ | 12.13 hrs , Volume= | $5,279 \mathrm{cf}$ |
| Outflow | $=$ | $0.85 \mathrm{cfs} @ 12.30 \mathrm{hrs}$, Volume $=$ | $5,158 \mathrm{cf}$, Atten $=46 \%$, Lag $=10.7 \mathrm{~min}$ |  |

Routed to Link AP2 : ANALYSIS POINT 2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity= $0.14 \mathrm{fps}, \mathrm{Min}$. Travel Time $=29.9 \mathrm{~min}$
Avg. Velocity $=0.05 \mathrm{fps}$, Avg. Travel Time $=85.1 \mathrm{~min}$
Peak Storage $=1,523$ cf @ 12.30 hrs
Average Depth at Peak Storage= 0.12' , Surface Width= 51.20'
Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 29.80 cfs
$50.00^{\prime} \times 1.00^{\prime}$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length $=250.0^{\prime}$ Slope $=0.0240$ '/'
Inlet Invert= 202.00', Outlet Invert= 196.00'


## Summary for Reach 14R: OVERLAND FLOW

Inflow Area = $52,768 \mathrm{sf}$, $0.60 \%$ Impervious, Inflow Depth > 2.55" for 25YR event Inflow $=2.25$ cfs @ 12.34 hrs, Volume $=11,215 \mathrm{cf}$ Outflow = 0.65 cfs @ 12.97 hrs , Volume $=10,062 \mathrm{cf}$, Atten= $71 \%$, Lag= 37.7 min

Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.13 \mathrm{fps}$, Min. Travel Time $=113.6 \mathrm{~min}$
Avg. Velocity $=0.07 \mathrm{fps}$, Avg. Travel Time $=199.3 \mathrm{~min}$
Peak Storage= 4,397 cf @ 12.97 hrs
Average Depth at Peak Storage= $0.10^{\prime}$, Surface Width= 52.02'
Bank-Full Depth= 1.00 Flow Area $=60.0$ sf, Capacity $=31.55$ cfs
50.00' x 1.00' deep channel, $\mathrm{n}=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 70.00'
Length=852.0' Slope= 0.0246 '/'
Inlet Invert= 207.00', Outlet Invert= 186.00'


## Summary for Reach 15R: OVERLAND FLOW

Inflow Area = 62,582 sf, 52.00\% Impervious, Inflow Depth > 1.86" for 25YR event Inflow = 0.23 cfs @ 15.93 hrs, Volume= 9,701 cf Outflow = 0.23 cfs @ 16.99 hrs, Volume=

8,871 cf, Atten= 0\%, Lag= 63.2 min
Routed to Link AP2 : ANALYSIS POINT 2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.08 \mathrm{fps}, \mathrm{Min}$. Travel Time $=63.7 \mathrm{~min}$
Avg. Velocity $=0.07 \mathrm{fps}$, Avg. Travel Time $=70.7 \mathrm{~min}$
Peak Storage= 872 cf @ 16.99 hrs
Average Depth at Peak Storage $=0.06$ ' , Surface Width $=50.58^{\prime}$
Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 27.21 cfs
50.00' x 1.00' deep channel, $\mathrm{n}=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length=300.0' Slope= 0.0200 '/'
Inlet Invert= 202.00', Outlet Invert= 196.00'


Summary for Reach 18R: OVERLAND FLOW
Inflow Area = 88,676 sf, 39.42\% Impervious, Inflow Depth $=1.01$ " for 25 YR event
Inflow $=3.39$ cfs @ 12.37 hrs, Volume= $\quad 7,468 \mathrm{cf}$
Outflow $=\quad 0.80 \mathrm{cfs}$ @ 12.90 hrs , Volume $=\quad 7,269 \mathrm{cf}$, Atten= $76 \%$, Lag $=31.8 \mathrm{~min}$
Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.14 \mathrm{fps}$, Min. Travel Time $=73.6 \mathrm{~min}$
Avg. Velocity $=0.06 \mathrm{fps}$, Avg. Travel Time $=163.7 \mathrm{~min}$
Peak Storage $=3,543$ cf @ 12.90 hrs
Average Depth at Peak Storage=0.11', Surface Width= 55.51'
Bank-Full Depth= 1.00' Flow Area= 75.0 sf, Capacity= 38.42 cfs
50.00' x 1.00' deep channel, $\mathrm{n}=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 100.00'
Length=609.0' Slope= 0.0279 '/'
Inlet Invert= 203.00', Outlet Invert= 186.00'


## Summary for Reach 20R: OVERLAND FLOW

| Inflow Area = | 72,222 sf, 68.72\% Impervious, | Inflow Depth > 3.44" for 25YR event |
| :---: | :---: | :---: |
| Inflow | 4.31 cfs @ 12.27 hrs , Volume= | 20,716 cf |
| Outflow | 1.57 cfs @ 13.10 hrs , Volume= | 19,942 cf, Atten= 63\%, Lag $=49.8 \mathrm{~min}$ |
| Routed to | 11R : 4x4 Open Bottom Culvert |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.13 \mathrm{fps}$, Min. Travel Time $=70.3 \mathrm{~min}$
Avg. Velocity $=0.07 \mathrm{fps}$, Avg. Travel Time $=138.3 \mathrm{~min}$
Peak Storage= 6,626 cf @ 13.10 hrs
Average Depth at Peak Storage= 0.23' , Surface Width= 52.31'
Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 18.54 cfs
50.00 ' $1.00^{\prime}$ deep channel, $n=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length=560.0' Slope= 0.0093 '/'
Inlet Invert= 200.00', Outlet Invert= 194.80'


## Summary for Reach 23R: OVERLAND FLOW



Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.30 \mathrm{fps}, \mathrm{Min}$. Travel Time $=13.2 \mathrm{~min}$
Avg. Velocity $=0.11 \mathrm{fps}$, Avg. Travel Time $=35.1 \mathrm{~min}$

Peak Storage $=6,778$ cf @ 12.72 hrs
Average Depth at Peak Storage= 0.48' , Surface Width= 69.20'
Bank-Full Depth= 1.00' Flow Area= 70.0 sf, Capacity $=31.93$ cfs
50.00' $\times 1.00^{\prime}$ deep channel, $n=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 20.0 '/' Top Width= 90.00'
Length $=237.0^{\prime}$ Slope $=0.0211^{\prime} / /$
Inlet Invert= 193.00', Outlet Invert= 188.00'


## Summary for Reach R202: OVERLAND FLOW

[62] Hint: Exceeded Reach SC1 OUTLET depth by 0.28 @ 12.95 hrs


Routed to Link AP2 : ANALYSIS POINT 2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.23 \mathrm{fps}$, Min. Travel Time $=50.5 \mathrm{~min}$
Avg. Velocity $=0.10 \mathrm{fps}$, Avg. Travel Time $=122.8 \mathrm{~min}$
Peak Storage= 41,142 cf @ 12.69 hrs
Average Depth at Peak Storage= $0.52^{\prime}$, Surface Width= 126.01'
Bank-Full Depth=1.00' Flow Area= 125.0 sf, Capacity= 42.56 cfs
$100.00 \times 1.00$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 25.0 '/' Top Width= 150.00'
Length $=700.0^{\prime}$ Slope $=0.0107 \mathrm{I} / \mathrm{\prime}$
Inlet Invert= 205.50', Outlet Invert= 198.00'


## Summary for Reach R211: OVERLAND FLOW

Inflow Area = 241,078 sf, $59.10 \%$ Impervious, Inflow Depth = 1.00" for 25 YR event Inflow = 11.59 cfs @ 12.30 hrs , Volume $=$ 20, 184 cf Outflow $=3.47$ cfs @ 12.70 hrs , Volume= Routed to Reach 11R : 4x4 Open Bottom Culvert

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.18 \mathrm{fps}$, Min. Travel Time $=54.3 \mathrm{~min}$
Avg. Velocity $=0.06 \mathrm{fps}$, Avg. Travel Time $=157.4 \mathrm{~min}$
Peak Storage $=11,311$ cf @ 12.70 hrs
Average Depth at Peak Storage= $0.45^{\prime}$, Surface Width= 48.54'
Bank-Full Depth=1.00' Flow Area= 50.0 sf, Capacity= 14.51 cfs
$35.00^{\prime} \times 1.00^{\prime}$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 15.0 '/' Top Width= 65.00'
Length=600.0' Slope= 0.0087 '/'
Inlet Invert= 200.00', Outlet Invert= 194.80'


## Summary for Reach SC1: Stream Crossing \#1

[52] Hint: Inlet/Outlet conditions not evaluated
[90] Warning: Qout>Qin may require smaller dt or Finer Routing

| Inflow Area = | 432,269 sf, 42.08\% Impervious, | Inflow Depth > 3.62" for 25YR event |
| :---: | :---: | :---: |
| Inflow | 26.72 cfs @ 12.32 hrs, Volume= | 130,245 cf |
| Outflow | 26.73 cfs @ 12.32 hrs, Volume= | 130,223 cf, Atten= 0\%, Lag= 0.1 min |
| Routed to | R202 : OVERLAND FLOW |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=3.86 \mathrm{fps}$, Min. Travel Time $=0.2 \mathrm{~min}$
Avg. Velocity $=1.28 \mathrm{fps}$, Avg. Travel Time $=0.6 \mathrm{~min}$
Peak Storage= 298 cf @ 12.32 hrs
Average Depth at Peak Storage= 0.43' , Surface Width= 16.00'
Bank-Full Depth= 5.00' Flow Area= 69.8 sf, Capacity= 722.91 cfs
192.0" W x 60.0" H, R=207.0" Arch Pipe
$n=0.030$ Stream, clean \& straight
Length= 43.1' Slope= 0.0200 '/'
Inlet Invert= 206.37', Outlet Invert= 205.51'


## Summary for Reach SC2: Stream Crossing \#2

[52] Hint: Inlet/Outlet conditions not evaluated

| Inflow A | 52,768 sf, | 0.60\% Impervious, | Inflow Depth > 2.55" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 2.25 cfs @ | 12.34 hrs , Volume= | 11,217 cf |
| Outflow | 2.25 cfs @ | 12.34 hrs , Volume= | $11,215 \mathrm{cf}$, Atten= 0\%, Lag= 0.3 mi |

Routed to Reach 14R : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity= 1.57 fps , Min. Travel Time $=0.4 \mathrm{~min}$
Avg. Velocity $=1.06 \mathrm{fps}$, Avg. Travel Time $=0.6 \mathrm{~min}$
Peak Storage= 52 cf @ 12.34 hrs
Average Depth at Peak Storage= 0.09' , Surface Width= 16.00'
Bank-Full Depth $=5.00^{\prime}$ Flow Area= 68.1 sf, Capacity= 768.96 cfs
192.0" W x 60.0" H, R=180.0" Arch Pipe
$\mathrm{n}=0.030$ Stream, clean \& straight
Length= 36.5' Slope= 0.0241 '/'
Inlet Invert= 208.52', Outlet Invert= 207.64'


## Summary for Pond 1P: DMH \#33

 Routed to Pond OCS6 : OCS \#6

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.34' @ 12.09 hrs
Flood Elev= 209.64'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $205.50 '$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=46.7^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $205.50^{\prime} / 204.33^{\prime} \quad \mathrm{S}=0.0251$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=2.15 cfs @ 12.09 hrs HW=206.33' TW=202.51' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls 2.15 cfs @ 3.10 fps )

Summary for Pond 3P: OCS \#8

| Inflow Area = | 12,684 sf, | 86.64\% Impervious, | Inflow Depth > 5.38" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.67 cfs @ | 12.09 hrs , Volume= | 5,685 cf |
| Outflow | 1.67 cfs @ | 12.09 hrs , Volume= | 5,685 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.67 cfs @ | 12.09 hrs , Volume= | 5,685 cf |

Routed to Pond P214 : STORMTECH INFILTRATION SYSTEM \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=202.09' @ 12.60 hrs
Flood Elev= 206.36'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | :---: | :---: | :---: | :---: |
| $\# 1$ | Primary | $200.62^{\prime}$ | $12.0^{\prime \prime}$ Vert. Orifice/Grate $\quad$ C= $0.600 \quad$ Limited to weir flow at low heads |  |

Primary OutFlow Max=1.67 cfs @ 12.09 hrs HW=201.39' TW=201.11' (Dynamic Tailwater)
—1=Orifice/Grate (Orifice Controls 1.67 cfs @ 2.57 fps)

## Summary for Pond 5R: TRENCH DRAIN



Primary OutFlow Max=1.45 cfs @ 12.09 hrs HW=198.30' TW=195.96' (Dynamic Tailwater)
——1=Culvert (Inlet Controls 1.45 cfs @ 4.15 fps )

## Summary for Pond 11P: YARD DRAIN



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev= 207.41' @ 12.17 hrs Surf.Area= 6,009 sf Storage= 883 cf
Plug-Flow detention time $=16.5 \mathrm{~min}$ calculated for 6,973 cf ( $99 \%$ of inflow)
Center-of-Mass det. time= $12.2 \mathrm{~min}(826.4-814.2$ )


Primary OutFlow Max=1.64 cfs @ 12.17 hrs HW=207.41' TW=203.36' (Dynamic Tailwater)
L-1=Culvert (Passes 1.64 cfs of 6.63 cfs potential flow)


## Summary for Pond CB10: CB \#10

 Routed to Pond D5 : DMH \#5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=210.46' @ 12.09 hrs
Flood Elev= 212.93'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $209.76 '$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=33.8^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 209.76' 209.59 ' $\mathrm{S}=0.0050 \mathrm{I} / \mathrm{l} \quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=0.93 cfs @ 12.09 hrs HW=210.44' TW=210.24' (Dynamic Tailwater)
——1=Culvert (Outlet Controls 0.93 cfs @ 2.30 fps )

## Summary for Pond CB11: CB \#11

| Inflow Area = | 7,173 sf | 00\% Impervious, | Inflow Depth > 6.05" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.99 cfs @ | 12.09 hrs , Volume= | 3,615 cf |
| Outflow | 0.99 cfs @ | 12.09 hrs , Volume= | $3,615 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | 0.99 cfs @ | 12.09 hrs , Volume= | 3,615 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 210.55' @ 12.09 hrs
Flood Elev= 213.13'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $209.94^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=26.3^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 209.94' $/ 209.67^{\prime} \quad \mathrm{S}=0.0103^{\prime} / /^{\prime} \quad \mathrm{Cc}=0.900$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=0.96 cfs @ 12.09 hrs HW=210.54' TW=210.24' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.96 cfs @ 2.83 fps )

## Summary for Pond CB12: CB \#12

| Inflow Area $=$ | 5,238 sf, $100.00 \%$ Impervious, | Inflow Depth $>$ | $6.05 "$ | for 25 YR event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.72 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $2,640 \mathrm{cf}$ |
| Outflow | $=$ | $0.72 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $2,640 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | $=$ | $0.72 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $2,640 \mathrm{cf}$ |

Routed to Pond 1P : DMH \#33
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=207.11' @ 12.09 hrs
Flood Elev=209.84'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $206.68^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=41.3^{\prime} \quad \mathrm{Ke}=0.500$ <br>  <br>  <br>  <br>  <br>  <br>  <br>  <br>  <br>  <br> $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.70 cfs @ 12.09 hrs HW=207.10' TW=206.33' (Dynamic Tailwater)
$\__{1=C u l v e r t ~(I n l e t ~ C o n t r o l s ~}^{0.70} \mathrm{cfs} @ 2.22 \mathrm{fps}$ )

## Summary for Pond CB13: CB \#13



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=207.35' @ 12.09 hrs
Flood Elev= 209.86'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | ---: | :--- | :--- |
| \#1 | Primary | $206.70^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=43.7^{\prime} \mathrm{Ke}=0.500$ |  |

Inlet / Outlet Invert= 206.70' / 205.61' S=0.0249 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=1.45 cfs @ 12.09 hrs HW=207.34' TW=206.33' (Dynamic Tailwater)
—1 $_{1=C u l v e r t ~(I n l e t ~ C o n t r o l s ~} 1.45$ cfs @ 2.72 fps)

## Summary for Pond CB14: CB \#14

| Inflow Area = | 12,099 sf, 8 | 86.22\% Impervious, | Inflow Depth > 5.24" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.57 cfs @ 12 | 12.09 hrs , Volume= | 5,281 cf |
| Outflow = | 1.57 cfs @ 12. | 12.09 hrs , Volume= | $5,281 \mathrm{cf}$, Atten= $0 \%$, Lag= 0.0 min |
| Primary = Routed to Pond | $\begin{aligned} & 1.57 \mathrm{cfs} @ 12 \\ & \text { D8 : DMH \#8 } \end{aligned}$ | 12.09 hrs , Volume= | $5,281 \mathrm{cf}$ |
| Routing by Dyn-S <br> Peak Elev= 201.9 <br> Flood Elev= 203 | or-Ind method, ' @ 12.09 hrs | Time Span= 0.00-2 | $.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | $200.79^{\prime}$ | 12.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { vert } \mathrm{L}=23.2^{\prime} \mathrm{Ke}=0.500 \\ & \mathrm{t}=200.79^{\prime} / 200.67^{\prime} \quad \mathrm{S}=0.0052^{\prime} / l^{\prime} \quad \mathrm{Cc}=0.900 \end{aligned}$ $\text { ted PE, smooth interior, Flow Area= } 0.79 \text { sf }$ |

Primary OutFlow Max=1.54 cfs @ 12.09 hrs HW=201.92' TW=201.76' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.54 cfs @ 1.96 fps)

## Summary for Pond CB15: CB \#15

 Routed to Pond D8: DMH \#8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=201.84' @ 12.09 hrs
Flood Elev= 203.95'


Primary OutFlow Max=0.91 cfs @ 12.09 hrs HW=201.81' TW=201.75' (Dynamic Tailwater)
_1=Culvert (Inlet Controls 0.91 cfs @ 1.16 fps )

## Summary for Pond CB16: CB \#16

| Inflow Area = | 8,516 | 64.88\% Impervious, | Depth > 3.94" for 25 YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.88 cfs @ | 12.09 hrs , Volume= | 2,794 cf |
| Outflow | 0.88 cfs @ | 12.09 hrs , Volume= | 2,794 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.88 cfs @ | 12.09 hrs , Volume= | 2,794 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 204.11' @ 12.09 hrs
Flood Elev= 206.64'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 203.47' | 12.0" Round Culvert L= 20.9' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=203.47' / 203.33' S=0.0067 '// Cc=0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.86 cfs @ 12.09 hrs HW=204.10' TW=203.91' (Dynamic Tailwater)
——1=Culvert (Outlet Controls 0.86 cfs @ 2.37 fps )

## Summary for Pond CB17: CB \#17



## Summary for Pond CB18: CB \#18

| Inflow Area = | 24,853 | pervious, | Inflow Depth > 4.50" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 2.77 cfs @ | 12.09 hrs , Volume= | 9,314 cf |
| Outflow | 2.77 cfs @ | 12.09 hrs , Volume= | $9,314 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 2.77 cfs @ | 12.09 hrs , Volume= | 9,314 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.80' @ 12.10 hrs
Flood Elev= 208.16'
Device Routing Invert Outlet Devices
\#1 Primary 204.72' 15.0" Round Culvert L= 25.1' Ke=0.500
Inlet / Outlet Invert= 204.72' / 204.59' S=0.0052 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.23 sf
Primary OutFlow Max=2.12 cfs @ 12.09 hrs HW=205.79' TW=205.59' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 2.12 cfs @ 2.55 fps )

## Summary for Pond CB20: CB \#20

| Inflow Area = | 11,939 sf, 88.95\% Impervious, Inflow Depth > 5.70" for 25YR event |  |  |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.62 cfs @ 12 | 12.09 hrs , Volume= | 5,667 cf |
| Outflow | 1.62 cfs @ 12 | 12.09 hrs , Volume= | $5,667 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary = Routed to Pon | $\begin{aligned} & 1.62 \mathrm{cfs} @ 12 \\ & \text { D12 : DMH \#1 } \end{aligned}$ | 12.09 hrs , Volume= 12 | 5,667 cf |
| Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$ Peak Elev= 204.88' @ 12.09 hrs <br> Flood Elev=207.13' |  |  |  |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 203.97' | 12.0" Round Cu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { lvert } \mathrm{L}=30.3^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=203.97 \prime^{\prime} / 203.81^{\prime} \quad \mathrm{S}=0.0053 \text { '/' } \mathrm{Cc}=0.900 \\ & \text { ted } \mathrm{PE} \text {, smooth interior, Flow Area= } 0.79 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=1.59 cfs @ 12.09 hrs HW=204.85' TW=204.55' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 1.59 cfs @ 2.90 fps )

## Summary for Pond CB21: CB \#21



Routed to Pond D12 : DMH \#12
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.05' @ 12.09 hrs
Flood Elev= 208.02'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $204.32^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert L=26.0' $\mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= 204.32' $/ 204.19^{\prime} \mathrm{S}=0.0050$ '/' $\mathrm{Cc}=0.900$ |  |
|  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area=0.79 sf |  |  |

Primary OutFlow Max=1.27 cfs @ 12.09 hrs HW=205.04' TW=204.55' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 1.27 cfs @ 2.96 fps )

## Summary for Pond CB22: CB \#22

| Inflow Area = | 12,001 sf, | 91.62\% Impervious, | Inflow Depth > | 81" for 25YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 1.64 cfs @ | 12.09 hrs , Volume= | 5,813 cf |  |
| Outflow | 1.64 cfs @ | 12.09 hrs , Volume= | 5,813 cf, | Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 1.64 cfs @ | 12.09 hrs , Volume= | 5,813 cf |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs $/ 3$

Peak Elev= 206.17' @ 12.09 hrs
Flood Elev= 208.50'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $205.33^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=16.1^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= $205.33^{\prime} / 205.25^{\prime} \quad \mathrm{S}=0.0050 \quad$ '/' $\quad \mathrm{Cc}=0.900$ |  |
| $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 0.79 sf |  |  |  |

Primary OutFlow Max=1.60 cfs @ 12.09 hrs HW=206.15' TW=205.32' (Dynamic Tailwater)
——1=Culvert (Barrel Controls 1.60 cfs @ 3.14 fps )

## Summary for Pond CB23: CB \#23



Routed to Pond D14 : DMH \#14
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=206.10' @ 12.09 hrs
Flood Elev=208.57'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $205.41^{\prime}$ | $\mathbf{1 2 . 0 "}$ Round Culvert $\mathrm{L}=16.3^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= $205.41^{\prime} / 205.32$ ' $\mathrm{S}=0.0055 \mathrm{I} / \mathrm{Cl} \quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=1.19 cfs @ 12.09 hrs HW=206.09' TW=205.32' (Dynamic Tailwater)
——1=Culvert (Barrel Controls 1.19 cfs @ 2.95 fps )

## Summary for Pond CB24: CB \#24

 Routed to Pond D16 : DMH \#16

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=205.98' @ 12.09 hrs
Flood Elev= 209.21'


Primary OutFlow Max=1.03 cfs @ 12.09 hrs HW=205.96' TW=205.82' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 1.03 cfs @ 2.26 fps )

## Summary for Pond CB25: CB \#25

| Inflow Area = | $8,487 \mathrm{sf}, 80$ | 80.92\% Impervious, | Inflow Depth > 5.58" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.14 cfs @ 12 | 12.09 hrs , Volume= | 3,947 cf |
| Outflow = | 1.14 cfs @ 12. | 12.09 hrs , Volume= | $3,947 \mathrm{cf}$, Atten $=0 \%, \mathrm{Lag}=0.0 \mathrm{~min}$ |
| Primary = Routed to Pond | 1.14 cfs @ 12 D16 : DMH \#16 | 12.09 hrs , Volume= 16 | 3,947 cf |
| Routing by Dyn-S <br> Peak Elev= 205. <br> Flood Elev= 208. | or-Ind method, ' @ 12.09 hrs | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 205.22' | 15.0" Round Culv Inlet / Outlet Inve $\mathrm{n}=0.012$ Corruga | $\begin{aligned} & \text { Ivert } \mathrm{L}=11.4^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{rt}=205.22^{\prime} / 205.16^{\prime} \quad \mathrm{S}=0.0053 \mathrm{I} / \mathrm{Cc}=0.900 \\ & \text { ated PP, smooth interior, Flow Area= } 1.23 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=1.11 cfs @ 12.09 hrs HW=205.95' TW=205.82' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 1.11 cfs @ 2.16 fps )

## Summary for Pond CB26: CB \#26

| Inflow Area = | 8,835 | ervious, | 5.24" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.15 cfs @ | 12.09 hrs , Volume= | 3,856 cf |
| Outflow | 1.15 cfs @ | 12.09 hrs , Volume= | $3,856 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.15 cfs @ | 12.09 hrs , Volume= | 3,856 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=202.43' @ 12.09 hrs
Flood Elev= 204.93'


Primary OutFlow Max=1.12 cfs @ 12.09 hrs HW=202.42' TW=201.32' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 1.12 cfs @ 2.93 fps )

## Summary for Pond CB27: CB \#27

| Inflow Area = | 6,111 | 91.90\% Impervious, | Wepth > 5.81" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.84 cfs @ | 12.09 hrs , Volume= | 2,960 cf |
| Outflow | 0.84 cfs @ | 12.09 hrs , Volume= | 2,960 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.84 cfs @ | 12.09 hrs , Volume= | 2,960 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 201.56' @ 12.09 hrs
Flood Elev= 204.16'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 201.00' | 12.0" Round Culvert L= 18.0' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert=201.00' / 200.90' S=0.0056 '/l' Cc= 0.900 <br> $n=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |

Primary OutFlow Max=0.82 cfs @ 12.09 hrs HW=201.55' TW=201.32' (Dynamic Tailwater)
——1=Culvert (Outlet Controls 0.82 cfs @ 2.65 fps )

## Summary for Pond CB28: CB \#28



## Summary for Pond CB29: CB \#29



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.22' @ 12.09 hrs
Flood Elev= 208.55'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $205.38^{\prime}$ | 12.0" Round Culvert $\mathrm{L}=13.5^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $205.38^{\prime} / 205.31^{\prime} \quad \mathrm{S}=0.0052$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=1.09 cfs @ 12.09 hrs HW=206.20' TW=206.07' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 1.09 cfs @ 2.13 fps )

## Summary for Pond CB30: CB \#30



Primary OutFlow Max=1.15 cfs @ 12.09 hrs HW=206.22' TW=206.07' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 1.15 cfs @ 2.20 fps )

## Summary for Pond CB31: CB \#31

| Inflow Area $=$ | $16,365 \mathrm{sf}$, | $68.64 \%$ Impervious, | Inflow Depth > 5.13" |
| :--- | :--- | :--- | :--- |
| for 25 YR event |  |  |  |
| Inflow | $=$ | $2.10 \mathrm{cfs} @$ | 12.09 hrs , Volume |
| Outflow | $=$ | $2.10 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |
| Primary | $=$ | $2.10 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |

Routed to Pond D21 : DMH \#21
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.18' @ 12.09 hrs
Flood Elev=207.36'


Primary OutFlow Max=2.04 cfs @ 12.09 hrs HW=205.17' TW=204.63' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 2.04 cfs @ 3.31 fps )

## Summary for Pond CB32: CB \#32

| Inflow Area $=$ | $12,710 \mathrm{sf}$, | $70.47 \%$ | Impervious, | Inflow Depth $>$ |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $1.65 \mathrm{cfs} @$ | 12.09 hrs , Volume= | for 25 YR event |
| Outflow | $=$ | $1.65 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $5,548 \mathrm{cf}$ |
| Primary | $=$ | $1.65 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $5,548 \mathrm{cf}$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 205.04' @ 12.09 hrs
Flood Elev= 207.35'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $204.19^{\prime}$ | 12.0" Round Culvert $\mathrm{L}=16.3^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= 204.19' $/ 204.11^{\prime} \quad \mathrm{S}=0.0049 \mathrm{I} / \mathrm{l}$ | $\mathrm{Cc}=0.900$ |
| $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |  |

Primary OutFlow Max=1.61 cfs @ 12.09 hrs HW=205.03' TW=204.63' (Dynamic Tailwater)
——1=Culvert (Barrel Controls 1.61 cfs @ 3.10 fps )

## Summary for Pond CB33: CB \#33



## Summary for Pond CB34: CB \#34

 Routed to Pond D22 : DMH \#22

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=206.04' @ 12.09 hrs
Flood Elev= 208.38'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $205.21^{\prime}$ | $\mathbf{1 2 . 0}$ " Round Culvert $\mathrm{L}=16.5^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $205.21^{\prime} / 205.13^{\prime} \quad \mathrm{S}=0.0048$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=1.12 cfs @ 12.09 hrs HW=206.02' TW=205.87' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 1.12 cfs @ 2.24 fps )

## Summary for Pond CB35: CB \#35

| Inflow Area = | 4,149 sf, 98.10\% Impervious, Inflow Depth > 6.05" for 25YR event |  |  |
| :---: | :---: | :---: | :---: |
| Inflow = | 0.57 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 2,091 cf |
| Outflow = | 0.57 cfs @ 12 | . 09 hrs, Volume= | $2,091 \mathrm{cf}$, Atten= $0 \%$, Lag 0.0 min |
| Primary = Routed to Pon | 0.57 cfs @ 12 D23 : DMH \#2 | .09 hrs , Volume= | 2,091 cf |
| Routing by Dyn-S <br> Peak Elev= 207.5 <br> Flood Elev= 210 | or-Ind method, ' @ 12.09 hrs $1^{\prime}$ | ime Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 207.04' | 12.0" Round Culv Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { lvert } \mathrm{L}=15.2^{\prime} \mathrm{Ke}=0.500 \\ & \mathrm{t}=207.04^{\prime} / 206.96^{\prime} \quad \mathrm{S}=0.0053^{\prime} / / \quad \mathrm{Cc}=0.900 \end{aligned}$ $\text { ted PE, smooth interior, Flow Area= } 0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.56 cfs @ 12.09 hrs HW=207.49' TW=207.30' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.56 cfs @ 2.38 fps )

## Summary for Pond CB36: CB \#36

| Inflow Area $=$ | $6,622 \mathrm{sf}, 100.00 \%$ Impervious, | Inflow Depth > 6.05" | for 25 YR event |  |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.92 \mathrm{cfs} @$ | 12.09 hrs , Volume | $3,337 \mathrm{cf}$ |
| Outflow | $=$ | $0.92 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $3,337 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | $=$ | $0.92 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume $=$ | $3,337 \mathrm{cf}$ |  | Routed to Pond D23 : DMH \#23

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.64' @ 12.09 hrs
Flood Elev=210.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $207.04^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=16.1^{\prime} \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 207.04' $/ 206.96^{\prime} \quad \mathrm{S}=0.0050$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=0.89 cfs @ 12.09 hrs HW=207.63' TW=207.30' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.89 cfs @ 2.66 fps )

## Summary for Pond CB38: CB \#38

| Inflow Area = | 7,637 | 00.00\% Impervious, | Inflow Depth > 6.05" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.06 cfs @ | 12.09 hrs , Volume= | 3,849 cf |
| Outflow | 1.06 cfs @ | 12.09 hrs , Volume= | $3,849 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.06 cfs @ | 12.09 hrs , Volume= | 3,849 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 210.83' @ 12.09 hrs
Flood Elev=212.86'


Primary OutFlow Max=1.03 cfs @ 12.09 hrs HW=210.80' TW=210.72' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.03 cfs @ 1.31 fps)

## Summary for Pond CB39: CB \#39



Routed to Pond D25: DMH \#25
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=210.83' @ 12.09 hrs
Flood Elev= 212.86'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $209.69^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=16.4^{\prime} \quad \mathrm{Ke}=0.500$ <br>  <br>  <br>  <br>  <br>  <br>  <br>  <br>  <br>  <br> $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=1.02 cfs @ 12.09 hrs HW=210.80' TW=210.72' (Dynamic Tailwater)
$\__{1=C u l v e r t ~(I n l e t ~ C o n t r o l s ~}^{1.02} \mathrm{cfs} @ 1.30 \mathrm{fps}$ )

## Summary for Pond CB40: CB \#40

| Inflow Area = | 4,211 sf,100.00\% Impervious, | Inflow Depth > 6.05" for 25YR event |
| :---: | :---: | :---: |
| Inflow | 0.58 cfs @ 12.09 hrs, Volume= | 2,122 cf |
| Outflow | 0.58 cfs @ 12.09 hrs, Volume= | $2,122 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.58 cfs @ 12.09 hrs, Volume= | 2,122 cf | Routed to Pond D27 : DMH \#27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=214.28' @ 12.09 hrs
Flood Elev= 217.04'
Device Routing Invert Outlet Devices
\#1 Primary 213.68' 12.0" Round Culvert L= 17.8' Ke=0.500
Inlet / Outlet Invert= 213.68' / 213.55' S=0.0073 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=0.57 cfs @ 12.09 hrs HW=214.26' TW=214.17' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.57 cfs @ 1.72 fps )

## Summary for Pond CB41: CB \#41

| Inflow Area $=$ | $5,586 \mathrm{sf}, 100.00 \%$ | Impervious, | Inflow Depth > 6.05" | for 25 YR event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.77 \mathrm{cfs} @$ | 12.09 hrs , Volume | $2,815 \mathrm{cf}$ |
| Outflow | $=$ | $0.77 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $2,815 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | $=$ | $0.77 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume $=$ | $2,815 \mathrm{cf}$ |  | Routed to Pond D27 : DMH \#27

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=214.43' @ 12.09 hrs
Flood Elev= 217.06'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 213.89' | 12.0" Round Culvert L= 18.4' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert= 213.89' / 213.80' S=0.0049 '/' Cc= 0.900 <br> $n=0.013$ Corrugated $P E$, smooth interior, Flow Area= 0.79 sf |

Primary OutFlow Max=0.75 cfs @ 12.09 hrs HW=214.42' TW=214.17' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.75 cfs @ 2.55 fps )

## Summary for Pond CB43: CB \#43

| Inflow Area $=$ | $3,109 \mathrm{sf}, 75.36 \%$ Impervious, | Inflow Depth $>5.01 "$ | for 25 YR event |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.39 \mathrm{cfs} @$ | 12.09 hrs , Volume |
| Outflow | $=$ | $0.39 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |
| Primary | $=$ | $0.39 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |

Routed to Pond D29: DMH \#29
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=220.52' @ 12.09 hrs
Flood Elev=223.17'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 220.00' | 12.0" Round Culvert L= 14.9' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 220.00' $/ 219.93$ ' S=0.0047 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |

Primary OutFlow Max=0.38 cfs @ 12.09 hrs HW=220.51' TW=220.44' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.38 cfs @ 1.40 fps )

## Summary for Pond CB44: CB \#44

| Inflow Area = | 1,978 | 3\% Impervious, | Inflow Depth > | 5.35" for 25YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 0.26 cfs @ | 12.09 hrs , Volume= | 882 cf |  |
| Outflow | 0.26 cfs @ | 12.09 hrs , Volume= | 882 cf | f, Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 0.26 cfs @ | 12.09 hrs , Volume= | 882 cf |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 220.49' @ 12.09 hrs
Flood Elev= 223.17'


Primary OutFlow Max=0.25 cfs @ 12.09 hrs HW=220.48' TW=220.44' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.25 cfs @ 1.01 fps )

## Summary for Pond CB45: CB \#45



Routed to Pond D30 : DMH \#30
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=221.59' @ 12.09 hrs
Flood Elev= 224.46'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $221.29^{\prime}$ | $\mathbf{1 2 . 0 "}$ Round Culvert $\mathrm{L}=18.2^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 221.29' $/ 221.20^{\prime} \quad \mathrm{S}=0.0049$ '/' $\quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=0.26 cfs @ 12.09 hrs HW=221.59' TW=221.42' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.26 cfs @ 1.94 fps )

## Summary for Pond CB46: CB \#46

 Routed to Pond D30 : DMH \#30

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=221.94' @ 12.09 hrs
Flood Elev= 224.69'
Device Routing Invert Outlet Devices
\#1 Primary 221.53' 12.0" Round Culvert L= 15.3' Ke=0.500
Inlet / Outlet Invert= 221.53' / 221.45' S=0.0052 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=0.46 cfs @ 12.09 hrs HW=221.94' TW=221.42' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.46 cfs @ 2.26 fps )

## Summary for Pond CB47: CB\#47

| Inflow Area = | 3,012 sf, 100.00\% Impervious, | Inflow Depth > 6.05" for 25YR event |
| :---: | :---: | :---: |
| Inflow | 0.42 cfs @ 12.09 hrs , Volume= | 1,518 cf |
| Outflow | 0.42 cfs @ 12.09 hrs , Volume= | $1,518 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | 0.42 cfs @ 12.09 hrs , Volume= | 1,518 cf | Routed to Pond D31 : DMH\#31

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=225.79' @ 12.16 hrs
Flood Elev= 230.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $225.05 '$ | $\mathbf{1 2 . 0}{ }^{\prime \prime}$ Round Culvert $\mathrm{L}=20.9^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= $225.05^{\prime} / 224.95^{\prime} \quad \mathrm{S}=0.0048 \mathrm{I} / \mathrm{l} \quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=0.42 cfs @ 12.09 hrs HW=225.66' TW=225.62' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.42 cfs @ 1.18 fps )

## Summary for Pond CB48: CB\#48

| Inflow Area = | 60,128 sf, 25.16\% Impervious, | Inflow Depth > 3.03" for 25YR event |
| :---: | :---: | :---: |
| Inflow | 4.02 cfs @ 12.17 hrs, Volume= | 15,183 cf |
| Outflow | 4.02 cfs @ 12.17 hrs, Volume= | 15,183 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 4.02 cfs @ 12.17 hrs, Volume= | 15,183 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=226.24' @ 12.16 hrs
Flood Elev=230.25'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $224.82^{\prime}$ | $15.0^{\prime \prime}$ Round Culvert $\mathrm{L}=16.9^{\prime}$ <br>  |
|  |  | Inlet / Outlet Invert= $224.82^{\prime} / 224.74^{\prime} \mathrm{S}=0.500$ |  |
| $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |  |  |  |

Primary OutFlow Max=3.94 cfs @ 12.17 hrs HW=226.21' TW=225.76' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 3.94 cfs @ 3.21 fps )

## Summary for Pond CB49: CB \#49

| Inflow Area = | 5,238 | \% Impervious, | 5.58" for 25 YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.70 cfs @ | 12.09 hrs , Volume= | 2,436 cf |
| Outflow | 0.70 cfs @ | 12.09 hrs , Volume= | $2,436 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.70 cfs @ | 12.09 hrs , Volume= | 2,436 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 203.65' @ 12.09 hrs
Flood Elev= 205.93'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.76' | 12.0" Round Culvert L= 15.5' Ke= 0.500 <br> Inlet / Outlet Invert= 202.76' / 202.68' S=0.0052 '// Cc= 0.900 <br> $n=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79$ sf |

Primary OutFlow Max=0.69 cfs @ 12.09 hrs HW=203.62' TW=203.57' (Dynamic Tailwater)
——1=Culvert (Outlet Controls 0.69 cfs @ 1.28 fps )

## Summary for Pond CB50: CB \#50



## Summary for Pond CB51: CB \#51

| Inflow Area $=$ | $6,823 \mathrm{sf}, 100.00 \%$ |  | Impervious, | Inflow Depth $>$ |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.94 \mathrm{cfs} @$ | 12.09 hrs , Volume= | for 25 YR event |
| Outflow | $=$ | $0.94 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $3,439 \mathrm{cf}$ |
| Primary | $=$ | $0.94 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $3,439 \mathrm{cf}$ | Routed to Pond OCS7 : OCS \#7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=203.36' @ 12.95 hrs
Flood Elev= 212.77'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.35' | 12.0" Round Culvert L=31.4' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 202.35' / 202.19' S=0.0051 '/' Cc= 0.900 $\mathrm{n}=0.013$, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.92 cfs @ 12.09 hrs HW=202.94' TW=202.62' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.92 cfs @ 2.76 fps )

## Summary for Pond CB52: CB \#52



Primary OutFlow Max=1.20 cfs @ 12.09 hrs HW=203.37' TW=202.62' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 1.20 cfs @ 2.92 fps)

## Summary for Pond CB53: CB \#53

| Inflow Area $=$ | $7,863 \mathrm{sf}$, | $86.52 \%$ Impervious, | Inflow Depth > 5.47" |
| :--- | :--- | :--- | :--- |
| for 25 YR event |  |  |  |
| Inflow | $=$ | $1.05 \mathrm{cfs} @$ | 12.09 hrs , Volume |
| Outflow | $=$ | $1.05 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |
| Primary | $=$ | $1.05 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | Routed to Pond 3P : OCS \#8

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=203.42' @ 12.09 hrs
Flood Elev=205.95'
Device Routing Invert Outlet Devices
\#1 Primary $\quad 202.78^{\prime} \quad 12.0 "$ Round Culvert L=32.0' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert=202.78' / 202.62' S=0.0050 '/' Cc=0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$
Primary OutFlow Max=1.02 cfs @ 12.09 hrs HW=203.41' TW=201.39' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 1.02 cfs @ 2.82 fps )

## Summary for Pond CB54: CB \#54

| Inflow Area = | 4,821 | 86.85\% Impervious, | w Depth > 5.24" for 25 YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.63 cfs @ | 12.09 hrs , Volume= | 2,104 cf |
| Outflow | 0.63 cfs @ | 12.09 hrs , Volume= | 2,104 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.63 cfs @ | 12.09 hrs , Volume= | 2,104 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 203.14' @ 12.09 hrs
Flood Elev= 205.82'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.66' | 12.0" Round Culvert L=36.7' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=202.66' / 202.48' S=0.0049 '// Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.61 cfs @ 12.09 hrs HW=203.13' TW=201.39' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.61 cfs @ 2.48 fps )

## Summary for Pond CB7: CB\#5

 Routed to Pond D4 : DMH\#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=213.04' @ 12.09 hrs
Flood Elev=215.79'


## Summary for Pond CB8: CB\#8

| Inflow Area = | 5,450 sf, | 88.75\% Impervious, | Inflow Depth > | 5.58" for 25YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow = | 0.73 cfs @ | 12.09 hrs , Volume= | 2,534 cf |  |
| Outflow | 0.73 cfs @ | 12.09 hrs , Volume= | 2,534 cf, | Atten $=0 \%, L a g=0.0 \mathrm{~min}$ |
| Primary | 0.73 cfs @ | 12.09 hrs , Volume= | 2,534 cf |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=214.27' @ 12.09 hrs
Flood Elev= 215.79'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | ---: | :--- | :--- |
| \#1 | Primary | $213.79^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=15.1^{\prime} \mathrm{Ke}=0.500$ |  |

Inlet / Outlet Invert= 213.79' / 213.64' S=0.0099 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.71 cfs @ 12.09 hrs HW=214.27' TW=211.65' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.71 cfs @ 2.84 fps )

## Summary for Pond CB9: CB \#9

| Inflow Area = | $16,307 \mathrm{sf}$, 93 | 93.95\% Impervious, | Inflow Depth > 5.93" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 2.24 cfs @ 12 | 12.09 hrs , Volume= | 8,058 cf |
| Outflow = | 2.24 cfs @ 12 | 12.09 hrs , Volume= | $8,058 \mathrm{cf}$, Atten= $0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pon | $\begin{aligned} & 2.24 \mathrm{cfs} @ 12 \end{aligned}$ | 12.09 hrs , Volume= | 8,058 cf |
| Routing by Dyn-S <br> Peak Elev= 210.9 <br> Flood Elev= 213 | or-Ind method, @ 12.09 hrs | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 210.10' | 12.0" Round Cu Inlet / Outlet Inver $\mathrm{n}=0.013$ Corruga | $\begin{aligned} & \text { Ivert } \mathrm{L}=19.9 ' \mathrm{Ke}=0.500 \\ & t=210.10 \text { ' } / 209.71 \text { ' } \mathrm{S}=0.0196 \text { '/' } \mathrm{Cc}=0.900 \\ & \text { ated PE, smooth interior, Flow Area= } 0.79 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=2.18 cfs @ 12.09 hrs HW=210.94' TW=210.24' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 2.18 cfs @ 3.11 fps )

## Summary for Pond D10: DMH \#10

| Inflow Area $=$ | $8,516 \mathrm{sf}, 64.88 \%$ Impervious, | Inflow Depth > 3.94" | for 25 YR event |  |
| :--- | :---: | :---: | :---: | :---: |
| Inflow | $=$ | $0.88 \mathrm{cfs} @$ | 12.09 hrs , Volume | $2,794 \mathrm{cf}$ |
| Outflow | $=$ | $0.88 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $2,794 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | $=$ | $0.88 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $2,794 \mathrm{cf}$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=203.91' @ 12.09 hrs
Flood Elev= 206.49'


Primary OutFlow Max=0.86 cfs @ 12.09 hrs HW=203.91' TW=197.78' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.86 cfs @ 2.65 fps )

## Summary for Pond D11: DMH \#11

| Inflow Area = | 36,689 | 73.28\% Impervious, | Inflow Depth > 4.77" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 4.32 cfs @ | 12.09 hrs , Volume= | 14,592 cf |
| Outflow | 4.32 cfs @ | 12.09 hrs , Volume= | 14,592 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 4.32 cfs @ | 12.09 hrs , Volume= | 14,592 cf | Routed to Pond OCS3 : OCS\#3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 205.64' @ 12.12 hrs
Flood Elev= 208.33'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $204.25^{\prime}$ | $\mathbf{1 8 . 0 "}$ Round Culvert $\mathrm{L}=44.6^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $204.25^{\prime} / 204.03^{\prime} \quad \mathrm{S}=0.0049$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=1.77 \mathrm{sf}$ |  |

Primary OutFlow Max=4.27 cfs @ 12.09 hrs HW=205.58' TW=205.19' (Dynamic Tailwater)
——1=Culvert (Outlet Controls 4.27 cfs @ 3.42 fps )

## Summary for Pond D12: DMH \#12



## Summary for Pond D13: DMH \#13

| Inflow Area = | 81,632 | 72.61\% Impervious, | Inflow Depth > 5.06" for 25 YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 9.39 cfs @ | 12.09 hrs , Volume= | 34,406 cf |
| Outflow | 9.39 cfs @ | 12.09 hrs , Volume= | $34,406 \mathrm{cf}, \mathrm{Atten}=0 \%$ Lag $=0.0 \mathrm{~min}$ |
| Primary | 9.39 cfs @ | 12.09 hrs , Volume= | 34,406 cf |

Routed to Pond P207 : INFILTRATION POND \#2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=203.59' @ 12.09 hrs
Flood Elev= 208.12'
Device Routing Invert Outlet Devices
\#1 Primary
201.95' 24.0" Round Culvert L=60.1' $\mathrm{Ke}=0.500$

Inlet / Outlet Invert= 201.95' / 201.65' S=0.0050 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 3.14 sf
Primary OutFlow Max=9.26 cfs @ 12.09 hrs HW=203.58' TW=197.80' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 9.26 cfs @ 4.61 fps )

## Summary for Pond D14: DMH \#14

| Inflow Area = | 38,112 sf, | . $0 \%$ Impervious, | Inflow Depth > 5.49" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 5.06 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 17,421 cf |
| Outflow | 5.06 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | $17,421 \mathrm{cf}$, Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pon | 5.06 cfs @ 12 d13 : DMH \#13 | .09 hrs , Volume= | 17,421 cf |
| Routing by Dyn-S <br> Peak Elev= 205.3 <br> Flood Elev= 208 | or-Ind method, @ 12.09 hrs | ime Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 204.13' | 18.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.012$ Corrug | $\begin{aligned} & \text { vert } \mathrm{L}=256.3^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=204.13^{\prime} / 202.85^{\prime} \quad \mathrm{S}=0.0050 \mathrm{l} / \mathrm{Cc}=0.900 \end{aligned}$ $\text { ted PP, smooth interior, Flow Area= } 1.77 \mathrm{sf}$ |

Primary OutFlow Max=4.93 cfs @ 12.09 hrs HW=205.32' TW=203.56' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 4.93 cfs @ 4.51 fps )

## Summary for Pond D16: DMH \#16

| Inflow Area | 16,417 sf, | .69\% Impervious, | Inflow Depth > 5.52" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 2.20 cfs @ | 12.09 hrs , Volume= | 7,558 cf |
| Outflow | 2.20 cfs @ | 12.09 hrs , Volume= | $7,558 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 2.20 cfs @ | 12.09 hrs , Volume= | 7,558 cf | Routed to Pond D14 : DMH \#14

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.84' @ 12.09 hrs
Flood Elev=208.59'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- | :--- |
| $\# 1$ | Primary | $204.90^{\prime}$ | $15.0^{\prime \prime}$ Round Culvert L=103.5' $\mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 204.90' $/ 204.38^{\prime} \quad \mathrm{S}=0.0050 \quad \mathrm{ll}$ | $\mathrm{Cc}=0.900$ |
|  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |  |

Primary OutFlow Max=2.14 cfs @ 12.09 hrs HW=205.82' TW=205.32' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 2.14 cfs @ 3.08 fps )

## Summary for Pond D17: DMH \#17

| Inflow Area $=$ | $14,946 \mathrm{sf}$, | $75.26 \%$ | Impervious, | Inflow Depth $>$ |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $1.98 \mathrm{cfs} @$ | 12.09 hrs , Volume= | for 25 YR event |
| Outflow | $=$ | $1.98 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $6,816 \mathrm{cf}$ |
| Primary | $=$ | $1.98 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $6,816 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ | Routed to Pond D18: DMH \#18

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 201.33' @ 12.09 hrs
Flood Elev= 204.84'


Primary OutFlow Max=1.93 cfs @ 12.09 hrs HW=201.32' TW=198.41' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.93 cfs @ 2.98 fps )

## Summary for Pond D18: DMH \#18



Routed to Pond OCS1 : OCS\#1
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 198.43' @ 12.09 hrs
Flood Elev= 201.13'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 197.44' | 15.0" Round Culvert L= 46.3' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 197.44' / 196.98' S=0.0099 '// Cc= 0.900 $n=0.013$ Corrugated PE, smooth interior, Flow Area= 1.23 sf |

Primary OutFlow Max=3.19 cfs @ 12.09 hrs HW=198.41' TW=196.32' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 3.19 cfs @ 4.29 fps )

## Summary for Pond D19: DMH \#19

| Inflow Area = | 17,428 | 83.29\% Impervious, | fow Depth > 5.58" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 2.34 cfs @ | 12.09 hrs , Volume= | 8,105 cf |
| Outflow | 2.34 cfs @ | 12.09 hrs , Volume= | $8,105 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 2.34 cfs @ | 12.09 hrs , Volume= | 8,105 cf |

Routed to Pond d20 : DMH \#20
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=206.09' @ 12.09 hrs
Flood Elev= 208.57'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $205.19^{\prime}$ | $\mathbf{1 2 . 0 "}$ Round Culvert $\mathrm{L}=82.5^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $205.19^{\prime} / 204.43^{\prime} \quad \mathrm{S}=0.0092$ |
|  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |  |

Primary OutFlow Max=2.28 cfs @ 12.09 hrs HW=206.07' TW=205.11' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 2.28 cfs @ 4.14 fps )

## Summary for Pond D2: DMH\#2

| Inflow Area = | 73,240 sf, | 2\% Impervious, | Inflow Depth > 3.54" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 5.38 cfs @ 12 | .14 hrs, Volume= | 21,579 cf |
| Outflow | 5.38 cfs @ 12 | 14 hrs , Volume= | $21,579 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary = Routed to Pon | $\begin{aligned} & 5.38 \mathrm{cfs} @ 12 \\ & \text { P205: INFILTF } \end{aligned}$ | 14 hrs , Volume= ATION POND \#3 | 21,579 cf |
| Routing by Dyn-S <br> Peak Elev= 208.3 <br> Flood Elev= 212 | r-Ind method, @ 12.14 hrs | ime Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 206.90' | 15.0" Round Culve Inlet / Outlet Inver $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \hline \text { vert } \mathrm{L}=38.2^{\prime} \mathrm{Ke}=0.500 \\ & \mathrm{t}=206.90^{\prime} / 206.52^{\prime} \mathrm{S}=0.0099^{\prime} / / \mathrm{Cc}=0.900 \end{aligned}$ $\text { ted PE, smooth interior, Flow Area= } 1.23 \mathrm{sf}$ |

Primary OutFlow Max=5.33 cfs @ 12.14 hrs HW=208.36' TW=206.33' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 5.33 cfs @ 4.68 fps )

## Summary for Pond D20: DMH \#20

| Inflow Area | 17,428 sf, | 83.29\% Impervious, | Inflo | 58 " for 25YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 2.34 cfs @ | 12.09 hrs , Volume= | 8,105 cf |  |
| Outflow | 2.34 cfs @ | 12.09 hrs , Volume= | 8,105 cf, | Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 2.34 cfs @ | 12.09 hrs , Volume= | 8,105 cf |  |

Routed to Pond D21 : DMH \#21
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.13' @ 12.09 hrs
Flood Elev= 207.68'


Primary OutFlow Max=2.28 cfs @ 12.09 hrs HW=205.11' TW=204.63' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 2.28 cfs @ 3.28 fps )

## Summary for Pond D21: DMH \#21

| Inflow Area = | 71,317 sf, 79.77\% Impervious, | Inflow Depth > 5.47" for 25YR event |
| :---: | :---: | :---: |
| Inflow | 9.46 cfs @ 12.09 hrs , Volume= | 32,518 cf |
| Outflow | 9.46 cfs @ 12.09 hrs , Volume= | 32,518 cf, Atten=0\%, Lag= 0.0 min |
| Primary | 9.46 cfs @ 12.09 hrs , Volume= | 32,518 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 204.66' @ 12.09 hrs
Flood Elev=207.55'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| \#1 | Primary | $203.02 '$ | 24.0" Round Culvert $\mathrm{L}=72.4^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 203.02' $/ 202.66^{\prime} \quad \mathrm{S}=0.0050$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area=3.14 sf |  |

Primary OutFlow Max=9.21 cfs @ 12.09 hrs HW=204.63' TW=202.18' (Dynamic Tailwater)
——1=Culvert (Barrel Controls 9.21 cfs @ 4.64 fps )

## Summary for Pond D22: DMH \#22

| Inflow Area = | 24,814 | 89.39\% Impervious, | Inflow Depth > 5.74" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 3.37 cfs @ | 12.09 hrs , Volume= | 11,876 cf |
| Outflow | 3.37 cfs @ | 12.09 hrs , Volume= | 11,876 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 3.37 cfs @ | 12.09 hrs , Volume= | 11,876 cf |

Routed to Pond d21: DMH \#21
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=205.89' @ 12.09 hrs
Flood Elev= 208.46'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 204.87' | 15.0" Round Culvert L= 134.2' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=204.87' / 203.92' S=0.0071 '// Cc= 0.900 $n=0.013$ Corrugated PE, smooth interior, Flow Area= 1.23 sf |

Primary OutFlow Max=3.28 cfs @ 12.09 hrs HW=205.87' TW=204.63' (Dynamic Tailwater)
——1=Culvert (Barrel Controls 3.28 cfs @ 4.28 fps )

## Summary for Pond D23: DMH \#23



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.31' @ 12.09 hrs
Flood Elev= 210.30'
Device Routing Invert Outlet Devices
\#1 Primary 206.70' 15.0" Round Culvert L= 173.3' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert= 206.70' / 204.97' S=0.0100 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.23 sf
Primary OutFlow Max=1.45 cfs @ 12.09 hrs HW=207.30' TW=205.87' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 1.45 cfs @ 3.63 fps )

## Summary for Pond D25: DMH \#25

| Inflow Area = | 36,995 sf, | .96\% Impervious, | Inflow Depth > 5.55" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 4.84 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 17,114 cf |
| Outflow | 4.84 cfs @ 12 | .09 hrs , Volume= | $17,114 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pon | $\begin{aligned} & 4.84 \mathrm{cfs} @ 12 \\ & \text { P210 : POCKE } \end{aligned}$ | .09 hrs , Volume= T WETLAND \#1 | 17,114 cf |
| Routing by Dyn-S <br> Peak Elev= 210.7 <br> Flood Elev= 213 | r-Ind method, @ 12.09 hrs | 「ime Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 209.36' | 15.0" Round Culu Inlet / Outlet Inve $n=0.012$ Corrug | $\begin{aligned} & \text { Ivert } \mathrm{L}=237.6^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=209.36^{\prime} / 208.17^{\prime} \quad \mathrm{S}=0.0050 \mathrm{l} / \mathrm{Cc}=0.900 \\ & \text { ated PP, smooth interior, Flow Area= } 1.23 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=4.72 cfs @ 12.09 hrs HW=210.73' TW=203.34' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 4.72 cfs @ 4.38 fps )

## Summary for Pond D27: DMH \#27



Routed to Pond D35 : DMH \#35
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=214.18' @ 12.09 hrs
Flood Elev=217.00'


Primary OutFlow Max=2.66 cfs @ 12.09 hrs HW=214.17' TW=213.11' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls 2.66 cfs @ 3.09 fps )

## Summary for Pond D28: DMH \#28

| Inflow Area = | 11,949 | 62.72\% Impervious, | $w$ Depth > 4.51" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.38 cfs @ | 12.09 hrs , Volume= | 4,491 cf |
| Outflow | 1.38 cfs @ | 12.09 hrs , Volume= | $4,491 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.38 cfs @ | 12.09 hrs , Volume= | 4,491 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 218.08' @ 12.09 hrs
Flood Elev= 220.17'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | :--- | :--- | :--- |
| $\# 1$ | Primary | $217.46^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=158.3^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |  |
|  |  | $\mathrm{Inlet} /$ Outlet Invert= $217.46^{\prime} / 214.29^{\prime} \quad \mathrm{S}=0.0200$ |  |  |
| $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |  |  |  |

Primary OutFlow Max=1.35 cfs @ 12.09 hrs HW=218.07' TW=214.17' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.35 cfs @ 2.67 fps )

## Summary for Pond D29: DMH \#29



Routed to Pond D28: DMH \#28
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=220.45' @ 12.09 hrs
Flood Elev= 223.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 219.83' | 12.0" Round Culvert L= 150.9' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=219.83' / $217.55^{\prime} \quad \mathrm{S}=0.0151$ '// Cc=0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=1.35 cfs @ 12.09 hrs HW=220.44' TW=218.07' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls $1.35 \mathrm{cfs} @ 2.67 \mathrm{fps}$ )

## Summary for Pond D30: DMH \#30

| Inflow Area = | 6,862 | 50.73\% Impervious, | Inflow Depth > 4.04" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.73 cfs @ | 12.09 hrs , Volume= | 2,311 cf |
| Outflow | 0.73 cfs @ | 12.09 hrs , Volume= | $2,311 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.73 cfs @ | 12.09 hrs , Volume= | 2,311 cf | Routed to Pond D29: DMH \#29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=221.43' @ 12.09 hrs
Flood Elev= 224.95'
Device Routing Invert Outlet Devices
\#1 Primary 220.92' 12.0" Round Culvert L= 184.2' Ke=0.500
Inlet / Outlet Invert= 220.92' / 220.00' S=0.0050 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=0.71 cfs @ 12.09 hrs HW=221.42' TW=220.44' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.71 cfs @ 2.64 fps )

## Summary for Pond D31: DMH\#31

| Inflow Area = | 63,140 sf, 2 | 28.73\% Impervious, | Inflow Depth > 3.17" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 4.31 cfs @ 12 | 12.16 hrs , Volume= | 16,701 cf |
| Outflow = | 4.31 cfs @ 12 | 12.16 hrs , Volume= | $16,701 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary = | 4.31 cfs @ 12 | 12.16 hrs , Volume= | 16,701 cf |
| Routed to Pond | D4 : DMH\#4 |  |  |
| Routing by Dyn-S <br> Peak Elev= 225.7 <br> Flood Elev= 229 | or-Ind method, @ 12.16 hrs | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 224.63' | 15.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.012$ Corrug | $\begin{aligned} & \text { Ivert } \mathrm{L}=288.5^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=224.63^{\prime} / 213.09^{\prime} \mathrm{S}=0.0400 \text { '/' } \mathrm{Cc}=0.900 \\ & \text { ated PP, smooth interior, Flow Area= } 1.23 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=4.25 cfs @ 12.16 hrs HW=225.77' TW=211.76' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 4.25 cfs @ 3.63 fps)

## Summary for Pond D34: DMH \#34

| Inflow Area = | 23,255 sf,100.00\% Impervious, | flow Depth > 6.05" for 25YR event |
| :---: | :---: | :---: |
| Inflow | 3.21 cfs @ 12.09 hrs , Volume= | 11,720 cf |
| Outflow | 3.21 cfs @ 12.09 hrs , Volume= | $11,720 \mathrm{cf}$, Atten $=0 \%, \mathrm{Lag}=0.0 \mathrm{~min}$ |
| Primary | 3.21 cfs @ 12.09 hrs , Volume= | 11,720 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 199.29' @ 12.09 hrs
Flood Elev=202.00'


Primary OutFlow Max=3.13 cfs @ 12.09 hrs HW=199.25' TW=196.32' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls 3.13 cfs @ 3.98 fps )

## Summary for Pond D35: DMH \#35

| Inflow Area = | 21,746 | pervious, | 5.20" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 2.73 cfs @ | 12.09 hrs , Volume= | 9,429 cf |
| Outflow | 2.73 cfs @ | 12.09 hrs , Volume= | $9,429 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 2.73 cfs @ | 12.09 hrs , Volume= | 9,429 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 213.12' @ 12.09 hrs
Flood Elev=215.70'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 212.28' | 15.0" Round Culvert L= 171.5' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert=212.28' / 209.71' S=0.0150 '// Cc= 0.900 $\mathrm{n}=0.012$, Flow Area $=1.23 \mathrm{sf}$ |

Primary OutFlow Max=2.66 cfs @ 12.09 hrs HW=213.11' TW=210.73' (Dynamic Tailwater) —1=Culvert (Inlet Controls 2.66 cfs @ 3.09 fps)

## Summary for Pond D4: DMH\#4

| Inflow Area = | 73,240 | \% Impervious, | th | 54" for 25YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 5.38 cfs @ | 12.14 hrs , Volume= | 21,579 cf |  |
| Outflow | 5.38 cfs @ | 12.14 hrs , Volume= | 21,579 cf, | Atten= 0\%, Lag= 0.0 min |
| Primary | 5.38 cfs @ | 12.14 hrs , Volume= | 21,579 cf |  | Routed to Pond D2: DMH\#2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=211.79' @ 12.14 hrs
Flood Elev=217.64'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 210.34' | 15.0" Round Culvert L= 222.3' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=210.34' / 207.01' S=0.0150 '//' Cc= 0.900 $n=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |

Primary OutFlow Max=5.33 cfs @ 12.14 hrs HW=211.78' TW=208.36' (Dynamic Tailwater)
_1=Culvert (Inlet Controls $5.33 \mathrm{cfs} @ 4.34 \mathrm{fps}$ )

## Summary for Pond D5: DMH \#5



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=210.26' @ 12.09 hrs
Flood Elev= 212.97'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 209.09' | 18.0" Round Culvert L= 183.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 209.09' / 208.17' S=0.0050 '/' Cc=0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.77 sf |

Primary OutFlow Max=4.08 cfs @ 12.09 hrs HW=210.24' TW=209.17' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 4.08 cfs @ 3.90 fps )

## Summary for Pond D6: DMH \#6

| Inflow Area = | 30,441 sf, 9 | 96.76\% Impervious, | Inflow Depth > 5.98" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 4.20 cfs @ 12 | 12.09 hrs , Volume= | 15,182 cf |
| Outflow | 4.20 cfs @ 12. | 12.09 hrs , Volume $=$ | $15,182 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary = Routed to Pon | 4.20 cfs @ 12 <br> D7 : DMH \#7 | 12.09 hrs , Volume= | 15,182 cf |
| Routing by Dyn-S <br> Peak Elev= 209. <br> Flood Elev= 214 | or-Ind method, @ 12.09 hrs ' | , Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 208.07' | 18.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { lvert } \mathrm{L}=299.7^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=208.077^{\prime} / 206.57^{\prime} \quad \mathrm{S}=0.0050 \mathrm{l} \end{aligned} \mathrm{Cc=0.900}$ $\text { ted PE, smooth interior, Flow Area= } 1.77 \text { sf }$ |

Primary OutFlow Max=4.08 cfs @ 12.09 hrs HW=209.17' TW=207.44' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 4.08 cfs @ 4.11 fps )

## Summary for Pond D7: DMH \#7

| Inflow Area | 30,441 sf, | 96.76\% Impervious, | Infl | 98" for 25YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 4.20 cfs @ | 12.09 hrs , Volume= | 15,182 cf |  |
| Outflow | 4.20 cfs @ | 12.09 hrs , Volume= | 15,182 cf, | Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 4.20 cfs @ | 12.09 hrs , Volume= | 15,182 cf |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.46' @ 12.09 hrs
Flood Elev=213.17'


Primary OutFlow Max=4.08 cfs @ 12.09 hrs HW=207.44' TW=202.18' (Dynamic Tailwater)
_1 $^{1}=$ Culvert (Inlet Controls 4.08 cfs @ 3.36 fps )

## Summary for Pond D8: DMH \#8



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 201.79' @ 12.09 hrs
Flood Elev= 204.72'


Primary OutFlow Max=2.43 cfs @ 12.09 hrs HW=201.76' TW=201.14' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 2.43 cfs @ 3.30 fps )

## Summary for Pond D9: DMH \#9

| Inflow Area | 18,765 sf, | pervio | Inflow Depth > 5.53" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 2.49 cfs @ | 12.09 hrs , Volume= | 8,641 cf |
| Outflow | 2.49 cfs @ | 12.09 hrs , Volume $=$ | $8,641 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | 2.49 cfs @ | 12.09 hrs , Volume= | 8,641 cf | Routed to Pond P207 : INFILTRATION POND \#2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=201.16' @ 12.09 hrs
Flood Elev= 204.80'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 200.03' | 12.0" Round Culvert L= 11.9' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert=200.03' / 199.97' S=0.0050 '// Cc= 0.900 <br> $n=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |

Primary OutFlow Max=2.43 cfs @ 12.09 hrs HW=201.14' TW=197.77' (Dynamic Tailwater)
——1=Culvert (Barrel Controls 2.43 cfs @ 3.48 fps )

## Summary for Pond DE61: DRIP \#61



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> $(\%)$ | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 212.19 | 665 | 0.0 | 0 | 0 |
| 212.20 | 665 | 40.0 | 3 | 3 |
| 214.19 | 665 | 40.0 | 529 | 532 |
| 214.20 | 665 | 100.0 | 7 | 539 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 214.10' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
| \#2 | Primary | 212.70' | Coef. (English) 2.802 .923 .083 .303 .32 |
|  |  |  | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 212.70' / 212.65' S=0.0050 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.20 sf |
| \#3 | Discarded | 212.19' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.00 cfs @ 4.10 hrs HW=212.21' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.48 cfs @ 12.14 hrs HW=213.33' TW=208.06' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.48 cfs @ 2.49 fps )

## Summary for Pond DE62: DRIP \#62

| Inflow Area = | 4,247 sf, | 92.68\% Impervious, | Inflow Depth > 5.81" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.58 cfs @ | 12.09 hrs , Volume= | 2,057 cf |
| Outflow | 0.48 cfs @ | 12.14 hrs , Volume= | 1,911 cf, Atten= 17\%, Lag= 3.4 min |
| Discarded $=$ | 0.00 cfs @ | 4.10 hrs , Volume= | 203 cf |
| Primary | 0.48 cfs @ | 12.14 hrs , Volume= | 1,708 cf | Routed to Reach 8R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=213.33' @ 12.14 hrs Surf.Area= 665 sf Storage= 303 cf
Plug-Flow detention time $=69.7 \mathrm{~min}$ calculated for 1,907 cf ( $93 \%$ of inflow)
Center-of-Mass det. time= 31.8 min (788.6-756.8)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $212.19^{\prime}$ | 539 cf | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 212.19 | 665 | 0.0 | 0 | 0 |
| 212.20 | 665 | 40.0 | 3 | 3 |
| 214.19 | 665 | 40.0 | 529 | 532 |
| 214.20 | 665 | 100.0 | 7 | 539 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 214.10' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 212.70' | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 212.70' $/ 212.65$ ' S=0.0050 '/l' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.20 \mathrm{sf}$ |
| \#3 | Discarded | 212.19' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01 ' |

Discarded OutFlow Max=0.00 cfs @ 4.10 hrs HW=212.21' (Free Discharge)
$\complement_{3=\text { Exfiltration (Exfiltration Controls } 0.00 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=0.48 cfs @ 12.14 hrs HW=213.33' TW=208.06' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs )
-2=Culvert (Barrel Controls 0.48 cfs @ 2.49 fps )

## Summary for Pond DE63: DRIP \#63



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 206.99 | 404 | 0.0 | 0 | 0 |
| 207.00 | 404 | 40.0 | 2 | 2 |
| 208.99 | 404 | 40.0 | 322 | 323 |
| 209.00 | 404 | 100.0 | 4 | 327 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 208.90' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.400 .60 \quad 0.801 .00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 207.50' | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 207.50' / 207.45 ' S=0.0050 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.20 \mathrm{sf}$ |
| \#3 | Discarded | 206.99' | 0.170 in/hr Exfiltration over Surface area Phase-In=0.01' |

Discarded OutFlow Max=0.00 cfs @ 4.30 hrs HW=207.01' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.36 cfs @ 12.12 hrs HW=208.00' TW=202.10' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
-2=Culvert (Barrel Controls 0.36 cfs @ 2.31 fps )

## Summary for Pond DE64: DRIP \#64

| Inflow Area = | 3,470 sf, | pervious, | Depth > 5 | 81" for 25YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 0.47 cfs @ | 12.09 hrs , Volume= | 1,681 cf |  |
| Outflow | 0.42 cfs @ | 12.13 hrs , Volume= | 1,578 cf, | Atten= 11\%, Lag= 2.5 min |
| Discarded | 0.00 cfs @ | 3.80 hrs , Volume= | 144 cf |  |
| Primary | 0.42 cfs @ | 12.13 hrs , Volume= | 1,433 cf |  | Routed to Reach 12R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.06' @ 12.13 hrs Surf.Area= 470 sf Storage= 201 cf
Plug-Flow detention time $=62.4 \mathrm{~min}$ calculated for 1,578 cf ( $94 \%$ of inflow)
Center-of-Mass det. time= 28.2 min (785.0-756.8)


Discarded OutFlow Max=0.00 cfs @ 3.80 hrs HW=205.01' (Free Discharge)
$L^{-}=$Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.41 cfs @ 12.13 hrs HW=206.05' TW=202.10' (Dynamic Tailwater)

- $1=$ Broad-Crested Rectangular Weir ( Controls 0.00 cfs )
$\mathbf{2 = C u l v e r t}$ (Barrel Controls 0.41 cfs @ 2.39 fps )


## Summary for Pond DE65: DRIP \#65

| Inflow Area = | 3,016 | 88.69\% Impervious, | Inflow Depth > 5.70" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.41 cfs @ | 12.09 hrs , Volume= | 1,432 cf |
| Outflow | 0.37 cfs @ | 12.12 hrs , Volume= | $1,343 \mathrm{cf}$, Atten= 8\%, Lag= 2.2 min |
| Discarded | 0.00 cfs @ | 4.30 hrs , Volume= | 121 cf |
| Primary | 0.37 cfs @ | 12.12 hrs , Volume= | 1,222 cf | Routed to Reach 12R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.01' @ 12.12 hrs Surf.Area= 404 sf Storage= 165 cf
Plug-Flow detention time $=60.7$ min calculated for 1,343 cf ( $94 \%$ of inflow)
Center-of-Mass det. time $=26.6 \mathrm{~min}(788.7-762.1)$


Discarded OutFlow Max=0.00 cfs @ 4.30 hrs HW=206.01' (Free Discharge)
L3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.36 cfs @ 12.12 hrs HW=207.00' TW=202.10' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
-2=Culvert (Barrel Controls 0.36 cfs @ 2.31 fps )

## Summary for Pond DE66: DRIP \#66

| Inflow Area = | 3,407 sf, | pervious, | Depth > | 81" for 25YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 0.47 cfs @ | 12.09 hrs , Volume= | 1,650 cf |  |
| Outflow | 0.42 cfs @ | 12.13 hrs , Volume= | 1,547 cf, | Atten= 11\%, Lag= 2.5 min |
| Discarded | 0.00 cfs @ | 3.15 hrs , Volume= | 144 cf |  |
| Primary | 0.41 cfs @ | 12.13 hrs , Volume= | 1,403 cf |  | Routed to Reach 12R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 208.85' @ 12.13 hrs Surf.Area= 470 sf Storage= 199 cf
Plug-Flow detention time= 62.6 min calculated for 1,544 cf ( $94 \%$ of inflow)
Center-of-Mass det. time= $28.5 \mathrm{~min}(785.2-756.8$ )

| Volume | Invert207.79 | t Avail.Storage |  | Storage Description |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| \#1 |  | ' 381 cf Custom Stage Data (Prismatic)Listed below (Recalc) |  |  |  |
| $\begin{array}{r} \text { Elevation } \\ \text { (feet) } \end{array}$ |  | Surf.Area (sq-ft) | Voids (\%) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
| 207.79 |  | 470 | 0.0 | 0 | 0 |
| 207.80 |  | 470 | 40.0 | 2 | 2 |
| 209.79 |  | 470 | 40.0 | 374 | 376 |
| 209.80 |  | 470 | 100.0 | 5 | 381 |
| Device R | Routing | Invert Outlet Devices |  |  |  |
| \#1 | Primary | 209.7 | $0^{\prime} \quad 180.0$ ' long x 0.5' breadth Broad-Crested Rectangular Weir <br> Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |  |  |
| \#2 | Primary | 208 | ( 6.0' Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |  |  |
|  |  |  | Inlet / Outlet Invert= 208.30' 208.25 ' S=0.0050 '/' Cc= 0.900 |  |  |
| \#3 | Discarded | d 207.79' 0.1 | $9^{\prime} \quad 0.170$ in/hr Exfiltration over Surface area Phase-In= 0.01 ' |  |  |

Discarded OutFlow Max=0.00 cfs @ 3.15 hrs HW=207.80' (Free Discharge)
$L^{-} 3=$ Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.41 cfs @ 12.13 hrs HW=208.84' TW=202.10' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.41 cfs @ 2.38 fps )

## Summary for Pond DE67: DRIP \#67



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 207.99 | 470 | 0.0 | 0 | 0 |
| 208.00 | 470 | 40.0 | 2 | 2 |
| 209.99 | 470 | 40.0 | 374 | 376 |
| 210.00 | 470 | 100.0 | 5 | 381 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| \#1 | Primary | $209.90 '$ | 180.0' long $\times$ 0.5' breadth Broad-Crested Rectangular Weir |
|  |  |  | Head (feet) 0.20 $0.400 .600 .80 \quad 1.00$ |
| Coef. (English) $2.80 \quad 2.923 .083 .303 .32$ |  |  |  |

Discarded OutFlow Max=0.00 cfs @ 3.80 hrs HW=208.01' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.41 cfs @ 12.13 hrs HW=209.05' TW=208.05' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
-2=Culvert (Barrel Controls 0.41 cfs @ 2.39 fps )

## Summary for Pond DE68: DRIP \#68

| Inflow Area = | 4,212 sf, | 92.62\% Impervious, | Inflow Depth > 5.81" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.58 cfs @ | 12.09 hrs , Volume= | 2,040 cf |
| Outflow | 0.49 cfs @ | 12.14 hrs , Volume= | 1,897 cf, Atten= 14\%, Lag= 3.1 min |
| Discarded | 0.00 cfs @ | 4.15 hrs , Volume= | 203 cf |
| Primary | 0.49 cfs @ | 12.14 hrs , Volume= | 1,694 cf | Routed to Pond OCS4 : OCS\#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 208.02' @ 12.14 hrs Surf.Area= 665 sf Storage= 274 cf
Plug-Flow detention time $=68.3 \mathrm{~min}$ calculated for 1,897 cf ( $93 \%$ of inflow)
Center-of-Mass det. time= $30.2 \mathrm{~min}(787.0-756.8)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $206.99^{\prime}$ | 539 cf | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 206.99 | 665 | 0.0 | 0 | 0 |
| 207.00 | 665 | 40.0 | 3 | 3 |
| 208.99 | 665 | 40.0 | 529 | 532 |
| 209.00 | 665 | 100.0 | 7 | 539 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 208.90' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 207.50' | 6.0" Round Culvert L=20.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 207.50' / 206.00' S= 0.0750 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.20 sf |
| \#3 | Discarded | 206.99' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01 ' |

Discarded OutFlow Max=0.00 cfs @ 4.15 hrs HW=207.01' (Free Discharge)
$L_{3=\text { Exfiltration (Exfiltration Controls } 0.00 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=0.49 cfs @ 12.14 hrs HW=208.01' TW=204.88' (Dynamic Tailwater)

- 1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs )
-2=Culvert (Inlet Controls 0.49 cfs @ 2.47 fps )


## Summary for Pond DE69: DRIP \#69



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 205.49 | 470 | 0.0 | 0 | 0 |
| 205.50 | 470 | 40.0 | 2 | 2 |
| 207.49 | 470 | 40.0 | 374 | 376 |
| 207.50 | 470 | 100.0 | 5 | 381 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 207.40' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 206.00' | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=206.00' 205.95 ' S=0.0050 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.20 sf |
| \#3 | Discarded | 205.49' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In=0.01' |

Discarded OutFlow Max=0.00 cfs @ 3.80 hrs HW=205.51' (Free Discharge)
$\complement_{3=\text { Exfiltration (Exfiltration Controls } 0.00 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=0.41 cfs @ 12.13 hrs HW=206.55' TW=202.43' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
-2=Culvert (Barrel Controls 0.41 cfs @ 2.39 fps )

## Summary for Pond DE70: DRIP \#70

| Inflow Area = | 3,476 | 60\% Impervious, | Inflow Depth > 5.81" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.48 cfs @ | 12.09 hrs , Volume= | 1,684 cf |
| Outflow | 0.42 cfs @ | 12.13 hrs , Volume= | 1,581 cf, Atten= 11\%, Lag= 2.5 min |
| Discarded = | 0.00 cfs @ | 3.15 hrs , Volume= | 144 cf |
| Primary | 0.42 cfs @ | 12.13 hrs , Volume= | 1,436 cf | Routed to Pond P212 : INFILTRATION POND \#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.96' @ 12.13 hrs Surf.Area= 470 sf Storage= 201 cf
Plug-Flow detention time $=61.8 \mathrm{~min}$ calculated for 1,577 cf ( $94 \%$ of inflow)
Center-of-Mass det. time= 28.2 min (785.0-756.8)


Discarded OutFlow Max=0.00 cfs @ 3.15 hrs HW=205.90' (Free Discharge)
$L^{-}=$Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.41 cfs @ 12.13 hrs HW=206.95' TW=202.43' (Dynamic Tailwater)

- 1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
$\mathbf{2 = C u l v e r t}$ (Barrel Controls 0.41 cfs @ 2.39 fps )


## Summary for Pond DE71: DRIP \#71

| Inflow Area = | 4,210 | 92.61\% Impervious, | Inflow Depth > 5.81" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.58 cfs @ | 12.09 hrs , Volume= | 2,039 cf |
| Outflow | 0.48 cfs @ | 12.14 hrs , Volume= | 1,893 cf, Atten= 17\%, Lag= 3.4 min |
| Discarded | 0.00 cfs @ | 4.50 hrs , Volume= | 203 cf |
| Primary | 0.48 cfs @ | 12.14 hrs , Volume= | 1,690 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 207.62' @ 12.14 hrs Surf.Area= 665 sf Storage= 302 cf
Plug-Flow detention time $=70.6 \mathrm{~min}$ calculated for 1,893 cf ( $93 \%$ of inflow)
Center-of-Mass det. time= 32.0 min (788.7-756.8)


Discarded OutFlow Max=0.00 cfs @ 4.50 hrs HW=206.52' (Free Discharge)
L $3=$ Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.47 cfs @ 12.14 hrs HW=207.62' TW=202.51' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.47 cfs @ 2.48 fps )

## Summary for Pond DECH: DRIP \#CH

| Inflow Area = | 6,262 sf | 92.70\% Impervious, | Depth > 5 | 5.81" for 25YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 0.86 cfs @ | 12.09 hrs , Volume= | 3,033 cf |  |
| Outflow | 0.45 cfs @ | 12.27 hrs , Volume= | 3,033 cf, | Atten $=47 \%, L a g=11.3 \mathrm{~min}$ |
| Discarded = | 0.04 cfs @ | 10.10 hrs , Volume= | 1,659 cf |  |
| Primary | 0.42 cfs @ | 12.27 hrs , Volume= | 1,373 cf |  | Routed to Pond CB18 : CB \#18

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 209.99' @ 12.22 hrs Surf.Area= 636 sf Storage= 508 cf
Plug-Flow detention time= 21.3 min calculated for 3,026 cf (100\% of inflow)
Center-of-Mass det. time= 21.1 min (777.9-756.8)


## Summary for Pond DMH32: DMH \#32



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=203.60' @ 12.09 hrs
Flood Elev=206.16'


## Summary for Pond OCS1: OCS\#1

| Inflow Area = | 48,573 s | 81.99\% Impervious, | Inflow Depth > 5.63" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 6.49 cfs @ | 12.09 hrs , Volume= | 22,774 cf |
| Outflow | 6.49 cfs @ | 12.09 hrs , Volume= | 22,774 cf, Atten=0\%, Lag= 0.0 min |
| Primary | 6.49 cfs @ | 12.09 hrs , Volume= | 22,774 cf |

Routed to Pond P206 : STORMTECH INFILTRATION SYSTEM \#2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev= 196.41' @ 12.16 hrs
Flood Elev= 201.48'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $195.00^{\prime}$ | $\mathbf{2 4 . 0} \mathbf{0}^{\prime \prime}$ Vert. Orifice/Grate $\quad$ C= $0.600 \quad$ Limited to weir flow at low heads |

Primary OutFlow Max=6.35 cfs @ 12.09 hrs HW=196.32' TW=195.96' (Dynamic Tailwater)
—1=Orifice/Grate (Orifice Controls 6.35 cfs @ 2.89 fps )

## Summary for Pond OCS3: OCS\#3



Primary OutFlow Max=6.59 cfs @ 12.09 hrs HW=205.18' TW=204.58' (Dynamic Tailwater)
—1=Orifice/Grate (Orifice Controls 6.59 cfs @ 3.73 fps )

## Summary for Pond OCS4: OCS\#4

| Inflow Area = | 17,972 sf, 28.85\% Impervious, | Inflow Depth > 3.83" for 25YR event |
| :---: | :---: | :---: |
| Inflow | 1.75 cfs @ 12.10 hrs , Volume= | 5,738 cf |
| Outflow | 1.75 cfs @ 12.10 hrs , Volume= | 5,738 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.75 cfs @ 12.10 hrs , Volume= | 5,738 cf |

Routed to Pond P204 : STORMTECH INFILTRATION SYSTEM \#1
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.10' @ 12.26 hrs
Flood Elev= 208.00'
Device Routing Invert Outlet Devices
\#1 Primary 203.10' 18.0" Vert. Orifice/Grate $\quad$ C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=1.75 cfs @ 12.10 hrs HW=204.69' TW=204.64' (Dynamic Tailwater)
_1 $^{1}$ Orifice/Grate (Orifice Controls 1.75 cfs @ 0.99 fps )

## Summary for Pond OCS6: OCS \#6



Primary OutFlow Max=2.15 cfs @ 12.09 hrs HW=202.51' TW=202.19' (Dynamic Tailwater)
—1=Orifice/Grate (Orifice Controls 2.15 cfs @ 2.74 fps )

## Summary for Pond OCS7: OCS \#7



Primary OutFlow Max=2.21 cfs @ 12.09 hrs HW=202.62' TW=202.19' (Dynamic Tailwater)
\&1=Orifice/Grate (Orifice Controls 2.21 cfs @ 3.13 fps )

## Summary for Pond P204: STORMTECH INFILTRATION SYSTEM \#1

| Inflow Area = | 72,222 s | 68.72\% Impervious, | Inflow Depth > 4.85" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 8.49 cfs @ | 12.09 hrs , Volume= | 29,181 cf |
| Outflow | 4.39 cfs @ | 12.27 hrs , Volume= | 26,780 cf, Atten= 48\%, Lag= 10.5 min |
| Discarded | 0.09 cfs @ | 8.30 hrs , Volume= | 6,064 cf |
| Primary | 4.31 cfs @ | 12.27 hrs , Volume= | 20,716 cf |

Routed to Reach 20r : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 205.09' @ 12.27 hrs Surf.Area= 5,670 sf Storage= 9,827 cf Flood Elev=208.75' Surf.Area=5,670 sf Storage= 13,379 cf

Plug-Flow detention time $=96.7 \mathrm{~min}$ calculated for 26,780 cf ( $92 \%$ of inflow)
Center-of-Mass det. time $=55.0 \mathrm{~min}$ ( 828.7-773.8)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1A | 202.50' | 5,923 cf | $77.50^{\prime} \mathrm{W} \times 67.70^{\prime} \mathrm{L} \times 4.08$ 'H STORMTECH SC-740 <br> 21,423 cf Overall $-6,615$ cf Embedded $=14,808$ cf $\times 40.0 \%$ Voids |
| \#2A | 203.08' | 6,615 cf | ADS_StormTech SC-740 +Capx 144 Inside \#1 <br> Effective Size= 44.6 "W x $30.0^{\prime \prime} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12 \mathrm{~L}=45.9 \mathrm{cf}$ Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with $0.44^{\prime}$ Overlap 144 Chambers in 16 Rows |
| \#3B | 202.50' | 427 cf | $6.25^{\prime} \mathrm{W} \times 67.70^{\prime} \mathrm{L} \times 3.50^{\prime} \mathrm{H}$ ISOLATOR ROW <br> 1,481 cf Overall -413 cf Embedded $=1,067$ cf $\times 40.0 \%$ Voids |
| \#4B | 203.00' | 413 cf | ADS_StormTech SC-740 +Capx 9 Inside \#3 <br> Effective Size $=44.6 \mathrm{~W} \mathrm{~W} \times 30.0^{\prime \prime} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12 \mathrm{~L}=45.9 \mathrm{cf}$ Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with $0.44^{\prime}$ Overlap |

13,379 cf Total Available Storage
Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.75' | 15.0" Round Culvert L=35.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 202.75' / 201.00' S=0.0500 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |
| \#2 | Device 1 | 204.75' | 4.0' long x 0.5 ' breadth Broad-Crested Rectangular Weir |
|  |  |  | Head (feet) 0.200 .400 .600 .801 .00 |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#3 | Device 1 | 203.25' | 8.0" Vert. Orifice/Grate C=0.600 Limited to weir flow at low heads |
| \#4 | Discarded | 202.50' | 0.660 in/hr Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.09 cfs @ 8.30 hrs HW=202.56' (Free Discharge)
L4=Exfiltration (Exfiltration Controls 0.09 cfs )
Primary OutFlow Max=4.25 cfs @ 12.27 hrs HW=205.08' TW=200.12' (Dynamic Tailwater)
4-1=Culvert (Passes 4.25 cfs of 7.72 cfs potential flow)
-2=Broad-Crested Rectangular Weir(Weir Controls 2.20 cfs @ 1.66 fps )
—3=Orifice/Grate (Orifice Controls 2.06 cfs @ 5.89 fps )

## Summary for Pond P205: INFILTRATION POND \#3

| Inflow Area = | 88,676 sf, | 39.42\% Impervious, | Depth > 3 | 3.61" for 25YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 6.85 cfs @ | 12.12 hrs , Volume= | 26,643 cf |  |
| Outflow | 3.69 cfs @ | 12.37 hrs , Volume= | 22,614 cf, | Atten $=46 \%, L a g=14.9 \mathrm{~min}$ |
| Discarded | 0.31 cfs @ | 12.37 hrs , Volume= | 15,146 cf |  |
| Primary | 3.39 cfs @ | 12.37 hrs , Volume= | 7,468 cf |  | Routed to Reach 18R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 206.84' @ 12.37 hrs Surf.Area= 5,490 sf Storage= 8,662 cf
Plug-Flow detention time $=169.0$ min calculated for 22,567 cf ( $85 \%$ of inflow)
Center-of-Mass det. time $=104.0 \mathrm{~min}$ ( 918.2 - 814.2 )


Discarded OutFlow Max=0.31 cfs @ 12.37 hrs HW=206.84' (Free Discharge)
—2=Exfiltration (Exfiltration Controls 0.31 cfs)
Primary OutFlow Max=3.31 cfs @ 12.37 hrs HW=206.84' TW=203.04' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir(Weir Controls 3.31 cfs @ 1.17 fps )

## Summary for Pond P206: STORMTECH INFILTRATION SYSTEM \#2

| Inflow Area = | 59,746 sf, | 80.70\% Impervious, | Inflow Depth > 5.60" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 7.97 cfs @ | 12.09 hrs , Volume= | 27,862 cf |
| Outflow | 4.16 cfs @ | 12.23 hrs , Volume= | $27,859 \mathrm{cf}$, Atten= 48\%, Lag= 8.5 min |
| Discarded = | 0.49 cfs @ | 11.15 hrs , Volume= | 21,285 cf |
| Primary | 3.67 cfs @ | 12.23 hrs , Volume= | 6,574 cf | Routed to Link AP4 : ANALYSIS POINT \#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 196.29' @ 12.23 hrs Surf.Area= 6,072 sf Storage= 6,969 cf
Plug-Flow detention time $=60.4$ min calculated for 27,801 cf ( $100 \%$ of inflow)
Center-of-Mass det. time=60.2 min ( 822.3-762.1)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1A | 194.67' | 1,786 cf | 39.50'W x 53.46'L x 3.33'H FIELD A |
|  |  |  | 7,038 cf Overall - 2,573 cf Embedded $=4,466$ cf $\times 40.0 \%$ Voids |
| \#2A | 195.00' | 2,573 cf | ADS_StormTech SC-740 +Capx 56 Inside \#1 |
|  |  |  | Effective Size $=44.6$ "W $\times 30.0$ "H $=>6.45 \mathrm{sf} \times 7.12 \mathrm{~L}=45.9 \mathrm{cf}$ |
|  |  |  | Overall Size $=51.0$ "W $\times 30.0$ 'H $\times 7.566^{\prime} \mathrm{L}$ with $0.44{ }^{\prime}$ Overlap |
|  |  |  | 56 Chambers in 8 Rows |
| \#3B | 194.67' | 3,296 cf | 58.50'W x 67.70'L x 3.33'H FIELD B |
|  |  |  | 13,201 cf Overall - 4,962 cf Embedded $=8,239$ cf $\times 40.0 \%$ Voids |
| \#4B | 195.00' | 4,962 cf | ADS_StormTech SC-740 +Capx 108 Inside \#3 |
|  |  |  | Effective Size $=44.6$ " $\mathrm{W} \times 30.0{ }^{\prime \prime} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12^{\prime} \mathrm{L}=45.9 \mathrm{cf}$ |
|  |  |  | Overall Size $=51.0$ "W x 30.0"H $\times 7.56{ }^{\prime} \mathrm{L}$ with 0.44 ' Overlap |
|  |  |  | 108 Chambers in 12 Rows |
|  |  | 12,616 cf | Total Available Storage |

Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 194.65' | 18.0" Round Culvert L=30.0' $\mathrm{Ke}=0.200$ |
|  |  |  | Inlet / Outlet Invert= 194.65' / 194.50' S=0.0050 '//' Cc= 0.900 $n=0.013$ Corrugated PE, smooth interior, Flow Area $=1.77$ sf |
| \#2 | Device 1 | 195.85' | 4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) |
| \#3 | Discarded | 194.67' | $3.500 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.49 cfs @ 11.15 hrs HW=194.71' (Free Discharge)
$\complement_{3=\text { Exfiltration (Exfiltration Controls } 0.49 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=3.64 cfs @ 12.23 hrs HW=196.28' TW=0.00' (Dynamic Tailwater)
亡-1=Culvert (Passes 3.64 cfs of 7.27 cfs potential flow)


## Summary for Pond P207: INFILTRATION POND \#2



| Prepared by Howard Stein Hudson Associates <br> HydroCAD® <br> 10.20-3c s/n 02930 <br> © 2023 HydroCAD Software Solution |  |  |  |
| ---: | ---: | ---: | ---: |
| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| 196.80 | 10,100 | 0 | 0 |
| 198.00 | 12,000 | 13,260 | 13,260 |
| 200.00 | 15,000 | 27,000 | 40,260 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 198.80' | 20.0' long x 21.0' breadth Broad-Crested Rectangular Weir Head (feet) $0.200 .400 .60 \quad 0.801 .001 .201 .401 .60$ |
| \#2 | Primary |  | Coef. (English) 2.682 .702 .702 .642 .632 .642 .642 .63 |
|  |  | 194.75' | 15.0" Round Culvert L= 40.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $194.75^{\prime} / 194.55^{\prime} \quad \mathrm{S}=0.0050$ '/l' Cc= 0.900 $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |
| \#3 | Device 2 | 198.80' | 6.0" x 6.0" Horiz. Orifice/Grate X 6.00 columns |
|  |  |  | X 6 rows C= 0.600 in 48.0" x 48.0" Grate (56\% open area) |
|  |  |  | Limited to weir flow at low heads |
| \#4 | Device 2 | 197.40' | 8.0" Vert. Orifice/Grate $\mathrm{C}=0.600$ Limited to weir flow at low heads |
| \#5 | Discarded | 196.80' | 3.690 in/hr Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=1.09 cfs @ 12.61 hrs HW=198.50' (Free Discharge)
-5=Exfiltration (Exfiltration Controls 1.09 cfs)
Primary OutFlow Max=1.47 cfs @ 12.61 hrs HW=198.50' TW=192.24' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs)
2=Culvert (Passes 1.47 cfs of 10.45 cfs potential flow)

- $3=$ Orifice/Grate ( Controls 0.00 cfs )

4=Orifice/Grate (Orifice Controls 1.47 cfs @ 4.22 fps)

## Summary for Pond P210: POCKET WETLAND \#1



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: |
| 199.00 | 218 | 0 | 0 |
| 200.00 | 294 | 256 | 256 |
| 201.00 | 376 | 335 | 591 |
| 202.00 | 3,991 | 2,184 | 2,775 |
| 204.00 | 8,073 | 12,064 | 14,839 |
| 206.00 | 13,272 | 21,345 | 36,184 |
| 206.50 | 14,753 | 7,006 | 43,190 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 205.10' | 20.0' long x 15.0' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.400 .60 \quad 0.801 .001 .201 .401 .60$ |
|  |  |  | Coef. (English) 2.682 .702 .702 .642 .632 .642 .642 .63 |
| \#2 | Primary | 202.25' | 12.0" Round Culvert L=44.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 202.25' / 202.03' S=0.0050 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| \#3 | Device 2 | 202.30' | 2.5" Vert. Orifice/Grate $\mathrm{C}=0.600$ Limited to weir flow at low heads |
| \#4 | Device 2 | 204.50' | 6.0 " x 6.0" Horiz. Orifice/Grate X 6.00 columns |
|  |  |  | X 6 rows $\mathrm{C}=0.600$ in 48.0 " $\times 48.0$ " Grate ( $56 \%$ open area) |
|  |  |  | Limited to weir flow at low heads |

Primary OutFlow Max=0.23 cfs @ 15.93 hrs HW=204.35' TW=202.06' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs )
$-2=C u l v e r t$ (Passes 0.23 cfs of 4.26 cfs potential flow)
-3=Orifice/Grate (Orifice Controls 0.23 cfs @ 6.71 fps)
4=Orifice/Grate (Controls 0.00 cfs)

## Summary for Pond P212: INFILTRATION POND \#1



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) | Wet.Area <br> (sq-ft) |
| ---: | ---: | ---: | ---: | ---: |
| 200.00 | 4,354 | 0 | 0 | 4,354 |
| 201.00 | 9,360 | 6,699 | 6,699 | 9,368 |
| 202.00 | 10,993 | 10,166 | 16,865 | 11,040 |
| 204.00 | 13,976 | 24,909 | 41,774 | 14,126 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.50' | 25.0' long x 20.0' breadth Broad-Crested Rectangular |
|  |  |  | Head (feet) 0.200 .400 .600 .801 .001 .201 .401 .60 |
|  |  |  | Coef. (English) 2.682 .702 .702 .642 .632 .642 .642 .63 |
| \#2 | Discarded | 200.00' | $5.130 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= $0.01^{\prime}$ |

Discarded OutFlow Max=1.44 cfs @ 12.30 hrs HW=202.81' (Free Discharge)
L2=Exfiltration (Exfiltration Controls 1.44 cfs)
Primary OutFlow Max=11.56 cfs @ 12.30 hrs HW=202.81' TW=200.19' (Dynamic Tailwater)
$L_{1=B r o a d-C r e s t e d ~ R e c t a n g u l a r ~ W e i r(W e i r ~ C o n t r o l s ~}^{11.56}$ cfs @ 1.50 fps )

## Summary for Pond P213: Stormtech Infiltration System \#3



Storage Group A created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.30' | 12.0" Round Culvert L=60.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 202.30' / 202.00' S=0.0050 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area= 0.79 sf |
| \#2 | Discarded | 200.95' | $5.130 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In=0.01' |
| \#3 | Device 1 | 204.25' | 4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) |

\#4 Device $1 \quad$ 203.35' 6.0" W x 4.0" H Vert. Orifice/Grate C= 0.600
Limited to weir flow at low heads
Discarded OutFlow Max=0.39 cfs @ 11.50 hrs HW=200.99' (Free Discharge)
L2=Exfiltration (Exfiltration Controls 0.39 cfs )
Primary OutFlow Max=0.00 cfs @ 12.96 hrs HW=203.36' TW=202.58' (Dynamic Tailwater)
-1=Culvert (Passes 0.00 cfs of 2.35 cfs potential flow)
-3=Sharp-Crested Rectangular Weir( Controls 0.00 cfs )
4=Orifice/Grate (Orifice Controls 0.00 cfs @ 0.30 fps)

## Summary for Pond P214: STORMTECH INFILTRATION SYSTEM \#4

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=3)


$$
6,797 \text { cf Total Available Storage }
$$

Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard
Device Routing Invert Outlet Devices
\#1 Primary 201.00' 12.0" Round Culvert L= 25.0' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert= 201.00' / 200.88' S=0.0048 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
\#2 Discarded 200.00' $8.280 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01'
\#3 Device $1 \quad 203.40$ 4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
\#4 Device $1 \quad 202.40^{\prime} \quad$ 8.0' Vert. Orifice/Grate $C=0.600$ Limited to weir flow at low heads
Discarded OutFlow Max=0.61 cfs @ 11.70 hrs HW=200.06' (Free Discharge)
L2=Exfiltration (Exfiltration Controls 0.61 cfs)


Summary for Link AP1: ANALYSIS POINT 1

| Inflow Area $=$ | $9,943 \mathrm{sf}$, 92.79\% Impervious, | Inflow Depth > 5.70" | for 25 YR event |  |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | 1.35 cfs @ | 12.09 hrs , Volume | $4,720 \mathrm{cf}$ |
| Primary | $=$ | $1.35 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $4,720 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Primary outflow $=$ Inflow, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$

## Summary for Link AP2: ANALYSIS POINT 2

| Inflow Area $=$ | $816,898 \mathrm{sf}$, | $39.51 \%$ Impervious, | Inflow Depth > 3.37" | for 25 YR event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $28.46 \mathrm{cfs} @$ | 12.40 hrs , Volume | $229,211 \mathrm{cf}$ |
| Primary | $=$ | $28.46 \mathrm{cfs} @$ | 12.40 hrs , Volume $=$ | $229,211 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP3: ANALYSIS POINT 3

| Inflow Area | 55,420 | s, | 3.03" for 25YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 4.43 cfs @ | 12.09 hrs , Volume= | 14,013 cf |
| Primary | 4.43 cfs @ | 12.09 hrs , Volume= | $14,013 \mathrm{cf}, \mathrm{Atten}=0 \%, \mathrm{Lag}=0.0 \mathrm{~min}$ |

Primary outflow $=$ Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP4: ANALYSIS POINT \#4

| Inflow Area $=$ | $1,691,659 \mathrm{sf}, 25.34 \%$ Impervious, | Inflow Depth $>$ | $2.13 "$ | for 25 YR event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $43.69 \mathrm{cfs} @$ | 12.54 hrs , Volume $=$ | $300,250 \mathrm{cf}$ |
| Primary | $=$ | $43.69 \mathrm{cfs} @$ | 12.54 hrs , Volume $=$ | $300,250 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Primary outflow $=$ Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Time span=0.00-24.00 hrs, $\mathrm{dt}=0.05 \mathrm{hrs}, 481$ points $\times 3$
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

SubcatchmentB1: MULTIFAMILYBLDG

## SubcatchmentB2: MULTIFAMILYBLDG

SubcatchmentB3: MULTIFAMILY

SubcatchmentC10: CB \#10

SubcatchmentC11: CB \#11

SubcatchmentC12: CB \#12

SubcatchmentC13: CB \#13

SubcatchmentC14: CB \#14

SubcatchmentC15: CB \#15

SubcatchmentC16: CB \#16

SubcatchmentC17: CB \#17

SubcatchmentC18: CB \#18

SubcatchmentC20: CB \#20

SubcatchmentC21: CB \#21

SubcatchmentC22: CB \#22

SubcatchmentC23: CB \#23

Runoff Area=23,255 sf $100.00 \%$ Impervious Runoff Depth $>8.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=4.64 cfs $17,082 \mathrm{cf}$

Runoff Area $=17,561$ sf $100.00 \%$ Impervious Runoff Depth $>8.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=$=3.50 \mathrm{cfs} 12,899 \mathrm{cf}$

Runoff Area=19,981 sf $100.00 \%$ Impervious Runoff Depth $>8.81$ " Tc=6.0 min CN=98 Runoff=3.99 cfs $14,677 \mathrm{cf}$

Runoff Area $=6,961$ sf $100.00 \%$ Impervious Runoff Depth $>8.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff= $1.39 \mathrm{cfs} 5,113 \mathrm{cf}$

Runoff Area $=7,173$ sf $100.00 \%$ Impervious Runoff Depth $>8.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=1.43 cfs $5,269 \mathrm{cf}$

Runoff Area $=5,238$ sf $100.00 \%$ Impervious Runoff Depth $>8.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=1.04 cfs $3,847 \mathrm{cf}$

Runoff Area=10,873 sf $90.78 \%$ Impervious Runoff Depth>8.57" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff= $2.16 \mathrm{cfs} 7,768 \mathrm{cf}$

Runoff Area=12,099 sf $86.22 \%$ Impervious Runoff Depth>7.97" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=91$ Runoff=2.33 cfs $8,033 \mathrm{cf}$

Runoff Area $=6,666$ sf $100.00 \%$ Impervious Runoff Depth $>8.81$ " $\mathrm{T}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=1.33 \mathrm{cfs} 4,896 \mathrm{cf}$

Runoff Area=8,516 sf $64.88 \%$ Impervious Runoff Depth $>6.50$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=79$ Runoff=1.43 cfs $4,611 \mathrm{cf}$

Runoff Area $=11,836$ sf $73.87 \%$ Impervious Runoff Depth $>8.09$ " Tc=6.0 min CN=92 Runoff=2.30 cfs 7,978 cf

Runoff Area=18,591 sf 66.35\% Impervious Runoff Depth>7.85" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=90$ Runoff=3.56 cfs $12,155 \mathrm{cf}$

Runoff Area $=11,939$ sf $88.95 \%$ Impervious Runoff Depth $>8.45$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff $=2.36 \mathrm{cfs} 8,409 \mathrm{cf}$

Runoff Area=10,174 sf $87.04 \%$ Impervious Runoff Depth $>7.85$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=90$ Runoff $=1.95 \mathrm{cfs} 6,652 \mathrm{cf}$

Runoff Area=12,001 sf $91.62 \%$ Impervious Runoff Depth $>8.57$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=2.38 \mathrm{cfs} 8,574 \mathrm{cf}$

Runoff Area=9,694 sf 61.00\% Impervious Runoff Depth>7.72" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=89$ Runoff=1.84 cfs $6,239 \mathrm{cf}$

SubcatchmentC24: CB \#24

## SubcatchmentC25: CB \#25

SubcatchmentC26: CB \#26

SubcatchmentC27: CB \#27

SubcatchmentC28: CB \#28

SubcatchmentC29: CB \#29

SubcatchmentC30: CB \#30

SubcatchmentC31: CB \#31

SubcatchmentC32: CB \#32

SubcatchmentC33: CB \#33

SubcatchmentC34: CB \#34

SubcatchmentC35: CB \#35

SubcatchmentC36: CB \#36

SubcatchmentC38: CB \#38

SubcatchmentC39: CB \#39

SubcatchmentC40: CB \#40

SubcatchmentC41: CB \#41

Runoff Area=7,930 sf 72.16\% Impervious Runoff Depth>8.21" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff=1.55 cfs $5,426 \mathrm{cf}$

Runoff Area=8,487 sf $80.92 \%$ Impervious Runoff Depth>8.33" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff=1.67 cfs $5,892 \mathrm{cf}$

Runoff Area=8,835 sf $63.75 \%$ Impervious Runoff Depth $>7.97$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=91$ Runoff= $1.70 \mathrm{cfs} 5,866 \mathrm{cf}$

Runoff Area=6,111 sf $91.90 \%$ Impervious Runoff Depth $>8.57$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=$=1.21 \mathrm{cfs} 4,366 \mathrm{cf}$

Runoff Area=10,372 sf $51.33 \%$ Impervious Runoff Depth>7.60" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=88$ Runoff=$=1.95 \mathrm{cfs} 6,570 \mathrm{cf}$

Runoff Area=8,495 sf $84.21 \%$ Impervious Runoff Depth>8.33" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff=1.67 cfs $5,898 \mathrm{cf}$

Runoff Area=8,933 sf $82.40 \%$ Impervious Runoff Depth>8.33" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff=1.76 cfs $6,202 \mathrm{cf}$

Runoff Area=16,365 sf 68.64\% Impervious Runoff Depth>7.85" Tc=6.0 min CN=90 Runoff=3.13 cfs $10,699 \mathrm{cf}$

Runoff Area $=12,710$ sf $70.47 \%$ Impervious Runoff Depth $>7.97$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=91$ Runoff $=2.45 \mathrm{cfs} 8,439 \mathrm{cf}$

Runoff Area=5,421 sf $83.90 \%$ Impervious Runoff Depth>8.33" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=94$ Runoff=1.07 cfs $3,764 \mathrm{cf}$

Runoff Area=8,622 sf $80.51 \%$ Impervious Runoff Depth $>8.21$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff=1.69 cfs $5,899 \mathrm{cf}$

Runoff Area=4,149 sf $98.10 \%$ Impervious Runoff Depth $>8.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.83 \mathrm{cfs} 3,048 \mathrm{cf}$

Runoff Area $=6,622$ sf $100.00 \%$ Impervious Runoff Depth $>8.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=1.32 cfs $4,864 \mathrm{cf}$

Runoff Area=7,637 sf $100.00 \%$ Impervious Runoff Depth>8.81" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=1.52 \mathrm{cfs} 5,610 \mathrm{cf}$

Runoff Area $=7,612$ sf $100.00 \%$ Impervious Runoff Depth $>8.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=98$ Runoff $=1.52 \mathrm{cfs} 5,591 \mathrm{cf}$

Runoff Area $=4,211$ sf $100.00 \%$ Impervious Runoff Depth $>8.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff $=0.84 \mathrm{cfs} 3,093 \mathrm{cf}$

Runoff Area $=5,586$ sf $100.00 \%$ Impervious Runoff Depth $>8.81$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=1.11 cfs $4,103 \mathrm{cf}$

SubcatchmentC43: CB \#43

## SubcatchmentC44: CB \#44

SubcatchmentC45: CB \#45

SubcatchmentC46: CB \#46

SubcatchmentC47: CB \#47

SubcatchmentC48: CB \#48

SubcatchmentC49: CB \#49

SubcatchmentC50: CB \#50

SubcatchmentC51: CB \#51

SubcatchmentC52: CB\#52

SubcatchmentC53: CB \#53

SubcatchmentC54: CB \#54

SubcatchmentC7: CB \#5

SubcatchmentC8: CB \#8

Subcatchment C9: CB \#9

SubcatchmentCH1: CLUBHOUSE

SubcatchmentMB1: MAIL KIOSK

Runoff Area=3,109 sf 75.36\% Impervious Runoff Depth>7.72" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=89$ Runoff=0.59 cfs 2,001 cf

Runoff Area=1,978 sf $84.43 \%$ Impervious Runoff Depth>8.09" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=92$ Runoff= $0.38 \mathrm{cfs} 1,333 \mathrm{cf}$

Runoff Area=2,465 sf $50.30 \%$ Impervious Runoff Depth $>6.62$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=80$ Runoff= $0.42 \mathrm{cfs} 1,360 \mathrm{cf}$

Runoff Area=4,397 sf $50.97 \%$ Impervious Runoff Depth $>6.62$ " Tc=6.0 min CN=80 Runoff=0.75 cfs 2,426 cf

Runoff Area=3,012 sf 100.00\% Impervious Runoff Depth>8.81" Tc=6.0 min $\mathrm{CN}=98$ Runoff=0.60 cfs 2,212 cf

Runoff Area=60,128 sf 25.16\% Impervious Runoff Depth>5.38" Flow Length=400' Tc=11.8 min CN=70 Runoff=7.16 cfs 26,943 cf

Runoff Area=5,238 sf $84.59 \%$ Impervious Runoff Depth>8.33" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=94$ Runoff=1.03 cfs 3,637 cf

Runoff Area $=15,040$ sf $77.20 \%$ Impervious Runoff Depth $>8.21$ " Tc=6.0 min CN=93 Runoff=2.94 cfs 10,290 cf

Runoff Area=6,823 sf 100.00\% Impervious Runoff Depth>8.81" Tc=6.0 min $\quad \mathrm{CN}=98$ Runoff=1.36 cfs 5,012 cf

Runoff Area=9,052 sf 87.14\% Impervious Runoff Depth>8.45" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=95$ Runoff=1.79 cfs 6,376 cf

Runoff Area=7,863 sf $86.52 \%$ Impervious Runoff Depth $>8.21^{\prime \prime}$ Tc=6.0 min CN=93 Runoff=1.54 cfs $5,380 \mathrm{cf}$

Runoff Area=4,821 sf 86.85\% Impervious Runoff Depth>7.97" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=91 \quad$ Runoff=$=0.93 \mathrm{cfs} 3,201 \mathrm{cf}$

Runoff Area=4,650 sf 100.00\% Impervious Runoff Depth>8.81" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=98$ Runoff=0.93 cfs $3,416 \mathrm{cf}$

Runoff Area=5,450 sf 88.75\% Impervious Runoff Depth>8.33" $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=94$ Runoff=1.07cfs $3,784 \mathrm{cf}$

Runoff Area=16,307 sf 93.95\% Impervious Runoff Depth>8.69" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=97$ Runoff=3.25 cfs 11,814 cf

Runoff Area=6,262 sf 92.70\% Impervious Runoff Depth>8.57" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=1.24 cfs $4,474 \mathrm{cf}$

Runoff Area=938 sf 100.00\% Impervious Runoff Depth>8.81" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=98$ Runoff=0.19 cfs 689 cf

SubcatchmentS201: SUMMER STREET Runoff Area=9,943 sf 92.79\% Impervious Runoff Depth>8.45" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff=1.96 cfs 7,004 cf

SubcatchmentS202: EXISTING WETLANDRunoff Area=432,269 sf $42.08 \%$ Impervious Runoff Depth>6.11" Flow Length=856' Tc=23.2 $\mathrm{min} \quad \mathrm{CN}=76$ Runoff=44.78 cfs 219,981 cf

SubcatchmentS203: POCKET WETLAND\#1 Runoff Area=25,587 sf $0.00 \%$ Impervious Runoff Depth $>5.63$ " Tc=6.0 min CN=72 Runoff=3.79 cfs $12,007 \mathrm{cf}$

SubcatchmentS204: EXISTING
Runoff Area=308,203 sf $31.07 \%$ Impervious Runoff Depth $>5.98$ " Flow Length=632' Tc=22.6 min CN=75 Runoff=31.68 cfs $153,689 \mathrm{cf}$

SubcatchmentS205: ISOLATEDWETLANDRunoff Area=55,420 sf $16.57 \%$ Impervious Runoff Depth $>5.38$ "
$\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=70$ Runoff=7.86 cfs $24,862 \mathrm{cf}$
SubcatchmentS206: OVERLANDFLOW Runoff Area=891,295 sf $2.91 \%$ Impervious Runoff Depth $>4.73$ " Flow Length=1,467' Tc=34.5 $\mathrm{min} \quad \mathrm{CN}=65$ Runoff=60.37 cfs $351,563 \mathrm{cf}$

SubcatchmentS207:INFILTRATIONPOND Runoff Area=20,803 sf $0.00 \%$ Impervious Runoff Depth>7.60" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=88$ Runoff $=3.91 \mathrm{cfs} 13,178 \mathrm{cf}$

## SubcatchmentS208: GRASS AREA

Runoff Area=13,760 sf $9.33 \%$ Impervious Runoff Depth>6.00" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=75$ Runoff= $2.16 \mathrm{cfs} 6,883 \mathrm{cf}$

SubcatchmentS209: WETLANDC Runoff Area=107,073 sf $0.38 \%$ Impervious Runoff Depth $>5.73$ " Flow Length=607' Slope=0.0150 '/l' Tc=28.9 min CN=73 Runoff=9.50 cfs 51,122 cf

SubcatchmentS210: INFILTRATIONPOND Runoff Area=75,890 sf $0.00 \%$ Impervious Runoff Depth>6.36" Flow Length=580' Slope $=0.0150$ '/' Tc=16.5 $\mathrm{min} \mathrm{CN}=78$ Runoff $=9.35 \mathrm{cfs} 40,231 \mathrm{cf}$

## SubcatchmentS211: S211

SubcatchmentS212: SWALE

## SubcatchmentS213: COURTYARD

## SubcatchmentT1: Trench Drain 1

## SubcatchmentT2: Drive Under B2

SubcatchmentTH1: TOWN HOUSE\#1

SubcatchmentTH10: TOWN HOUSE \#10

Runoff Area $=15,436$ sf $47.47 \%$ Impervious Runoff Depth $>6.50^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=79$ Runoff $=2.59 \mathrm{cfs} 8,357 \mathrm{cf}$

Runoff Area=52,768 sf $0.60 \%$ Impervious Runoff Depth $>4.755^{\prime \prime}$ Flow Length=418' Tc=23.1 min CN=65 Runoff=4.28 cfs 20,867 cf

Runoff Area=21,407 sf $48.10 \%$ Impervious Runoff Depth $>6.50$ " $\mathrm{Tc}=6.0 \mathrm{~min} \quad \mathrm{CN}=79$ Runoff $=3.59 \mathrm{cfs} 11,590 \mathrm{cf}$

Runoff Area=11,173 sf $75.10 \%$ Impervious Runoff Depth $>8.21$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=93$ Runoff $=2.18 \mathrm{cfs} 7,644 \mathrm{cf}$

Runoff Area=4,445 sf $64.30 \%$ Impervious Runoff Depth $>6.37$ " $\mathrm{T}=6.0 \mathrm{~min} \quad \mathrm{CN}=78$ Runoff $=0.73 \mathrm{cfs} 2,361 \mathrm{cf}$

Runoff Area=4,247 sf $92.68 \%$ Impervious Runoff Depth>8.57" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.84 \mathrm{cfs} 3,034 \mathrm{cf}$

Runoff Area=3,476 sf $91.60 \%$ Impervious Runoff Depth>8.57" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.69 \mathrm{cfs} 2,483 \mathrm{cf}$
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## Subcatchment TH2: TOWN HOUSE \#2

SubcatchmentTH3: TOWN HOUSE\#3

SubcatchmentTH4: TOWN HOUSE \#4

SubcatchmentTH5: TOWN HOUSE \#5

SubcatchmentTH6: TOWN HOUSE \#6

SubcatchmentTH7: TOWN HOUSE\#7

SubcatchmentTH8: TOWN HOUSE \#8

SubcatchmentTH9: TOWN HOUSE \#9

Runoff Area=4,247 sf $92.68 \%$ Impervious Runoff Depth $>8.57$ " $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.84 \mathrm{cfs} 3,034 \mathrm{cf}$

Runoff Area $=3,013$ sf $88.68 \%$ Impervious Runoff Depth $>8.45{ }^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff $=0.60 \mathrm{cfs} 2,122 \mathrm{cf}$

Runoff Area=3,470 sf $91.59 \%$ Impervious Runoff Depth>8.57" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=$=0.69 \mathrm{cfs} 2,479 \mathrm{cf}$

Runoff Area=3,016 sf $88.69 \%$ Impervious Runoff Depth $>8.45{ }^{\prime \prime}$ $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=95$ Runoff= $0.60 \mathrm{cfs} 2,124 \mathrm{cf}$

Runoff Area=3,407 sf $91.46 \%$ Impervious Runoff Depth>8.57" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.68 \mathrm{cfs} 2,434 \mathrm{cf}$

Runoff Area=3,481 sf $91.61 \%$ Impervious Runoff Depth>8.57" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff= $0.69 \mathrm{cfs} 2,487 \mathrm{cf}$

Runoff Area=4,212 sf $92.62 \%$ Impervious Runoff Depth>8.57" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff $=0.84 \mathrm{cfs} 3,009 \mathrm{cf}$

Runoff Area=3,480 sf $91.61 \%$ Impervious Runoff Depth>8.57" $\mathrm{Tc}=6.0 \mathrm{~min} \mathrm{CN}=96$ Runoff=$=0.69 \mathrm{cfs} 2,486 \mathrm{cf}$

Reach 8R: OVERLANDFLOW Avg. Flow Depth=0.11' Max Vel=0.12 fps Inflow=1.97 cfs 7,579 cf $\mathrm{n}=0.400 \mathrm{~L}=563.0$ ' $\mathrm{S}=0.0213$ '/' Capacity=28.09 cfs Outflow=0.70 cfs 7,164 cf

Reach 9R: OVERLANDFLOW Avg. Flow Depth=0.16' Max Vel=0.27 fps Inflow=1.22 cfs $2,322 \mathrm{cf}$ $\mathrm{n}=0.400 \mathrm{~L}=211.0$ ' $\mathrm{S}=0.0652$ '/' Capacity $=23.45 \mathrm{cfs}$ Outflow=0.97 cfs $2,322 \mathrm{cf}$

Reach 10R: OVERLANDFLOW Avg. Flow Depth=0.69' Max Vel=0.47 fps Inflow=10.24 cfs 31,579 cf $\mathrm{n}=0.400 \mathrm{~L}=164.0$ ' $\mathrm{S}=0.0366$ '/' Capacity= 17.57 cfs Outflow=8.88 cfs $31,578 \mathrm{cf}$

Reach 11R: 4x4 Open Bottom CulvertAvg. Flow Depth=2.22' Max Vel=2.73 fps Inflow=24.22 cfs $144,775 \mathrm{cf}$ 48.0 " $\times 48.0$ " Box Pipe $n=0.069 \quad L=30.0$ ' $\quad S=0.0150$ '/' Capacity=42.20 cfs Oufflow=24.22 cfs $144,761 \mathrm{cf}$

Reach 12R: OVERLANDFLOW Avg. Flow Depth=0.16' Max Vel=0.17 fps Inflow=2.22 cfs 8,218 cf $\mathrm{n}=0.400 \mathrm{~L}=250.0$ ' $\mathrm{S}=0.0240$ '/' Capacity $=29.80 \mathrm{cfs}$ Outflow=1.36 cfs $8,061 \mathrm{cf}$

Reach 14R: OVERLANDFLOW Avg. Flow Depth=0.18' Max Vel=0.18 fps Inflow=4.27 cfs $20,864 \mathrm{cf}$ $\mathrm{n}=0.400 \mathrm{~L}=852.0^{\prime} \mathrm{S}=0.0246$ '//' Capacity=31.55 cfs Outflow=1.64 cfs $19,373 \mathrm{cf}$

Reach 15R: OVERLANDFLOW Avg. Flow Depth=0.21' Max Vel=0.18 fps Inflow=3.51 cfs 20,698 cf


Reach 18R: OVERLANDFLOW Avg. Flow Depth=0.26' Max Vel=0.24 fps Inflow=10.22 cfs 22,242 cf n=0.400 L=609.0' $\mathrm{S}=0.0279$ '/l' Capacity=38.42 cfs Outflow=3.56 cfs 21,983 cf

Reach 20R: OVERLANDFLOW Avg. Flow Depth=0.39' Max Vel=0.19 fps Inflow=8.98 cfs $35,788 \mathrm{cf}$ $\mathrm{n}=0.400 \mathrm{~L}=560.0$ ' $\mathrm{S}=0.0093$ '/' Capacity=18.54 cfs Outflow=3.75 cfs $34,661 \mathrm{cf}$

Reach 23R: OVERLANDFLOW Avg. Flow Depth=0.83' Max Vel=0.41 fps Inflow=24.22 cfs 144,761 cf n=0.400 L=237.0' $\mathrm{S}=0.0211$ '/' Capacity=31.93 cfs Outflow=22.58 cfs 143,931 cf

Reach R202: OVERLANDFLOW Avg. Flow Depth=0.75' Max Vel=0.29 fps Inflow=44.78 cfs 219,947 cf $\mathrm{n}=0.400 \mathrm{~L}=700.0$ ' $\mathrm{S}=0.0107 \mathrm{I} / \mathrm{I} \quad$ Capacity=42.56 cfs Outflow=25.82 cfs $212,555 \mathrm{cf}$

Reach R211: OVERLANDFLOW Avg. Flow Depth=0.88' Max Vel=0.27 fps Inflow=27.69 cfs $57,014 \mathrm{cf}$ $\mathrm{n}=0.400 \mathrm{~L}=600.0$ ' $\mathrm{S}=0.0087$ '/' Capacity=14.51 cfs Outflow=11.38 cfs $56,631 \mathrm{cf}$

Reach SC1: Stream Crossing\#1 Avg. Flow Depth=0.59' Max Vel=4.71 fps Inflow=44.78 cfs 219,981 cf 192.0 " $\times 60.0$ ", R=207.0" Arch Pipe $n=0.030$ L=43.1' $\mathrm{S}=0.0200$ '/' Capacity=722.91 cfs Outflow=44.78 cfs $219,947 \mathrm{cf}$

Reach SC2: Stream Crossing\#2 Avg. Flow Depth=0.13' Max Vel=2.01 fps Inflow=4.28 cfs 20,867 cf 192.0 " $\times 60.0$ ", $\mathrm{R}=180.0$ " Arch Pipe $\mathrm{n}=0.030 \mathrm{~L}=36.5^{\prime} \mathrm{S}=0.0241$ '/' Capacity=768.96 cfs Outflow=4.27 cfs 20,864 cf

Pond 1P: DMH \#33

Pond 3P: OCS \#8

## Pond 5R: TRENCH DRAIN

Pond 11P: YARD DRAIN

Pond CB10: CB \#10

Pond CB11: CB \#11

Pond CB12: CB \#12

Pond CB13: CB \#13

Pond CB14: CB \#14

Pond CB15: CB \#15

Pond CB16: CB \#16

Peak Elev=206.71' Inflow=3.20 cfs 11,616 cf 12.0" Round Culvert n=0.013 L=46.7' S=0.0251 '/' Outflow=3.20 cfs 11,616 cf

Peak Elev=203.30' Inflow=2.47 cfs 8,581 cf Outflow=2.47 cfs 8,581 cf

Peak Elev=199.24' Inflow=2.18 cfs 7,644 cf 8.0" Round Culvert n=0.012 L=36.0' S=0.0200 '/' Outflow=2.18 cfs 7,644 cf

Peak Elev=207.48' Storage=1,296 cf Inflow=3.59 cfs 11,590 cf Outflow=2.82 cfs $11,525 \mathrm{cf}$

Peak Elev=210.78' Inflow=1.39 cfs 5,113 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=33.8^{\prime} \mathrm{S}=0.0050$ '/' Outflow=1.39 cfs $5,113 \mathrm{cf}$

Peak Elev=210.83' Inflow=1.43 cfs 5,269 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=26.3^{\prime} \mathrm{S}=0.0103$ '/' Outflow=1.43 cfs $5,269 \mathrm{cf}$

Peak Elev=207.24' Inflow=1.04 cfs 3,847 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=41.3^{\prime} \mathrm{S}=0.0249$ '/' Outflow=1.04 cfs $3,847 \mathrm{cf}$

Peak Elev=207.53' Inflow=2.16 cfs 7,768 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=43.7^{\prime} \mathrm{S}=0.0249$ '/' Outflow=2.16 cfs $7,768 \mathrm{cf}$

Peak Elev=203.42' Inflow=2.33 cfs 8,033 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=23.2^{\prime} \mathrm{S}=0.0052$ '//' Outflow=2.33 cfs $8,033 \mathrm{cf}$

Peak Elev=203.16' Inflow=1.33 cfs 4,896 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=15.6^{\prime} \mathrm{S}=0.0051$ '/' Outflow=1.33 cfs $4,896 \mathrm{cf}$

Peak Elev=204.33' Inflow=1.43 cfs 4,611 cf 12.0" Round Culvert n=0.013 L=20.9' S=0.0067 '/' Outflow=1.43 cfs 4,611 cf
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Pond CB17: CB \#17

Pond CB18: CB \#18

Pond CB20: CB \#20

Pond CB21: CB \#21

Pond CB22: CB \#22

Pond CB23: CB \#23

Pond CB24: CB \#24

Pond CB25: CB \#25

Pond CB26: CB \#26

Pond CB27: CB \#27

Pond CB28: CB \#28

Pond CB29: CB \#29

Pond CB30: CB \#30

Pond CB31: CB \#31

Pond CB32: CB \#32

Pond CB33: CB \#33

Pond CB34: CB \#34

Peak Elev=207.31' Inflow=2.30 cfs 7,978 cf
12.0" Round Culvert n=0.013 L=13.8' S=0.0094 '/' Outflow=2.30 cfs 7,978 cf

Peak Elev=207.38' Inflow=3.90 cfs 14,596 cf 15.0" Round Culvert $n=0.013$ L=25.1' $S=0.0052$ '//' Outflow=3.90 cfs 14,596 cf

Peak Elev=205.83' Inflow=2.36 cfs 8,409 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=30.3^{\prime} \mathrm{S}=0.0053$ '/' Outflow=2.36 cfs $8,409 \mathrm{cf}$

Peak Elev=205.70' Inflow=1.95 cfs 6,652 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=26.0^{\prime} \mathrm{S}=0.0050$ '/' Outflow=1.95 cfs 6,652 cf

Peak Elev=206.41' Inflow=2.38 cfs 8,574 cf 12.0" Round Culvert n=0.012 L=16.1' S=0.0050 '/' Outflow=2.38 cfs 8,574 cf

Peak Elev=206.30' Inflow=1.84 cfs 6,239 cf 12.0" Round Culvert n=0.012 L=16.3' S=0.0055 '/' Outflow=1.84 cfs 6,239 cf

Peak Elev=206.37' Inflow=1.55 cfs 5,426 cf 12.0" Round Culvert n=0.012 L=12.1' S=0.0050 '/' Outflow=1.55 cfs 5,426 cf

Peak Elev=206.30' Inflow=1.67 cfs 5,892 cf 15.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=11.4$ ' $\mathrm{S}=0.0053$ '/' Outflow=1.67 cfs 5,892 cf

Peak Elev=202.61' Inflow=1.70 cfs 5,866 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=42.5$ ' $\mathrm{S}=0.0052$ '/' Outflow=1.70 cfs 5,866 cf

Peak Elev=201.82' Inflow=1.21 cfs 4,366 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=18.0^{\prime} \mathrm{S}=0.0056$ '/' Outflow=1.21 cfs $4,366 \mathrm{cf}$

Peak Elev=199.02' Inflow=1.95 cfs 6,570 cf 12.0" Round Culvert n=0.013 L=13.7' S=0.0044 '/' Outflow=1.95 cfs 6,570 cf

Peak Elev=206.87' Inflow=1.67 cfs 5,898 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=13.5$ ' $\mathrm{S}=0.0052$ '/' Outflow=1.67 cfs 5,898 cf

Peak Elev=206.89' Inflow=1.76 cfs 6,202 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=17.5^{\prime} \mathrm{S}=0.0051$ '/' Outflow=1.76 cfs 6,202 cf 12.0" Round Culvert $n=0.013$ L=16.4' $\mathrm{S}=0.0049$ '/' Outflow=3.13 cfs 10,699 cf

Peak Elev=205.58' Inflow=2.45 cfs 8,439 cf 12.0" Round Culvert n=0.013 L=16.3' S=0.0049 '/' Outflow=2.45 cfs 8,439 cf

Peak Elev=206.39' Inflow=1.07 cfs 3,764 cf 12.0" Round Culvert $n=0.013$ L=11.7' $S=0.0051$ '/' Outflow=1.07 cfs 3,764 cf

Peak Elev=206.50' Inflow=1.69 cfs 5,899 cf 12.0" Round Culvert n=0.013 L=16.5' S=0.0048 '/' Outflow=1.69 cfs 5,899 cf

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Pond CB54: CB \#54

## Pond CB7: CB\#5

Pond CB8: CB\#8

Pond CB9: CB \#9

Pond D10: DMH \#10

Pond D11: DMH \#11

Pond D12: DMH \#12

Pond D13: DMH \#13

Pond D14: DMH \#14

Pond D16: DMH \#16

Pond D17: DMH \#17

Pond D18: DMH \#18

Pond D19: DMH \#19

Pond D2: DMH\#2

Pond D20: DMH \#20

Pond D21: DMH \#21

Pond D22: DMH \#22

Peak Elev=203.32' Inflow=0.93 cfs 3,201 cf 12.0" Round Culvert n=0.013 L=36.7' S=0.0049 '/' Outflow=0.93 cfs 3,201 cf

Peak Elev=215.06' Inflow=0.93 cfs 3,416 cf 12.0" Round Culvert $n=0.012$ L=15.1' $S=0.0099$ '/' Outflow=0.93 cfs 3,416 cf

Peak Elev=215.06' Inflow=1.07 cfs 3,784 cf 12.0" Round Culvert $n=0.013$ L=15.1' S=0.0099 '//' Outflow=1.07 cfs 3,784 cf

Peak Elev=211.34' Inflow=3.25 cfs 11,814 cf 12.0" Round Culvert n=0.013 L=19.9' S=0.0196 '/' Outflow=3.25 cfs 11,814 cf

Peak Elev=204.11' Inflow=1.43 cfs 4,611 cf 12.0" Round Culvert n=0.013 L=15.6' S=0.0051 '/' Outflow=1.43 cfs 4,611 cf

Peak Elev=207.27' Inflow=6.20 cfs 22,574 cf 18.0" Round Culvert $n=0.013$ L=44.6' $S=0.0049$ '/' Outflow=6.20 cfs 22,574 cf

Peak Elev=205.44' Inflow=4.31 cfs 15,061 cf 12.0" Round Culvert $n=0.013$ L=41.9' $S=0.0050$ '/' Outflow=4.31 cfs 15,061 cf

Peak Elev=204.14' Inflow=14.18 cfs 52,717 cf 24.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=60.1^{\prime} \mathrm{S}=0.0050$ '/' Outflow=14.18 cfs 52,717 cf

Peak Elev=205.77' Inflow=7.44 cfs 26,131 cf 18.0" Round Culvert n=0.012 L=256.3' S=0.0050 '/' Outflow=7.44 cfs 26,131 cf

Peak Elev=206.22' Inflow=3.22 cfs 11,318 cf 15.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=103.5^{\prime} \mathrm{S}=0.0050$ '/' Outflow=3.22 cfs 11,318 cf

Peak Elev=201.64' Inflow=2.92 cfs 10,232 cf 12.0" Round Culvert $n=0.013$ L=91.6' $S=0.0312$ '/' Outflow=2.92 cfs 10,232 cf

Peak Elev=198.76' Inflow=4.87 cfs 16,802 cf 15.0" Round Culvert n=0.013 L=46.3' S=0.0099 '/' Outflow=4.87 cfs 16,802 cf

Peak Elev=206.75' Inflow=3.43 cfs 12,100 cf 12.0" Round Culvert $n=0.013$ L=82.5' $S=0.0092$ '/' Outflow=3.43 cfs 12,100 cf

Peak Elev=209.92' Inflow=9.15 cfs 36,354 cf 15.0" Round Culvert n=0.013 L=38.2' S=0.0099 '/' Outflow=9.15 cfs 36,354 cf

Peak Elev=205.57' Inflow=3.43 cfs 12,100 cf 15.0" Round Culvert $n=0.013$ L=63.5' $S=0.0050$ '/' Outflow=3.43 cfs 12,100 cf

Peak Elev=205.17' Inflow=13.91 cfs 48,812 cf 24.0" Round Culvert $n=0.013$ L=72.4' $S=0.0050$ '/' Outflow=13.91 cfs 48,812 cf

Peak Elev=206.31' Inflow=4.90 cfs 17,574 cf 15.0" Round Culvert n=0.013 L=134.2' S=0.0071 '/' Outflow=4.90 cfs 17,574 cf
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Pond D23: DMH \#23

Pond D25: DMH \#25

Pond D27: DMH \#27

Pond D28: DMH \#28

Pond D29: DMH \#29

Pond D30: DMH \#30

Pond D31: DMH\#31

Pond D34: DMH \#34

Pond D35: DMH \#35

Pond D4: DMH\#4

Pond D5: DMH \#5

Pond D6: DMH \#6

Pond D7: DMH \#7

Pond D8: DMH \#8

Pond D9: DMH \#9

Pond DE61: DRIP \#61

Pond DE62: DRIP \#62

Peak Elev=207.50' Inflow=2.15 cfs 7,912 cf 15.0" Round Culvert n=0.013 L=173.3' $\mathrm{S}=0.0100$ '/' Outflow=2.15 cfs 7,912 cf

Peak Elev=212.68' Inflow=7.14 cfs 25,517 cf 15.0" Round Culvert n=0.012 L=237.6' S=0.0050 '/' Outflow=7.14 cfs 25,517 cf

Peak Elev=214.46' Inflow=4.10 cfs 14,316 cf 15.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=63.9^{\prime} \mathrm{S}=0.0150$ '/' Outflow=4.10 cfs $14,316 \mathrm{cf}$

Peak Elev=218.28' Inflow=2.14 cfs 7,120 cf 12.0" Round Culvert n=0.013 L=158.3' $\mathrm{S}=0.0200$ '/' Outflow=2.14 cfs 7,120 cf

Peak Elev=220.65' Inflow=2.14 cfs 7,120 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=150.9$ ' $\mathrm{S}=0.0151$ '/' Outflow=2.14 cfs $7,120 \mathrm{cf}$

Peak Elev=221.60' Inflow=1.17 cfs 3,786 cf 12.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=184.2^{\prime} \mathrm{S}=0.0050$ '/' Outflow=1.17 cfs $3,786 \mathrm{cf}$

Peak Elev=226.91' Inflow=7.60 cfs 29,155 cf 15.0" Round Culvert n=0.012 L=288.5' S=0.0400 '/' Outflow=7.60 cfs 29,155 cf

Peak Elev=200.07' Inflow=4.64 cfs 17,082 cf 12.0" Round Culvert n=0.012 L=52.0' $\mathrm{S}=0.0200$ '/' Outflow=4.64 cfs 17,082 cf

Peak Elev=213.61' Inflow=4.10 cfs 14,316 cf 15.0" Round Culvert n=0.012 L=171.5' S=0.0150 '/' Outflow=4.10 cfs 14,316 cf

Peak Elev=215.03' Inflow=9.15 cfs 36,354 cf 15.0" Round Culvert $\mathrm{n}=0.012 \mathrm{~L}=222.3^{\prime} \mathrm{S}=0.0150$ '/' Outflow=9.15 cfs $36,354 \mathrm{cf}$

Peak Elev=210.62' Inflow=6.07 cfs 22,196 cf 18.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=183.0^{\prime} \mathrm{S}=0.0050$ '/' Outflow=6.07 cfs 22,196 cf

Peak Elev=209.51' Inflow=6.07 cfs 22,196 cf 18.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=299.7$ ' $\mathrm{S}=0.0050$ '/' Outflow=6.07 cfs 22,196 cf

Peak Elev=207.73' Inflow=6.07 cfs 22,196 cf 18.0" Round Culvert n=0.013 L=44.2' $\mathrm{S}=0.0550$ '/' Outflow=6.07 cfs 22,196 cf

Peak Elev=203.04' Inflow=3.66 cfs 12,929 cf 12.0" Round Culvert n=0.013 L=87.7' S=0.0050 '/' Outflow=3.66 cfs 12,929 cf

Peak Elev=201.60' Inflow=3.66 cfs 12,929 cf 12.0" Round Culvert n=0.013 L=11.9' S=0.0050 '// Outflow=3.66 cfs 12,929 cf

Peak Elev=213.59' Storage=372 cf Inflow=0.84 cfs 3,034 cf Discarded=0.00 cfs 211 cf Primary $=0.69$ cfs 2,673 cf Outflow=0.69 cfs 2,884 cf

Peak Elev=213.59' Storage=372 cf Inflow=0.84 cfs 3,034 cf Discarded=0.00 cfs 211 cf Primary $=0.69$ cfs 2,673 cf Outflow $=0.69$ cfs 2,884 cf
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Pond DE63: DRIP \#63
Discarded=0.00 cfs 127 cf Primary= 0.52 cfs 1,906 cf Outflow=0.52 cfs 2,032 cf
Peak Elev=206.28' Storage=242 cf Inflow=0.69 cfs 2,479 cf Discarded=0.00 cfs 149 cf Primary= 0.60 cfs 2,224 cf Outflow=0.60 cfs 2,374 cf

Peak Elev=207.21' Storage=196 cf Inflow=0.60 cfs 2,124 cf Discarded=0.00 cfs 127 cf Primary= 0.52 cfs 1,908 cf Outflow $=0.52$ cfs 2,034 cf

Peak Elev=209.07' Storage=240 cf Inflow=0.68 cfs 2,434 cf Discarded=0.00 cfs 149 cf Primary $=0.58$ cfs 2,180 cf Outflow=0.59 cfs 2,329 cf

Peak Elev=209.28' Storage=242 cf Inflow=0.69 cfs 2,487 cf Discarded=0.00 cfs 149 cf Primary= 0.60 cfs 2,232 cf Outflow=0.60 cfs 2,382 cf

Peak Elev=208.26' Storage=338 cf Inflow=0.84 cfs 3,009 cf Discarded=0.00 cfs 211 cf Primary= 0.68 cfs 2,652 cf Outflow=0.68 cfs 2,863 cf

Peak Elev=206.78' Storage=242 cf Inflow=0.69 cfs 2,486 cf Discarded=0.00 cfs 149 cf Primary $=0.60$ cfs 2,232 cf Outflow $=0.60$ cfs 2,381 cf

Peak Elev=207.18' Storage=242 cf Inflow=0.69 cfs 2,483 cf Discarded=0.00 cfs 149 cf Primary= 0.60 cfs 2,229 cf Outflow=0.60 cfs 2,378 cf

Peak Elev=207.88' Storage=370 cf Inflow=0.84 cfs 3,008 cf Discarded=0.00 cfs 211 cf Primary=0.68 cfs 2,647 cf Outflow=0.69 cfs 2,858 cf

Pond DECH: DRIP \#CH

Pond DMH32: DMH \#32

Pond OCS1: OCS\#1

Pond OCS3: OCS\#3

Pond OCS4: OCS\#4

Pond OCS6: OCS \#6

Pond OCS7: OCS \#7
Peak Elev=210.92' Storage=746 cf Inflow=1.24 cfs 4,474 cf Discarded=0.04 cfs 2,032 cf Primary=1.05 cfs 2,441 cf Outflow=1.08 cfs 4,473 cf

Peak Elev=204.19' Inflow=3.97 cfs 13,927 cf 12.0" Round Culvert n=0.013 L=19.2' S=0.0531 '/' Outflow=3.97 cfs 13,927 cf

Peak Elev=196.96' Inflow=9.51 cfs 33,884 cf Outflow=9.51 cfs 33,884 cf

Peak Elev=206.75' Inflow=9.71 cfs 35,473 cf Outflow=9.71 cfs $35,473 \mathrm{cf}$

Peak Elev=205.75' Inflow=2.79 cfs 9,535 cf Outflow=2.79 cfs 9,535 cf

Peak Elev=204.53' Inflow=3.20 cfs 11,616 cf Outflow=3.20 cfs 11,616 cf

Peak Elev=204.53' Inflow=3.15 cfs 11,388 cf Outflow=3.15 cfs $11,388 \mathrm{cf}$

Pond P204: STORMTECHINFILTRATION Peak Elev=205.68' Storage=11,436 cf Inflow=12.49 cfs $45,008 \mathrm{cf}$ Discarded=0.09 cfs 6,554 cf Primary=8.98 cfs 35,788 cf Outflow=9.06 cfs 42,342 cf
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Pond P205: INFILTRATIONPOND \#3 Peak Elev=207.05' Storage=9,818 cf Inflow=11.50 cfs $44,711 \mathrm{cf}$
Discarded= $=0.32$ cfs 16,679 cf Primary=10.22 cfs 22,242 cf Outflow=10.54 cfs 38,922 cf
Pond P206: STORMTECHINFILTRATION Peak Elev=196.64' Storage=8,490 cf Inflow=11.69 cfs $41,528 \mathrm{cf}$ Discarded=0.49 cfs 26,156 cf Primary=8.72 cfs 15,367 cf Outflow= 9.22 cfs 41,523 cf

Pond P207: INFILTRATIONPOND \#2 Peak Elev=198.98' Storage=25,795 cf Inflow=23.17 cfs $83,435 \mathrm{cf}$ Discarded=1.15 cfs 51,829 cf Primary=10.24 cfs 31,579 cf Outflow=11.39 cfs 83,408 cf

Pond P210: POCKET WETLAND\#1
Peak Elev=204.66' Storage=20,706 cf Inflow=10.93 cfs 37,524 cf Outflow=3.51 cfs 20,698 cf

Pond P212: INFILTRATIONPOND \#1 Peak Elev=203.05' Storage=29,225 cf Inflow=32.18 cfs 135,950 cf Discarded=1.49 cfs $74,502 \mathrm{cf}$ Primary=27.69 cfs $57,014 \mathrm{cf}$ Outflow=29.18 cfs 131,516 cf

Pond P213: Stormtech Infiltration System Peak Elev=204.44' Storage=7,117 cf Inflow=6.35 cfs 23,003 cf Discarded=0.39 cfs 19,325 cf Primary=1.88 cfs 3,677 cf Outflow=2.27 cfs 23,001 cf

Pond P214: STORMTECHINFILTRATION Peak Elev=203.26' Storage=6,493 cf Inflow=6.45 cfs $23,257 \mathrm{cf}$ Discarded=0.61 cfs 20,934 cf Primary=1.22 cfs 2,322 cf Outflow=1.84 cfs 23,256 cf

## Link AP1: ANALYSISPOINT 1

## Link AP2: ANALYSISPOINT 2

Inflow=1.96 cfs 7,004 cf Primary $=1.96$ cfs 7,004 cf

Inflow=52.47 cfs 394,820 cf Primary=52.47 cfs 394,820 cf

## Link AP3: ANALYSISPOINT 3

## Link AP4: ANALYSISPOINT \#4

Inflow=7.86 cfs 24,862 cf Primary $=7.86$ cfs 24,862 cf

Inflow=98.54 cfs 593,282 cf Primary $=98.54$ cfs 593,282 cf

Total Runoff Area $=2,573,920$ sf Runoff Volume $=1,283,918$ cf Average Runoff Depth $=5.99$ " 70.09\% Pervious $=1,803,997$ sf $29.91 \%$ Impervious $=769,923$ sf

## Summary for Subcatchment B1: MULTIFAMILY BLDG \#1

Runoff $=\quad 4.64$ cfs @ 12.09 hrs, Volume= 17,082 cf, Depth> 8.81"
Routed to Pond D34 : DMH \#34
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 20,156 | 98 | Roofs, HSG C |
| 3,099 | 98 | Roofs, HSG D |
| 23,255 | 98 | Weighted Average |
| 23,255 |  | 100.00\% Impervious Area |
| Tc | Length <br> $(\mathrm{min})$ | Slope <br> (feet) |
| (ft/ft) | Velocity <br> (ft/sec) | Capacity <br> (cfs) |

## Summary for Subcatchment B2: MULTIFAMILY BLDG \#2

```
Runoff = 3.50 cfs @ 12.09 hrs, Volume= 12,899 cf, Depth> 8.81"
```

    Routed to Pond OCS3 : OCS\#3
    Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | a (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 7,873 | 9898 | Roofs, HSG A |  |  |
|  | 9,688 |  | Roofs, HSG C |  |  |
|  | 17,561 | 98 | Weighted Average 100.00\% Impervious Area |  |  |
|  | 17,561 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment B3: MULTIFAMILY BUILDING \#3

Runoff $=\quad 3.99$ cfs @ 12.09 hrs, Volume= 14,677 cf, Depth> 8.81"
Routed to Pond P214 : STORMTECH INFILTRATION SYSTEM \#4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 608 | 98 | Roofs, HSG A |
| 19,373 | 98 | Roofs, HSG C |
| 19,981 | 98 | Weighted Average |
| 19,981 |  | $100.00 \%$ Impervious Area |



Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 339 | 98 | Paved roads w/curbs \& sewers, HSG B |
| 5,703 | 98 | Paved roads w/curbs \& sewers, HSG C |
| 919 | 98 | Paved roads w/curbs \& sewers, HSG D |

## Summary for Subcatchment C11: CB \#11

Runoff $=\quad 1.43$ cfs @ 12.09 hrs, Volume= $5,269 \mathrm{cf}$, Depth> 8.81"

Routed to Pond CB11 : CB \#11
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


## Summary for Subcatchment C12: CB \#12

Runoff $=1.04$ cfs @ 12.09 hrs, Volume $=\quad 3,847$ cf, Depth> 8.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"
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## Summary for Subcatchment C13: CB \#13

Runoff $=\quad 2.16$ cfs @ 12.09 hrs, Volume= $\quad 7,768 \mathrm{cf}$, Depth> 8.57"
Routed to Pond CB13 : CB \#13
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,003 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 7,547 | 98 | Paved parking, HSG C |
| 2,323 | 98 | Roofs, HSG C |

Summary for Subcatchment C14: CB \#14
Runoff $=\quad 2.33$ cfs @ 12.09 hrs, Volume= 8,033 cf, Depth> 7.97"
Routed to Pond CB14 : CB \#14
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,195 | 39 | >75\% Grass cover, Good, HSG A |  |  |
|  | 7,649 | 98 P | Paved parking, HSG A |  |  |
|  | 472 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 2,783 | 98 P | Paved parking, HSG C |  |  |
|  | 12,099 | 91 | Weighted Average |  |  |
|  | 1,667 |  | 13.78\% Pervious Area |  |  |
|  | 10,432 |  | 86.22\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) |  | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C15: CB \#15

Runoff $=\quad 1.33$ cfs @ 12.09 hrs, Volume= 4,896 cf, Depth> 8.81"

Routed to Pond CB15 : CB \#15
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & 5,000 \\ & 1,666 \end{aligned}$ | $\begin{array}{ll} \hline 98 & F \\ 98 & F \end{array}$ | Paved parking, HSG A Paved parking, HSG C |  |  |
|  | $\begin{aligned} & \hline 6,666 \\ & 6,666 \end{aligned}$ | 98 | Weighted Average 100.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |

## Summary for Subcatchment C16: CB \#16

```
Runoff = 1.43 cfs @ 12.09 hrs, Volume= 4,611 cf, Depth> 6.50"
```

    Routed to Pond CB16 : CB \#16
    Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 2,467 | 39 | $>75 \%$ Grass cover, Good, HSG A |
| 4,380 | 98 | Paved parking, HSG A |
| 524 | 74 | >75\% Grass cover, Good, HSG C |
| 1,145 | 98 | Paved parking, HSG C |
| 8,516 | 79 | Weighted Average |
| 2,991 |  | $35.12 \%$ Pervious Area |
| 5,525 |  | $64.88 \%$ Impervious Area |
| Tc | Length | Slope <br> (ft/ft) |
| Velocity  <br> (ft/sec) Capacity <br> (min) (cfs) |  |  |

Summary for Subcatchment C17: CB \#17
Runoff $=\quad 2.30$ cfs @ 12.09 hrs, Volume $=\quad 7,978 \mathrm{cf}$, Depth> 8.09"
Routed to Pond CB17 : CB \#17
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


Direct Entry,

## Summary for Subcatchment C20: CB \#20

Runoff $=\quad 2.36$ cfs @ 12.09 hrs, Volume= $8,409 \mathrm{cf}$, Depth> 8.45"
Routed to Pond CB20 : CB \#20
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,319 | 98 | Paved parking, HSG A |
| 1,319 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 7,301 | 98 | Paved parking, HSG C |
| 11,939 | 95 | Weighted Average |
| 1,319 |  | 11.05\% Pervious Area |
| 10,620 |  | $88.95 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ |
| ---: | ---: | ---: | ---: | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description | Direct Entry, |
| :--- |

## Summary for Subcatchment C21: CB \#21

Runoff $=\quad 1.95$ cfs @ 12.09 hrs, Volume= 6,652 cf, Depth> 7.85"
Routed to Pond CB21 : CB \#21
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,319 | 39 > | >75\% Grass cover, Good, HSG A Paved parking, HSG A Paved parking, HSG C |  |  |
|  | 7,301 | 98 P |  |  |  |
|  | 1,554 | 98 P |  |  |  |
|  | 10,174 | 90 | Weighted Average <br> 12.96\% Pervious Area <br> 87.04\% Impervious Area |  |  |
|  | 1,319 |  |  |  |  |
|  | 8,855 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |

## Summary for Subcatchment C22: CB \#22

Runoff $=\quad 2.38$ cfs @ 12.09 hrs, Volume $=\quad 8,574$ cf, Depth> 8.57"
Routed to Pond CB22 : CB \#22
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| :---: | :---: | :---: |
| 2,946 | 98 | Paved parking, HSG A |
| 177 | 74 | >75\% Grass cover, Good, HSG C |
| 2,641 | 98 | Paved parking, HSG C |
| 829 | 80 | >75\% Grass cover, Good, HSG D |
| 5,408 | 98 | Paved parking, HSG D |
| 12,001 | 96 | Weighted Average |
| 1,006 |  | 8.38\% Pervious Area |
| 10,995 |  | 91.62\% Impervious Area |
| Tc Length (min) (feet) | Slope (ft/ft) | Velocity Capacity Description (ft/sec) (cfs) |

## Summary for Subcatchment C23: CB \#23

Runoff = 1.84 cfs @ 12.09 hrs, Volume=
6,239 cf, Depth> 7.72"
Routed to Pond CB23 : CB \#23
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG A |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 242 | 39 > |  |  |  |
|  | 3,016 | 98 P | Paved parking, HSG A |  |  |
|  | 1,267 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 218 | 98 P | Paved parking, HSG C |  |  |
|  | 2,272 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 2,679 | 98 P | Paved parking, HSG D |  |  |
|  | 9,694 | 89 | Weighted Average |  |  |
|  | 3,781 |  | 39.00\% Pervious Area |  |  |
|  | 5,913 |  | 61.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C24: CB \#24

Runoff $=\quad 1.55$ cfs @ 12.09 hrs, Volume= $\quad 5,426 \mathrm{cf}$, Depth> 8.21"

Routed to Pond CB24 : CB \#24
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG A |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 211 | 39 > |  |  |  |
|  | 519 | 98 P | Paved parking, HSG A |  |  |
|  | 15 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 300 | 98 P | Paved parking, HSG C |  |  |
|  | 1,393 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 6,049 | 98 P | Paved parking, HSG D |  |  |
|  | 8,487 | 94 | Weighted Average |  |  |
|  | 1,619 |  | 19.08\% Pervious Area |  |  |
|  | 6,868 |  | 80.92\% Im | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C26: CB \#26

Runoff = 1.70 cfs @ 12.09 hrs, Volume= 5,866 cf, Depth> 7.97"

Routed to Pond CB26 : CB \#26
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,203 | 80 | >75\% Grass cover, Good, HSG D |
| 5,632 | 98 | Paved parking, HSG D |

## Summary for Subcatchment C27: CB \#27

Runoff $=\quad 1.21 \mathrm{cfs}$ @ 12.09 hrs, Volume= 4,366 cf, Depth> 8.57"
Routed to Pond CB27 : CB \#27
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 98 | 39 | $>75 \%$ Grass cover, Good, HSG A |
| 131 | 98 | Paved parking, HSG A |
| 397 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 5,485 | 98 | Paved parking, HSG D |
| 6,111 | 96 | Weighted Average |
| 495 |  | 8.10\% Pervious Area |
| 5,616 |  | $91.90 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ |
| ---: | ---: | ---: | ---: | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description | Direct Entry, |
| :--- |

## Summary for Subcatchment C28: CB \#28

Runoff $=\quad 1.95$ cfs @ 12.09 hrs, Volume $=\quad 6,570 \mathrm{cf}$, Depth> 7.60"
Routed to Pond CB28 : CB \#28
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 2,751 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 2,841 | 98 | Paved parking, HSG C |
| 2,297 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 2,483 | 98 | Paved parking, HSG D |

Runoff $=\quad 1.67$ cfs @ 12.09 hrs, Volume= $\quad 5,898$ cf, Depth> 8.33"
Routed to Pond CB29 : CB \#29
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 100YR Rainfall=9.06"

| $\begin{array}{r} \text { Area (sf) } \\ \hline 1,341 \end{array}$ |  | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 74 |  |  |  |
|  | 5,330 | 98 P | >75\% Grass cover, Good, HSG C Paved parking, HSG C |  |  |
|  | 1,824 | 98 R | Roofs, HSG |  |  |
|  | 8,495 | 94 | Weighted | verage |  |
|  | 1,341 |  | 15.79\% Pe | vious Area |  |
|  | 7,154 |  | 84.21\% Im | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |

## Summary for Subcatchment C30: CB \#30

Runoff $=\quad 1.76$ cfs @ 12.09 hrs, Volume= 6,202 cf, Depth> 8.33"

Routed to Pond CB30 : CB \#30
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,572 | 74 |  |  |  |
|  | 6,310 | 98 P | Paved parking, HSG C |  |  |
|  | 1,051 | 98 | Roofs, HSG C |  |  |
|  | 8,933 | 94 | Weighted Average |  |  |
|  | 1,572 |  | 17.60\% Pervious Area |  |  |
|  | 7,361 |  | 82.40\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C31: CB \#31

Runoff $=\quad 3.13$ cfs @ 12.09 hrs, Volume= 10,699 cf, Depth> 7.85"
Routed to Pond CB31 : CB \#31
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 5,132 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 9,132 | 98 P | Paved parking, HSG C |  |  |
|  | 2,101 | 98 R | Roofs, HSG C |  |  |
|  | 16,365 | 90 | Weighted Average |  |  |
|  | 5,132 |  | 31.36\% Pervious Area |  |  |
|  | 11,233 |  | 68.64\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ (\mathrm{cfs}) \\ \hline \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment C32: CB \#32

Runoff $=\quad 2.45$ cfs @ 12.09 hrs, Volume $=\quad 8,439 \mathrm{cf}$, Depth> 7.97"
Routed to Pond CB32 : CB \#32
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


Direct Entry,

## Summary for Subcatchment C34: CB \#34

Runoff $=\quad 1.69$ cfs @ 12.09 hrs, Volume $=\quad 5,899 \mathrm{cf}$, Depth> 8.21"
Routed to Pond CB34 : CB \#34
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,680 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 5,115 | 98 | Paved parking, HSG C |
| 1,827 | 98 | Roofs, HSG C |
| 8,622 | 93 | Weighted Average |
| 1,680 |  | $19.49 \%$ Pervious Area |
| 6,942 |  | $80.51 \%$ Impervious Area |



Routed to Pond CB35 : CB \#35
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,578 | 98 P |  |  |  |
|  | 79 | 80 > | Paved parking, HSG C <br> >75\% Grass cover, Good, HSG D |  |  |
|  | 492 | 98 P |  |  |  |
|  | 4,149 | 98 V | Weighted Average |  |  |
|  | 79 |  | 1.90\% Pervious Area |  |  |
|  | 4,070 |  | 98.10\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C36: CB \#36

Runoff $=\quad 1.32$ cfs @ 12.09 hrs, Volume= $\quad 4,864$ cf, Depth> 8.81"
Routed to Pond CB36 : CB \#36
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | ea (sf) | CN Description |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 6,622 | 98 | aved park | ng, HSG C |  |
| 6,622 |  | 100.00\% Impervious Area |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity <br> (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

Summary for Subcatchment C38: CB \#38
Runoff = 1.52 cfs @ 12.09 hrs, Volume= $5,610 \mathrm{cf}$, Depth> 8.81"
Routed to Pond CB38 : CB \#38
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 6,647 | 98 | Paved parking, HSG B |  |  |
|  | 392 | 98 | Paved parking, HSG C |  |  |
|  | 598 | 98 | Paved parking, HSG D |  |  |
|  | 7,637 | 98 | Weighted Average |  |  |
|  | 7,637 |  | 100.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity <br> (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment C39: CB \#39

Runoff $=\quad 1.52$ cfs @ 12.09 hrs, Volume $=\quad 5,591 \mathrm{cf}$, Depth> 8.81"
Routed to Pond CB39 : CB \#39
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

6.0

Direct Entry,

## Summary for Subcatchment C40: CB \#40

Runoff $=\quad 0.84$ cfs @ 12.09 hrs, Volume $=3,093 \mathrm{cf}$, Depth> 8.81"
Routed to Pond CB40 : CB \#40
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


## Summary for Subcatchment C41: CB \#41

Runoff =
$=\quad 1.11$ cfs @ 12.09 hrs, Volume=
4,103 cf, Depth> 8.81"
Routed to Pond CB41 : CB \#41
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


## Summary for Subcatchment C43: CB \#43

Runoff $=\quad 0.59$ cfs @ 12.09 hrs, Volume $=\quad 2,001 \mathrm{cf}$, Depth> 7.72"
Routed to Pond CB43 : CB \#43
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,343 | 98 P | Paved parking, HSG B $>75 \%$ Grass cover, Good, HSG B |  |  |
|  | 766 | $61>$ |  |  |  |
|  | 3,109 | 89 V | Weighted Average |  |  |
|  | 766 |  | 24.64\% Pervious Area |  |  |
|  | 2,343 |  | 75.36\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

Summary for Subcatchment C44: CB \#44
Runoff $=\quad 0.38$ cfs @ 12.09 hrs, Volume= $1,333 \mathrm{cf}$, Depth> 8.09"
Routed to Pond CB44 : CB \#44
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 1,670 | 98 | Paved parking, HSG B |
| 308 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 1,978 | 92 | Weighted Average |
| 308 |  | 15.57\% Pervious Area |
| 1,670 |  | $84.43 \%$ Impervious Area |



Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & 1,240 \\ & 1,225 \end{aligned}$ | $\begin{array}{ll} 98 \\ 61 \end{array}$ | Paved parking, HSG B $>75 \%$ Grass cover, Good, HSG B |  |  |
|  | $\begin{aligned} & 2,465 \\ & 1,225 \\ & 1,240 \end{aligned}$ | 80 | Weighted Average 49.70\% Pervious Area 50.30\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |

## Summary for Subcatchment C46: CB \#46

Runoff $=\quad 0.75$ cfs @ 12.09 hrs, Volume $=\quad 2,426 \mathrm{cf}$, Depth> 6.62"

Routed to Pond CB46 : CB \#46
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 2,241 | 98 | Paved parking, HSG B |
| 2,156 | 61 | >75\% Grass cover, Good, HSG B |
| 4,397 | 80 | Weighted Average |
| 2,156 |  | $49.03 \%$ Pervious Area |
| 2,241 | $50.97 \%$ Impervious Area |  |
| Tc Length Slope Velocity Capacity <br> (min) (feet) (ft/ft) (ft/sec) (cfs) |  |  |
| 6.0 |  | Direct Entry, |

Summary for Subcatchment C47: CB \#47
Runoff $=\quad 0.60$ cfs @ 12.09 hrs, Volume= $\quad 2,212 \mathrm{cf}$, Depth> 8.81"
Routed to Pond CB47 : CB\#47
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


## Summary for Subcatchment C48: CB \#48

Runoff $=\quad 7.16$ cfs @ 12.16 hrs, Volume= $\quad 26,943 \mathrm{cf}$, Depth> 5.38"
Routed to Pond CB48 : CB\#48
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) |  | CN | Description |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{array}{r} 3,877 \\ 56,251 \\ \hline \end{array}$ |  | Paved roads w/curbs \& sewers, HSG B 1 acre lots, $20 \%$ imp, HSG B |  |  |  |
|  | $\begin{aligned} & 60,128 \\ & 45,001 \\ & 15,127 \end{aligned}$ | 70 | Weighted Average <br> 74.84\% Pervious Area <br> 25.16\% Impervious Area |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | Capacity (cfs) | Description |  |
| 7.0 | 50 | 0.0800 | 0.12 |  | Sheet Flow, <br> Woods: Light underbrush $n=0.400$ | P2=3.27" |
| 4.8 | 350 | 0.0600 | 1.22 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |  |

11.8400 Total

Summary for Subcatchment C49: CB \#49
Runoff = 1.03 cfs @ 12.09 hrs, Volume= 3,637 cf, Depth> 8.33"
Routed to Pond CB49 : CB \#49
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 4,431 | 98 | Paved roads w/curbs \& sewers, HSG C |
| 807 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 5,238 | 94 | Weighted Average |
| 807 |  | 15.41\% Pervious Area |
| 4,431 |  | $84.59 \%$ Impervious Area |



Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| $\begin{array}{r} \text { Area }(\mathrm{sf}) \\ \hline 3,429 \end{array}$ |  | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 74 > | $>75 \%$ Grass cover, Good, HSG C Paved roads w/curbs \& sewers, HSG C |  |  |
|  | 11,611 | 98 P |  |  |  |
|  | 15,040 | 93 V | Weighted A | verage |  |
|  | 3,429 |  | 22.80\% Per | vious Area |  |
|  | 11,611 |  | 77.20\% Imp | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{gathered} \text { Capacity } \\ \text { (cfs) } \end{gathered}$ | Description |

Direct Entry,
Summary for Subcatchment C51: CB \#51
Runoff $=\quad 1.36$ cfs @ 12.09 hrs, Volume= 5,012 cf, Depth> 8.81"
Routed to Pond CB51 : CB \#51
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,147 | 98 R | Roofs, HSG C |  |  |
|  | 3,676 | 98 P | Paved parking, HSG C |  |  |
|  | 6,823 | 98 | Weighted Average |  |  |
|  | 6,823 |  | 100.00\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

Summary for Subcatchment C52: CB\#52
Runoff = 1.79 cfs @ 12.09 hrs, Volume= 6,376 cf, Depth> 8.45"
Routed to Pond CB52 : CB \#52
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & \hline 1,164 \\ & 7,888 \end{aligned}$ | $\begin{array}{ll} \hline 74 & 7 \\ 98 & F \end{array}$ | >75\% Grass cover, Good, HSG C Paved parking, HSG C |  |  |
|  | $\begin{aligned} & 9,052 \\ & 1,164 \\ & 7,888 \end{aligned}$ | $95 \quad 1$ | Weighted Average 12.86\% Pervious Area 87.14\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entr |

## Summary for Subcatchment C53: CB \#53

Runoff $=\quad 1.54$ cfs @ 12.09 hrs, Volume= 5,380 cf, Depth> 8.21"

Routed to Pond CB53 : CB \#53
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 287 | 39 > | >75\% Grass cover, Good, HSG A |  |  |
|  | 3,287 | 98 P | Paved parking, HSG A |  |  |
|  | 773 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 3,516 | 98 P | Paved parking, HSG C |  |  |
|  | 7,863 | 93 V | Weighted Average |  |  |
|  | 1,060 |  | 13.48\% Pervious Area |  |  |
|  | 6,803 |  | 86.52\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ftft) | Velocity (ft/sec) | Capacity (cfs) | Description |

6.0

Direct Entry,

## Summary for Subcatchment C54: CB \#54

Runoff $=\quad 0.93$ cfs @ 12.09 hrs, Volume= 3,201 cf, Depth> 7.97"
Routed to Pond CB54 : CB \#54
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 550 | 39 | $>75 \%$ Grass cover, Good, HSG A |
| 4,176 | 98 | Paved parking, HSG A |
| 84 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 11 | 98 | Paved parking, HSG C |
| 4,821 | 91 | Weighted Average |
| 634 |  | $13.15 \%$ Pervious Area |
| 4,187 |  | $86.85 \%$ Impervious Area |



Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 100YR Rainfall=9.06"

Area (sf) CN Description

|  | 4,650 | 98 Paved roads w/curbs \& sewers, HSG B |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 4,650 |  |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry, |

## Summary for Subcatchment C8: CB \#8

Runoff $=1.07$ cfs @ 12.09 hrs, Volume= 3,784 cf, Depth> 8.33"
Routed to Pond CB8 : CB\#8
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | Description <br> Paved roads w/curbs \& sewers, HSG B |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 4,837 |  |  |  |  |
|  | 613 |  | Paved roads w/curbs \& sewers, HSG B $>75 \%$ Grass cover, Good, HSG B |  |  |
|  | 5,450 | $94 \begin{array}{rr} \\ \\ & 11 \\ & 8\end{array}$ | Weighted Average <br> 11.25\% Pervious Area <br> 88.75\% Impervious Area |  |  |
|  | 613 |  |  |  |  |
|  | 4,837 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | $\begin{gathered} \text { Slope } \\ (\mathrm{ft} / \mathrm{ft}) \end{gathered}$ | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  | - |  |  | Direct Entry, |

## Summary for Subcatchment C9: CB \#9

Runoff $=\quad 3.25$ cfs @ 12.09 hrs, Volume $=\quad 11,814$ cf, Depth> 8.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG B |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 31 | 61 > |  |  |  |
|  | 433 | 98 P | Paved roads w/curbs \& sewers, HSG B |  |  |
|  | 904 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 12,073 | 98 P | Paved parking, HSG C |  |  |
|  | 2,305 | 98 R | Roofs, HSG C |  |  |
|  | 52 | $80>$ | >75\% Grass cover, Good, HSG D |  |  |
|  | 509 | 98 P | Paved parking, HSG D |  |  |
|  | 16,307 | 97 V | Weighted Average |  |  |
|  | 987 |  | 6.05\% Pervious Area |  |  |
|  | 15,320 |  | 93.95\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{rr} c & \text { Length } \\ \text { ) } & \text { (feet) } \\ \hline \end{array}$ | Slope $(\mathrm{ft} / \mathrm{ft})$ | $\begin{array}{r} \text { Velocity } \\ (\mathrm{ft} / \mathrm{sec}) \\ \hline \end{array}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry, |

## Summary for Subcatchment CH1: CLUBHOUSE

Runoff = 1.24 cfs @ 12.09 hrs, Volume= 4,474 cf, Depth> 8.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ |
| ---: | ---: | ---: | ---: | | Capacity |
| ---: |
| $(\mathrm{cfs})$ |$\quad$ Description | Direct Entry, |
| :--- |

## Summary for Subcatchment S201: SUMMER STREET ACCESS APRON

Runoff = 1.96 cfs @ 12.09 hrs, Volume= 7,004 cf, Depth> 8.45"
Routed to Link AP1 : ANALYSIS POINT 1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 717 | 61 | >75\% Grass cover, Good, HSG B |
| 9,226 | 98 | Paved parking, HSG B |

6.0

Direct Entry,

## Summary for Subcatchment S202: EXISTING WETLAND

Runoff $=\quad 44.78$ cfs @ 12.31 hrs, Volume $=219,981$ cf, Depth> 6.11"
Routed to Reach SC1 : Stream Crossing \#1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 136,496 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 83,935 | 55 | Woods, Good, HSG B |
| 29 | 98 | Paved parking, HSG B |
| 13,946 | 98 | Roofs, HSG B |
| 9,038 | 48 | Brush, Good, HSG B |
| 2,573 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 17,121 | 70 | Woods, Good, HSG C |
| 98 | 98 | Paved parking, HSG C |
| 1,097 | 65 | Brush, Good, HSG C |
| 126 | 80 | $>75 \%$ Grass cover, Good, HSG D |
| 132 | 98 | Paved parking, HSG D |
| 167,678 | 98 | Water Surface, HSG D |
| 432,269 | 76 | Weighted Average |
| 250,386 |  | 57.92\% Pervious Area |
| 181,883 |  | $42.08 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ | Description |
| ---: | ---: | ---: | ---: | :--- | :--- |
| 5.6 | 50 | 0.0200 | 0.15 | Sheet Flow, <br> Grass: Short $\mathrm{n}=0.150 \quad \mathrm{P} 2=3.27 "$ <br> 1.4 | 118 |
| 0.0400 | 1.40 | Shallow Concentrated Flow, <br> Short Grass Pasture Kv=7.0 fps <br> Shallow Concentrated Flow, <br> Woodland Kv=5.0 fps |  |  |  |
| 16.2 | 688 | 0.0200 | 0.71 |  |  |

## Summary for Subcatchment S203: POCKET WETLAND \#1

Runoff $=\quad 3.79$ cfs @ 12.09 hrs, Volume $=12,007$ cf, Depth> 5.63"

Routed to Pond p210 : POCKET WETLAND \#1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 12,682 | 61 | $>75 \%$ Grass cover, Good, HSG B |
| 1,060 | 98 | Water Surface, 0\% imp, HSG B |
| 7,785 | 74 | >75\% Grass cover, Good, HSG C |
| 4,060 | 98 | Water Surface, 0\% imp, HSG C |

## Summary for Subcatchment S204: EXISTING WETLANDS

Runoff $=31.68$ cfs @ 12.31 hrs, Volume $=153,689 \mathrm{cf}$, Depth> 5.98"
Routed to Link ap2 : ANALYSIS POINT 2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

$22.6 \quad 632$ Total

## Summary for Subcatchment S205: ISOLATED WETLAND

Runoff $=\quad 7.86$ cfs @ 12.09 hrs, Volume= $\quad 24,862$ cf, Depth> 5.38"
Routed to Link AP3 : ANALYSIS POINT 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 10,910 | 30 | Woods, Good, HSG A |
| 3,684 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 2,275 | 70 | Woods, Good, HSG C |
| 171 | 98 | Paved parking, HSG C |
| 1,706 | 65 | Brush, Good, HSG C |
| 1,940 | 80 | >75\% Grass cover, Good, HSG D |
| 23,513 | 77 | Woods, Good, HSG D |
| 393 | 98 | Paved parking, HSG D |
| 2,208 | 73 | Brush, Good, HSG D |
| 8,620 | 98 | Water Surface, HSG D |
| 55,420 | 70 | Weighted Average |
| 46,236 |  | 83.43\% Pervious Area |
| 9,184 |  | $16.57 \%$ Impervious Area |


| Tc <br> $(\mathrm{min})$ | Length <br> $(\mathrm{feet})$ | Slope <br> $(\mathrm{ft} / \mathrm{ft})$ | Velocity <br> $(\mathrm{ft} / \mathrm{sec})$ | Capacity <br> $(\mathrm{cfs})$ |
| ---: | ---: | ---: | ---: | :--- | Description | Direct Entry, |
| :--- |
| 6.0 |
|  |

Runoff $=60.37$ cfs @ 12.48 hrs, Volume $=\quad 351,563 \mathrm{cf}$, Depth> 4.73"
Routed to Link AP4 : ANALYSIS POINT \#4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 16,514 | 39 | >75\% Grass cover, Good, HSG A |
| 118,226 | 30 | Woods, Good, HSG A |
| 713 | 98 | Paved parking, HSG A |
| 41,148 | 30 | Brush, Good, HSG A |
| 17,568 | 51 | 1 acre lots, 20\% imp, HSG A |
| 37,410 | 61 | >75\% Grass cover, Good, HSG B |
| 13,900 | 55 | Woods, Good, HSG B |
| 54,538 | 48 | Brush, Good, HSG B |
| 91,202 | 68 | 1 acre lots, 20\% imp, HSG B |
| 77,444 | 74 | >75\% Grass cover, Good, HSG C |
| 114,763 | 70 | Woods, Good, HSG C |
| 3,493 | 98 | Paved parking, HSG C |
| 57,740 | 65 | Brush, Good, HSG C |
| 5,763 | 80 | >75\% Grass cover, Good, HSG D |
| 126,141 | 77 | Woods, Good, HSG D |
| 114,732 | 98 | Water Surface, 0\% imp, HSG D |

## Summary for Subcatchment S207: INFILTRATION POND \#2

Runoff $=3.91$ cfs @ 12.09 hrs , Volume=
13,178 cf, Depth> 7.60"
Routed to Pond P207 : INFILTRATION POND \#2
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 839 | 98 | Water Surface, 0\% imp, HSG A |
| 8,802 | 74 | >75\% Grass cover, Good, HSG C |
| 11,162 | 98 | Water Surface, 0\% imp, HSG C |

Summary for Subcatchment S208: GRASS AREA
Runoff $=\quad 2.16$ cfs @ 12.09 hrs, Volume $=\quad 6,883 \mathrm{cf}$, Depth> 6.00"
Routed to Pond OCS4 : OCS\#4
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN D | Description |
| :---: | :---: | :---: |
| 476 | $39>$ | >75\% Grass cover, Good, HSG A |
| 12,000 | $74>$ | >75\% Grass cover, Good, HSG C |
| 168 | 98 P | Paved parking, HSG A |
| 1,116 | 98 P | Paved parking, HSG C |
| 13,760 | 75 | Weighted Average |
| 12,476 |  | 90.67\% Pervious Area |
| 1,284 |  | 9.33\% Impervious Area |
| Tc Length (min) (feet) | Slope (ft/ft) | Velocity Capacity Description (ft/sec) (cfs) |

6.0

## Direct Entry,

## Summary for Subcatchment S209: WETLAND C

Runoff $=\quad 9.50$ cfs @ 12.40 hrs, Volume= 51,122 cf, Depth> 5.73"
Routed to Reach 11R : 4x4 Open Bottom Culvert
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


## Summary for Subcatchment S210: INFILTRATION POND \#1

Runoff $=\quad 9.35$ cfs @ 12.22 hrs, Volume $=\quad 40,231 \mathrm{cf}$, Depth> 6.36"

Routed to Pond P212 : INFILTRATION POND \#1
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


## Summary for Subcatchment S211: S211

Runoff $=\quad 2.59$ cfs @ 12.09 hrs, Volume $=8,357 \mathrm{cf}$, Depth> 6.50"

Routed to Pond P205 : INFILTRATION POND \#3
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


### 23.1418 Total

## Summary for Subcatchment S213: COURTYARD

Runoff $=\quad 3.59 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume $=$
Routed to Pond 11P :YARD DRAIN

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | >75\% Grass cover, Good, HSG A |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 5,047 | 39 > |  |  |  |
|  | 1,678 | 98 P | Paved parking, HSG A |  |  |
|  | 168 | 98 R | Roofs, HSG A |  |  |
|  | 532 | 98 V | Water Surface, 0\% imp, HSG A |  |  |
|  | 4,518 | 74 > | >75\% Grass cover, Good, HSG C |  |  |
|  | 7,080 | 98 P | Paved parking, HSG C |  |  |
|  | 878 | 98 R | Roofs, HSG C |  |  |
|  | 718 | 98 | Water Surface, 0\% imp, HSG C |  |  |
|  | 296 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 492 | 98 P | Paved parking, HSG D |  |  |
|  | 21,407 | 79 | Weighted Average |  |  |
|  | 11,111 |  | 51.90\% Pervious Area |  |  |
|  | 10,296 |  | 48.10\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment T1: Trench Drain 1

Runoff $=\quad 2.18 \mathrm{cfs} @ 12.09 \mathrm{hrs}$, Volume $=\quad 7,644 \mathrm{cf}$, Depth> 8.21"
Routed to Pond 5R : TRENCH DRAIN
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN | Description $>75 \%$ Grass cover, Good, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1,443 | 74 |  |  |  |
|  | 4,228 | 98 | Paved parking, HSG C |  |  |
|  | 1,339 | 80 | >75\% Grass cover, Good, HSG D |  |  |
|  | 4,163 | 98 | Paved parking, HSG D |  |  |
|  | 11,173 | 93 | Weighted Average |  |  |
|  | 2,782 |  | 24.90\% Pervious Area |  |  |
|  | 8,391 |  | 75.10\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | $\begin{array}{rr} c & \text { Length } \\ \text { 1) } & \text { (feet) } \\ \hline \end{array}$ | Slope $(\mathrm{ft} \mid \mathrm{ft})$ | $\begin{array}{rr} \text { e } \begin{array}{r} \text { Velocity } \\ \text { t) } \\ (\mathrm{ft} / \mathrm{sec}) \end{array} \\ \hline \end{array}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment T2: Drive Under B2

Runoff $=\quad 0.73$ cfs @ 12.09 hrs, Volume= $2,361 \mathrm{cf}$, Depth> 6.37"

Routed to Reach 11R : 4x4 Open Bottom Culvert
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"


Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN | Roofs, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,936 | 98 |  |  |  |
|  | 311 | 74 | Roofs, HSG C <br> $>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 4,247 | 96 | Weighted Average |  |  |
|  | 311 |  | 7.32\% Pervious Area |  |  |
|  | 3,936 |  | 92.68\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |

## Summary for Subcatchment TH10: TOWN HOUSE \#10

Runoff $=\quad 0.69$ cfs @ 12.09 hrs, Volume $=\quad 2,483 \mathrm{cf}$, Depth> 8.57"
Routed to Pond DE70 : DRIP \#70
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,184 | 98 | Roofs, HSG C |  |  |
|  | 292 | 74 | Roofs, HSG C <br> $>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 3,476 | 96 | Weighted Average |  |  |
|  | 292 |  | 8.40\% Pervious Area |  |  |
|  | 3,184 |  | 91.60\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{aligned} & \text { Length } \\ & \text { (feet) } \end{aligned}$ | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |

6.0 Direct Entry,

## Summary for Subcatchment TH11: TOWN HOUSE \#11

Runoff $=\quad 0.84$ cfs @ 12.09 hrs, Volume= $\quad 3,008 \mathrm{cf}$, Depth> 8.57" Routed to Pond DE71 : DRIP \#71

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,899 | 98 R | Roofs, HSG C$>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 311 | $74>$ |  |  |  |
|  | 4,210 | 96 | Weighted Average |  |  |
|  | 311 |  | 7.39\% Pervious Area |  |  |
|  | 3,899 |  | 92.61\% Im | ervious Ar |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{rr} c & \text { Length } \\ \text { ) } & \text { (feet) } \\ \hline \end{array}$ | Slope $(\mathrm{ft} / \mathrm{ft})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |

6.0

Direct Entry,

## Summary for Subcatchment TH2: TOWN HOUSE \#2

Runoff $=\quad 0.84$ cfs @ 12.09 hrs, Volume $=\quad 3,034$ cf, Depth> 8.57"
Routed to Pond DE62 : DRIP \#62
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,936 | 98 R | Roofs, HSG C |  |  |
|  | 311 | $74>$ | >75\% Grass cover, Good, HSG C |  |  |
|  | 4,247 | 96 V | Weighted Average |  |  |
|  | 311 |  | 7.32\% Pervious Area |  |  |
|  | 3,936 |  | 92.68\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH3: TOWN HOUSE \#3

Runoff $=\quad 0.60$ cfs @ 12.09 hrs, Volume= 2,122 cf, Depth> 8.45"
Routed to Pond DE63 : DRIP \#63
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN D | Rescript, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,672 | 9874 |  |  |  |
|  | 341 |  | Roofs, HSG C$>75 \%$ Grass cover Good HSG C |  | od, HSG C |
|  | 3,013 | $95 \quad 1$ | Weighted Average 11.32\% Pervious Area 88.68\% Impervious Area |  |  |
|  | 341 |  |  |  |  |
|  | 2,672 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | $\begin{array}{rr} c & \text { Length } \\ \text { (feet) } \\ \hline \end{array}$ | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH4: TOWN HOUSE \#4

Runoff $=\quad 0.69 \mathrm{cfs} @ 12.09$ hrs, Volume $=\quad 2,479 \mathrm{cf}$, Depth> 8.57"
Routed to Pond DE64 : DRIP \#64
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}$ Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,178 | 98 | Roofs, HSG C |
| 292 | 74 | >75\% Grass cover, Good, HSG C |
| 3,470 | 96 | Weighted Average |
| 292 |  | $8.41 \%$ Pervious Area |
| 3,178 |  | $91.59 \%$ Impervious Area |
| Tc | Length | Slope  <br> (ft/ft) Velocity <br> (ft/sec) Capacity <br> (min) (cfs) |

6.0

Direct Entry,

## Summary for Subcatchment TH5: TOWN HOUSE \#5

Runoff $=0.60$ cfs @ 12.09 hrs, Volume=
2,124 cf, Depth> 8.45"
Routed to Pond DE65 : DRIP \#65
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2,675 |  | Roofs, HSG C <br> $>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 341 |  |  |  |  |
|  | 3,016 | 95 | Weighted Average 11.31\% Pervious Area 88.69\% Impervious Area |  |  |
|  | 341 |  |  |  |  |
|  | 2,675 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH6: TOWN HOUSE \#6

Runoff $=\quad 0.68$ cfs @ 12.09 hrs, Volume $=\quad 2,434 \mathrm{cf}$, Depth> 8.57"
Routed to Pond DE66 : DRIP \#66
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-24.00 \mathrm{hrs}$, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,116 | 98 | Roofs, HSG C |
| 291 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 3,407 | 96 | Weighted Average |
| 291 |  | $8.54 \%$ Pervious Area |
| 3,116 |  | $91.46 \%$ Impervious Area |
| Tc | Length | Slope  <br> (ft/ft) Velocity <br> (ft/sec) Capacity <br> (min) (cfs) |

6.0

Direct Entry,

## Summary for Subcatchment TH7: TOWN HOUSE \#7

Runoff $=\quad 0.69$ cfs @ 12.09 hrs, Volume $=\quad 2,487 \mathrm{cf}$, Depth> 8.57"
Routed to Pond DE67 : DRIP \#67
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,189 | 98 | Roofs, HSG C |  |  |
|  | 292 | 74 | >75\% Grass cover, Good, HSG C |  |  |
|  | 3,481 | 96 | Weighted Average |  |  |
|  | 292 |  | 8.39\% Pervious Area |  |  |
|  | 3,189 |  | 91.61\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH8: TOWN HOUSE \#8

Runoff $=\quad 0.84$ cfs @ 12.09 hrs, Volume $=\quad 3,009 \mathrm{cf}$, Depth> 8.57"

Routed to Pond DE68 : DRIP \#68
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

|  | Area (sf) | CN | Roofs, HSG C |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3,901 | 98 R |  |  |  |
|  | 311 | $74>$ | Roofs, HSG C <br> $>75 \%$ Grass cover, Good, HSG C |  |  |
|  | 4,212 | $96 \begin{array}{r}\text { V } \\ 7 \\ \\ 9\end{array}$ | Weighted Average |  |  |
|  | 311 |  | 7.38\% Pervious Area |  |  |
|  | 3,901 |  | 92.62\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 6.0 |  |  |  |  | Direct Entry |

## Summary for Subcatchment TH9: TOWN HOUSE \#9

Runoff $=\quad 0.69$ cfs @ 12.09 hrs, Volume $=\quad 2,486 \mathrm{cf}$, Depth> 8.57"
Routed to Pond DE69 : DRIP \#69
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100YR Rainfall=9.06"

| Area (sf) | CN | Description |
| ---: | ---: | :--- |
| 3,188 | 98 | Roofs, HSG C |
| 292 | 74 | $>75 \%$ Grass cover, Good, HSG C |
| 3,480 | 96 | Weighted Average |
| 292 |  | $8.39 \%$ Pervious Area |
| 3,188 |  | $91.61 \%$ Impervious Area |
| Tc | Length | Slope  <br> (fft/ft) Velocity <br> (ft/sec)  |
| Capacity <br> (cfs) | Description |  |
| 6.0 |  | Direct Entry, |

## Summary for Reach 8R: OVERLAND FLOW

Inflow Area $=11,975$ sf, $92.37 \%$ Impervious, Inflow Depth > 7.60" for 100YR event Inflow $=1.97$ cfs @ 12.14 hrs, Volume $=\quad 7,579 \mathrm{cf}$ Outflow $=\quad 0.70 \mathrm{cfs}$ @ 12.48 hrs , Volume $=\quad 7,164 \mathrm{cf}$, Atten= 64\%, Lag= 20.2 min Routed to Link AP4 : ANALYSIS POINT \#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.12 \mathrm{fps}$, Min. Travel Time $=75.2 \mathrm{~min}$
Avg. Velocity $=0.05 \mathrm{fps}$, Avg. Travel Time $=175.9 \mathrm{~min}$

Peak Storage= 3,174 cf @ 12.48 hrs
Average Depth at Peak Storage= 0.11' , Surface Width= 51.12'
Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 28.09 cfs
50.00' x 1.00' deep channel, $\mathrm{n}=0.400$

Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length=563.0' Slope= 0.0213 '/'
Inlet Invert= 208.00', Outlet Invert= 196.00'


## Summary for Reach 9R: OVERLAND FLOW

Inflow Area $=\quad 32,665$ sf, $94.81 \%$ Impervious, Inflow Depth $=0.85^{\prime \prime}$ for 100YR event Inflow $=1.22$ cfs @ 12.42 hrs, Volume= 2,322 cf Outflow = $0.97 \mathrm{cfs} @ 12.60 \mathrm{hrs}$, Volume= $2,322 \mathrm{cf}$, Atten= $21 \%$, Lag= 10.5 min

Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.27 \mathrm{fps}, \mathrm{Min}$. Travel Time $=13.0 \mathrm{~min}$
Avg. Velocity $=0.06 \mathrm{fps}$, Avg. Travel Time $=56.1 \mathrm{~min}$
Peak Storage= 752 cf @ 12.60 hrs
Average Depth at Peak Storage $=0.16$ ' , Surface Width= 23.29'
Bank-Full Depth= $1.00^{\prime}$ Flow Area 30.0 sf, Capacity= 23.45 cfs
20.00' x 1.00' deep channel, $n=0.400$

Side Slope Z-value= 10.0 '/' Top Width= 40.00'
Length= 211.0' Slope= 0.0652 '/'
Inlet Invert= 201.75', Outlet Invert= 188.00'


## Summary for Reach 10R: OVERLAND FLOW

Inflow Area = 129,716 sf, 63.13\% Impervious, Inflow Depth = 2.92" for 100YR event Inflow = 10.24 cfs @ 12.28 hrs, Volume $=31,579 \mathrm{cf}$ Outflow $=8.88$ cfs @ 12.37 hrs, Volume $=31,578 \mathrm{cf}$, Atten= $13 \%$, Lag= 5.7 min

Routed to Link AP4 : ANALYSIS POINT \#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.47 \mathrm{fps}, \mathrm{Min}$. Travel Time $=5.8 \mathrm{~min}$
Avg. Velocity $=0.13 \mathrm{fps}$, Avg. Travel Time $=20.8 \mathrm{~min}$
Peak Storage= 3,056 cf @ 12.37 hrs
Average Depth at Peak Storage=0.69' , Surface Width= 33.85'
Bank-Full Depth=1.00' Flow Area= 30.0 sf, Capacity= 17.57 cfs
20.00' x 1.00' deep channel, $n=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 10.0 '/' Top Width= 40.00'
Length= 164.0' Slope= 0.0366 '/'
Inlet Invert= 192.00', Outlet Invert= 186.00'


Summary for Reach 11R: 4x4 Open Bottom Culvert
[52] Hint: Inlet/Outlet conditions not evaluated
[62] Hint: Exceeded Reach 20R OUTLET depth by 1.03' @ 12.45 hrs
[64] Warning: Exceeded Reach 20R outlet bank by 0.42' @ 12.47 hrs
[62] Hint: Exceeded Reach R211 OUTLET depth by 0.56 @ 12.45 hrs
[64] Warning: Exceeded Reach R211 outlet bank by 0.42 @ 12.47 hrs

| Inflow Area = | 424,818 | 45.99\% Impervious, | Depth > 4.09" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 24.22 cfs @ | 12.47 hrs , Volume= | 144,775 cf |
| Outflow | 24.22 cfs @ | 12.47 hrs , Volume= | 144,761 cf, Atten= 0\%, Lag= 0.1 m |

Routed to Reach 23R : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=2.73 \mathrm{fps}$, Min. Travel Time $=0.2 \mathrm{~min}$
Avg. Velocity $=0.95 \mathrm{fps}$, Avg. Travel Time $=0.5 \mathrm{~min}$
Peak Storage= 266 cf @ 12.47 hrs
Average Depth at Peak Storage= $2.22^{\prime}$, Surface Width= 4.00'
Bank-Full Depth= 4.00' Flow Area= 16.0 sf, Capacity= 42.20 cfs
48.0" W x 48.0" H Box Pipe
$\mathrm{n}=0.069$ Riprap, 6 -inch
Length=30.0' Slope $=0.0150$ '/'
Inlet Invert= 194.00', Outlet Invert= 193.55'


## Summary for Reach 12R: OVERLAND FLOW



Routed to Link AP2 : ANALYSIS POINT 2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity= $0.17 \mathrm{fps}, \mathrm{Min}$. Travel Time $=24.9 \mathrm{~min}$
Avg. Velocity $=0.06 \mathrm{fps}$, Avg. Travel Time $=74.2 \mathrm{~min}$
Peak Storage= 2,026 cf @ 12.29 hrs
Average Depth at Peak Storage= 0.16' , Surface Width= 51.60'
Bank-Full Depth $=1.00$ ' Flow Area $=55.0$ sf, Capacity= 29.80 cfs
$50.00^{\prime} \times 1.00$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length $=250.0^{\prime}$ Slope $=0.0240$ '/'
Inlet Invert= 202.00', Outlet Invert= 196.00'


## Summary for Reach 14R: OVERLAND FLOW

Inflow Area $=\quad 52,768$ sf, $0.60 \%$ Impervious, Inflow Depth > 4.74" for 100YR event Inflow $=4.27$ cfs @ 12.33 hrs , Volume $=\quad 20,864 \mathrm{cf}$ Outflow = 1.64 cfs @ 12.80 hrs , Volume $=19,373 \mathrm{cf}$, Atten= 62\%, Lag= 28.4 min

Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.18 \mathrm{fps}, \mathrm{Min}$. Travel Time $=79.0 \mathrm{~min}$
Avg. Velocity $=0.08 \mathrm{fps}$, Avg. Travel Time $=169.1 \mathrm{~min}$
Peak Storage= 7,796 cf @ 12.80 hrs
Average Depth at Peak Storage=0.18' , Surface Width= 53.54'
Bank-Full Depth= 1.00 Flow Area $=60.0$ sf, Capacity $=31.55$ cfs
$50.00^{\prime} \times 1.00$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 10.0 '/' Top Width= 70.00'
Length $=852.0$ ' Slope $=0.0246$ '/'
Inlet Invert= 207.00', Outlet Invert= 186.00'


## Summary for Reach 15R: OVERLAND FLOW

| Inflow Area = | 62,582 sf, | merviou | $3.97{ }^{\prime \prime}$ for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 3.51 cfs @ | 12.40 hrs , Volume= | 20,698 cf |
| Outflow | 1.89 cfs @ | 12.67 hrs , Volume= | 19,826 cf, Atten= 46\%, Lag= 16.2 m |

Routed to Link AP2 : ANALYSIS POINT 2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.18 \mathrm{fps}, \mathrm{Min}$. Travel Time $=27.8 \mathrm{~min}$
Avg. Velocity $=0.09 \mathrm{fps}$, Avg. Travel Time $=57.7 \mathrm{~min}$
Peak Storage= 3,139 cf @ 12.67 hrs
Average Depth at Peak Storage=0.21' , Surface Width= 52.05'
Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 27.21 cfs
$50.00^{\prime} \times 1.00$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length=300.0' Slope= 0.0200 '/'
Inlet Invert= 202.00', Outlet Invert= 196.00'


## Summary for Reach 18R: OVERLAND FLOW

Inflow Area $=\quad 88,676$ sf, $39.42 \%$ Impervious, Inflow Depth $=3.01$ " for 100YR event
Inflow = 10.22 cfs @ 12.18 hrs , Volume= $\quad 22,242 \mathrm{cf}$
Outflow $=\quad 3.56 \mathrm{cfs} @ 12.56 \mathrm{hrs}$, Volume $=\quad 21,983 \mathrm{cf}$, Atten= $65 \%$, Lag= 22.9 min
Routed to Link AP4 : ANALYSIS POINT \#4
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.24 \mathrm{fps}$, Min. Travel Time $=42.7 \mathrm{~min}$
Avg. Velocity $=0.09 \mathrm{fps}$, Avg. Travel Time $=116.4 \mathrm{~min}$
Peak Storage= 9,121 cf @ 12.56 hrs
Average Depth at Peak Storage= 0.26 ' , Surface Width= 63.23'
Bank-Full Depth= 1.00' Flow Area= 75.0 sf, Capacity= 38.42 cfs
$50.00^{\prime} \times 1.00$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 25.0 ' $/$ ' Top Width= 100.00'
Length=609.0' Slope= 0.0279 '/'
Inlet Invert= 203.00', Outlet Invert= 186.00'


## Summary for Reach 20R: OVERLAND FLOW

| low | 2 sf , 68.72\% Imperviou | Inflow Depth > 5.95" for 100YR event |
| :---: | :---: | :---: |
| Inflow | 8.98 cfs @ 12.18 hrs, Volume= | 35,788 cf |
| Outflow | 3.75 cfs @ 12.53 hrs, Volume= | 34,661 cf, Atten= 58\%, Lag= 20.8 min |

Routed to Reach 11R : 4x4 Open Bottom Culvert
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.19 \mathrm{fps}, \mathrm{Min}$. Travel Time $=50.3 \mathrm{~min}$
Avg. Velocity $=0.08 \mathrm{fps}$, Avg. Travel Time $=118.6 \mathrm{~min}$
Peak Storage= 11,298 cf @ 12.53 hrs
Average Depth at Peak Storage= 0.39' , Surface Width= 53.88'
Bank-Full Depth= 1.00' Flow Area= 55.0 sf, Capacity= 18.54 cfs
$50.00^{\prime} \times 1.00$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 5.0 '/' Top Width= 60.00'
Length $=560.0$ ' Slope $=0.0093$ '/'
Inlet Invert= 200.00', Outlet Invert= 194.80'


## Summary for Reach 23R: OVERLAND FLOW

[61] Hint: Exceeded Reach 11R outlet invert by 0.28 @ 12.60 hrs

| Inflow Area | 424,818 sf, 45.99\% Impervious, | w Depth > 4.09" for 100YR event |
| :---: | :---: | :---: |
| Inflow | 24.22 cfs @ 12.47 hrs, Volume= | 144,761 cf |
| Outflow | 22.58 cfs @ 12.60 hrs , Volume= | 143,931 cf, Atten= 7\%, Lag= 8.0 min |

Routed to Link AP4 : ANALYSIS POINT \#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.41 \mathrm{fps}, \mathrm{Min}$. Travel Time $=9.6 \mathrm{~min}$
Avg. Velocity $=0.13 \mathrm{fps}$, Avg. Travel Time $=29.8 \mathrm{~min}$
Peak Storage= 13,052 cf @ 12.60 hrs
Average Depth at Peak Storage=0.83' , Surface Width= 83.10'
Bank-Full Depth=1.00' Flow Area= 70.0 sf, Capacity= 31.93 cfs
$50.00^{\prime} \times 1.00^{\prime}$ deep channel, $n=0.400$ Sheet flow: Woods+light brush
Side Slope Z-value= 20.0 '/' Top Width= 90.00'
Length= 237.0' Slope= 0.0211 '/'
Inlet Invert= 193.00', Outlet Invert= 188.00'


## Summary for Reach R202: OVERLAND FLOW

[55] Hint: Peak inflow is 105\% of Manning's capacity
[62] Hint: Exceeded Reach SC1 OUTLET depth by 0.41' @ 12.90 hrs

| Inflow Area = | 432,269 sf, 42.08\% Impervious, | 6.11" for 100YR event |
| :---: | :---: | :---: |
| Inflow | 44.78 cfs @ 12.32 hrs, Volume= | 219,947 cf |
| Outflow | 25.82 cfs @ 12.63 hrs, Volume= | $212,555 \mathrm{cf}$, Atten= $42 \%$, Lag $=18.6$ min | Routed to Link AP2 : ANALYSIS POINT 2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=0.29 \mathrm{fps}, \mathrm{Min}$. Travel Time $=40.5 \mathrm{~min}$
Avg. Velocity $=0.11 \mathrm{fps}$, Avg. Travel Time $=106.8 \mathrm{~min}$
Peak Storage= 62,638 cf @ 12.63 hrs
Average Depth at Peak Storage= $0.75^{\prime}$, Surface Width= $137.65{ }^{\prime}$
Bank-Full Depth= 1.00' Flow Area= 125.0 sf, Capacity= 42.56 cfs
100.00 x 1.00 deep channel, $n=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 25.0 '/' Top Width= 150.00'
Length=700.0' Slope= $0.0107{ }^{\prime} / /$
Inlet Invert= 205.50', Outlet Invert= 198.00'


## Summary for Reach R211: OVERLAND FLOW

[55] Hint: Peak inflow is $191 \%$ of Manning's capacity

| Inflow Area $=$ | 241,078 sf, $59.10 \%$ Impervious, | Inflow Depth $=2.84 "$ | for 100 YR event |  |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $27.69 \mathrm{cfs} @$ | 12.16 hrs , Volume $=$ | $57,014 \mathrm{cf}$ |
| Outflow | $=$ | $11.38 \mathrm{cfs} @$ | 12.54 hrs , Volume $=$ | $56,631 \mathrm{cf}$, Atten $=59 \%$, Lag $=22.7 \mathrm{~min}$ |

Routed to Reach 11R : 4x4 Open Bottom Culvert
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity= 0.27 fps , Min. Travel Time= 37.1 min
Avg. Velocity $=0.09 \mathrm{fps}$, Avg. Travel Time $=117.0 \mathrm{~min}$
Peak Storage= 25,331 cf @ 12.54 hrs
Average Depth at Peak Storage $=0.88^{\prime}$, Surface Width= 61.30'
Bank-Full Depth= 1.00 ' Flow Area $=50.0$ sf, Capacity= 14.51 cfs
35.00 ' $1.00^{\prime}$ deep channel, $n=0.400$ Sheet flow: Woods+light brush

Side Slope Z-value= 15.0 '/' Top Width= 65.00'
Length=600.0' Slope= 0.0087 '/'
Inlet Invert= 200.00', Outlet Invert= 194.80'


## Summary for Reach SC1: Stream Crossing \#1

[52] Hint: Inlet/Outlet conditions not evaluated

| Inflow Area = | @, | 6.11 for 100YR event |
| :---: | :---: | :---: |
| Inflo | 44.78 cfs @ 12.31 hrs, Volume= | 219,981 cf |
| Outflow | 44.78 cfs @ 12.32 hrs , Volume= | 219,947 cf, Atten= 0\%, Lag= 0.1 min |

Routed to Reach R202 : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span $=0.00-24.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$
Max. Velocity $=4.71 \mathrm{fps}$, Min. Travel Time $=0.2 \mathrm{~min}$
Avg. Velocity $=1.47 \mathrm{fps}$, Avg. Travel Time $=0.5 \mathrm{~min}$
Peak Storage= 409 cf @ 12.32 hrs
Average Depth at Peak Storage= 0.59' , Surface Width= 16.00'
Bank-Full Depth= 5.00' Flow Area= 69.8 sf, Capacity= 722.91 cfs
192.0" W x 60.0" H, R=207.0" Arch Pipe
$\mathrm{n}=0.030$ Stream, clean \& straight
Length= 43.1' Slope= 0.0200 '/'
Inlet Invert= 206.37', Outlet Invert= 205.51'

## Summary for Reach SC2: Stream Crossing \#2

[52] Hint: Inlet/Outlet conditions not evaluated

| In | 8 | 0.60\% Impervious, | Inflow Depth > 4.75" for 100YR event |
| :---: | :---: | :---: | :---: |
| Infl | 4.28 cfs @ | 12.32 hrs , Volume= | 20,867 cf |
| Outflow | 4.27 cfs @ | 12.33 hrs , Volume= | 20,864 cf, Atten= 0\%, Lag= 0.1 min |

Routed to Reach 14R : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Max. Velocity $=2.01 \mathrm{fps}$, Min. Travel Time $=0.3 \mathrm{~min}$
Avg. Velocity $=1.08 \mathrm{fps}$, Avg. Travel Time $=0.6 \mathrm{~min}$
Peak Storage= 77 cf @ 12.33 hrs
Average Depth at Peak Storage= 0.13' , Surface Width= 16.00'
Bank-Full Depth= 5.00' Flow Area= 68.1 sf, Capacity= 768.96 cfs
192.0" W x 60.0" H, R=180.0" Arch Pipe
$\mathrm{n}=0.030$ Stream, clean \& straight
Length= 36.5' Slope $=0.0241$ '/'
Inlet Invert= 208.52', Outlet Invert= 207.64'


## Summary for Pond 1P: DMH \#33

| Inflow Area = | 16,111 sf, 93.77\% Impervious, | w Depth > 8.65" for 100YR event |
| :---: | :---: | :---: |
| Inflow | 3.20 cfs @ 12.09 hrs , Volume= | 11,616 cf |
| Outflow | 3.20 cfs @ 12.09 hrs , Volume= | 11,616 cf, Atten= 0\%, Lag= 0.0 |
| Primary | 3.20 cfs @ 12.09 hrs , Volume= | 11,616 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=206.71' @ 12.09 hrs
Flood Elev= 209.64'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 205.50' | 12.0" Round Culvert L= 46.7' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 205.50' $/ 204.33$ ' S=0.0251 '/' Cc= 0.900 |

$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$
Primary OutFlow Max=3.12 cfs @ 12.09 hrs HW=206.68' TW=203.63' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls 3.12 cfs @ 3.97 fps )

## Summary for Pond 3P: OCS \#8



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=203.30' @ 12.41 hrs
Flood Elev=206.36'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | ---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $200.62^{\prime}$ | $12.0^{\prime \prime}$ Vert. Orifice/Grate | $C=0.600 \quad$ Limited to weir flow at low heads |

Primary OutFlow Max=2.40 cfs @ 12.09 hrs HW=202.18' TW=201.77' (Dynamic Tailwater)
$廿_{1=O r i f i c e / G r a t e ~(O r i f i c e ~ C o n t r o l s ~}^{2.40} \mathrm{cfs} @ 3.06 \mathrm{fps}$ )

## Summary for Pond 5R: TRENCH DRAIN

| Inflow Area = | 11,173 sf, | mpervious, | Inflow Depth > 8.21" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 2.18 cfs @ | 12.09 hrs , Volume= | 7,644 cf |
| Outflow | 2.18 cfs @ | 12.09 hrs , Volume= | $7,644 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 2.18 cfs @ | 12.09 hrs , Volume= | 7,644 cf |

Routed to Pond P206 : STORMTECH INFILTRATION SYSTEM \#2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev= 199.24' @ 12.09 hrs
Flood Elev= 199.50'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $197.22^{\prime}$ | $8.0^{\prime \prime}$ Round Culvert $\mathrm{L}=36.0^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= $197.22^{\prime} / 196.50^{\prime} \quad \mathrm{S}=0.0200 \quad \mathrm{I} / \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 0.35 sf |  |

Primary OutFlow Max=2.13 cfs @ 12.09 hrs HW=199.16' TW=196.52' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls $2.13 \mathrm{cfs} @ 6.09 \mathrm{fps}$ )

## Summary for Pond 11P: YARD DRAIN

| Inflow Area = | 21,407 | pervious, | flow Depth > 6.50" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 3.59 cfs @ | 12.09 hrs , Volume= | 11,590 cf |
| Outflow | 2.82 cfs @ | 12.16 hrs , Volume= | $11,525 \mathrm{cf}$, Atten= 22\%, Lag= 4.2 min |
| Primary | 2.82 cfs @ | 12.16 hrs , Volume= | 11,525 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 207.48' @ 12.16 hrs Surf.Area= 6,408 sf Storage= 1,296 cf
Plug-Flow detention time $=14.4 \mathrm{~min}$ calculated for $11,501 \mathrm{cf}$ ( $99 \%$ of inflow)
Center-of-Mass det. time $=10.9 \mathrm{~min}$ ( 811.0-800.1)


Primary OutFlow Max=2.78 cfs @ 12.16 hrs HW=207.47' TW=203.83' (Dynamic Tailwater)
\&1=Culvert (Passes 2.78 cfs of 6.70 cfs potential flow)
$\complement_{2=O r i f i c e / G r a t e ~(W e i r ~ C o n t r o l s ~}^{2.78} \mathrm{cfs} @ 1.55 \mathrm{fps}$ )

## Summary for Pond CB10: CB \#10

| Inflow Area = | 6,961 sf, 100 | .00\% Impervious, | Inflow Depth > 8.81" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.39 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 5,113 cf |
| Outflow | 1.39 cfs @ 12, | . $09 \mathrm{hrs}$, Volume= | $5,113 \mathrm{cf}$, Atten= $0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pond | $\begin{aligned} & 1.39 \mathrm{cfs} @ 12 . \\ & \text { D5 : DMH \#5 } \end{aligned}$ | 09 hrs , Volume= | $5,113 \mathrm{cf}$ |
| Routing by Dyn-S <br> Peak Elev= 210.7 <br> Flood Elev= 212 | or-Ind method, $8^{\prime}$ @ 12.09 hrs | ime Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 209.76' | 12.0" Round Cu Inlet / Outlet Inver $\mathrm{n}=0.013$ Corruga | $\begin{aligned} & \text { vert } \mathrm{L}=33.8^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=209.76^{\prime} / 209.59^{\prime} \quad \mathrm{S}=0.0050 \mathrm{l} \end{aligned}$ $\text { ted PE, smooth interior, Flow Area= } 0.79 \text { sf }$ |

Primary OutFlow Max=1.33 cfs @ 12.09 hrs HW=210.75' TW=210.59' (Dynamic Tailwater)
-1=Culvert (Outlet Controls 1.33 cfs @ 2.13 fps )

## Summary for Pond CB11: CB \#11

| Inflow Area = | 7,173 sf, 1 | .00\% Impervious, | Inflow Depth > 8.81" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.43 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 5,269 cf |
| Outflow = | 1.43 cfs @ 12 | . $09 \mathrm{hrs}$, , Volume= | $5,269 \mathrm{cf}, \mathrm{Atten}=0 \%, \mathrm{Lag}=0.0 \mathrm{~min}$ |
| Primary = Routed to Pon | $1.43 \mathrm{cfs} @ 12$ | .09 hrs , Volume= | 5,269 cf |
| Routing by Dyn-S <br> Peak Elev= 210.83 <br> Flood Elev= 213 | or-Ind method, @ 12.09 hrs | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 209.94' | $\begin{aligned} & \text { 12.0" Round Cul } \\ & \text { Inlet / Outlet Inver } \\ & \mathrm{n}=0.013 \text { Corruga } \end{aligned}$ | $\begin{aligned} & \text { lvert } \mathrm{L}=26.3^{\prime} \mathrm{Ke}=0.500 \\ & \mathrm{t}=209.94^{\prime} / 209.67^{\prime} \quad \mathrm{S}=0.0103^{\prime} / \prime \quad \mathrm{Cc}=0.900 \\ & \text { ated PE, smooth interior, Flow Area= } 0.79 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=1.38 cfs @ 12.09 hrs HW=210.81' TW=210.59' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 1.38 cfs @ 2.55 fps )

## Summary for Pond CB12: CB \#12

| Inflow Area $=$ | $5,238 \mathrm{sf}, 100.00 \%$ | Impervious, | Inflow Depth > 8.81" | for 100 YR event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $1.04 \mathrm{cfs} @$ | 12.09 hrs , Volume | $3,847 \mathrm{cf}$ |
| Outflow | $=$ | $1.04 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $3,847 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | $=$ | $1.04 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $3,847 \mathrm{cf}$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.24' @ 12.09 hrs
Flood Elev=209.84'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 206.68' | 12.0" Round Culvert L= 41.3' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 206.68' / 205.65' S=0.0249'/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 s |

Primary OutFlow Max=1.02 cfs @ 12.09 hrs HW=207.22' TW=206.68' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 1.02 cfs @ 3.38 fps )

## Summary for Pond CB13: CB \#13

| Inflow Area $=$ | $10,873 \mathrm{sf}, 90.78 \%$ | Impervious, | Inflow Depth $>88.57 "$ |
| :--- | :--- | :--- | :--- |
| for 100 YR event |  |  |  |
| Inflow | $=$ | $2.16 \mathrm{cfs} @$ | 12.09 hrs , Volume |
| Outflow | $=$ | $2.16 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |
| Primary | $=$ | $2.16 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 207.53' @ 12.09 hrs
Flood Elev=209.86'


Primary OutFlow Max=2.10 cfs @ 12.09 hrs HW=207.51' TW=206.68' (Dynamic Tailwater)
—1=Culvert (Inlet Controls $2.10 \mathrm{cfs} @ 3.07 \mathrm{fps}$ )

## Summary for Pond CB14: CB \#14



## Summary for Pond CB15: CB \#15



Primary OutFlow Max=1.29 cfs @ 12.09 hrs HW=203.05' TW=202.93' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.29 cfs @ 1.65 fps )

## Summary for Pond CB16: CB \#16

| Inflow Area = | 8,516 sf, 64.88\% Impervious, Inflow Depth > 6.50" for 100YR event |  |  |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.43 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 4,611 cf |
| Outflow = | 1.43 cfs @ 12 | . 09 hrs, Volume= | $4,611 \mathrm{cf}$, Atten= $0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pond | $\begin{aligned} & 1.43 \mathrm{cfs} @ 12 \\ & \text { D10 : DMH \#1 } \end{aligned}$ | .09 hrs , Volume= | 4,611 cf |
| Routing by Dyn-S <br> Peak Elev= 204. <br> Flood Elev= 206 | or-Ind method, @ 12.09 hrs | ime Span= 0.00-2 | $.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 203.47' | 12.0" Round Culv Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { vert } \mathrm{L}=20.9^{\prime} \mathrm{Ke}=0.500 \\ & \mathrm{t}=203.47^{\prime} / 203.33^{\prime} \mathrm{S}=0.0067^{\prime} / l^{\prime} \quad \mathrm{Cc}=0.900 \end{aligned}$ $\text { ted PE, smooth interior, Flow Area }=0.79 \mathrm{sf}$ |

Primary OutFlow Max=1.40 cfs @ 12.09 hrs HW=204.32' TW=204.09' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 1.40 cfs @ 2.65 fps )

## Summary for Pond CB17: CB \#17

| Inflow Area $=$ | $11,836 \mathrm{sf}$, | $73.87 \%$ | Impervious, |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | 2.30 cfs @ | 12.09 hrs , Volume $=$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.31' @ 12.10 hrs
Flood Elev= 208.16'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $204.99^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=13.8^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= 204.99' $/ 204.86^{\prime} \quad \mathrm{S}=0.0094$ '/' $\quad \mathrm{Cc}=0.900$ |  |
|  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |

Primary OutFlow Max=0.64 cfs @ 12.09 hrs HW=207.15' TW=207.12' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 0.64 cfs @ 0.82 fps)

## Summary for Pond CB18: CB \#18



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 207.38' @ 12.11 hrs
Flood Elev=208.16'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 204.72' | 15.0" Round Culvert L=25.1' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert=204.72' / 204.59' S=0.0052 '/l' Cc=0.900 <br> $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.23 sf |

Primary OutFlow Max=1.87 cfs @ 12.09 hrs HW=207.25' TW=207.15' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.87 cfs @ 1.52 fps)

## Summary for Pond CB20: CB \#20



Primary OutFlow Max=2.30 cfs @ 12.09 hrs HW=205.72' TW=205.35' (Dynamic Tailwater)
$\left\llcorner_{1=C u l v e r t ~(I n l e t ~ C o n t r o l s ~}^{2.30} \mathrm{cfs} @ 2.92 \mathrm{fps}\right.$ )

## Summary for Pond CB21: CB \#21

| Inflow Area $=$ | $10,174 \mathrm{sf}$, | $87.04 \%$ | Impervious, | Inflow Depth $>$ |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $1.95 \mathrm{cfs} @$ | 12.09 hrs , Volume= | for 100 YR event |
| Outflow | $=$ | $1.95 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $6,652 \mathrm{cf}$ |
| Primary | $=$ | $1.95 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $6,652 \mathrm{cf}$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.70' @ 12.09 hrs
Flood Elev= 208.02'


Primary OutFlow Max=1.91 cfs @ 12.09 hrs HW=205.61' TW=205.35' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.91 cfs @ 2.44 fps )

## Summary for Pond CB22: CB \#22

| Inflow Area = | $12,001 \mathrm{sf}$, 9 | 91.62\% Impervious, | Inflow Depth > 8.57" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 2.38 cfs @ 12 | 12.09 hrs , Volume= | 8,574 cf |
| Outflow | 2.38 cfs @ 12. | 12.09 hrs , Volume= | $8,574 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary = Routed to Pond | $\begin{aligned} & 2.38 \mathrm{cfs} @ 12 \\ & \mathrm{D} 14 \text { : DMH \#1 } \end{aligned}$ | 12.09 hrs , Volume= 14 | 8,574 cf |
| Routing by Dyn-S <br> Peak Elev= 206.4 <br> Flood Elev= 208 | or-Ind method, ' @ 12.09 hrs | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 205.33' | 12.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.012$ Corrug | $\begin{aligned} & \text { vert } \mathrm{L}=16.1^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=205.33^{\prime} / 205.25^{\prime} \quad \mathrm{S}=0.0050 \text { '/' } \mathrm{Cc}=0.900 \end{aligned}$ $\text { ted PP, smooth interior, Flow Area }=0.79 \mathrm{sf}$ |

Primary OutFlow Max=2.32 cfs @ 12.09 hrs HW=206.39' TW=205.73' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 2.32 cfs @ 3.48 fps )

## Summary for Pond CB23: CB \#23



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.30' @ 12.09 hrs
Flood Elev=208.57'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $205.41^{\prime}$ | $\mathbf{1 2 . 0 "}$ Round Culvert $\mathrm{L}=16.3^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $205.41^{\prime} / 205.32^{\prime} \quad \mathrm{S}=0.0055 \mathrm{I} / /^{\prime} \quad \mathrm{Cc}=0.900$ |
|  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=1.79 cfs @ 12.09 hrs HW=206.29' TW=205.73' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 1.79 cfs @ 3.27 fps )

## Summary for Pond CB24: CB \#24

| Inflow Area = | 7,930 | 72.16\% Imperviou | Depth > 8.21" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.55 cfs @ | 12.09 hrs , Volume= | 5,426 cf |
| Outflow | 1.55 cfs @ | 12.09 hrs , Volume= | $5,426 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.55 cfs @ | 12.09 hrs , Volume= | 5,426 cf | Routed to Pond D16 : DMH \#16

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 206.37' @ 12.09 hrs
Flood Elev=209.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- | :--- |
| $\# 1$ | Primary | $205.21^{\prime}$ | $\mathbf{1 2 . 0}$ " Round Culvert $\mathrm{L}=12.1^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $205.21^{\prime} / 205.15^{\prime} \quad \mathrm{S}=0.0050$ |
|  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |

Primary OutFlow Max=1.45 cfs @ 12.09 hrs HW=206.33' TW=206.19' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.45 cfs @ 1.84 fps )

## Summary for Pond CB25: CB \#25



## Summary for Pond CB26: CB \#26



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=202.61' @ 12.09 hrs
Flood Elev= 204.93'
Device Routing Invert Outlet Devices
\#1 Primary 201.77' 12.0" Round Culvert L=42.5' Ke=0.500
Inlet / Outlet Invert= 201.77' / 201.55' S=0.0052 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.66 cfs @ 12.09 hrs HW=202.60' TW=201.61' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 1.66 cfs @ 3.23 fps )

## Summary for Pond CB27: CB \#27

| Inflow Area = | 6,111 sf, 91.90\% Impervious, Inflow Depth > 8.57" for 100YR event |  |  |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.21 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 4,366 cf |
| Outflow = | 1.21 cfs @ 12 | . 09 hrs, Volume= | $4,366 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pon | 1.21 cfs @ 12 D17 : DMH \#1 | .09 hrs , Volume= | 4,366 cf |
| Routing by Dyn-S <br> Peak Elev= 201.8 <br> Flood Elev= 204 | or-Ind method, @ 12.09 hrs ${ }^{\prime}$ | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 201.00' | 12.0" Round Cul Inlet / Outlet Inver $\mathrm{n}=0.013$ Corruga | $\begin{aligned} & \text { lvert } \mathrm{L}=18.0^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=201.00 \text { ' } 200.90^{\prime} \quad \mathrm{S}=0.0056 \mathrm{l} / \mathrm{Cc}=0.900 \end{aligned}$ $\text { ted PE, smooth interior, Flow Area= } 0.79 \mathrm{sf}$ |

Primary OutFlow Max=1.18 cfs @ 12.09 hrs HW=201.79' TW=201.61' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 1.18 cfs @ 2.42 fps )

## Summary for Pond CB28: CB \#28

| Inflow Area = | 10,372 | rviou | 7.60" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.95 cfs @ | 12.09 hrs , Volume= | 6,570 cf |
| Outflow | 1.95 cfs @ | 12.09 hrs , Volume= | $6,570 \mathrm{cf}$, Atten $=0 \%, L a g=0.0 \mathrm{~min}$ |
| Primary | 1.95 cfs @ | 12.09 hrs , Volume= | 6,570 cf |

Routed to Pond D18: DMH \#18
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 199.02' @ 12.09 hrs
Flood Elev= 200.92'


Primary OutFlow Max=1.90 cfs @ 12.09 hrs HW=198.98' TW=198.73' (Dynamic Tailwater)
_1=Culvert (Inlet Controls 1.90 cfs @ 2.42 fps )

## Summary for Pond CB29: CB \#29



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 206.87' @ 12.09 hrs
Flood Elev=208.55'


Primary OutFlow Max=1.35 cfs @ 12.09 hrs HW=206.79' TW=206.66' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.35 cfs @ 1.71 fps)

## Summary for Pond CB30: CB \#30



Routed to Pond D19: DMH \#19
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=206.89' @ 12.09 hrs
Flood Elev= 208.54'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $205.38^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=17.5^{\prime} \quad \mathrm{Ke}=0.500$ <br>  <br>  <br>  <br>  <br>  <br>  <br>  <br>  <br>  <br> $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=1.45 cfs @ 12.09 hrs HW=206.81' TW=206.66' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls $1.45 \mathrm{cfs} @ 1.84 \mathrm{fps}$ )

## Summary for Pond CB31: CB \#31

| Inflow Area = | $16,365 \mathrm{sf}$, 68 | 68.64\% Impervious, | Inflow Depth > 7.85" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 3.13 cfs @ 12 | 12.09 hrs , Volume= | 10,699 cf |
| Outflow = | 3.13 cfs @ 12 | 12.09 hrs , Volume= | 10,699 cf, Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pon | $\begin{aligned} & 3.13 \mathrm{cfs} @ 12 \\ & \mathrm{x} \text { D21 : DMH \#2 } \end{aligned}$ | 12.09 hrs , Volume= 21 | 10,699 cf |
| Routing by Dyn-S <br> Peak Elev= 205.8 <br> Flood Elev= 207 | or-Ind method, ' @ 12.09 hrs | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | $t$ Outlet Devices |  |
| \#1 Primary | 204.19' | 12.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { Ivert } \mathrm{L}=16.4^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=204.19^{\prime} / 204.11^{\prime} \quad \mathrm{S}=0.0049 \mathrm{I} / \mathrm{Cc}=0.900 \\ & \text { ated PE, smooth interior, Flow Area= } 0.79 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=3.05 cfs @ 12.09 hrs HW=205.77' TW=205.12' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 3.05 cfs @ 3.89 fps )

## Summary for Pond CB32: CB \#32

| Inflow Area $=$ | $12,710 \mathrm{sf}$, | $70.47 \%$ | Impervious, | Inflow Depth $>$ |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $2.45 \mathrm{cfs} @$ | 12.09 hrs , Volume | for 100 YR event |
| Outflow | $=$ | $2.45 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $8,439 \mathrm{cf}$ |
| Primary | $=$ | $2.45 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $8,439 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ | Routed to Pond D21 : DMH \#21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=205.58' @ 12.09 hrs
Flood Elev= 207.35'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $204.19^{\prime}$ | 12.0" Round Culvert L= 16.3' $\mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 204.19' $/ 204.11^{\prime} \quad \mathrm{S}=0.0049$ '/' Cc= 0.900 |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=2.39 cfs @ 12.09 hrs HW=205.52' TW=205.12' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 2.39 cfs @ 3.04 fps )

## Summary for Pond CB33: CB \#33



Routed to Pond D22 : DMH \#22
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.39' @ 12.09 hrs
Flood Elev=208.45'


Primary OutFlow Max=1.04 cfs @ 12.09 hrs HW=206.35' TW=206.27' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls 1.04 cfs @ 1.33 fps )

## Summary for Pond CB34: CB \#34

| Inflow Area = | 8,622 sf, | 80.51\% Impervious, | Inflow Depth > 8.21" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.69 cfs @ | 12.09 hrs , Volume= | 5,899 cf |
| Outflow | 1.69 cfs @ | 12.09 hrs , Volume= | $5,899 \mathrm{cf}$, Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 1.69 cfs @ | 12.09 hrs , Volume= | 5,899 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 206.50' @ 12.09 hrs
Flood Elev=208.38'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $205.21^{\prime}$ | $\mathbf{1 2 . 0}$ " Round Culvert $\mathrm{L}=16.5^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $205.21^{\prime} / 205.13^{\prime} \quad \mathrm{S}=0.0048 \mathrm{I} / /^{\prime} \quad \mathrm{Cc}=0.900$ |
| $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |  |

Primary OutFlow Max=1.64 cfs @ 12.09 hrs HW=206.46' TW=206.27' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.64 cfs @ 2.09 fps)

## Summary for Pond CB35: CB \#35

| Inflow Area = | 4,149 sf, 98.10\% Impervious, Inflow Depth > 8.81" for 100YR event |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 0.83 cfs @ 12 | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 3,048 cf |  |
| Outflow | 0.83 cfs @ 12 | . 09 hrs , Volume= | $3,048 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |  |
| Primary $=\quad 0.83$ cfs @ 12.09 hrs , Volume= Routed to Pond D23 : DMH \#23 |  |  |  |  |
|  |  |  |  |  |
| Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$ <br> Peak Elev= 207.66' @ 12.09 hrs <br> Flood Elev=210.21' |  |  |  |  |
|  |  |  |  |  |
|  |  |  |  |  |
| Device Routing | Invert Outlet Devices |  |  |  |
| Inlet / Outlet Invert= 207.04' / 206.96' S=0.0053 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |  |  |  |  |
| Primary OutFlow Max=0.79 cfs @ 12.09 hrs HW=207.65' TW=207.48' (Dynamic Tailwater) $\left\llcorner_{1}=\right.$ Culvert (Outlet Controls 0.79 cfs @ 2.27 fps ) |  |  |  |  |

## Summary for Pond CB36: CB \#36



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=207.78' @ 12.09 hrs
Flood Elev= 210.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $207.04 '$ | $\mathbf{1 2 . 0}$ " Round Culvert L= 16.1' $\quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 207.04' $/ 206.96 ' \quad \mathrm{~S}=0.0050$ '/' $\quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=1.29 cfs @ 12.09 hrs HW=207.77' TW=207.48' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 1.29 cfs @ 2.93 fps )

## Summary for Pond CB38: CB \#38

| Inflow Area = | 7,637 sf,100 | 100.00\% Impervious, | Inflow Depth > 8.81" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 1.52 cfs @ 12 | 12.09 hrs , Volume= | 5,610 cf |
| Outflow = | 1.52 cfs @ 12 | 12.09 hrs , Volume= | $5,610 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary = Routed to Pond | 1.52 cfs @ 12 D25 : DMH \#2 | 12.09 hrs , Volume= 25 | 5,610 cf |
| Routing by Dyn- <br> Peak Elev= 212.8 <br> Flood Elev= 212. | or-Ind method, $4^{\prime}$ @ 12.09 hrs ' | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 209.69' | 12.0" Round Culv Inlet / Outlet Inve $\mathrm{n}=0.012$ Corruga | $\begin{aligned} & \text { Ivert } \mathrm{L}=16.7^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{rt}=209.69^{\prime} / 209.61 ' \quad \mathrm{~S}=0.0048 \text { '/' } \quad \mathrm{Cc}=0.900 \\ & \text { ated PP, smooth interior, Flow Area= } 0.79 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=1.48 cfs @ 12.09 hrs HW=212.67' TW=212.52' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 1.48 cfs @ 1.89 fps )

## Summary for Pond CB39: CB \#39

| Inflow Area = | 7,612 | 00.00\% Impervious, | De | for 100YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 1.52 cfs @ | 12.09 hrs , Volume= | 5,591 cf |  |
| Outflow | 1.52 cfs @ | 12.09 hrs , Volume= | 5,591 cf, | Atten $=0 \%, L a g=0.0 \mathrm{~min}$ |
| Primary | 1.52 cfs @ | 12.09 hrs , Volume= | 5,591 cf |  |

Routed to Pond D25 : DMH \#25
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=212.84' @ 12.09 hrs
Flood Elev=212.86'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $209.69^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=16.4^{\prime}$ <br>  <br>  <br>  |
|  |  | $\mathrm{Ke}=0.500$ |  |
| $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |  |

Primary OutFlow Max=1.48 cfs @ 12.09 hrs HW=212.67' TW=212.52' (Dynamic Tailwater)
$\left\llcorner_{1=C u l v e r t ~(I n l e t ~ C o n t r o l s ~}^{1.48} \mathrm{cfs} @ 1.88 \mathrm{fps}\right.$ )

## Summary for Pond CB40: CB \#40



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 214.52' @ 12.09 hrs
Flood Elev=217.04'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $213.68^{\prime}$ | $\mathbf{1 2 . 0 "}$ Round Culvert $\mathrm{L}=17.8^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= $213.68^{\prime} / 213.55^{\prime} \quad \mathrm{S}=0.0073 \mathrm{I} / /^{\prime} \quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=0.74 cfs @ 12.09 hrs HW=214.50' TW=214.43' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.74 cfs @ 1.47 fps )

## Summary for Pond CB41: CB \#41



Routed to Pond D27 : DMH \#27
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=214.63' @ 12.09 hrs
Flood Elev=217.06'


## Summary for Pond CB43: CB \#43

| Inflow Area = | 3,109 | .36\% Imperviou | Inflow Depth > 7.72" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.59 cfs @ | 12.09 hrs , Volume= | 2,001 cf |
| Outflow | 0.59 cfs @ | 12.09 hrs , Volume= | $2,001 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.59 cfs @ | 12.09 hrs , Volume= | 2,001 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=220.71' @ 12.09 hrs
Flood Elev= 223.17'
Device Routing Invert Outlet Devices
\#1 Primary 220.00' 12.0" Round Culvert L= 14.9' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert= 220.00' / 219.93' S=0.0047 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=0.57 cfs @ 12.09 hrs HW=220.70' TW=220.64' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 0.57 cfs @ 1.38 fps )

## Summary for Pond CB44: CB \#44

| Inflow Area = | 1,978 sf, | 84.43\% Impervious, | Inflow Depth > 8.09" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.38 cfs @ | 12.09 hrs , Volume= | 1,333 cf |
| Outflow | 0.38 cfs @ | 12.09 hrs , Volume= | $1,333 \mathrm{cf}, \mathrm{Atten}=0 \%$, Lag= 0.0 min |
| Primary | 0.38 cfs @ | 12.09 hrs , Volume= | 1,333 cf | Routed to Pond D29: DMH \#29

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=220.68' @ 12.09 hrs
Flood Elev= 223.17'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- | :--- |
| $\# 1$ | Primary | $220.00^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=14.9^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= $220.00^{\prime} / 219.93^{\prime} \quad \mathrm{S}=0.0047 \mathrm{I} / /^{\prime} \quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=0.37 cfs @ 12.09 hrs HW=220.67' TW=220.64' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.37 cfs @ 0.95 fps )

## Summary for Pond CB45: CB \#45



Routed to Pond D30 : DMH \#30
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=221.73' @ 12.09 hrs
Flood Elev=224.46'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $221.29^{\prime}$ | $\mathbf{1 2 . 0}{ }^{\prime \prime}$ Round Culvert $\mathrm{L}=18.2^{\prime} \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 221.29' $/ 221.20^{\prime} \mathrm{S}=0.0049$ '/' $\mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area=0.79 sf |  |

Primary OutFlow Max=0.41 cfs @ 12.09 hrs HW=221.72' TW=221.59' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 0.41 cfs @ 1.88 fps )

## Summary for Pond CB46: CB \#46

| Inflow Area = | 4,397 | .97\% Impervious, | w Depth > 6.62" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.75 cfs @ | 12.09 hrs , Volume= | 2,426 cf |
| Outflow | 0.75 cfs @ | 12.09 hrs , Volume= | $2,426 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 0.75 cfs @ | 12.09 hrs , Volume= | 2,426 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 222.06' @ 12.09 hrs
Flood Elev= 224.69'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 221.53' | 12.0" Round Culvert L= 15.3' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert= 221.53' / 221.45' S=0.0052 '// Cc=0.900 <br> $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=0.73 cfs @ 12.09 hrs HW=222.06' TW=221.59' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 0.73 cfs @ 2.55 fps )

## Summary for Pond CB47: CB\#47



Routed to Pond D31 : DMH\#31
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=226.92' @ 12.16 hrs
Flood Elev=230.21'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 225.05' | 12.0" Round Culvert L= 20.9' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=225.05' / 224.95' S=0.0048 '// Cc=0.900 $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 0.79 sf |

Primary OutFlow Max=0.60 cfs @ 12.09 hrs HW=226.42' TW=226.40' (Dynamic Tailwater)
—1=Culvert (Inlet Controls $0.60 \mathrm{cfs} @ 0.76 \mathrm{fps}$ )

## Summary for Pond CB48: CB\#48

| Inflow Area = | 60,128 | 25.16\% Impervious, | Inflow Depth > 5.38" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 7.16 cfs @ | 12.16 hrs , Volume= | 26,943 cf |
| Outflow | 7.16 cfs @ | 12.16 hrs , Volume= | 26,943 cf, Atten=0\%, Lag= 0.0 min |
| Primary | 7.16 cfs @ | 12.16 hrs , Volume= | 26,943 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=228.37' @ 12.16 hrs
Flood Elev= 230.25'
Device Routing Invert Outlet Devices
\#1 Primary 224.82' 15.0" Round Culvert L= 16.9' Ke=0.500
Inlet / Outlet Invert= 224.82' / 224.74' S=0.0047 '/' Cc= 0.900
$\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf
Primary OutFlow Max=7.03 cfs @ 12.16 hrs HW=228.27' TW=226.85' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 7.03 cfs @ 5.73 fps )

## Summary for Pond CB49: CB \#49

| Inflow Area = | 5,238 sf | .59\% Impervious, | Inflow Depth > 8.33" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.03 cfs @ | 12.09 hrs , Volume= | 3,637 cf |
| Outflow | 1.03 cfs @ | 12.09 hrs , Volume= | $3,637 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.03 cfs @ | 12.09 hrs , Volume= | 3,637 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=204.26' @ 12.09 hrs
Flood Elev= 205.93'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.76' | 12.0" Round Culvert L= 15.5' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert= 202.76' / 202.68' S=0.0052 '/' Cc=0.900 <br> $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=1.00 cfs @ 12.09 hrs HW=204.21' TW=204.14' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 1.00 cfs @ 1.28 fps )

## Summary for Pond CB50: CB \#50

| Inflow Area = | 15,040 | 7.20\% Impervious, | Inflow Depth > 8.21" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 2.94 cfs @ | 12.09 hrs , Volume= | 10,290 cf |
| Outflow | 2.94 cfs @ | 12.09 hrs , Volume= | 10,290 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 2.94 cfs @ | 12.09 hrs , Volume= | 10,290 cf | Routed to Pond DMH32 : DMH \#32

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=204.79' @ 12.09 hrs
Flood Elev= 205.93'
Device Routing Invert Outlet Devices
\#1 Primary $\quad 202.78^{\prime} \quad 12.0 "$ Round Culvert L= 15.3' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert=202.78' / 202.70' S=0.0052 '/l' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$
Primary OutFlow Max=2.86 cfs @ 12.09 hrs HW=204.71' TW=204.14' (Dynamic Tailwater)
_1=Culvert (Inlet Controls 2.86 cfs @ 3.65 fps )

## Summary for Pond CB51: CB \#51

| Inflow Area $=$ | $6,823 \mathrm{sf}, 100.00 \%$ Impervious, | Inflow Depth > 8.81" | for 100 YR event |  |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $1.36 \mathrm{cfs} @$ | 12.09 hrs , Volume | $5,012 \mathrm{cf}$ |
| Outflow | $=$ | $1.36 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $5,012 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary | $=$ | $1.36 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $5,012 \mathrm{cf}$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 204.55' @ 12.34 hrs
Flood Elev= 212.77'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.35' | 12.0" Round Culvert L=31.4' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert=202.35' / 202.19' S=0.0051 '// Cc= 0.900 $\mathrm{n}=0.013$. Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=1.36 cfs @ 12.09 hrs HW=203.74' TW=203.61' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 1.36 cfs @ 1.73 fps)

## Summary for Pond CB52: CB \#52



Routed to Pond OCS7 : OCS \#7
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=204.56' @ 12.33 hrs
Flood Elev=205.84'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.68' | 12.0" Round Culvert L=25.5' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert=202.68' / 202.55' S=0.0051 '// Cc=0.900 <br> $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=1.87 cfs @ 12.09 hrs HW=203.85' TW=203.61' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls 1.87 cfs @ 2.38 fps )

## Summary for Pond CB53: CB \#53



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=203.58' @ 12.09 hrs
Flood Elev= 205.95'
Device Routing Invert Outlet Devices
\#1 Primary 202.78' 12.0" Round Culvert L=32.0' Ke=0.500
Inlet / Outlet Invert= 202.78' / 202.62' S=0.0050 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.50 cfs @ 12.09 hrs HW=203.57' TW=202.18' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 1.50 cfs @ 3.10 fps )

## Summary for Pond CB54: CB \#54

| Inflow Area = | 4,821 sf, | 6.85\% Impervious, | Inflow Depth > 7.97" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 0.93 cfs @ 12. | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | 3,201 cf |
| Outflow | 0.93 cfs @ 12. | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | $3,201 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pond | $\begin{aligned} & 0.93 \mathrm{cfs} @ 12 \\ & 3 \mathrm{P}: \text { OCS \#8 } \end{aligned}$ | .09 hrs , Volume= | 3,201 cf |
| Routing by Dyn-St <br> Peak Elev= 203.32 <br> Flood Elev= 205.8 | r-Ind method, @ 12.40 hrs | 「ime Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 202.66' | 12.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { Ivert } \mathrm{L}=36.7^{\prime} \quad \mathrm{Ke}=0.500 \\ & t=202.66^{\prime} / 202.48^{\prime} \quad \mathrm{S}=0.00499^{\prime} / \mathrm{Cc}=0.900 \end{aligned}$ $\text { ted PE, smooth interior, Flow Area }=0.79 \text { sf }$ |

Primary OutFlow Max=0.91 cfs @ 12.09 hrs HW=203.24' TW=202.18' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 0.91 cfs @ 2.74 fps )

## Summary for Pond CB7: CB\#5

| Inflow Area $=$ | $4,650 \mathrm{sf}, 100.00 \%$ Impervious, | Inflow Depth > 8.81" | for 100 YR event |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.93 \mathrm{cfs} @$ | 12.09 hrs , Volume |
| Outflow | $=$ | $0.93 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ |
| Primary | $=$ | $0.93 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | Routed to Pond D4 : DMH\#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=215.06' @ 12.14 hrs
Flood Elev=215.79'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $212.60^{\prime}$ | $\mathbf{1 2 . 0}{ }^{\prime \prime}$ Round Culvert $\mathrm{L}=15.1^{\prime} \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 212.60 ' $/ 212.45$ ' $\mathrm{S}=0.0099$ '/' $\mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 0.79 sf |  |

Primary OutFlow Max=1.92 cfs @ 12.09 hrs HW=214.04' TW=213.78' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls 1.92 cfs @ 2.44 fps )

## Summary for Pond CB8: CB\#8

| Inflow Area = | 5,450 | 88.75\% Impervious, | w Depth > 8.33" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.07 cfs @ | 12.09 hrs , Volume= | 3,784 cf |
| Outflow | 1.07 cfs @ | 12.09 hrs , Volume= | $3,784 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.07 cfs @ | 12.09 hrs , Volume= | 3,784 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs $/ 3$

Peak Elev=215.06' @ 12.14 hrs
Flood Elev=215.79'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 213.79' | 12.0" Round Culvert L= 15.1' $\mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert=213.79' / 213.64' S=0.0099 '//' Cc= 0.900 <br> $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=1.30 cfs @ 12.09 hrs HW=214.47' TW=213.79' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 1.30 cfs @ 3.24 fps )

## Summary for Pond CB9: CB \#9



Routed to Pond D5 : DMH \#5
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=211.34' @ 12.09 hrs
Flood Elev=213.27'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $210.10^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=19.9^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= $210.10^{\prime} / 209.71^{\prime} \quad \mathrm{S}=0.0196$ | $I^{\prime} / \quad \mathrm{Cc}=0.900$ |
| $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |  |  |  |

Primary OutFlow Max=3.16 cfs @ 12.09 hrs HW=211.30' TW=210.59' (Dynamic Tailwater)
\&1=Culvert (Inlet Controls 3.16 cfs @ 4.03 fps )

## Summary for Pond D10: DMH \#10

| Inflow Area = | 8,516 sf, | 64.88\% Impervious, | Inflow Depth > 6.50" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.43 cfs @ | 12.09 hrs , Volume= | 4,611 cf |
| Outflow | 1.43 cfs @ | 12.09 hrs , Volume= | $4,611 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary | 1.43 cfs @ | 12.09 hrs , Volume= | 4,611 cf |

Routed to Pond P207 : INFILTRATION POND \#2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=204.11' @ 12.09 hrs
Flood Elev= 206.49'
Device Routing Invert Outlet Devices
\#1 Primary 203.33' 12.0" Round Culvert L= 15.6' Ke=0.500
Inlet / Outlet Invert= 203.33' / 203.25' S=0.0051 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf
Primary OutFlow Max=1.40 cfs @ 12.09 hrs HW=204.09' TW=198.46' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 1.40 cfs @ 3.00 fps )

## Summary for Pond D11: DMH \#11

| Inflow Area = | 36,689 sf, 73.28\% Impervious, Inflow Depth > 7.38" for 100YR event |  |  |
| :---: | :---: | :---: | :---: |
| Inflow = | 6.20 cfs @ 12 | . 09 hrs , Volume= | 22,574 cf |
| Outflow = | 6.20 cfs @ 12. | . 09 hrs , Volume $=$ | $22,574 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pon | $\begin{aligned} & 6.20 \mathrm{cfs} @ 12 \\ & \text { OCS3: OCS\# } \end{aligned}$ | .09 hrs , Volume= | 22,574 cf |
| Routing by Dyn-S <br> Peak Elev= 207.2 <br> Flood Elev= 208 | or-Ind method, @ 12.10 hrs | ime Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | Outlet Devices |  |
| \#1 Primary | 204.25' | 18.0" Round Cu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { Ivert } \mathrm{L}=44.6^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{rt=} 204.25^{\prime} / 204.03 \text { ' } \mathrm{S}=0.0049 \text { '/' } \quad \mathrm{Cc}=0.900 \\ & \text { ated PE, smooth interior, Flow Area= } 1.77 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=6.09 cfs @ 12.09 hrs HW=207.14' TW=206.62' (Dynamic Tailwater)
L-1=Culvert (Inlet Controls 6.09 cfs @ 3.44 fps )

## Summary for Pond D12: DMH \#12



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.44' @ 12.09 hrs
Flood Elev=207.78'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $203.21^{\prime}$ | $\mathbf{1 2 . 0 "}$ Round Culvert $\mathrm{L}=41.9^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $203.21^{\prime} / 203.00^{\prime} \quad \mathrm{S}=0.0050$ |
|  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |  |

Primary OutFlow Max=4.20 cfs @ 12.09 hrs HW=205.35' TW=204.10' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 4.20 cfs @ 5.35 fps )

## Summary for Pond D13: DMH \#13

| flow Area = | 81,632 | .61\% Impervious, | w Depth > 7.75" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 14.18 cfs @ | 12.09 hrs , Volume= | 52,717 cf |
| Outflow | 14.18 cfs @ | 12.09 hrs , Volume= | $52,717 \mathrm{cf}$, Atten $=0 \%$ Lag $=0.0 \mathrm{~min}$ |
| Primary | 14.18 cfs @ | 12.09 hrs , Volume= | 52,717 cf | Routed to Pond P207 : INFILTRATION POND \#2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 204.14' @ 12.09 hrs
Flood Elev=208.12'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 201.95' | 24.0" Round Culvert L=60.1' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=201.95' / 201.65' S=0.0050 '// Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area=3.14 sf |

Primary OutFlow Max=14.01 cfs @ 12.09 hrs HW=204.12' TW=198.49' (Dynamic Tailwater) —1=Culvert (Barrel Controls 14.01 cfs @ 5.11 fps)

## Summary for Pond D14: DMH \#14

| Inflow Area = |  | 38,112 sf, 77.40\% Impervious, Inflow Depth > 8.23" for 100YR event |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | $=$ | 7.44 cfs @ 12 | . 09 hrs , Volume= | 26,131 cf |
| Outflow | = | 7.44 cfs @ 12. | . $09 \mathrm{hrs}, \mathrm{Volume=}$ | $26,131 \mathrm{cf}$, Atten= 0\%, Lag= 0.0 min |
| Primary Route | $=$ do Pon | 7.44 cfs @ 12 d13 : DMH \#1 | .09 hrs , Volume= | 26,131 cf |
| Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$ Peak Elev=205.77' @ 12.09 hrs <br> Flood Elev= 208.78' |  |  |  |  |
| Device | Routing | Invert | Outlet Devices |  |
| \#1 | Primary | 204.13' | 18.0" Round Cul Inlet / Outlet Inve $n=0.012$ Corrug | $\begin{aligned} & \text { Ivert } \mathrm{L}=256.3^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{rt}=204.13^{\prime} / 202.85 \mathrm{~S}=0.0050 \mathrm{I} / \mathrm{Cc}=0.900 \\ & \text { ated PP, smooth interior, Flow Area= } 1.77 \mathrm{sf} \end{aligned}$ |
| Primary OutFlow Max=7.26 cfs @ 12.09 hrs HW=205.73' TW=204.10' (Dynamic Tailwater) <br> ——1=Culvert (Outlet Controls 7.26 cfs @ 4.79 fps ) |  |  |  |  |

## Summary for Pond D16: DMH \#16

| Inflow Area = | 16,417 | 76.69\% Imperviou | Inflow Depth > 8.27" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 3.22 cfs @ | 12.09 hrs , Volume= | 11,318 cf |
| Outflow | 3.22 cfs @ | 12.09 hrs , Volume= | $11,318 \mathrm{cf}, \mathrm{Atten}=0 \%, \mathrm{Lag}=0.0 \mathrm{~min}$ |
| Primary | 3.22 cfs @ | 12.09 hrs , Volume= | 11,318 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.22' @ 12.09 hrs
Flood Elev= 208.59'
Device Routing Invert Outlet Devices
\#1 Primary 204.90' 15.0" Round Culvert L= 103.5' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert=204.90' / 204.38' S=0.0050 '/' Cc= 0.900
$\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf
Primary OutFlow Max=3.14 cfs @ 12.09 hrs HW=206.19' TW=205.73' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 3.14 cfs @ 3.09 fps )

## Summary for Pond D17: DMH \#17

| Inflow Area = | 14,946 sf, 75.26\% Impervious, Inflow Depth > 8.22" for 100YR event |  |  |
| :---: | :---: | :---: | :---: |
| Inflow = | 2.92 cfs @ 12. | 12.09 hrs , Volume= | 10,232 cf |
| Outflow | 2.92 cfs @ 12 | 12.09 hrs , Volume= | 10,232 cf, Atten= 0\%, Lag= 0.0 min |
| Primary = Routed to Pon | $2.92 \text { cfs @ } 12$ | 12.09 hrs , Volume= \#18 | 10,232 cf |
| Routing by Dyn-S <br> Peak Elev= 201.6 <br> Flood Elev= 204 | or-Ind method, @ 12.09 hrs | , Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | $t$ Outlet Devices |  |
| \#1 Primary | 200.55' | 12.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { Ivert } \mathrm{L}=91.6^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{rt}=200.55^{\prime} / 197.69^{\prime} \mathrm{S}=0.0312 \text { '/' } \quad \mathrm{Cc}=0.900 \\ & \text { ated PE, smooth interior, Flow Area= } 0.79 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=2.84 cfs @ 12.09 hrs HW=201.61' TW=198.73' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 2.84 cfs @ 3.62 fps)

## Summary for Pond D18: DMH \#18



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 198.76' @ 12.09 hrs
Flood Elev= 201.13'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $197.44^{\prime}$ | $\mathbf{1 5 . 0 ^ { \prime \prime }}$ Round Culvert $\mathrm{L}=46.3^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $197.44^{\prime} / 196.98^{\prime} \quad \mathrm{S}=0.0099$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=1.23 \mathrm{sf}$ |  |

Primary OutFlow Max=4.74 cfs @ 12.09 hrs HW=198.73' TW=196.91' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 4.74 cfs @ 4.65 fps )

## Summary for Pond D19: DMH \#19



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 206.75' @ 12.09 hrs
Flood Elev=208.57'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 205.19' | 12.0" Round Culvert L=82.5' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=205.19' / 204.43' S=0.0092 '// Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=3.31 cfs @ 12.09 hrs HW=206.66' TW=205.53' (Dynamic Tailwater)
——1=Culvert (Outlet Controls 3.31 cfs @ 4.22 fps )

## Summary for Pond D2: DMH\#2

 Routed to Pond P205 : INFILTRATION POND \#3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=209.92' @ 12.14 hrs
Flood Elev=212.00'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $206.90^{\prime}$ | $15.0^{\prime \prime}$ Round Culvert $\mathrm{L}=38.2^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= 206.90' $/ 206.52^{\prime} \quad \mathrm{S}=0.0099$ | '/' $\quad \mathrm{Cc}=0.900$ |
|  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 1.23 sf |  |  |

Primary OutFlow Max=9.07 cfs @ 12.14 hrs HW=209.88' TW=207.03' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 9.07 cfs @ 7.39 fps)

## Summary for Pond D20: DMH \#20

| Inflow Area = | 17,428 | 83.29\% Impervious, | Inflow Depth > 8.33" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 3.43 cfs @ | 12.09 hrs , Volume= | 12,100 cf |
| Outflow | 3.43 cfs @ | 12.09 hrs , Volume= | $12,100 \mathrm{cf}, \mathrm{Atten}=0 \%, \mathrm{Lag}=0.0 \mathrm{~min}$ |
| Primary | 3.43 cfs @ | 12.09 hrs , Volume= | 12,100 cf | Routed to Pond D21 : DMH \#21

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.57' @ 12.09 hrs
Flood Elev=207.68'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $204.19^{\prime}$ | $\mathbf{1 5 . 0 "}$ Round Culvert $\mathrm{L}=63.5^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= $204.19^{\prime} / 203.87^{\prime} \quad \mathrm{S}=0.0050$ |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.23 \mathrm{sf}$ |  |

Primary OutFlow Max=3.34 cfs @ 12.09 hrs HW=205.53' TW=205.12' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 3.34 cfs @ 3.16 fps )

## Summary for Pond D21: DMH \#21

| Inflow Area = | 71,317 sf, | 7\% Impervious, | Inflow Depth > | 8.21" for 100YR event |
| :---: | :---: | :---: | :---: | :---: |
| Inflow | 13.91 cfs @ | 12.09 hrs , Volume= | $48,812 \mathrm{cf}$ |  |
| Outflow | 13.91 cfs @ | 12.09 hrs , Volume= | 48,812 cf | , Atten= 0\%, Lag= 0.0 min |
| Primary | 13.91 cfs @ | 12.09 hrs , Volume= | 48,812 cf |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=205.17' @ 12.09 hrs
Flood Elev= 207.55'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | $203.02^{\prime}$ | $24.0^{\prime \prime}$ Round Culvert $\mathrm{L}=72.4^{\prime} \quad \mathrm{Ke}=0.500$ |
|  |  | Inlet / Outlet Invert= 203.02' $/ 202.66^{\prime} \quad \mathrm{S}=0.0050 \quad$ '/' $\quad \mathrm{Cc}=0.900$ |  |
|  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 3.14 sf |  |

Primary OutFlow Max=13.55 cfs @ 12.09 hrs HW=205.12' TW=202.97' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 13.55 cfs @ 5.10 fps )

## Summary for Pond D22: DMH \#22



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.31' @ 12.09 hrs
Flood Elev= 208.46'


Primary OutFlow Max=4.78 cfs @ 12.09 hrs HW=206.27' TW=205.12' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 4.78 cfs @ 4.34 fps )

## Summary for Pond D23: DMH \#23

| Inflow Area = | 10,771 | \% Impervious, | 8.81" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 2.15 cfs @ | 12.09 hrs , Volume= | 7,912 cf |
| Outflow | 2.15 cfs @ | 12.09 hrs , Volume= | $7,912 \mathrm{cf}$, Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 2.15 cfs @ | 12.09 hrs , Volume= | 7,912 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
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Peak Elev= 207.50' @ 12.09 hrs
Flood Elev=210.30'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 206.70' | 15.0" Round Culvert L= 173.3' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=206.70' / 204.97' S=0.0100 '// Cc=0.900 $n=0.013$ Corrugated PE , smooth interior, Flow Area= 1.23 sf |

Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=207.48' TW=206.27' (Dynamic Tailwater)
——1=Culvert (Outlet Controls 2.09 cfs @ 3.69 fps )

## Summary for Pond D25: DMH \#25



Routed to Pond P210 : POCKET WETLAND \#1
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=212.68' @ 12.09 hrs
Flood Elev=213.11'


Primary OutFlow Max=6.95 cfs @ 12.09 hrs HW=212.52' TW=204.08' (Dynamic Tailwater)
—1=Culvert (Barrel Controls 6.95 cfs @ 5.67 fps )

## Summary for Pond D27: DMH \#27

| Inflow Area = | 21,746 sf, | 79.51\% Impervious, | Inflow Depth > 7.90" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 4.10 cfs @ | 12.09 hrs , Volume= | 14,316 cf |
| Outflow | 4.10 cfs @ | 12.09 hrs , Volume= | $14,316 \mathrm{cf}$, Atten $=0 \%$ Lag $=0.0 \mathrm{~min}$ |
| Primary | 4.10 cfs @ | 12.09 hrs , Volume= | 14,316 cf | Routed to Pond D35 : DMH \#35

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=214.46' @ 12.09 hrs
Flood Elev= 217.00'
Device Routing Invert Outlet Devices
\#1 Primary 213.34' 15.0" Round Culvert L= 63.9' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert= 213.34' / 212.38 ' S=0.0150 '/' Cc= 0.900
$\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf
Primary OutFlow Max=4.04 cfs @ 12.09 hrs HW=214.44' TW=213.55' (Dynamic Tailwater)
—1=Culvert (Outlet Controls 4.04 cfs @ 4.72 fps )

## Summary for Pond D28: DMH \#28



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=218.28' @ 12.09 hrs
Flood Elev= 220.17'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $217.46^{\prime}$ | $12.0^{\prime \prime}$ Round Culvert $\mathrm{L}=158.3^{\prime} \quad \mathrm{Ke}=0.500$ <br>  |
|  |  | Inlet / Outlet Invert= $217.46^{\prime} / 214.29^{\prime} \quad \mathrm{S}=0.0200$ |  |
|  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |  |  |

Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=218.27' TW=214.44' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 2.09 cfs @ 3.07 fps )

## Summary for Pond D29: DMH \#29

| Inflow Area = | 11,949 sf | perviou | Depth > 7.15" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 2.14 cfs @ | 12.09 hrs , Volume= | 7,120 cf |
| Outflow | 2.14 cfs @ | 12.09 hrs , Volume= | $7,120 \mathrm{cf}$, Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | 2.14 cfs @ | 12.09 hrs , Volume= | 7,120 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=220.65' @ 12.09 hrs
Flood Elev=223.21'
Device Routing Invert Outlet Devices
\#1 Primary $\quad 219.83^{\prime} \quad 12.0 "$ Round Culvert $\mathrm{L}=150.9^{\prime} \mathrm{Ke}=0.500$
Inlet / Outlet Invert=219.83' / 217.55' S=0.0151 '// Cc=0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$
Primary OutFlow Max=2.09 cfs @ 12.09 hrs HW=220.64' TW=218.27' (Dynamic Tailwater)
_1=Culvert (Inlet Controls 2.09 cfs @ 3.07 fps )

## Summary for Pond D30: DMH \#30

| Inflow Area $=$ | $6,862 \mathrm{sf}$, | $50.73 \%$ | Impervious, | Inflow Depth $>$ |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $1.17 \mathrm{cfs} @$ | 12.09 hrs , Volume= | for 100 YR event |
| Outflow | $=$ | $1.17 \mathrm{cfs} @$ | 12.09 hrs , Volume= | $3,786 \mathrm{cf}$ |
| Primary | $=$ | $1.17 \mathrm{cfs} @$ | 12.09 hrs , Volume $=$ | $3,786 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 221.60' @ 12.09 hrs
Flood Elev=224.95'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 220.92' | 12.0" Round Culvert L= 184.2' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=220.92' / 220.00' S=0.0050 '// Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |

Primary OutFlow Max=1.14 cfs @ 12.09 hrs HW=221.59' TW=220.64' (Dynamic Tailwater) —1=Culvert (Outlet Controls 1.14 cfs @ 2.90 fps )

## Summary for Pond D31: DMH\#31



Primary OutFlow Max=7.50 cfs @ 12.16 hrs HW=226.87' TW=214.77' (Dynamic Tailwater)
$\__{1=C u l v e r t ~(I n l e t ~ C o n t r o l s ~}^{7.50} \mathrm{cfs} @ 6.12 \mathrm{fps}$ )

## Summary for Pond D34: DMH \#34

| Inflow Area = | 23,255 | 00.00\% Imperviou | Inflow Depth > 8.81" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 4.64 cfs @ | 12.09 hrs , Volume= | 17,082 cf |
| Outflow | 4.64 cfs @ | 12.09 hrs , Volume= | 17,082 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 4.64 cfs @ | 12.09 hrs , Volume= | 17,082 cf | Routed to Pond OCS1 : OCS\#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=200.07' @ 12.09 hrs
Flood Elev= 202.00'


Primary OutFlow Max=4.52 cfs @ 12.09 hrs HW=200.00' TW=196.90' (Dynamic Tailwater)
—1=Culvert (Inlet Controls 4.52 cfs @ 5.75 fps )

## Summary for Pond D35: DMH \#35



## Summary for Pond D4: DMH\#4

| Inflow Area | 73,240 sf, | vious, | Inflow Depth > 5.96" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 9.15 cfs @ | 12.14 hrs , Volume= | 36,354 cf |
| Outflow | 9.15 cfs @ | 12.14 hrs , Volume= | $36,354 \mathrm{cf}, \mathrm{Atten}=0 \%, \mathrm{Lag}=0.0 \mathrm{~min}$ |
| Primary | 9.15 cfs @ | 12.14 hrs , Volume= | 36,354 cf | Routed to Pond D2 : DMH\#2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=215.03' @ 12.14 hrs
Flood Elev=217.64'


Primary OutFlow Max=9.07 cfs @ 12.14 hrs HW=214.90' TW=209.88' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 9.07 cfs @ 7.39 fps )

## Summary for Pond D5: DMH \#5

| Inflow Area = | 30,441 sf, 96.76\% Impervious, | Weph > 8.75 for 100 |
| :---: | :---: | :---: |
| Inflow | 6.07 cfs @ 12.09 hrs, Volume= | 22,196 cf |
| Outflow | 6.07 cfs @ 12.09 hrs, Volume= | 22,196 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 6.07 cfs @ 12.09 hrs, Volume= | 22,196 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 210.62' @ 12.09 hrs
Flood Elev=212.97'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 209.09' | 18.0" Round Culvert L= 183.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=209.09' / 208.17' S=0.0050 '// Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE , smooth interior, Flow Area $=1.77 \mathrm{sf}$ |

Primary OutFlow Max=5.90 cfs @ 12.09 hrs HW=210.59' TW=209.48' (Dynamic Tailwater)
——1=Culvert (Outlet Controls 5.90 cfs @ 4.16 fps )

## Summary for Pond D6: DMH \#6

 Routed to Pond D7 : DMH \#7

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=209.51' @ 12.09 hrs
Flood Elev= 214.82'

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 208.07' | 18.0" Round Culvert L= 299.7' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=208.07' / 206.57' S=0.0050 '// Cc= 0.900 $n=0.013$ Corrugated PE, smooth interior, Flow Area= 1.77 sf |
| $\underbrace{P r i m a r}_{1=C}$ | OutFIow lvert (O | 90 cfs trols 5.90 | 12.09 hrs HW=209.48' TW=207.71' (Dynamic Tailwater) cfs @ 4.42 fps ) |

## Summary for Pond D7: DMH \#7

| Inflow Area = | 30,441 sf, | 96.76\% Impervious, | Inflow Depth > 8.75" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 6.07 cfs @ | 12.09 hrs , Volume= | 22,196 cf |
| Outflow | 6.07 cfs @ | 12.09 hrs , Volume= | 22,196 cf, Atten=0\%, Lag= 0.0 min |
| Primary | 6.07 cfs @ | 12.09 hrs , Volume= | 22,196 cf |

Routed to Pond P212 : INFILTRATION POND \#1
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.73' @ 12.09 hrs
Flood Elev= 213.17'


Primary OutFlow Max=5.90 cfs @ 12.09 hrs HW=207.71' TW=202.97' (Dynamic Tailwater)
—1=Culvert (Inlet Controls $5.90 \mathrm{cfs} @ 3.79 \mathrm{fps}$ )

## Summary for Pond D8: DMH \#8

| Inflow Area = | $18,765 \mathrm{sf}, 9$ | 91.12\% Impervious, | Inflow Depth > 8.27" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow = | 3.66 cfs @ 12 | 12.09 hrs , Volume= | 12,929 cf |
| Outflow | 3.66 cfs @ 12. | 12.09 hrs , Volume= | $12,929 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |
| Primary = Routed to Pond | $\begin{aligned} & 3.66 \mathrm{cfs} @ 12 \end{aligned}$ | 12.09 hrs , Volume= | 12,929 cf |
| Routing by Dyn-S <br> Peak Elev= 203.04 <br> Flood Elev= 204 | or-Ind method, ' @ 12.09 hrs ' | Time Span= 0.00-2 | $4.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs} / 3$ |
| Device Routing | Invert | $t$ Outlet Devices |  |
| \#1 Primary | $200.57{ }^{\prime}$ | 12.0" Round Culu Inlet / Outlet Inve $\mathrm{n}=0.013$ Corrug | $\begin{aligned} & \text { lvert } \mathrm{L}=87.7^{\prime} \quad \mathrm{Ke}=0.500 \\ & \mathrm{t}=200.577^{\prime} / 200.13^{\prime} \mathrm{S}=0.0050 \text { '/' } \mathrm{Cc}=0.900 \\ & \text { ated PE, smooth interior, Flow Area= } 0.79 \mathrm{sf} \end{aligned}$ |

Primary OutFlow Max=3.57 cfs @ 12.09 hrs HW=202.93' TW=201.57' (Dynamic Tailwater)
L-1=Culvert (Outlet Controls 3.57 cfs @ 4.54 fps )

## Summary for Pond D9: DMH \#9

| Inflow Area = | 18,7 | ervious, | 27" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 3.66 cfs @ | 12.09 hrs , Volume= | 12,929 cf |
| Outflow | 3.66 cfs @ | 12.09 hrs , Volume= | 12,929 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 3.66 cfs @ | 12.09 hrs , Volume= | 12,929 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=201.60' @ 12.09 hrs
Flood Elev= 204.80'
Device Routing Invert Outlet Devices
\#1 Primary $\quad 200.03^{\prime} \quad 12.0 "$ Round Culvert L= 11.9' $\mathrm{Ke}=0.500$
Inlet / Outlet Invert=200.03' / 199.97' S=0.0050 '/' Cc= 0.900
$\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.79 \mathrm{sf}$
Primary OutFlow Max=3.57 cfs @ 12.09 hrs HW=201.57' TW=198.45' (Dynamic Tailwater)
L-1=Culvert (Barrel Controls 3.57 cfs @ 4.54 fps )

## Summary for Pond DE61: DRIP \#61

| Inflow Area = | 4,247 sf, | 92.68\% Impervious, | Inflow Depth > 8.57" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.84 cfs @ | 12.09 hrs , Volume= | 3,034 cf |
| Outflow | 0.69 cfs @ | 12.15 hrs , Volume= | 2,884 cf, Atten= 18\%, Lag $=3.6 \mathrm{~min}$ |
| Discarded = | 0.00 cfs @ | 2.75 hrs , Volume= | 211 cf |
| Primary | 0.69 cfs @ | 12.15 hrs , Volume= | 2,673 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Prepared by Howard Stein Hudson Associates Printed 6/19/2023

Peak Elev= 213.59' @ 12.15 hrs Surf.Area= 665 sf Storage= 372 cf
Plug-Flow detention time= 56.7 min calculated for 2,884 cf ( $95 \%$ of inflow)
Center-of-Mass det. time= 27.9 min (777.6-749.6)


Discarded OutFlow Max=0.00 cfs @ 2.75 hrs HW=212.21' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.69 cfs @ 12.15 hrs HW=213.58' TW=208.08' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.69 cfs @ 3.49 fps )

## Summary for Pond DE62: DRIP \#62

| Inflow Area = | 4,247 sf, | 92.68\% Impervious, | Inflow Depth > 8.57" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.84 cfs @ | 12.09 hrs , Volume= | 3,034 cf |
| Outflow | 0.69 cfs @ | 12.15 hrs , Volume= | 2,884 cf, Atten= 18\%, Lag= 3.6 min |
| Discarded | 0.00 cfs @ | 2.75 hrs , Volume= | 211 cf |
| Primary | 0.69 cfs @ | 12.15 hrs , Volume= | 2,673 cf | Routed to Reach 8R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=213.59' @ 12.15 hrs Surf.Area= 665 sf Storage= 372 cf
Plug-Flow detention time $=56.7$ min calculated for 2,884 cf ( $95 \%$ of inflow)
Center-of-Mass det. time= 27.9 min (777.6-749.6)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $212.19^{\prime}$ | 539 cf | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 212.19 | 665 | 0.0 | 0 | 0 |
| 212.20 | 665 | 40.0 | 3 | 3 |
| 214.19 | 665 | 40.0 | 529 | 532 |
| 214.20 | 665 | 100.0 | 7 | 539 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 214.10' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.801 .00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 212.70' | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 212.70' / 212.65 ' S=0.0050 '/' Cc= 0.900 $n=0.013$ Corrugated PE, smooth interior, Flow Area $=0.20$ sf |
| \#3 | Discarded | 212.19' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.00 cfs @ 2.75 hrs HW=212.21' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.69 cfs @ 12.15 hrs HW=213.58' TW=208.08' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.69 cfs @ 3.49 fps)

## Summary for Pond DE63: DRIP \#63

| Inflow Area = | 3,013 sf, | 88.68\% Impervious, | Inflow Depth > 8.45" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.60 cfs @ | 12.09 hrs , Volume= | 2,122 cf |
| Outflow | 0.52 cfs @ | 12.14 hrs , Volume= | 2,032 cf, Atten= 12\%, Lag= 3.0 min |
| Discarded = | 0.00 cfs @ | 2.95 hrs , Volume= | 127 cf |
| Primary | 0.52 cfs @ | 12.14 hrs , Volume= | 1,906 cf | Routed to Reach 12R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=208.20' @ 12.13 hrs Surf.Area= 404 sf Storage= 196 cf
Plug-Flow detention time $=48.1$ min calculated for 2,032 cf ( $96 \%$ of inflow)
Center-of-Mass det. time $=23.0 \min (777.0-754.1)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $206.99^{\prime}$ | 327 cf | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 206.99 | 404 | 0.0 | 0 | 0 |
| 207.00 | 404 | 40.0 | 2 | 2 |
| 208.99 | 404 | 40.0 | 322 | 323 |
| 209.00 | 404 | 100.0 | 4 | 327 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 208.90' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 207.50' | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 207.50' / 207.45' S=0.0050 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.20 sf |
| \#3 | Discarded | 206.99' | $\mathbf{0 . 1 7 0} \mathbf{i n} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= $0.01{ }^{\prime}$ |

Discarded OutFlow Max=0.00 cfs @ 2.95 hrs HW=207.01' (Free Discharge)
$\complement_{3=\text { Exfiltration (Exfiltration Controls } 0.00 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=0.51 cfs @ 12.14 hrs HW=208.19' TW=202.14' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.51 cfs @ 2.62 fps)

## Summary for Pond DE64: DRIP \#64



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 204.99 | 470 | 0.0 | 0 | 0 |
| 205.00 | 470 | 40.0 | 2 | 2 |
| 206.99 | 470 | 40.0 | 374 | 376 |
| 207.00 | 470 | 100.0 | 5 | 381 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 206.90' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 205.50' | 6.0" Round Culvert $\mathrm{L}=10.0{ }^{\prime} \mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 205.50' / 205.45' S=0.0050 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.20 \mathrm{sf}$ |
| \#3 | Discarded | 204.99' | 0.170 in/hr Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.00 cfs @ 2.55 hrs HW=205.01' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.59 cfs @ 12.14 hrs HW=206.27' TW=202.13' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.59 cfs @ 2.98 fps )

## Summary for Pond DE65: DRIP \#65

| Inflow Area = | 3,016 sf, | 88.69\% Impervious, | Inflow Depth > 8.45" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.60 cfs @ | 12.09 hrs , Volume= | 2,124 cf |
| Outflow | 0.52 cfs @ | 12.14 hrs , Volume= | 2,034 cf, Atten= 12\%, Lag $=3.0 \mathrm{~min}$ |
| Discarded | 0.00 cfs @ | 2.95 hrs , Volume= | 127 cf |
| Primary | 0.52 cfs @ | 12.14 hrs , Volume= | 1,908 cf | Routed to Reach 12R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.21' @ 12.13 hrs Surf.Area= 404 sf Storage= 196 cf
Plug-Flow detention time $=47.7 \mathrm{~min}$ calculated for 2,030 cf ( $96 \%$ of inflow)
Center-of-Mass det. time= 23.0 min (777.0-754.1)


Discarded OutFlow Max=0.00 cfs @ 2.95 hrs HW=206.01' (Free Discharge)
$\complement_{3=\text { Exfiltration (Exfiltration Controls } 0.00 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=0.51 cfs @ 12.14 hrs HW=207.19' TW=202.14' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs )
$\mathbf{2 = C u l v e r t}$ (Barrel Controls 0.51 cfs @ 2.62 fps )

## Summary for Pond DE66: DRIP \#66



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=209.07' @ 12.14 hrs Surf.Area= 470 sf Storage= 240 cf
Plug-Flow detention time $=50.3 \mathrm{~min}$ calculated for $2,329 \mathrm{cf}$ ( $96 \%$ of inflow)
Center-of-Mass det. time= $24.6 \min (774.2-749.6$ )


Discarded OutFlow Max=0.00 cfs @ 2.10 hrs HW=207.80' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.57 cfs @ 12.14 hrs HW=209.05' TW=202.14' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.57 cfs @ 2.93 fps )

## Summary for Pond DE67: DRIP \#67

| Inflow Area = | 3,481 sf, | 1\% Impervious, | Inflow Depth > 8.57" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.69 cfs @ | 12.09 hrs , Volume= | 2,487 cf |
| Outflow | 0.60 cfs @ | 12.14 hrs , Volume= | 2,382 cf, Atten= 13\%, Lag $=3.0 \mathrm{~min}$ |
| Discarded | 0.00 cfs @ | 2.55 hrs , Volume= | 149 cf |
| Primary | 0.60 cfs @ | 12.14 hrs , Volume= | 2,232 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 209.28' @ 12.14 hrs Surf.Area= 470 sf Storage= 242 cf
Plug-Flow detention time= 49.2 min calculated for 2,377 cf ( $96 \%$ of inflow)
Center-of-Mass det. time= 24.3 min (773.9-749.6)


Discarded OutFlow Max=0.00 cfs @ 2.55 hrs HW=208.01' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.59 cfs @ 12.14 hrs HW=209.27' TW=208.08' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.59 cfs @ 2.99 fps)

## Summary for Pond DE68: DRIP \#68



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 206.99 | 665 | 0.0 | 0 | 0 |
| 207.00 | 665 | 40.0 | 3 | 3 |
| 208.99 | 665 | 40.0 | 529 | 532 |
| 209.00 | 665 | 100.0 | 7 | 539 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 208.90' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ <br> Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 207.50' | 6.0" Round Culvert $\mathrm{L}=20.0^{\prime} \mathrm{Ke}=0.500$ <br> Inlet / Outlet Invert=207.50' $/ 206.00^{\prime} \quad \mathrm{S}=0.0750$ '/l' Cc= 0.900 $n=0.013$ Corrugated $P E$, smooth interior, Flow Area $=0.20$ sf |
| \#3 | Discarded | 206.99' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.00 cfs @ 2.75 hrs HW=207.01' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.67 cfs @ 12.15 hrs HW=208.26' TW=205.72' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Inlet Controls 0.67 cfs @ 3.44 fps)

## Summary for Pond DE69: DRIP \#69

| Inflow Area = | 3,480 sf, | \% Impervious, | Inflow Depth > 8.57" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.69 cfs @ | 12.09 hrs , Volume= | 2,486 cf |
| Outflow | 0.60 cfs @ | 12.14 hrs , Volume= | 2,381 cf, Atten= 13\%, Lag= 3.0 min |
| Discarded | 0.00 cfs @ | 2.55 hrs , Volume= | 149 cf |
| Primary | 0.60 cfs @ | 12.14 hrs , Volume= | 2,232 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.78' @ 12.14 hrs Surf.Area= 470 sf Storage= 242 cf
Plug-Flow detention time $=49.2$ min calculated for 2,376 cf ( $96 \%$ of inflow)
Center-of-Mass det. time $=24.3 \min (773.9-749.6)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $205.49^{\prime}$ | 381 cf | Custom Stage Data (Prismatic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 205.49 | 470 | 0.0 | 0 | 0 |
| 205.50 | 470 | 40.0 | 2 | 2 |
| 207.49 | 470 | 40.0 | 374 | 376 |
| 207.50 | 470 | 100.0 | 5 | 381 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 207.40' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 206.00' | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 206.00' / 205.95' S=0.0050 '/l' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.20 \mathrm{sf}$ |
| \#3 | Discarded | 205.49' | $0.170 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= 0.01 ' |

Discarded OutFlow Max=0.00 cfs @ 2.55 hrs HW=205.51' (Free Discharge)
$\complement_{3=\text { Exfiltration (Exfiltration Controls } 0.00 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=0.59 cfs @ 12.14 hrs HW=206.77' TW=203.04' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs )
-2=Culvert (Barrel Controls 0.59 cfs @ 2.99 fps )

## Summary for Pond DE70: DRIP \#70



| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Voids <br> (\%) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: |
| 205.89 | 470 | 0.0 | 0 | 0 |
| 205.90 | 470 | 40.0 | 2 | 2 |
| 207.89 | 470 | 40.0 | 374 | 376 |
| 207.90 | 470 | 100.0 | 5 | 381 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 207.80' | 180.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) $0.20 \quad 0.40 \quad 0.60 \quad 0.80 \quad 1.00$ |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#2 | Primary | 206.40' | 6.0" Round Culvert L= 10.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=206.40' / 206.35' S=0.0050 '/' Cc= 0.900 $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area $=0.20 \mathrm{sf}$ |
| \#3 | Discarded | 205.89' | 0.170 in/hr Exfiltration over Surface area Phase-In= 0.01' |

Discarded OutFlow Max=0.00 cfs @ 2.05 hrs HW=205.90' (Free Discharge)
—3=Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.59 cfs @ 12.14 hrs HW=207.17' TW=203.04' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)
-2=Culvert (Barrel Controls 0.59 cfs @ 2.99 fps )

## Summary for Pond DE71: DRIP \#71

| Inflow Area = | 4,210 sf, | 92.61\% Impervious, | Inflow Depth > 8.57" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 0.84 cfs @ | 12.09 hrs , Volume= | 3,008 cf |
| Outflow | 0.69 cfs @ | 12.15 hrs , Volume= | $2,858 \mathrm{cf}$, Atten $=18 \%$ Lag $=3.6 \mathrm{~min}$ |
| Discarded | 0.00 cfs @ | 3.10 hrs , Volume= | 211 cf |
| Primary | 0.68 cfs @ | 12.15 hrs , Volume= | 2,647 cf | Routed to Pond P212 : INFILTRATION POND \#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.88' @ 12.15 hrs Surf.Area= 665 sf Storage= 370 cf
Plug-Flow detention time $=56.6$ min calculated for 2,852 cf ( $95 \%$ of inflow)
Center-of-Mass det. time= 28.0 min (777.7-749.6)


Discarded OutFlow Max=0.00 cfs @ 3.10 hrs HW=206.52' (Free Discharge)
$L^{-}=$Exfiltration (Exfiltration Controls 0.00 cfs )
Primary OutFlow Max=0.68 cfs @ 12.15 hrs HW=207.88' TW=203.05' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir ( Controls 0.00 cfs )
$\mathbf{2 = C u l v e r t}$ (Barrel Controls 0.68 cfs @ 3.47 fps)

## Summary for Pond DECH: DRIP \#CH

| Inflow Area = | 6,262 sf, | 92.70\% Impervious, | Inflow Depth > 8.57" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.24 cfs @ | 12.09 hrs , Volume= | 4,474 cf |
| Outflow | 1.08 cfs @ | 12.20 hrs , Volume= | $4,473 \mathrm{cf}$, Atten= 13\%, Lag= 6.6 min |
| Discarded = | 0.04 cfs @ | 8.85 hrs , Volume= | 2,032 cf |
| Primary | 1.05 cfs @ | 12.20 hrs , Volume= | 2,441 cf |
|  | B18 : CB |  |  |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=210.92' @ 12.18 hrs Surf.Area= 636 sf Storage= 746 cf
Plug-Flow detention time $=23.0 \mathrm{~min}$ calculated for $4,464 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time $=22.8 \mathrm{~min}$ (772.5-749.6)


Discarded OutFlow Max=0.04 cfs @ 8.85 hrs HW=208.02' (Free Discharge)
$L_{3}=$ Exfiltration (Exfiltration Controls 0.04 cfs )
Primary OutFlow Max=1.03 cfs @ 12.20 hrs HW=210.91' TW=206.78' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir(Weir Controls 0.62 cfs @ 0.31 fps)
-2=Culvert (Outlet Controls 0.40 cfs @ 4.64 fps )

## Summary for Pond DMH32: DMH \#32



Routed to Pond P212 : INFILTRATION POND \#1
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$

Peak Elev= 204.19' @ 12.09 hrs
Flood Elev=206.16'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :--- | :--- |
| $\# 1$ | Primary | $202.59^{\prime}$ | 12.0" Round Culvert $\mathrm{L}=19.2^{\prime} \quad \mathrm{Ke}=0.500$ <br>  <br>  <br>  <br>  <br>  <br>  <br>  <br>  <br> $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= $=0.79 \mathrm{sf}$ |

Primary OutFlow Max=3.87 cfs @ 12.09 hrs HW=204.14' TW=202.97' (Dynamic Tailwater)
——1=Culvert (Inlet Controls 3.87 cfs @ 4.92 fps )

## Summary for Pond OCS1: OCS\#1

| Inflow A |  | 48,573 sf, | 81.99\% Impervious, | Inflow Depth > | 8.37" for 100YR event |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Inflow | = | 9.51 cfs @ | 12.09 hrs , Volume= | 33,884 cf |  |
| Outflow | = | 9.51 cfs @ | 12.09 hrs , Volume= | 33,884 cf | Atten= 0\%, Lag $=0.0 \mathrm{~min}$ |
| Primary | = | 9.51 cfs @ | 12.09 hrs , Volume= | 33,884 cf |  |

Routed to Pond P206 : STORMTECH INFILTRATION SYSTEM \#2
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 196.96' @ 12.11 hrs
Flood Elev= 201.48'

| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | ---: | :--- |
| $\# 1$ | Primary | 195.00 | $\mathbf{2 4 . 0}$ Vert. Orifice/Grate |
| C= $=0.600$ | Limited to weir flow at low heads |  |  |

Primary OutFlow Max=9.29 cfs @ 12.09 hrs HW=196.91' TW=196.52' (Dynamic Tailwater)
—1=Orifice/Grate (Orifice Controls 9.29 cfs @ 3.01 fps)

## Summary for Pond OCS3: OCS\#3

 Routed to Pond p204 : STORMTECH INFILTRATION SYSTEM \#1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=206.75' @ 12.11 hrs
Flood Elev= 209.00'
Device Routing Invert Outlet Devices
\#1 Primary 203.10' 18.0" Vert. Orifice/Grate $C=0.600$ Limited to weir flow at low heads
Primary OutFlow Max=9.49 cfs @ 12.09 hrs HW=206.62' TW=205.37' (Dynamic Tailwater)
_1=Orifice/Grate (Orifice Controls 9.49 cfs @ 5.37 fps )

## Summary for Pond OCS4: OCS\#4

| Inflow Area | 17,972 s | 28.85\% Impervious, | Inflow Depth > 6.37" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 2.79 cfs @ | 12.10 hrs , Volume= | 9,535 cf |
| Outflow | 2.79 cfs @ | 12.10 hrs , Volume= | 9,535 cf, Atten= 0\%, Lag= 0.0 min |
| Primary | 2.79 cfs @ | 12.10 hrs , Volume= | 9,535 cf |

Routed to Pond P204 : STORMTECH INFILTRATION SYSTEM \#1
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev=205.75' @ 12.17 hrs
Flood Elev= 208.00'

| Device | Routing | Invert | Outlet Devices |  |
| :---: | :--- | :---: | :---: | :---: | :---: |
| $\# 1$ | Primary | $203.10^{\prime}$ | $18.0 "$ Vert. Orifice/Grate | $C=0.600 \quad$ Limited to weir flow at low heads |

Primary OutFlow Max=2.77 cfs @ 12.10 hrs HW=205.54' TW=205.44' (Dynamic Tailwater)
——1=Orifice/Grate (Orifice Controls 2.77 cfs @ 1.57 fps )

## Summary for Pond OCS6: OCS \#6



Primary OutFlow Max=3.12 cfs @ 12.09 hrs HW=203.63' TW=202.95' (Dynamic Tailwater)
L1=Orifice/Grate (Orifice Controls 3.12 cfs @ 3.97 fps )

## Summary for Pond OCS7: OCS \#7



Primary OutFlow Max=3.07 cfs @ 12.09 hrs HW=203.61' TW=202.95' (Dynamic Tailwater)
\&1=Orifice/Grate (Orifice Controls 3.07 cfs @ 3.90 fps)

## Summary for Pond P204: STORMTECH INFILTRATION SYSTEM \#1



Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.75' | 15.0" Round Culvert L=35.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 202.75' / 201.00' S=0.0500 '/l' Cc= 0.900 $\mathrm{n}=0.012$ Corrugated PP, smooth interior, Flow Area= 1.23 sf |
| \#2 | Device 1 | 204.75' | 4.0' long x 0.5' breadth Broad-Crested Rectangular Weir |
|  |  |  | Head (feet) 0.200 .400 .600 .801 .00 |
|  |  |  | Coef. (English) 2.802 .923 .083 .303 .32 |
| \#3 | Device 1 | 203.25' | 8.0" Vert. Orifice/Grate $\mathrm{C}=0.600$ Limited to weir flow at low heads |
| \#4 | Discarded | 202.50' | $0.660 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= $0.01{ }^{\text {' }}$ |

Discarded OutFlow Max=0.09 cfs @ 6.70 hrs HW=202.56' (Free Discharge)
$L_{4}=$ Exfiltration (Exfiltration Controls 0.09 cfs )
Primary OutFlow Max=8.95 cfs @ 12.18 hrs HW=205.67' TW=200.25' (Dynamic Tailwater)
$乙_{1=C u l v e r t ~(I n l e t ~ C o n t r o l s ~} 8.95$ cfs @ 7.29 fps)
-2=Broad-Crested Rectangular Weir(Passes < 11.63 cfs potential flow)
$\square_{3=O r i f i c e / G r a t e ~(P a s s e s ~<~}^{2.43}$ cfs potential flow)

## Summary for Pond P205: INFILTRATION POND \#3

| Inflow Area $=$ | $88,676 \mathrm{sf}$, | $39.42 \%$ | Impervious, |
| :--- | :--- | ---: | :--- |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=207.05' @ 12.18 hrs Surf.Area= 5,673 sf Storage= 9,818 cf
Plug-Flow detention time $=106.9$ min calculated for 38,841 cf ( $87 \%$ of inflow)
Center-of-Mass det. time $=49.0 \mathrm{~min}(852.1-803.0)$

| Volume | Invert Av | vail.Storage Stor | cription |
| :---: | :---: | :---: | :---: |
| \#1 | 205.00' | 15,630 cf Custom Stage Data (P |  |
| Elevation <br> (feet) | Surf.Area (sq-ft) | Inc.Store (cubic-feet) | Cum.Store (cubic-feet) |
| 205.00 | 3,939 | 39 | 0 |
| 206.00 | 4,756 | 4,348 | 4,348 |
| 207.00 | 5,627 | ( 5,192 | 9,539 |
| 208.00 | 6,555 | 55 6,091 | 15,630 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 206.65' | 15.0' long x 15.0' breadth Broad-Crested Rectangul |
|  |  |  | Head (feet) 0.200 .400 .600 .801 .001 .201 .401 .60 |
|  |  |  | Coef. (English) 2.682 .702 .702 .642 .632 .642 .642 .63 |
| \#2 | Discarded | 205.00' | $2.410 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= $0.01{ }^{\prime}$ |

Discarded OutFlow Max=0.32 cfs @ 12.18 hrs HW=207.05' (Free Discharge)
—2=Exfiltration (Exfiltration Controls 0.32 cfs )
Primary OutFlow Max=10.09 cfs @ 12.18 hrs HW=207.05' TW=203.11' (Dynamic Tailwater)


## Summary for Pond P206: STORMTECH INFILTRATION SYSTEM \#2

| Inflow Area = | 59,746 sf | 80.70\% Impervious, | Inflow Depth > 8.34" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 11.69 cfs @ | 12.09 hrs , Volume= | 41,528 cf |
| Outflow | 9.22 cfs @ | 12.16 hrs , Volume= | $41,523 \mathrm{cf}$, Atten= $21 \%$, Lag= 4.2 min |
| Discarded | 0.49 cfs @ | 10.15 hrs , Volume $=$ | 26,156 cf |
| Primary | 8.72 cfs @ | 12.16 hrs , Volume= | 15,367 cf | Routed to Link AP4 : ANALYSIS POINT \#4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 196.64' @ 12.16 hrs Surf.Area= 6,072 sf Storage= 8,490 cf
Plug-Flow detention time $=54.2 \mathrm{~min}$ calculated for $41,523 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time=54.1 min (808.9-754.8)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1A | 194.67' | 1,786 cf | 39.50'W x 53.46'L x 3.33'H FIELD A |
|  |  |  | 7,038 cf Overall - 2,573 cf Embedded $=4,466$ cf $\times 40.0 \%$ Voids |
| \#2A | 195.00' | 2,573 cf | ADS_StormTech SC-740 +Capx 56 Inside \#1 |
|  |  |  | Effective Size $=44.6$ "W $\times 30.0$ "H $=>6.45 \mathrm{sf} \times 7.12 \mathrm{~L}=45.9 \mathrm{cf}$ |
|  |  |  | Overall Size $=51.0$ "W $\times 30.0$ 'H $\times 7.56{ }^{\prime} \mathrm{L}$ with $0.44{ }^{\text {' Overlap }}$ |
|  |  |  | 56 Chambers in 8 Rows |
| \#3B | 194.67' | $3,296 \mathrm{cf}$ | 58.50'W x 67.70'L x 3.33'H FIELD B |
|  |  |  | 13,201 cf Overall - 4,962 cf Embedded $=8,239$ cf $\times 40.0 \%$ Voids |
| \#4B | 195.00' | 4,962 cf | ADS_StormTech SC-740 +Capx 108 Inside \#3 |
|  |  |  | Effective Size $=44.6$ " $\mathrm{W} \times 30.0$ " $\mathrm{H}=>6.45 \mathrm{sf} \times 7.12 \mathrm{~L}=45.9 \mathrm{cf}$ |
|  |  |  | Overall Size $=51.0$ "W x 30.0"H x 7.56'L with 0.44 ' Overlap |
|  |  |  | 108 Chambers in 12 Rows |
| 12,616 cf Total Available Storage |  |  |  |
| Storage Group A created with Chamber Wizard |  |  |  |
| Storage Group B created with Chamber Wizard |  |  |  |
| Device | Routing | Invert Ou | Outlet Devices |
| \#1 | Primary | 194.65' $\begin{array}{ll}18.0 \\ \text { Inle }\end{array}$ | 8.0" Round Culvert L=30.0' $\mathrm{Ke}=0.200$ |
|  |  |  | / Outlet Invert= 194.65' $/ 194.50$ ' S= 0.0050 '/' Cc= 0.900 |
|  |  |  | .013 Corrugated PE, smooth interior, Flow Area= 1.77 sf |
| \#2 | Device 1 | 195.85' 4.0' | long Sharp-Crested Rectangular Weir 2 End Contraction(s) |
| \#3 | Discarded | 194.67' 3.50 | $0 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= $0.01{ }^{\prime}$ |

Discarded OutFlow Max=0.49 cfs @ 10.15 hrs HW=194.71' (Free Discharge)

Primary OutFlow Max=8.69 cfs @ 12.16 hrs HW=196.63' TW=0.00' (Dynamic Tailwater)
L-1=Culvert (Passes 8.69 cfs of 8.71 cfs potential flow)


## Summary for Pond P207: INFILTRATION POND \#2

| Inflow Area | 129,716 | 13\% Impervious, | Inflow Depth > 7.72" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 23.17 cfs @ | 12.09 hrs , Volume= | 83,435 cf |
| Outflow | 11.39 cfs @ | 12.28 hrs , Volume= | 83,408 cf, Atten $=51 \%$, Lag $=11.2 \mathrm{~min}$ |
| Discarded = | 1.15 cfs @ | 12.28 hrs , Volume= | 51,829 cf |
| Primary | 10.24 cfs @ | 12.28 hrs , Volume= | 31,579 cf |

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 198.98' @ 12.28 hrs Surf.Area= 13,476 sf Storage= 25,795 cf
Plug-Flow detention time $=81.6 \mathrm{~min}$ calculated for $83,235 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time= $81.2 \mathrm{~min}(853.3-772.1)$


Discarded OutFlow Max=1.15 cfs @ 12.28 hrs HW=198.98' (Free Discharge)
$L_{5=\text { Exfiltration (Exfiltration Controls } 1.15 \mathrm{cfs} \text { ) }}$
Primary OutFlow Max=10.13 cfs @ 12.28 hrs HW=198.98' TW=192.58' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir(Weir Controls 4.17 cfs @ 1.14 fps )
-2=Culvert (Passes 5.95 cfs of 11.22 cfs potential flow)
——3=Orifice/Grate (Weir Controls 4.07 cfs @ 1.40 fps )
—4=Orifice/Grate (Orifice Controls 1.88 cfs @ 5.38 fps )

## Summary for Pond P210: POCKET WETLAND \#1



Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Starting Elev=201.00' Surf.Area= 376 sf Storage= 591 cf
Peak Elev= 204.66' @ 12.40 hrs Surf.Area= 9,782 sf Storage $=20,706$ cf ( 20,115 cf above start)
Plug-Flow detention time $=286.6$ min calculated for 20,065 cf (53\% of inflow)
Center-of-Mass det. time $=160.9 \mathrm{~min}$ ( 933.1-772.2 )


Primary OutFlow Max=3.50 cfs @ 12.40 hrs HW=204.66' TW=202.13' (Dynamic Tailwater)
-1=Broad-Crested Rectangular Weir( Controls 0.00 cfs)
$\mathbf{2 = C u l v e r t}$ (Passes 3.50 cfs of 4.74 cfs potential flow)
-3=Orifice/Grate (Orifice Controls 0.25 cfs @ 7.23 fps)
-4=Orifice/Grate (Weir Controls 3.26 cfs @ 1.30 fps )

## Summary for Pond P212: INFILTRATION POND \#1

| Inflow Area = | 241,078 sf, | 59.10\% Impervious | Inflow Depth > 6.77" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 32.18 cfs @ | 12.10 hrs , Volume= | 135,950 cf |
| Outflow | 29.18 cfs @ | 12.16 hrs , Volume= | 131,516 cf, Atten= 9\%, Lag= 3.6 min |
| Discarded | 1.49 cfs @ | 12.16 hrs , Volume= | 74,502 cf |
| Primary | 27.69 cfs @ | 12.16 hrs , Volume= | 57,014 cf |

Routed to Reach R211 : OVERLAND FLOW
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs / 3
Peak Elev= 203.05' @ 12.16 hrs Surf.Area= 12,518 sf Storage= 29,225 cf
Plug-Flow detention time $=101.4$ min calculated for 131,516 cf ( $97 \%$ of inflow)
Center-of-Mass det. time= $81.9 \mathrm{~min}(855.2-773.3)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $200.00^{\prime}$ | 41,774 cf | Custom Stage Data (Conic)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> $($ sq-ft $)$ | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) | Wet.Area <br> $($ sq-ft) |
| ---: | ---: | ---: | ---: | ---: |
| 200.00 | 4,354 | 0 | 0 | 4,354 |
| 201.00 | 9,360 | 6,699 | 6,699 | 9,368 |
| 202.00 | 10,993 | 10,166 | 16,865 | 11,040 |
| 204.00 | 13,976 | 24,909 | 41,774 | 14,126 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 202.50' | 25.0' long x 20.0' breadth Broad-Crested Rectangu |
|  |  |  | Head (feet) 0.200 .400 .600 .801 .001 .201 .401 .60 |
|  |  |  | Coef. (English) 2.682 .702 .702 .642 .632 .642 .642 .63 |
| \#2 | Discarded | 200.00' | $5.130 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area Phase-In= $0.01^{\prime}$ |

Discarded OutFlow Max=1.49 cfs @ 12.16 hrs HW=203.05' (Free Discharge)
L2=Exfiltration (Exfiltration Controls 1.49 cfs )
Primary OutFlow Max=27.29 cfs @ 12.16 hrs HW=203.05' TW=200.43' (Dynamic Tailwater)
L-1=Broad-Crested Rectangular Weir(Weir Controls 27.29 cfs @ 2.00 fps )

## Summary for Pond P213: Stormtech Infiltration System \#3



Routed to Pond P212 : INFILTRATION POND \#1
Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev= 204.44' @ 12.36 hrs Surf.Area= 3,317 sf Storage= 7,117 cf
Plug-Flow detention time $=91.4 \mathrm{~min}$ calculated for $23,001 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time= 91.3 min ( 838.2-746.9)


Discarded OutFlow Max=0.39 cfs @ 10.95 hrs HW=200.99' (Free Discharge)
L2=Exfiltration (Exfiltration Controls 0.39 cfs )
Primary OutFlow Max=1.83 cfs @ 12.36 hrs HW=204.44' TW=202.93' (Dynamic Tailwater)
t-1=Culvert (Passes 1.83 cfs of 4.11 cfs potential flow)

- 3=Sharp-Crested Rectangular Weir(Weir Controls 1.06 cfs @ 1.42 fps)
—4=Orifice/Grate (Orifice Controls 0.77 cfs @ 4.62 fps )
Summary for Pond P214: STORMTECH INFILTRATION SYSTEM \#4
[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=2)
Inflow Area $=\quad 32,665$ sf, $94.81 \%$ Impervious, Inflow Depth > 8.54" for 100YR event
Inflow $=\quad 6.45 \mathrm{cfs}$ @ 12.09 hrs , Volume $=\quad 23,257 \mathrm{cf}$
Outflow = 1.84 cfs @ 12.42 hrs , Volume= $23,256 \mathrm{cf}$, Atten= $72 \%$, Lag= 20.2 min
Discarded $=\quad 0.61 \mathrm{cfs}$ @ 11.50 hrs , Volume $=\quad 20,934 \mathrm{cf}$
Primary = 1.22 cfs @ 12.42 hrs, Volume= $2,322 \mathrm{cf}$ Routed to Reach 9R : OVERLAND FLOW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= $0.05 \mathrm{hrs} / 3$
Peak Elev=203.26' @ 12.42 hrs Surf.Area= 3,201 sf Storage= $6,493 \mathrm{cf}$
Plug-Flow detention time $=52.8 \mathrm{~min}$ calculated for $23,208 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time $=52.6 \mathrm{~min}(801.2-748.6)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1A | 200.00' | 1,851 cf | $30.00^{\prime} \mathrm{W} \times 67.70^{\prime} \mathrm{L} \times 3.50$ 'H Field A <br> 7,108 cf Overall - 2,481 cf Embedded $=4,627$ cf $\times 40.0 \%$ Voids |
| \#2A | 200.50' | 2,481 cf | ADS_StormTech SC-740 +Capx 54 Inside \#1 Effective Size $=44.6^{\prime \prime} \mathrm{W} \times 30.0 \mathrm{O} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12 \mathrm{~L}=45.9 \mathrm{cf}$ Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with 0.44 ' Overlap 54 Chambers in 6 Rows |
| \#3B | 200.00' | 1,087 cf | 25.25'W x 46.34'L x 3.50'H Field B <br> 4,095 cf Overall $-1,378$ cf Embedded $=2,717$ cf $\times 40.0 \%$ Voids |
| \#4B | 200.50' | 1,378 cf | ADS_StormTech SC-740 +Capx 30 Inside \#3 Effective Size $=44.6^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H}=>6.45 \mathrm{sf} \times 7.12^{\prime} \mathrm{L}=45.9 \mathrm{cf}$ Overall Size $=51.0^{\prime \prime} \mathrm{W} \times 30.0^{\prime \prime} \mathrm{H} \times 7.56^{\prime} \mathrm{L}$ with 0.44 ' Overlap 30 Chambers in 5 Rows |
|  |  | 6,797 cf | Total Available Storage |

Storage Group A created with Chamber Wizard
Storage Group B created with Chamber Wizard

| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 201.00' | 12.0" Round Culvert L= 25.0' $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert=201.00' 200.88 ' S=0.0048 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$ Corrugated PE, smooth interior, Flow Area= 0.79 sf |
| \#2 | Discarded | 200.00' | $8.280 \mathrm{in} / \mathrm{hr} \mathrm{Exfiltration} \mathrm{over} \mathrm{Surface} \mathrm{area} \mathrm{Phase-In=0.01'}$ |
| \#3 | Device 1 | 203.40' | 4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s) |
| \#4 | Device 1 | 202.40' | 8.0" Vert. Orifice/Grate $\mathrm{C}=0.600$ Limited to weir flow at low heads |

Discarded OutFlow Max=0.61 cfs @ 11.50 hrs HW=200.04' (Free Discharge)
—2=Exfiltration (Exfiltration Controls 0.61 cfs)
Primary OutFlow Max=1.22 cfs @ 12.42 hrs HW=203.26' TW=201.88' (Dynamic Tailwater)
L-1=Culvert (Passes 1.22 cfs of 4.44 cfs potential flow)

- $3=$ Sharp-Crested Rectangular Weir( Controls 0.00 cfs)
—4=Orifice/Grate (Orifice Controls 1.22 cfs @ 3.48 fps )


## Summary for Link AP1: ANALYSIS POINT 1



Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP2: ANALYSIS POINT 2

| Inflow A | 816 | , | 5.80" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 52.47 cfs @ | 12.41 hrs , Volume= | 394,820 cf |
| Primary | 52.47 cfs @ | 12.41 hrs, Volume= | 394,820 cf, Atten= 0\%, Lag= 0.0 |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP3: ANALYSIS POINT 3

| Inflow Area | 55,420 | 16.57\% Impervious, | w Depth > 5.38" for 100YR event |
| :---: | :---: | :---: | :---: |
| Inflow | 7.86 cfs @ | 12.09 hrs , Volume= | 24,862 cf |
| Primary | 7.86 cfs @ | 12.09 hrs , Volume= | 24,862 cf, Atten= 0\%, Lag= 0.0 m |

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

## Summary for Link AP4: ANALYSIS POINT \#4

| Inflow Area = | ¢, 25.34\% impervious, | > 4.21" for 100YR event |
| :---: | :---: | :---: |
| Infl | 98.54 cfs @ 12.49 hrs, Volume= | 593,282 cf |
| Primary | 98.54 cfs @ 12.49 hrs, Volume= | 593,282 cf, Atten= 0\%, Lag= 0.0 min |

Primary outflow $=$ Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Stage-Area-Storage for Pond P204: STORMTECH INFILTRATION SYSTEM \#1

| Elevation (feet) | $\begin{array}{r} \text { Surface } \\ (\mathrm{sq}-\mathrm{ft}) \end{array}$ | Storage (cubic-feet) | Elevation (feet) | $\begin{gathered} \text { Surface } \\ (\mathrm{sq}-\mathrm{ft}) \end{gathered}$ | Storage (cubic-feet) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 202.50 | 5,670 | 0 | 207.70 | 5,670 | 13,379 |
| 202.60 | 5,670 | 227 | 207.80 | 5,670 | 13,379 |
| 202.70 | 5,670 | 454 | 207.90 | 5,670 | 13,379 |
| 202.80 | 5,670 | 680 | 208.00 | 5,670 | 13,379 |
| 202.90 | 5,670 | 907 | 208.10 | 5,670 | 13,379 |
| 203.00 | 5,670 | 1,134 | 208.20 | 5,670 | 13,379 |
| 203.10 | 5,670 | 1,413 | 208.30 | 5,670 | 13,379 |
| 203.20 | 5,670 | 1,883 | 208.40 | 5,670 | 13,379 |
| 203.30 | 5,670 | 2,350 | 208.50 | 5,670 | 13,379 |
| 203.40 | 5,670 | 2,815 | 208.60 | 5,670 | 13,379 |
| 203.50 | 5,670 | 3,277 | 208.70 | 5,670 | 13,379 |
| 203.60 | 5,670 | 3,735 |  |  |  |
| 203.70 | 5,670 | 4,189 |  |  |  |
| 203.80 | 5,670 | 4,638 |  |  |  |
| 203.90 | 5,670 | 5,083 |  |  |  |
| 204.00 | 5,670 | 5,524 |  |  |  |
| 204.10 | 5,670 | 5,958 |  |  |  |
| 204.20 | 5,670 | 6,387 |  |  |  |
| 204.30 | 5,670 | 6,810 |  |  |  |
| 204.40 | 5,670 | 7,225 |  |  |  |
| 204.50 | 5,670 | 7,633 |  |  |  |
| 204.60 | 5,670 | 8,033 |  |  |  |
| 204.70 | 5,670 | 8,425 |  |  |  |
| 204.80 | 5,670 | 8,805 |  |  |  |
| 204.90 | 5,670 | 9,174 |  |  |  |
| 205.00 | 5,670 | 9,531 |  |  |  |
| 205.10 | 5,670 | 9,873 |  |  |  |
| 205.20 | 5,670 | 10,199 |  |  |  |
| 205.30 | 5,670 | 10,500 |  |  |  |
| 205.40 | 5,670 | 10,770 |  |  |  |
| 205.50 | 5,670 | 11,016 |  |  |  |
| 205.60 | 5,670 | 11,248 |  |  |  |
| 205.70 | 5,670 | 11,474 |  |  |  |
| 205.80 | 5,670 | 11,701 |  |  |  |
| 205.90 | 5,670 | 11,928 |  |  |  |
| 206.00 | 5,670 | 12,155 |  |  |  |
| 206.10 | 5,670 | 12,365 |  |  |  |
| 206.20 | 5,670 | 12,574 |  |  |  |
| 206.30 | 5,670 | 12,784 |  |  |  |
| 206.40 | 5,670 | 12,994 |  |  |  |
| 206.50 | 5,670 | 13,204 |  |  |  |
| 206.60 | 5,670 | 13,379 |  |  |  |
| 206.70 | 5,670 | 13,379 |  |  |  |
| 206.80 | 5,670 | 13,379 |  |  |  |
| 206.90 | 5,670 | 13,379 |  |  |  |
| 207.00 | 5,670 | 13,379 |  |  |  |
| 207.10 | 5,670 | 13,379 |  |  |  |
| 207.20 | 5,670 | 13,379 |  |  |  |
| 207.30 | 5,670 | 13,379 |  |  |  |
| 207.40 | 5,670 | 13,379 |  |  |  |
| 207.50 | 5,670 | 13,379 |  |  |  |
| 207.60 | 5,670 | 13,379 |  |  |  |

Stage-Area-Storage for Pond P205: INFILTRATION POND \#3

| Elevation | Surface (sq-ft) | Storage (cubic-feet) | Elevation (feet) | Surface (sq-ft) | Storage (cubic-feet) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 205.00 | 3,939 | 0 | 207.60 | 6,184 | 13,082 |
| 205.05 | 3,980 | 198 | 207.65 | 6,230 | 13,393 |
| 205.10 | 4,021 | 398 | 207.70 | 6,277 | 13,705 |
| 205.15 | 4,062 | 600 | 207.75 | 6,323 | 14,020 |
| 205.20 | 4,102 | 804 | 207.80 | 6,369 | 14,338 |
| 205.25 | 4,143 | 1,010 | 207.85 | 6,416 | 14,657 |
| 205.30 | 4,184 | 1,218 | 207.90 | 6,462 | 14,979 |
| 205.35 | 4,225 | 1,429 | 207.95 | 6,509 | 15,303 |
| 205.40 | 4,266 | 1,641 | 208.00 | 6,555 | 15,630 |
| 205.45 | 4,307 | 1,855 |  |  |  |
| 205.50 | 4,348 | 2,072 |  |  |  |
| 205.55 | 4,388 | 2,290 |  |  |  |
| 205.60 | 4,429 | 2,510 |  |  |  |
| 205.65 | 4,470 | 2,733 |  |  |  |
| 205.70 | 4,511 | 2,957 |  |  |  |
| 205.75 | 4,552 | 3,184 |  |  |  |
| 205.80 | 4,593 | 3,413 |  |  |  |
| 205.85 | 4,633 | 3,643 |  |  |  |
| 205.90 | 4,674 | 3,876 |  |  |  |
| 205.95 | 4,715 | 4,111 |  |  |  |
| 206.00 | 4,756 | 4,348 |  |  |  |
| 206.05 | 4,800 | 4,586 |  |  |  |
| 206.10 | 4,843 | 4,827 |  |  |  |
| 206.15 | 4,887 | 5,071 |  |  |  |
| 206.20 | 4,930 | 5,316 |  |  |  |
| 206.25 | 4,974 | 5,564 |  |  |  |
| 206.30 | 5,017 | 5,813 |  |  |  |
| 206.35 | 5,061 | 6,065 |  |  |  |
| 206.40 | 5,104 | 6,320 |  |  |  |
| 206.45 | 5,148 | 6,576 |  |  |  |
| 206.50 | 5,192 | 6,834 |  |  |  |
| 206.55 | 5,235 | 7,095 |  |  |  |
| 206.60 | 5,279 | 7,358 |  |  |  |
| 206.65 | 5,322 | 7,623 |  |  |  |
| 206.70 | 5,366 | 7,890 |  |  |  |
| 206.75 | 5,409 | 8,159 |  |  |  |
| 206.80 | 5,453 | 8,431 |  |  |  |
| 206.85 | 5,496 | 8,705 |  |  |  |
| 206.90 | 5,540 | 8,981 |  |  |  |
| 206.95 | 5,583 | 9,259 |  |  |  |
| 207.00 | 5,627 | 9,539 |  |  |  |
| 207.05 | 5,673 | 9,822 |  |  |  |
| 207.10 | 5,720 | 10,106 |  |  |  |
| 207.15 | 5,766 | 10,393 |  |  |  |
| 207.20 | 5,813 | 10,683 |  |  |  |
| 207.25 | 5,859 | 10,975 |  |  |  |
| 207.30 | 5,905 | 11,269 |  |  |  |
| 207.35 | 5,952 | 11,565 |  |  |  |
| 207.40 | 5,998 | 11,864 |  |  |  |
| 207.45 | 6,045 | 12,165 |  |  |  |
| 207.50 | 6,091 | 12,469 |  |  |  |
| 207.55 | 6,137 | 12,774 |  |  |  |

Stage-Area-Storage for Pond P206: STORMTECH INFILTRATION SYSTEM \#2

| Elevation (feet) | $\begin{array}{r} \text { Surface } \\ (\mathrm{sq}-\mathrm{ft}) \end{array}$ | Storage (cubic-feet) | Elevation (feet) | Surface (sq-ft) | Storage (cubic-feet) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 194.67 | 6,072 | 0 | 197.27 | 6,072 | 10,788 |
| 194.72 | 6,072 | 121 | 197.32 | 6,072 | 10,930 |
| 194.77 | 6,072 | 243 | 197.37 | 6,072 | 11,064 |
| 194.82 | 6,072 | 364 | 197.42 | 6,072 | 11,194 |
| 194.87 | 6,072 | 486 | 197.47 | 6,072 | 11,320 |
| 194.92 | 6,072 | 607 | 197.52 | 6,072 | 11,442 |
| 194.97 | 6,072 | 729 | 197.57 | 6,072 | 11,564 |
| 195.02 | 6,072 | 893 | 197.62 | 6,072 | 11,685 |
| 195.07 | 6,072 | 1,145 | 197.67 | 6,072 | 11,807 |
| 195.12 | 6,072 | 1,397 | 197.72 | 6,072 | 11,928 |
| 195.17 | 6,072 | 1,648 | 197.77 | 6,072 | 12,050 |
| 195.22 | 6,072 | 1,898 | 197.82 | 6,072 | 12,171 |
| 195.27 | 6,072 | 2,147 | 197.87 | 6,072 | 12,292 |
| 195.32 | 6,072 | 2,396 | 197.92 | 6,072 | 12,414 |
| 195.37 | 6,072 | 2,644 | 197.97 | 6,072 | 12,535 |
| 195.42 | 6,072 | 2,891 |  |  |  |
| 195.47 | 6,072 | 3,137 |  |  |  |
| 195.52 | 6,072 | 3,382 |  |  |  |
| 195.57 | 6,072 | 3,626 |  |  |  |
| 195.62 | 6,072 | 3,868 |  |  |  |
| 195.67 | 6,072 | 4,110 |  |  |  |
| 195.72 | 6,072 | 4,350 |  |  |  |
| 195.77 | 6,072 | 4,590 |  |  |  |
| 195.82 | 6,072 | 4,827 |  |  |  |
| 195.87 | 6,072 | 5,064 |  |  |  |
| 195.92 | 6,072 | 5,299 |  |  |  |
| 195.97 | 6,072 | 5,533 |  |  |  |
| 196.02 | 6,072 | 5,765 |  |  |  |
| 196.07 | 6,072 | 5,996 |  |  |  |
| 196.12 | 6,072 | 6,225 |  |  |  |
| 196.17 | 6,072 | 6,453 |  |  |  |
| 196.22 | 6,072 | 6,678 |  |  |  |
| 196.27 | 6,072 | 6,902 |  |  |  |
| 196.32 | 6,072 | 7,124 |  |  |  |
| 196.37 | 6,072 | 7,343 |  |  |  |
| 196.42 | 6,072 | 7,561 |  |  |  |
| 196.47 | 6,072 | 7,777 |  |  |  |
| 196.52 | 6,072 | 7,990 |  |  |  |
| 196.57 | 6,072 | 8,201 |  |  |  |
| 196.62 | 6,072 | 8,410 |  |  |  |
| 196.67 | 6,072 | 8,616 |  |  |  |
| 196.72 | 6,072 | 8,818 |  |  |  |
| 196.77 | 6,072 | 9,018 |  |  |  |
| 196.82 | 6,072 | 9,214 |  |  |  |
| 196.87 | 6,072 | 9,407 |  |  |  |
| 196.92 | 6,072 | 9,597 |  |  |  |
| 196.97 | 6,072 | 9,783 |  |  |  |
| 197.02 | 6,072 | 9,965 |  |  |  |
| 197.07 | 6,072 | 10,142 |  |  |  |
| 197.12 | 6,072 | 10,315 |  |  |  |
| 197.17 | 6,072 | 10,481 |  |  |  |
| 197.22 | 6,072 | 10,639 |  |  |  |

Stage-Area-Storage for Pond P207: INFILTRATION POND \#2

| Elevation <br> (feet) | Surface <br> (sq-ft) | Storage <br> (cubic-feet) | Elevation <br> (feet) | Surface <br> (sq-ft) | Storage <br> (cubic-feet) |
| ---: | ---: | ---: | ---: | ---: | ---: |
| 196.80 | 10,100 | 0 | 199.40 | 14,100 | 31,530 |
| 196.85 | 10,179 | 507 | 199.45 | 14,175 | 32,237 |
| 196.90 | 10,258 | 1,018 | 199.50 | 14,250 | 32,947 |
| 196.95 | 10,338 | 1,533 | 199.55 | 14,325 | 33,662 |
| 197.00 | 10,417 | 2,052 | 199.60 | 14,400 | 34,380 |
| 197.05 | 10,496 | 2,574 | 199.65 | 14,475 | 35,102 |
| 197.10 | 10,575 | 3,101 | 199.70 | 14,550 | 35,828 |
| 197.15 | 10,654 | 3,632 | 199.75 | 14,625 | 36,557 |
| 197.20 | 10,733 | 4,167 | 199.80 | 14,700 | 37,290 |
| 197.25 | 10,812 | 4,705 | 199.85 | 14,775 | 38,027 |
| 197.30 | 10,892 | 5,248 | 199.90 | 14,850 | 38,767 |
| 197.35 | 10,971 | 5,794 | 199.95 | 14,925 | 39,512 |
| 197.40 | 11,050 | 6,345 | 200.00 | 15,000 | 40,260 |
| 197.45 | 11,129 | 6,899 |  |  |  |
| 197.50 | 11,208 | 7,458 |  |  |  |
| 197.55 | 11,288 | 8,020 |  |  |  |
| 197.60 | 11,367 | 8,587 |  |  |  |
| 197.65 | 11,446 | 9,157 |  |  |  |
| 197.70 | 11,525 | 9,731 |  |  |  |
| 197.75 | 11,604 | 10,309 |  |  |  |
| 197.80 | 11,683 | 10,892 |  |  |  |
| 197.85 | 11,763 | 11,478 |  |  |  |
| 197.90 | 11,842 | 12,068 |  |  |  |
| 197.95 | 11,921 | 12,662 |  |  |  |
| 198.00 | 12,000 | 13,260 |  |  |  |
| 198.05 | 12,075 | 13,862 |  |  |  |
| 198.10 | 12,150 | 14,468 |  |  |  |
| 198.15 | 12,525 | 15,077 |  |  |  |
| 198.20 | 12,300 | 15,690 |  |  |  |
| 198.25 | 12,375 | 16,307 |  |  |  |
| 198.30 | 12,450 | 16,928 |  |  |  |
| 198.35 | 12,525 | 17,552 |  |  |  |
| 198.40 | 12,600 | 18,180 |  |  |  |
| 198.45 | 12,675 | 18,812 |  |  |  |
| 198.50 | 12,750 | 19,447 |  |  |  |
| 198.55 | 12,825 | 20,087 |  |  |  |
| 198.60 | 12,900 | 20,730 |  |  |  |
| 198.65 | 12,975 | 21,377 |  |  |  |
| 198.70 | 13,050 | 22,028 |  |  |  |
| 198.75 | 13,125 | 22,682 |  |  |  |
| 198.80 | 13,200 | 23,840 |  |  |  |
| 198.85 | 13,275 | 24,002 |  |  |  |
| 198.90 | 13,350 | 24,667 |  |  |  |
| 198.95 | 13,425 | 25,337 |  |  |  |
| 19900 | 13,500 | 26,010 |  |  |  |
| 199.05 | 13,575 | 26,687 |  |  |  |
| 199.10 | 13,650 | 27,368 |  |  |  |
| 199.15 | 13,725 | 28,052 |  |  |  |
| 199.20 | 13,800 | 28,740 |  |  |  |
| 199.25 | 13,875 | 29,432 |  |  |  |
| 199.30 | 13,950 | 30,128 |  |  |  |
| 199.35 | 14,025 | 30,827 |  |  |  |
|  |  |  |  |  |  |

Stage-Area-Storage for Pond P210: POCKET WETLAND \#1

| Elevation (feet) | $\begin{array}{r} \text { Surface } \\ (\mathrm{sq}-\mathrm{ft}) \end{array}$ | Storage (cubic-feet) | Elevation (feet) | Surface (sq-ft) | Storage (cubic-feet) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 199.00 | 218 | 0 | 204.20 | 8,593 | 16,505 |
| 199.10 | 226 | 22 | 204.30 | 8,853 | 17,377 |
| 199.20 | 233 | 45 | 204.40 | 9,113 | 18,276 |
| 199.30 | 241 | 69 | 204.50 | 9,373 | 19,200 |
| 199.40 | 248 | 93 | 204.60 | 9,633 | 20,150 |
| 199.50 | 256 | 119 | 204.70 | 9,893 | 21,126 |
| 199.60 | 264 | 144 | 204.80 | 10,153 | 22,129 |
| 199.70 | 271 | 171 | 204.90 | 10,413 | 23,157 |
| 199.80 | 279 | 199 | 205.00 | 10,673 | 24,211 |
| 199.90 | 286 | 227 | 205.10 | 10,932 | 25,291 |
| 200.00 | 294 | 256 | 205.20 | 11,192 | 26,398 |
| 200.10 | 302 | 286 | 205.30 | 11,452 | 27,530 |
| 200.20 | 310 | 316 | 205.40 | 11,712 | 28,688 |
| 200.30 | 319 | 348 | 205.50 | 11,972 | 29,872 |
| 200.40 | 327 | 380 | 205.60 | 12,232 | 31,083 |
| 200.50 | 335 | 413 | 205.70 | 12,492 | 32,319 |
| 200.60 | 343 | 447 | 205.80 | 12,752 | 33,581 |
| 200.70 | 351 | 482 | 205.90 | 13,012 | 34,869 |
| 200.80 | 360 | 517 | 206.00 | 13,272 | 36,184 |
| 200.90 | 368 | 554 | 206.10 | 13,568 | 37,526 |
| 201.00 | 376 | 591 | 206.20 | 13,864 | 38,897 |
| 201.10 | 737 | 647 | 206.30 | 14,161 | 40,298 |
| 201.20 | 1,099 | 738 | 206.40 | 14,457 | 41,729 |
| 201.30 | 1,461 | 866 | 206.50 | 14,753 | 43,190 |
| 201.40 | 1,822 | 1,031 |  |  |  |
| 201.50 | 2,184 | 1,231 |  |  |  |
| 201.60 | 2,545 | 1,467 |  |  |  |
| 201.70 | 2,906 | 1,740 |  |  |  |
| 201.80 | 3,268 | 2,049 |  |  |  |
| 201.90 | 3,630 | 2,393 |  |  |  |
| 202.00 | 3,991 | 2,775 |  |  |  |
| 202.10 | 4,195 | 3,184 |  |  |  |
| 202.20 | 4,399 | 3,614 |  |  |  |
| 202.30 | 4,603 | 4,064 |  |  |  |
| 202.40 | 4,807 | 4,534 |  |  |  |
| 202.50 | 5,012 | 5,025 |  |  |  |
| 202.60 | 5,216 | 5,536 |  |  |  |
| 202.70 | 5,420 | 6,068 |  |  |  |
| 202.80 | 5,624 | 6,620 |  |  |  |
| 202.90 | 5,828 | 7,193 |  |  |  |
| 203.00 | 6,032 | 7,786 |  |  |  |
| 203.10 | 6,236 | 8,399 |  |  |  |
| 203.20 | 6,440 | 9,033 |  |  |  |
| 203.30 | 6,644 | 9,687 |  |  |  |
| 203.40 | 6,848 | 10,362 |  |  |  |
| 203.50 | 7,053 | 11,057 |  |  |  |
| 203.60 | 7,257 | 11,773 |  |  |  |
| 203.70 | 7,461 | 12,508 |  |  |  |
| 203.80 | 7,665 | 13,265 |  |  |  |
| 203.90 | 7,869 | 14,041 |  |  |  |
| 204.00 | 8,073 | 14,839 |  |  |  |
| 204.10 | 8,333 | 15,659 |  |  |  |

Stage-Area-Storage for Pond P212: INFILTRATION POND \#1

| Elevation (feet) | $\begin{array}{r} \text { Surface } \\ (\mathrm{sq}-\mathrm{ft}) \end{array}$ | Storage (cubic-feet) | Elevation (feet) | Surface (sq-ft) | Storage (cubic-feet) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 200.00 | 4,354 | 0 | 202.60 | 11,850 | 23,716 |
| 200.05 | 4,559 | 223 | 202.65 | 11,923 | 24,311 |
| 200.10 | 4,769 | 456 | 202.70 | 11,996 | 24,909 |
| 200.15 | 4,984 | 700 | 202.75 | 12,070 | 25,510 |
| 200.20 | 5,204 | 955 | 202.80 | 12,143 | 26,116 |
| 200.25 | 5,428 | 1,220 | 202.85 | 12,217 | 26,725 |
| 200.30 | 5,657 | 1,497 | 202.90 | 12,291 | 27,337 |
| 200.35 | 5,891 | 1,786 | 202.95 | 12,365 | 27,954 |
| 200.40 | 6,129 | 2,087 | 203.00 | 12,440 | 28,574 |
| 200.45 | 6,372 | 2,399 | 203.05 | 12,514 | 29,198 |
| 200.50 | 6,620 | 2,724 | 203.10 | 12,589 | 29,825 |
| 200.55 | 6,873 | 3,061 | 203.15 | 12,665 | 30,457 |
| 200.60 | 7,130 | 3,411 | 203.20 | 12,740 | 31,092 |
| 200.65 | 7,393 | 3,774 | 203.25 | 12,815 | 31,731 |
| 200.70 | 7,659 | 4,151 | 203.30 | 12,891 | 32,373 |
| 200.75 | 7,931 | 4,540 | 203.35 | 12,967 | 33,020 |
| 200.80 | 8,207 | 4,944 | 203.40 | 13,044 | 33,670 |
| 200.85 | 8,488 | 5,361 | 203.45 | 13,120 | 34,324 |
| 200.90 | 8,774 | 5,793 | 203.50 | 13,197 | 34,982 |
| 200.95 | 9,065 | 6,239 | 203.55 | 13,274 | 35,644 |
| 201.00 | 9,360 | 6,699 | 203.60 | 13,351 | 36,309 |
| 201.05 | 9,439 | 7,169 | 203.65 | 13,428 | 36,979 |
| 201.10 | 9,517 | 7,643 | 203.70 | 13,506 | 37,652 |
| 201.15 | 9,597 | 8,121 | 203.75 | 13,584 | 38,329 |
| 201.20 | 9,676 | 8,603 | 203.80 | 13,662 | 39,011 |
| 201.25 | 9,756 | 9,089 | 203.85 | 13,740 | 39,696 |
| 201.30 | 9,836 | 9,578 | 203.90 | 13,818 | 40,385 |
| 201.35 | 9,917 | 10,072 | 203.95 | 13,897 | 41,077 |
| 201.40 | 9,997 | 10,570 | 204.00 | 13,976 | 41,774 |
| 201.45 | 10,079 | 11,072 |  |  |  |
| 201.50 | 10,160 | 11,578 |  |  |  |
| 201.55 | 10,242 | 12,088 |  |  |  |
| 201.60 | 10,324 | 12,602 |  |  |  |
| 201.65 | 10,407 | 13,120 |  |  |  |
| 201.70 | 10,489 | 13,643 |  |  |  |
| 201.75 | 10,572 | 14,169 |  |  |  |
| 201.80 | 10,656 | 14,700 |  |  |  |
| 201.85 | 10,740 | 15,235 |  |  |  |
| 201.90 | 10,824 | 15,774 |  |  |  |
| 201.95 | 10,908 | 16,317 |  |  |  |
| 202.00 | 10,993 | 16,865 |  |  |  |
| 202.05 | 11,063 | 17,416 |  |  |  |
| 202.10 | 11,134 | 17,971 |  |  |  |
| 202.15 | 11,204 | 18,530 |  |  |  |
| 202.20 | 11,275 | 19,092 |  |  |  |
| 202.25 | 11,346 | 19,657 |  |  |  |
| 202.30 | 11,418 | 20,226 |  |  |  |
| 202.35 | 11,489 | 20,799 |  |  |  |
| 202.40 | 11,561 | 21,375 |  |  |  |
| 202.45 | 11,633 | 21,955 |  |  |  |
| 202.50 | 11,705 | 22,538 |  |  |  |
| 202.55 | 11,778 | 23,126 |  |  |  |

Stage-Area-Storage for Pond P213: Stormtech Infiltration System \#3

| Elevation (feet | Surface (sq-ft) | Storage (cubic-feet) | Elevation (feet) | Surface (sq-ft) | Storage (cubic-feet) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 200.95 | 3,317 | 0 | 203.55 | 3,317 | 5,836 |
| 201.00 | 3,317 | 66 | 203.60 | 3,317 | 5,928 |
| 201.05 | 3,317 | 133 | 203.65 | 3,317 | 6,016 |
| 201.10 | 3,317 | 199 | 203.70 | 3,317 | 6,099 |
| 201.15 | 3,317 | 265 | 203.75 | 3,317 | 6,178 |
| 201.20 | 3,317 | 332 | 203.80 | 3,317 | 6,252 |
| 201.25 | 3,317 | 398 | 203.85 | 3,317 | 6,324 |
| 201.30 | 3,317 | 464 | 203.90 | 3,317 | 6,394 |
| 201.35 | 3,317 | 531 | 203.95 | 3,317 | 6,461 |
| 201.40 | 3,317 | 597 | 204.00 | 3,317 | 6,528 |
| 201.45 | 3,317 | 663 | 204.05 | 3,317 | 6,594 |
| 201.50 | 3,317 | 801 | 204.10 | 3,317 | 6,660 |
| 201.55 | 3,317 | 939 | 204.15 | 3,317 | 6,727 |
| 201.60 | 3,317 | 1,076 | 204.20 | 3,317 | 6,793 |
| 201.65 | 3,317 | 1,214 | 204.25 | 3,317 | 6,859 |
| 201.70 | 3,317 | 1,350 | 204.30 | 3,317 | 6,926 |
| 201.75 | 3,317 | 1,487 | 204.35 | 3,317 | 6,992 |
| 201.80 | 3,317 | 1,623 | 204.40 | 3,317 | 7,058 |
| 201.85 | 3,317 | 1,758 | 204.45 | 3,317 | 7,125 |
| 201.90 | 3,317 | 1,893 |  |  |  |
| 201.95 | 3,317 | 2,027 |  |  |  |
| 202.00 | 3,317 | 2,161 |  |  |  |
| 202.05 | 3,317 | 2,294 |  |  |  |
| 202.10 | 3,317 | 2,427 |  |  |  |
| 202.15 | 3,317 | 2,558 |  |  |  |
| 202.20 | 3,317 | 2,690 |  |  |  |
| 202.25 | 3,317 | 2,820 |  |  |  |
| 202.30 | 3,317 | 2,950 |  |  |  |
| 202.35 | 3,317 | 3,079 |  |  |  |
| 202.40 | 3,317 | 3,207 |  |  |  |
| 202.45 | 3,317 | 3,335 |  |  |  |
| 202.50 | 3,317 | 3,461 |  |  |  |
| 202.55 | 3,317 | 3,587 |  |  |  |
| 202.60 | 3,317 | 3,712 |  |  |  |
| 202.65 | 3,317 | 3,836 |  |  |  |
| 202.70 | 3,317 | 3,958 |  |  |  |
| 202.75 | 3,317 | 4,080 |  |  |  |
| 202.80 | 3,317 | 4,201 |  |  |  |
| 202.85 | 3,317 | 4,320 |  |  |  |
| 202.90 | 3,317 | 4,439 |  |  |  |
| 202.95 | 3,317 | 4,556 |  |  |  |
| 203.00 | 3,317 | 4,672 |  |  |  |
| 203.05 | 3,317 | 4,787 |  |  |  |
| 203.10 | 3,317 | 4,900 |  |  |  |
| 203.15 | 3,317 | 5,012 |  |  |  |
| 203.20 | 3,317 | 5,121 |  |  |  |
| 203.25 | 3,317 | 5,229 |  |  |  |
| 203.30 | 3,317 | 5,335 |  |  |  |
| 203.35 | 3,317 | 5,440 |  |  |  |
| 203.40 | 3,317 | 5,542 |  |  |  |
| 203.45 | 3,317 | 5,643 |  |  |  |
| 203.50 | 3,317 | 5,741 |  |  |  |

Stage-Area-Storage for Pond P214: STORMTECH INFILTRATION SYSTEM \#4

| Elevation (feet) | $\begin{array}{r} \text { Surface } \\ (\mathrm{sq}-\mathrm{ft}) \end{array}$ | Storage (cubic-feet) | Elevation (feet) | $\begin{gathered} \text { Surface } \\ (\mathrm{sq}-\mathrm{ft}) \end{gathered}$ | Storage (cubic-feet) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 200.00 | 3,201 | 0 | 202.60 | 3,201 | 5,556 |
| 200.05 | 3,201 | 64 | 202.65 | 3,201 | 5,644 |
| 200.10 | 3,201 | 128 | 202.70 | 3,201 | 5,728 |
| 200.15 | 3,201 | 192 | 202.75 | 3,201 | 5,808 |
| 200.20 | 3,201 | 256 | 202.80 | 3,201 | 5,884 |
| 200.25 | 3,201 | 320 | 202.85 | 3,201 | 5,955 |
| 200.30 | 3,201 | 384 | 202.90 | 3,201 | 6,024 |
| 200.35 | 3,201 | 448 | 202.95 | 3,201 | 6,091 |
| 200.40 | 3,201 | 512 | 203.00 | 3,201 | 6,156 |
| 200.45 | 3,201 | 576 | 203.05 | 3,201 | 6,220 |
| 200.50 | 3,201 | 640 | 203.10 | 3,201 | 6,284 |
| 200.55 | 3,201 | 771 | 203.15 | 3,201 | 6,349 |
| 200.60 | 3,201 | 902 | 203.20 | 3,201 | 6,413 |
| 200.65 | 3,201 | 1,032 | 203.25 | 3,201 | 6,477 |
| 200.70 | 3,201 | 1,162 | 203.30 | 3,201 | 6,541 |
| 200.75 | 3,201 | 1,292 | 203.35 | 3,201 | 6,605 |
| 200.80 | 3,201 | 1,421 | 203.40 | 3,201 | 6,669 |
| 200.85 | 3,201 | 1,550 | 203.45 | 3,201 | 6,733 |
| 200.90 | 3,201 | 1,679 | 203.50 | 3,201 | 6,797 |
| 200.95 | 3,201 | 1,807 |  |  |  |
| 201.00 | 3,201 | 1,934 |  |  |  |
| 201.05 | 3,201 | 2,061 |  |  |  |
| 201.10 | 3,201 | 2,187 |  |  |  |
| 201.15 | 3,201 | 2,313 |  |  |  |
| 201.20 | 3,201 | 2,438 |  |  |  |
| 201.25 | 3,201 | 2,563 |  |  |  |
| 201.30 | 3,201 | 2,687 |  |  |  |
| 201.35 | 3,201 | 2,810 |  |  |  |
| 201.40 | 3,201 | 2,932 |  |  |  |
| 201.45 | 3,201 | 3,054 |  |  |  |
| 201.50 | 3,201 | 3,175 |  |  |  |
| 201.55 | 3,201 | 3,296 |  |  |  |
| 201.60 | 3,201 | 3,415 |  |  |  |
| 201.65 | 3,201 | 3,534 |  |  |  |
| 201.70 | 3,201 | 3,651 |  |  |  |
| 201.75 | 3,201 | 3,768 |  |  |  |
| 201.80 | 3,201 | 3,884 |  |  |  |
| 201.85 | 3,201 | 3,998 |  |  |  |
| 201.90 | 3,201 | 4,112 |  |  |  |
| 201.95 | 3,201 | 4,225 |  |  |  |
| 202.00 | 3,201 | 4,336 |  |  |  |
| 202.05 | 3,201 | 4,447 |  |  |  |
| 202.10 | 3,201 | 4,556 |  |  |  |
| 202.15 | 3,201 | 4,664 |  |  |  |
| 202.20 | 3,201 | 4,770 |  |  |  |
| 202.25 | 3,201 | 4,874 |  |  |  |
| 202.30 | 3,201 | 4,977 |  |  |  |
| 202.35 | 3,201 | 5,078 |  |  |  |
| 202.40 | 3,201 | 5,178 |  |  |  |
| 202.45 | 3,201 | 5,276 |  |  |  |
| 202.50 | 3,201 | 5,371 |  |  |  |
| 202.55 | 3,201 | 5,465 |  |  |  |

## Pocket Wetland Sizing

P210
Site Location: 55 Summer Street - Walpole, MA

| Design Criteria | Pocket Wetland (req.) | Pocket Wetland (P205) |
| :---: | :---: | :---: |
| Minimum Drainage Area (Ac.) | $\geq 1$ to 10 | 1.4 |
| Constructed Wetland Surface Area/Watershed Ratio | $\geq 0.01$ | 0.14 |
| Length to Width Ratio (min.) | $\geq 2: 1$ | 12:1 |
| Extended Detention (ED) | OPTIONAL | NO |
| Allocation of WQv Volume (wet pools/low and high marsh/ED) in \% | 20/80/02 | 24/76/0 |
| Allocation of Surface Area (wet pools/low marsh/high marsh/semi-wet) in \% | 10/45/40/5 | 9/43/43/5 |
| Sediment Forebay | REQUIRED | YES |
| Micropool | REQUIRED | YES |
| Oulet Configuration | Hooded Broad-Crested Weir | Multi-Stage Discharge Outlet Structure |
| Target Allocations | Pocket Wetland | Pocket Wetland |
| \% Surface Area (Req.) |  | 8,465 |
| Sediment Forebay | 5\% | 5\% |
| Micropool | 5\% | 4\% |
| Deep Water Channel | 0\% | 0\% |
| Lo Marsh | 45\% | 43\% |
| High Marsh | 40\% | 43\% |
| Semi-Wet | 5\% | 5\% |
| \% WQv Volume (Req.) |  | 2,619 |
| Sediment Forebay | 10\% | 10\% |
| Micropool | 10\% | 14\% |
| Deep Water Channel | 0\% | 0\% |
| Lo Marsh and High Marsh | 80\% | 76\% |

Date: 06/20/2023
By: PB
Checked: KE

| Designed Surface Area (sf) |  |
| :---: | :---: |
| Sediment Forebay | 392 |
| Micropool | 376 |
| Deep Water | 0 |
| Lo Marsh | 3615 |
| High Marsh | 3674 |
| Semi-Wet | 408 |


| Designed Water Quality Volume (cf) |  |
| :---: | :---: |
| Sediment Forebay | 267 |
| Micropool | 376 |


[^0]:    *Evidence of maintenance (ie. receipts) must be provided.

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